BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

NYRSTAR

NANISIVIK MINE, NUNAVUT

2014 ANNUAL GEOTECHNICAL INSPECTION

FINAL

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February 28, 2015
Project No.: 0255-024

Mr. Johan Skoglund
Group Environment Manager, Americas
Nyrstar
Tessinerplatz 7
8002 Zurich, Switzerland

Dear Johan,

Re: 2014 Annual Geotechnical Inspection Nanisivik Mine, Nunavut

Please find attached our above captioned report on the 2014 Annual Geotechnical Inspection undertaken at Nanisivik Mine. The report has been finalized upon receipt of your review comments.

If there are any questions or comments regarding this report, please contact the undersigned at your convenience.

Yours sincerely,

BGC ENGINEERING INC.
per:

Geoff Claypool, M.Eng., P.Eng.
Senior Geological Engineer

EXECUTIVE SUMMARY

The Annual Geotechnical Inspection was undertaken at the Nanisivik Mine site in 2014. The program involved the following:

- Completion of a site inspection by a professional geotechnical engineer. The site inspection included visual observations of all reclamation measures undertaken at the Nanisivik Mine site since 2004.
- Download of air temperature monitoring equipment and the re-installation and subsequent download of water level monitoring equipment (in the Reservoir).
- Collection of geotechnical monitoring data from various instruments located around the mine site. The data was collected generally in line with the frequency outlined in the Water License.
- Review of the geotechnical monitoring data to assess the performance of various reclamation measures with respect to the original design intent.

The following paragraphs provide a summary of the significant observations, conclusions and recommendations based on the results of the 2014 Geotechnical Inspection Program undertaken at the Nanisivik Mine site.

Embankments

The breaches in the remnant dykes at the East Adit Treatment Facility continue to permit natural drainage of the area with only limited ponding in the Retention Pond. The size of this ponding is gradually decreasing with time as the low spots in the pond area backfill with natural sediment transported from upslope areas. The Main Tank Farm berm was nearly completely removed at the time of the inspection and no ponding was occurring in the area. The West Twin Dyke was physically stable with no signs of erosion or seepage.

Hydraulic Structures

The West Twin Dyke Spillway continues to effectively drain surface water from the Surface Cell cover into the Reservoir and is generally functioning as intended. Additional erosion of the left bank was noted in 2014, likely in response to elevated flows during freshet. Due to the nature of the flows causing the erosion, it is likely that this erosion will continue without additional enhancement of the left bank armour. If not addressed, this may lead to decline in the effectiveness of the spillway over time.

As observed during previous inspections, a small head pond developed on the Surface Cell cover at the spillway inlet. The presence of the head pond is not considered to negatively impact the overall cover performance; hence the elimination of the head pond is not considered necessary. Reduction in pond size may continue over time as the intact rock in the base of the spillway inlet becomes more permeable due to freeze thaw effects.

Seepage losses continued at the West Twin Outlet wall in 2014. Despite these seepage losses, the water level in the remnant polishing pond upstream of the wall remained near the invert of the outlet wall throughout the open water season in 2014. The stable water level in the Reservoir is thought to reflect the generally wetter than normal conditions at site in 2014 as opposed to being indicative of reduction in seepage losses. Increased instability in the shoreline of the polishing pond was also noted in 2014. Based on the water quality measurements collected since the East Twin Lake access road was breached in 2008, the seepage losses from the polishing pond do not appear to be affecting the overall performance of the West Twin Area reclamation measures. The seepage losses should continue to be monitored and additional seepage control measures should be considered.

Some erosion at the East Twin Diversion Dyke was noted during the inspection. The erosion was likely due to elevated flows during freshet in combination with snow blockage in the channel. The erosion appeared to be similar to what was observed in 2013. In general the dyke remains in stable condition. However, it is recommended that additional re-sloping and armouring of select areas of the channel be undertaken to enhance long term performance. As has been observed during inspections over the past couple of years, seepage was observed at the toe of the East Twin Diversion Dyke in 2014 upstream of the convergence of flow from West Twin. This seepage is likely originating in the remnant polishing pond. There is no concern with respect to the integrity of the dyke due to the presence of this seepage. However, it is recommended that this seepage point be monitored for increased flows in 2015.

Thermal Covers

The thermal covers were generally performing well in 2014 similar to the performance observed in 2013. Only minor erosion, cracking and thermokarsting/settlement were observed, which were not seen to negatively impact the thermal performance nor the water quality in the surrounding areas. Many of these features have been noted previously and have been observed to be physically stable for the past several years. For the most part, the active layer thaw was contained within the thermal cover at most locations. Continued improvement has also been observed in the quality of the surface water runoff from the Surface Cell cover system. This is an indication of the beneficial impacts related to improved geothermal performance of the cover system. Additionally, the water quality observed at the final discharge point for the West Twin Disposal Area has also been observed to remain well below the discharge criteria.

It is anticipated that, over time, the ice saturation will continue to progress within the base of the cover materials. As this occurs, the geothermal performance of the covers will continue to improve, further confining the active layer thaw within the cover. No maintenance is required for the thermal covers at this time; however, visual monitoring is recommended to check for additional surface deformation.

Talik Freeze-back

Freeze-back of the talik in the Surface Cell and Test Cell appears to be proceeding at anticipated rates. Cooling of the entire geothermal profile was observed in most instruments at similar or accelerated rates of cooling compared to recent years.

Most piezometers within the surface cell have frozen back due to continued downward migration of the freezing front. Where the piezometers remain in thawed tailings, the pore pressures continue to increase. The increasing pore pressures are related to continued freeze-back of the tailings. The pore pressures are lowest near the dyke and highest in the centre of the talik. The piezometers in the Test Cell talik have shown pore pressures increasing at a greater rate than in the past. This may be due to constraining of the hydraulic connection between the Test Cell talik and Reservoir due to downward migration of the freezing front.

Mine Openings, Crown Pillars and Raises

The covers constructed over the mine openings generally appear to be physically stable. Cracking at the East Open Pit crown pillar does not appear to have progressed since 2011. No maintenance is recommended, but continued visual inspection is warranted.

Shale and Armour Borrow Areas

In general, the shale and armour borrow areas appear to be physically stable and are not causing any significant ponding to occur. No maintenance was recommended at any of the borrow areas.

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LIMITATIONS

BGC Engineering Inc. (BGC) prepared this document for the account of Nyrstar. The material in it reflects the judgment of BGC staff in light of the information available to BGC at the time of document preparation. Any use which a third party makes of this document or any reliance on decisions to be based on it is the responsibility of such third parties. BGC accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this document.

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1.0 INTRODUCTION

Nanisivik Mine is currently owned by Nyrstar, who obtained the property through its purchase of Breakwater Resources Ltd. (Breakwater) in 2011. Nanisivik Mine is located on the Borden Peninsula on northern Baffin Island, just south of Strathcona Sound, as shown on Drawing 1.

Mining operations at Nanisivik ceased in September 2002. Site operations in 2014 were conducted under Nunavut Water Board License 1AR-NAN0914 (the License), dated April 1, 2009 which entitles CanZinco (the Licensee and a subsidiary of Breakwater Resources Ltd.) to use water and dispose of waste associated with the closure and reclamation activities and post-closure monitoring at the Nanisivik Mine. Part I, Item 5 of the License states the following:

> *"The Licensee shall undertake a geotechnical inspection, to be carried out annually by a Geotechnical Engineer, during the months of July, August or September and reported as set out in Part I, Item 6. The inspection shall be conducted in accordance with the Canadian Dam Safety Guidelines, where applicable and be consistent with the "2008 Annual Geotechnical Inspection" (BGC Engineering Inc., January 30, 2009), taking into account all major earthworks and any changes to the project."*

Additionally, the Nunavut Water Board's Letter of Approval for the Mine Reclamation Plan (NWB 2004) states the following:

> *"In addition to the requirements outlined in Part H, item 6, of the previous License (NWB1NAN0208), the Licensee shall include inspection of all portals, audits, mine openings, ventilation shafts associated with the mine and an examination of the area around the Industrial Complex ice lens."*

Also, the approved Nanisivik Mine Reclamation and Closure Monitoring Plan (Gartner Lee Ltd. [GLL] 2004a) states that the annual geotechnical inspection will include "all earth structures, water diversions, rock slopes and soil covers by a professional engineer." As per the Monitoring Plan, "the engineers report will include a professional review and assessment of all thermal monitoring information and flow information relevant to a physical stability assessment."

In fulfillment of these regulatory requirements, Mr. Johan Skoglund, Group Manager Environment, for Nyrstar, requested that BGC Engineering Inc. (BGC), conduct an inspection visit. The current report provides a summary of the conditions observed and any resulting recommendations and maintenance issues. The report also provides a comprehensive review of geotechnical monitoring data collected on site in 2014. Table 1 provides a list of the structures that were included within the inspection.

A new Water License (1AR-NAN1419) was recently issued by the Nunavut Water Board to Nyrstar which covers site operations between December 23, 2014 and December 22, 2019.

Items within the new Water License affecting future geotechnical monitoring requirements at the Nanisivik mine site are referenced throughout this document, where appropriate.

Table 1. Summary of Inspection Items.

Facility Type	Inspection Item
Embankments and Containment Structures	West Twin Dyke
	Test Cell Dyke
	East Twin Creek Diversion Dyke
	East Adit Treatment Facility
	Day Tank Farm Area
	Main Tank Farm Spill Containment Berm
	West Twin Outlet Channel
Thermal Covers	Surface Cell
	Test Cell
	Toe of Test Cell Dyke
	Toe of West Twin Dyke
	Landfill
	West Open Pit
	East Open Pit
	Oceanview Open Pit
	Area 14
	Upper Dump Pond
	Industrial Complex
Mine Openings, Crown Pillars and Raises	00/01 Portals and crown pillar
	17 N Portal
	Oceanview Portal
	K-Baseline Portal
	Area 14 Portal
	9S Portal
	Former Portal to Mill Foundation
	Lower Adit
	Shale Hill Raise
	Oceanview East and West Raises
	Area 14 Raise
Shale and Armour Borrow Areas	Mt. Fuji Shale Borrow Area
	West Twin Shale Borrow Area
	East Twin Shale Borrow Area
	Area 14 Shale Borrow Area

Facility Type	Inspection Item
	Townsite Shale Borrow Area
	Shale Hill Shale Borrow Area
	Twin Lakes Delta Armour Borrow Area
	Kuhulu Lake Road Borrow Area
	09S/17N Armour Borrow Area
	Chris Creek Armour Borrow Area
Other	Concentrate Storage Shed

All pre-1998 site investigation work, geotechnical design and construction monitoring of the West Twin and East Adit containment structures was carried out by Terratech, a division of SNC Inc. and Mr. Frank Tordon, P.Eng. Initial design work relating to the East Twin diversion dyke and channel, and the dump containment ponds was provided by Kilborn Engineering Ltd. Golder Associates Ltd. (Golder) prepared the annual inspection reports for the waste containment dykes in 1998 and 1999, while BGC has provided the annual inspection reports since 2000. These other reports should be reviewed, in combination with this current report, for the sake of consistency regarding performance and maintenance issues.

2.0 MINE RECLAMATION ACTIVITIES

The Final Closure and Reclamation Plan (FCRP) was submitted to the Nunavut Water Board for review and approval in March 2004. The review process included a technical meeting in Yellowknife in May 2004 and a public hearing in Arctic Bay in June 2004. The Board conveyed its approval of the plan in a letter to Breakwater dated July 6, 2004.

The reclamation of the mine site has been ongoing since August 2004, with the bulk of reclamation completed between 2004 and 2008. Since 2004, the following reclamation activities related to the geotechnical inspection have been completed:

- Permafrost aggradation or "thermal" covers were constructed over tailings in the Surface Cell, Test Cell, toe of the Test Cell Dyke, toe of the West Twin Dyke, and at the Upper Dump Pond.
- Thermal covers were constructed over waste rock in the West Open Pit, East Open Pit, Oceanview Pit, and Area 14 Waste rock pile.
- A thermal cover was constructed over the Nanisivik Landfill.
- A thermal cover was constructed over the Industrial Complex Foundation, which had been backfilled with metals contaminated soil.
- Portal plugs and thermal covers were constructed at the Lower Adit, 09 South, Oceanview, K-Baseline, 17 North, 88 North, 00 and 01 Portals.
- A fill pillar was constructed beneath the 00/01 crown/rib pillar.
- The West Twin Dyke Spillway was constructed to convey water from the Surface Cell to the Reservoir.
- The West Twin Outlet Channel was constructed to control the water level in the Reservoir.
- Additional armour was applied to the face of the East Twin Creek Diversion Berm.
- The East Adit Treatment Facility was decommissioned by breaching each of the dykes.
- The Day Tank Farm was decommissioned by removing the tanks, liner and berms.
- The Lower Dump Pond was decommissioned by removing the berms, tailings and the liner and transporting them to the underground mine workings.
- The Concentrate Storage Shed was dismantled and a surficial cover was constructed over the concrete floor slab.
- The road separating the Polishing Pond and the Reservoir was breached, and the culverts and stop log control structure were removed.
- The Main Tank Farm was demolished and remediation of the hydrocarbon impacted soils from beneath the tank farm is ongoing.

Most of these reclamation measures are summarized in the project completion report (BGC 2009b) and documented in detail in various as-built reports referred to throughout this document. With the exception of completion of the remediation of the hydrocarbon impacted soils from beneath the Main Tank Farm, and some minor maintenance and ongoing monitoring programs, reclamation of the Nanisivik Mine site is essentially complete. Post closure monitoring of the site is on-going and has been conducted in many areas of the site since 2006.

3.0 CLIMATE REVIEW

Climatic data was collected at the Nanisivik Airport by Environment Canada from 1976 to 2010, which is located approximately 10 km south of the West Twin Disposal Area (WTDA) and approximately 250 m higher in elevation. The recorded climate data were previously analyzed in Golder (1998) to provide a basis for deriving the existing climate parameters such as precipitation, air temperatures and lake evaporation at the project site. The following list is a summary of the main climatic parameters based on the data available in 1998:

- The mean annual air temperature (MAAT) was estimated to be -15.2°C.
- The mean annual precipitation total was estimated to be 240 mm.
- The 24 hour Probable Maximum Precipitation (PMP) value was estimated to range from 140 to 210 mm.
- The mean annual lake evaporation value, as measured at the Nanisivik site, was approximately 200 mm.

Climate monitoring was discontinued at the Nanisivik Airport in January 2011. Currently, the nearest available climate monitoring station is located at Arctic Bay, approximately 30 km from Nanisivik. Data collected from Arctic Bay since 2011 is provided on Drawing 2. The MAAT recorded at the Arctic Bay weather station in 2014 was approximately -13.9°C. This is approximately 0.4°C cooler than 2013, and more than 3°C cooler than MAAT values determined for 2011 and 2012. Additionally, the average summer air temperatures (July and August) in 2014 were approximately 1.4°C warmer than those recorded in 2013. This indicates a higher potential for active layer thaw in 2014 compared to 2013. This is further supported by the air thaw index (ATI) value calculated for 2014. The ATI calculated for 2014 was approximately 409 degree Celsius days (°C•d). This value has increased compared to 2013 (312 T °C•d but significantly less than values calculated for 2011 and 2012 (650 and 589 °C•d, respectively).

Due to the difference in elevation and the proximity to the coast line, the climate data sets from Arctic Bay and the historical data collected from the Nanisivik airport station are not directly comparable. In order to put the climate data collected from Arctic Bay since 2011 in historical context, a correction factor is required. Based on the overlap of data sets while monitoring data was collected from both sites in 2010 and 2011, and the data collected on site since 2013, a correction factor of -2°C has been estimated (Nanisivik 2°C cooler than Arctic Bay). This correction factor was applied to the climate data collected from Arctic Bay between 2011 and 2014, as shown on Drawing 2. The results suggest that 2013 and 2014 were both cooler than the long term average (-14.3°C, calculated between 1977 and 2013) in terms of both MAAT and ATI. These two years were also the coolest since reclamation of the site was initiated in 2004.

An assessment of regional air temperatures was conducted to further support the climate assessment. The closest regional weather station is located at Pond Inlet, Nunavut (located 235 km to the east). Pond Inlet experienced similar cool conditions in 2014. The MAAT recorded at Pond Inlet in 2014 was -14.0 °C, which is nominally equivalent to the MAAT in 2013 (-14.1°C) and only 1.1 °C warmer than the published 1970 to 2000 Normal of -15.1 °C.

It was noted by site monitoring staff, Mr. Claude Lavallee, that 2014 was nominally average in terms of precipitation on site compared to recent years. The site observations are supported by rain gauge data collected on site in 2014 by Mr. Lavallee. The rainfall data collected on site since 2010 is summarized in Table 2. As can be seen, the rainfall at site in the summers of the last three years is more than double the rainfall amounts in 2010 and 2011.

Table 2. Nanisivik Site Rain Gauge Data (June to September 2010-2014).

	2010	**2011**	**2012**	**2013**	**2014**
Rainfall (mm)	45.6	37.0	109.8	109.0	97.0

The rainfall measured at site in 2014 was also higher than the published normal at the Nanisivik airport from 1971-2000 (61.5 mm). Along with the elevated rainfall amounts experienced at site in 2014, Mr. Lavallee noted that spring run-off occurred earlier than normal in 2014 compared to most years; continuing a trend he has noticed in the area in recent years. Based on observations made during the site inspection, the early run-off contributed to increased erosion at various areas around the site such as the East Twin Creek Diversion Dyke. These observations of erosion are discussed in greater detail throughout this report.

In summary, climatic conditions at site in 2014 were cooler and wetter than normal. The impacts of these climatic conditions are seen in improved cover performance, continued and accelerated cooling and freeze back of mine wastes and erosion features noted at various hydraulic structures and cover systems around site. Additional discussion on these performance observations are included throughout this report, where appropriate.

4.0 MINE DESCRIPTION

Mining was conducted at Nanisivik Mine between 1976 and 2002. During that time, mining was conducted at four small open pits (West Open Pit, East Open Pit, East Trench, and Oceanview Open Pit) and underground (Main Lens, Area 14, Oceanview, and K-Baseline). The locations of each mining area are illustrated on Drawing 1. The ore was processed at the mill and the tailings were transported to the West Twin Disposal Area (WTDA), located at West Twin Lake.

The WTDA is comprised of an upper, solids retention pond, named the Surface Cell, and a lower, water retention pond, called the Reservoir, as shown on Drawing 3. The Surface Cell and Reservoir are separated by the West Twin Dyke, a frozen-core, rockfill dyke. Prior to construction of the West Twin Dyke, tailings were deposited throughout the original West Twin Lake. After construction of the dyke, tailings were primarily deposited into the Surface Cell. Excess water was then siphoned or pumped from the Surface Cell into the Reservoir from where it was reclaimed for use in the mill. The Reservoir was subsequently further separated by constructing the Test Cell Dyke, creating additional tailings disposal capacity in what was known as the Test Cell.

The Reservoir and a final polishing pond were separated by a rockfill causeway and stop log structure. Water from the polishing pond was then discharged to Twin Lakes Creek at the decant structure located at the outlet from the pond. The decant structure was replaced in 2005 by the West Twin Outlet Channel. In September 2008, the stop log structure was removed and the rockfill causeway was breached allowing the Reservoir and Polishing Pond to behave as a flow-through hydraulic system.

5.0 REVIEW OF 2013 MAINTENANCE RECOMMENDATIONS

The 2013 Annual Geotechnical Inspection Report (BGC 2014) outlined a number of maintenance recommendations. These recommendations, and their status, as observed during the 2014 inspection, are provided in Table 3.

Table 3. Summary and Status of Previous (2013) Recommendations.

Inspection Item	Recommended Action items (2013)	2014 Comments / Actions
West Twin Dyke Spillway	The area should continue to be visually monitored for any signs of erosion or permafrost degradation induced deformation of the side slopes. Review feasibility of methods to reduce quantity of snow in spillway.	Reduction of snow levels in spillway does not appear to be feasible following observations made by site personnel. Based on observations recorded during freshet, erosion along left bank will likely continue to be an issue in the future. Suggest reviewing additional enhancements to armouring along left bank of spillway.
West Twin Outlet Channel	Monitor the water elevation in the Reservoir. Consider seepage control measures such as backfill in the polishing pond upstream of the wall.	Water levels in 2014 remained within an acceptable range during periods of open water. Water levels should continue to be monitored in 2015. Further consideration should be given to seepage control measures in polishing pond to enhance long term performance.
East Twin Creek Diversion Channel	No immediate maintenance required. Additional armouring may be required at some point to address noted erosion. Monitor creek flows during freshet to assess causes of erosion. Monitor seepage discharge point at toe of dyke.	No observable change in left bank erosion occurred in 2014. Additional armouring of left bank should be considered to enhance long term performance of channel. Seepage at toe of dyke should continue to be monitored.
Surface Cell Tailings Cover	Continue to monitor thermokarst areas for additional deformation.	No additional thermokarst features noted in 2014. Pre-existing thermokarst features do not appear to have progressed in 2014. Continue to monitor cover for development of additional thermokarst features and further progression of old ones.

Inspection Item	Recommended Action items (2013)	2014 Comments / Actions
East Open Pit/ East Trench Waste Rock Cover	Continue to visually monitor cracking in EOP crown pillar and minor surficial erosion of EOP and East Trench covers.	No observable progression of crown pillar cracking in 2014. Some additional rill erosion noted on lower slope of cover system. Continue to visually monitor crown pillar for progression of cracking and surface of cover for erosion.
Oceanview Pit Waste Rock Cover	Continue to visually monitor surficial erosion along backslope.	No observable progression of erosion of back slope noted in 2014. Continue to monitor for progression of erosion in 2015.
00/01 Portals and crown pillar	Continue to visually monitor cracking in WOP crown pillar.	No observable progression of crown pillar cracking in 2014.
Instrumentation/ Monitoring	Re-install water level logger in Reservoir during 2014 inspection.	Water level logger was successfully installed in 2014 and removed at the end of open water season in early September. Logger should be re-installed in 2015. Frost gauges require maintenance prior to 2015 monitoring season.

Any recommendations identified in previous inspections that have yet to be addressed, or have only been partially addressed, have been carried forward as recommendations for 2014. More information regarding the 2014 recommendations is provided in Section 6.

6.0 2014 INSPECTION CONDITIONS

Mr. Geoff Claypool, P.Eng., conducted the geotechnical site inspection between July 29 and August 1, 2014. Each of the elements from Table 1 was inspected on foot. Pertinent observations concerning the physical condition of each element were recorded by photograph. Select photos are provided in Appendix I. The following sections review the results of the geotechnical inspection and geotechnical instrumentation monitoring program at the various site facilities.

6.1. Embankments and Containment Structures

Several embankments and containment structures were constructed at Nanisivik Mine throughout its history for various purposes. These include the following:

- West Twin Dyke;
- Test Cell Dyke;
- East Adit Treatment Pond Dyke;
- East Adit Retention Pond Dyke;
- Day Tank Farm Spill Containment Berm; and
- Main Tank Farm Spill Containment Berm.

The following sections provide a summary of the inspection conditions at each of the structures mentioned above. It should be noted that since the West Twin Dyke and Test Cell Dyke have been incorporated into the Surface Cell and Test Cell tailings covers, respectively, the inspection conditions for these structures are reviewed in Section 6.4 (Thermal Covers).

6.1.1. East Adit Treatment Facility Dykes

Construction Details

The East Adit Treatment Facility was located approximately 3 km east of the mill, downslope from the East Adit area, as shown on Drawing 1. The facility was comprised of a Treatment Pond and a Retention Pond, both of which employed earthen dykes to retain surface water flow. Water that flows through this area is runoff water from the surrounding drainage basin, where the water quality is affected and/or impacted by natural mineralized soil and rock.

Both dykes are shale rockfill structures mixed with a combination of one or more of the following: regional talus, glacial till, marine clay and/or bentonitic clay. The Treatment Pond Dyke is approximately 5 m above the surrounding ground surface. The Retention Pond Dyke is approximately 3 m above the surrounding ground surface.

Commitments were made in the 2004 Reclamation and Closure Plan (CanZinco 2004) to restore natural drainage by breaching/removing any man made diversions or catchments. As such, both dykes were breached in late 2006. The breaches are approximately 5 m wide at the base and are sloped back to the remnant crest of the dykes at an angle of approximately 3(H):1(V).

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-1). The main observations made during the inspection are summarized by the following:

- Both the breach of the East Adit Retention Pond and the breach of the East Adit Treatment Pond appear to be free of sediment and able to promote drainage of the ponds as intended.
- As observed during previous inspections, a small remnant pond was observed in the East Adit Retention Pond area which has been present since breaching. The ponding has been reduced in aerial extent over time due to continued natural backfilling of the area by sediments, mostly weathered shale, being transported and deposited in this area from upslope.
- Some channelization of remnant sediments was observed in the bottom of the East Adit Treatment Pond.

As expected, natural sedimentation has steadily been reducing the aerial extent of remnant ponding in the East Adit Retention Pond. As such, additional remedial actions at the East Adit Treatment Facility are not considered necessary.

6.1.2. Day Tank Farm Spill Containment Berm

Construction Details

The Day Tank Farm Spill Containment Berm was located just uphill from the Industrial Complex, as shown on Drawing 1. The berm provided contingency storage for fuels in the event of that the day tanks leak or spill. The berm was approximately 4 m high, at its highest point, and had a crest width of approximately 2 m.

CanZinco (2004) details the reclamation plan for the facility which involves the following:

- The liner is to be removed and disposed of underground according to the Waste Disposal Plan.
- Hydrocarbon contaminated soils are to be relocated to the underground mine, in accordance with the Underground Waste Disposal Plan.
- The areas which are disturbed during reclamation are to be backfilled and/or contoured to their surroundings.

In 2007, the day tank farm was decommissioned. According to site staff, the tanks, liner and berm were removed and transported to the underground mine workings. Site staff also indicated that some hydrocarbon contaminated soil was excavated and was also transported to the underground mine workings.

Inspection Conditions

The main observation made during the inspection was that the area where the day tank farm used to be was well drained and no ponding of surface water and no erosion of surface materials was observed.

No additional maintenance was recommended for this area, in terms of geotechnical requirements. As documented in SRK (2009), all geo-environmental considerations at the Day Tank farm area have been addressed.

6.1.3. Main Tank Farm Spill Containment Berm

Construction Details

The Main Tank Farm Spill Containment Berm was located adjacent to the loading dock at Strathcona Sound, just west of the concentrate storage building (Drawing 1). The purpose of the berm was to provide contingency storage for fuels should the fuel storage tanks leak or spill.

The berm was approximately 5 m high at its highest point and has a crest width of 1-3 m. The side slopes of the berm are approximately 1.5H:1V to 2H:1V.

Decommissioning of the Main Tank farm was initiated in 2010, with the removal of the tanks and removal of the containment berms completed in 2011. A remnant portion of one arm of the perimeter berm remains to aid in surface drainage control. Remediation of the hydrocarbon impacted soils from beneath the Main Tank farm is ongoing under the direction of SRK, the mine's geo-environmental consultant.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-2). The main observations made during the inspection are summarized by the following:

- No ponding of surface water was noted in the former tank farm area. Ponded water was observed in a collection pond excavation downslope of the biopile area.
- The hydrocarbon impacted soils were being placed in lined biopile cells to permit remediation of these soils. The screened cobble sized material was being stockpiled outside of the tank farm footprint, downslope of the remnant perimeter dyke.
- Screened soil was being stockpiled on the "Con Shed" floor prior to placement in biopile. Diversion berms were constructed around this material to divert surface water around the stockpiles.

The remediation of the hydrocarbon impacted soils is being overseen by the project's environmental consultant. Thus, no detailed inspection of these measures was undertaken by BGC.

No additional maintenance was recommended for this area, in terms of geotechnical requirements.

6.2. Water Conveyance Structures

Several water conveyance structures were constructed at Nanisivik Mine, both prior to, and during the reclamation process. These include the following structures:

- West Twin Dyke Spillway

- West Twin Outlet Channel
- East Twin Creek Diversion Berm and Channel.

The following sections provide a summary of the inspection conditions at each of the structures mentioned above.

6.2.1. West Twin Dyke Spillway

Construction Details

The West Twin Dyke Spillway is located at the south end of the Surface Cell, as shown on Drawing 3. The spillway conveys water from the Surface Cell to the Reservoir. The physical details of the spillway are summarized by the following:

- The spillway is approximately 550 m long.
- The bottom of the spillway is approximately 6 m wide.
- The grade of the spillway bottom varies from 0% at the inlet to 7% near the middle of the spillway and 2% at the outlet.
- The bottom of the spillway is founded on rock from the inlet to 100 m down gradient of the access ramp. The remainder of the spillway bottom is comprised of rockfill.
- The side slopes of the spillway vary from near vertical in rock to approximately 3(H):1(V) in soil side slopes.
- Side slopes composed of poor quality rock or soil are armoured with riprap.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-3). The main observations are summarized by the following:

- A small pond was observed at the spillway inlet on the Surface Cell cover. The maximum depth of the pond was estimated to be between 20 and 30 cm. The pond was a similar size to what has been observed in previous years.
- Surface flow seemed to travel further down the spillway than previous years before going subsurface into the rockfill, suggesting that the subgrade may be sanding-up / freezing-back.
- Erosion of the left bank materials had occurred previously during the year (Photo 8), during freshet following breaching of the ice-plug at the spillway entrance. Mr. Lavallee was on-site in the spring when the ice-plug breach occurred. The observations collected during this time provide further insight into the erosive event that occurs annually. Based on his observations collected during the event, the flow following the breaching of the ice-plug becomes focused along the left bank at an elevated level 1 to 2 m above the base of the spillway. Draining of the water that backs up on the Surface Cell cover occurred quickly, over a period of less than 4 hours. Some erosion of the bank armour materials occurred before the flow re-entering the spillway towards the outlet, depositing eroded material in the base of the spillway.

- Some armour material from the left bank was also observed to have raveled into the bottom of the spillway in response to the freshet erosion, partially restricting the width of the spillway base over a 10 m length of the spillway.

While the spillway currently remains effective in directing and confining flow from the Surface Cell to the Reservoir, it is recommended that additional armouring of the left bank be considered to address these types of flow events in the future. Regarding the base of the spillway to ensure the full design width is available for flow should be completed at the same time. Until these issues are addressed, it is recommended that the spillway channel continue to be inspected annually for stability of the side slopes and erosion of the side slopes and channel bottom.

6.2.2. West Twin Lake Outlet Channel

Construction Details

The West Twin Lake Outlet Channel is located in the northeast corner of the WTDA, as shown on Drawing 3. The channel conveys water from the Reservoir into Twin Lakes Creek. The channel replaces the former decant station located in the same area. The main design feature of the channel is a steel-reinforced, concrete wall which provides water retention and elevation control for the water level in the Reservoir. The physical details of the channel are summarized by the following:

- The concrete wall is approximately 17 m long and 0.3 m thick.
- The central portion of the wall where the flow occurs is 7 m wide and has an invert elevation of approximately 370.2 m.
- The wall contains sloping side walls (4(H):1(V)) and the elevation of the top of the side walls is approximately 370.8 m.
- The concrete wall is founded into the underlying bedrock via a steel-reinforced, concrete footing.
- The channel is armoured to approximately 370.8 m elevation, upstream and downstream of the wall.
- A plunge pool is located downstream of the wall to provide energy dissipation during flooding events.

In 2007, a geosynthetic clay liner (GCL) was installed upstream of the concrete wall, as suggested in the 2006 inspection report. This was completed in response to observations of declining water levels upstream of the wall during periods of low inflows. The declining water levels were inferred to suggest seepage losses through the foundation of the wall. As such, the GCL approach was suggested as a way to limit the potential for these seepage losses.

In September 2008, the culverts in the East Twin access road that had previously impeded the flow from the Reservoir to the Polishing Pond were removed and the Reservoir is now considered to be a fully flow-through hydraulic system at the normal water level. Due to the limited depth of the breach, the former East Twin access road restricts flow into the former polishing pond area at lower water levels.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-4). The main observations are summarized by the following:

- During the inspection, water was flowing over the invert of the wall with a flow depth of between 1 and 2 cm.
- Seepage was not observed downstream of the wall as has been observed in previous years. This may indicate that the seepage pathways under the wall remained frozen at the time of the inspection.
- Additional instability was noted along the shoreline of the remnant polishing pond in the form of tension cracks and sinkholes within the pond itself. The instability appeared to be further advanced compared to 2013.

Water level measurements relative to the invert elevation were manually taken by site staff throughout 2014 (Drawing 4). Based on these measurements, the water level upstream of the wall was within approximately 5 cm of the invert elevation between mid-June and late August, though only a limited number of readings were obtained. It is also likely that the water levels dropped below the invert of the wall during the early part of September when lower inflows are typically experienced. The water level upstream of the wall was consistently equal to or above the crest of the outlet wall in 2014, which is also shown on Drawing 4. This is likely reflective of the wet conditions experienced at site in 2014.

The water level logger was re-installed in the Reservoir during the 2014 inspection trip. Between July 31 and September 19, the water levels in the Reservoir were recorded hourly. The logger was removed in early-September after the formation of ice on the Reservoir. The data collected from the logger indicates the following:

- The water level in the Reservoir fluctuated by approximately 10 to 15 cm during the time it was installed.
- Due to limited readings at the outlet wall in 2014, only the late July and early September readings can be compared to the Reservoir water levels. As to be expected, water levels at both the outlet wall and Reservoir are decreasing later in the summer when inflows into the Reservoir are reduced.

It is recommended that the water level logger be re-installed during the 2015 inspection trip to continue to enhance the understanding of the relationship between the water levels in the Reservoir and remnant polishing pond. Weekly readings of the water level at the wall should be undertaken as part of monitoring activities.

The observed seepage losses and stability issues along the edge of the remnant polishing pond are thought to be related to the excavation of sediments from the polishing pond during site reclamation activities in 2005. Excavation of these sediments created a deeper pocket of water upstream of the wall and provided a heat source which likely resulted in thawing of the ground beneath the pond.

Based on the water quality measurements collected since the East Twin Access Road was breached in 2008, the seepage losses through the foundation of the wall do not appear to be influencing the overall performance of the West Twin Area reclamation measures. As suggested by the water level data discussed previously, the impact of seepage losses on the main water level in the Reservoir is minimized by the various breaches creating flow restrictions between the Reservoir and the remnant Polishing Pond. However, the breaches only act to slow the lowering of the water level and, therefore, should not be considered a permanent solution in mitigating seepage losses from the West Twin Area. In light of continuing seepage losses and increasing slope instability noted along the perimeter of the remnant polishing pond, it is recommended that measures to address these concerns be considered. This may involve backfilling of the Polishing Pond immediately upstream of the wall. This backfilling would be intended to provide the following benefits with respect to seepage control and stability:

- Enhance stability of slopes along the perimeter of the pond by buttressing these slopes.
- Lengthen the seepage pathway to reduce impacts of seepage losses on water levels upstream of the wall.
- Promote freeze-back of the foundation beneath the pond which would be expected to result in a corresponding reduction in seepage losses.

Detailed design specifications such as fill materials / volumes and surface grades and water management plans will need to be developed prior to implementation of any mitigation plan.

6.2.3. East Twin Creek Diversion Dyke and Channel

Construction Details

The East Twin Creek Diversion Dyke is located along Twin Lakes Creek between East Twin Lake and the West Twin Outlet Channel, as shown on Drawing 3. The diversion dyke and channel deflect flow from East Twin Lake away from its previous drainage course, which drained directly into West Twin Lake (the Reservoir). The flow from the diversion channel combines with the flow from the Reservoir downstream of the West Twin Outlet Channel.

The diversion dyke is approximately 2 m above the adjacent ground level and is comprised of sand, gravel and cobbles derived from the nearby Twin Lakes sand and gravel deposit. A portion of the dyke was re-graded during reclamation construction to be less susceptible to erosion. Additionally, the re-graded portion of the dyke was armoured with riprap to prevent future erosion from occurring.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-5). During the inspection, the area where erosion of the left bank of the channel was observed during previous inspections was reviewed. The erosion was found to be in similar condition compared to 2013. The observed erosion at this location may be the result of a similar scenario as the erosion due to freshet in the West Twin Dyke Spillway. It appears that the flow in East Twin Creek was elevated near the crest of the diversion dyke by snow blockage and concentrated flow resulted in erosion of the berm as it re-entered the main channel of East Twin Creek. It is recommended that the noted area should be re-sloped and additional amour rock should be applied to enhance long term stability of this area.

It should be noted that a seepage discharge spring was observed at the toe of the left bank of the dyke, as it has been during the inspections undertaken since 2012. The seepage water is likely originating in the polishing pond and is also likely related to excavation of polishing pond sediments during site reclamation. The seepage flow rate has increased compared to previous years. No mitigation efforts to address the observed seepage are recommended at this time, but the discharge point should continue to be visually monitored.

6.3. Thermal Covers

The following sections provide information regarding each of the thermal covers constructed at site. Each section provides details regarding the construction of each cover, a summary of the observations made during the 2014 inspection and reviews the monitoring data collected from each area in 2014. A table documenting the instrumentation monitoring schedule undertaken in 2014 is provided in Appendix II. It should be noted that the scheduled quarterly readings were not attempted in April or December 2014 due to access constraints and limited interpretive value provided by these readings. No additional attempts to collect data are planned prior to July 2015, in-line with the monitoring schedule defined within the new Water License.

An additional note on the interpretation of the geothermal monitoring data; for the purposes of interpretations made in this report, the following assumptions were made:

- In the active layer, ground conditions are considered “thawed” when ground temperatures of 0°C and warmer are observed.
- In the taliks, ground conditions are considered “thawed” when ground temperatures of warmer than -0.5°C are observed. This is to account for freezing point depression effects which have been noted in the tailings talik at the site.

6.3.1. Surface Cell Tailings Cover

Construction Details

A thermal cover was constructed over the tailings in the Surface Cell in 2004 and 2005. The thermal cover consists of a 1 m thick layer (minimum thickness) of granular shale overlain by a 0.25 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. It should be noted that, due to surface grade considerations, significantly increased shale thicknesses (greater than 2 m thick) were applied to approximately 30% of the Surface Cell. Additionally, due to practical construction considerations related to the grain size distribution of the armour materials, the armour layer thickness regularly exceeded 0.25 m. The shale was sourced from the Mt. Fuji and Area 14 borrow areas (Drawing 1). The armour material was sourced from the Twin Lakes Delta deposit (Drawing 1). The Surface Cell tailings cover is drained by a series of swales which convey surface water to the spillway inlet located at the south end of the Surface Cell. The spillway transfers water draining off of the Surface Cell into the Reservoir. It should be noted that surface flow is not regularly observed on the Surface Cell and that the majority of the water flow occurs sub-surface due to the coarse nature of the cover materials.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-6). The main observations are summarized by the following:

- The head pond at the entrance to the spillway at the south end of the cover was observed to be similar in size compared to previous years.
- Similar to 2013, some minor thermokarsting was noted along the south shoreline, along the east edge of the main drainage swale and just north of the E/W trench. This localized thermokarsting is not considered to be negatively impacting the overall performance of the cover system. Additionally, the same thermokarst features have been observed is past inspections and do not appear to be changing with time suggesting they have physically stabilized.
- No erosion of the cover materials was noted, but some minor settlement deformation in the bottom of the main drainage swale was observed closer to the spillway entrance.

No maintenance was recommended for the Surface Cell cover system, though the surface of the cover should continue to be visually inspected for additional deformation.

Monitoring Data

The Surface Cell is instrumented with ten thermistors, seven vibrating wire piezometers, six frost gauges and two monitoring wells. The location of each of these instruments is provided on Drawing 5. Select plots providing the results of the monitoring are provided, for interpretation purposes, on Drawings 6 through 12.

The instrumentation has two main purposes; monitor the depth of the active layer thaw in the cover and monitor the ground temperatures, pore pressures and water quality in the talik during freeze-back.

Drawing 6 provides data from the thermistors installed along the periphery of the Surface Cell talik. The graphs illustrate the following:

- The entire tailings profile along the periphery of the talik appears to be frozen, even at thermistor 03-07 where the tailings profile extends to 25 m depth.
- The tailings profile continues to cool over time. The rate of cooling observed in 2014 appears to be similar to what has been observed in previous years.

Drawing 7 provides data from thermistors installed closer to the middle of the talik. The graphs illustrate the following:

- Generally, the upper 18 to 22 m of the ground profile appears to be frozen, as illustrated by data recorded from Thermistors BGC03-09, BGC03-10 and BGC05-05.

- The rate of cooling of the geothermal profile appears to have accelerated compared to previous years.
- Rate of downward progression of the -0.5°C isotherm continues to advance at similar or increased rates compared to previous years, as illustrated by data from Thermistors BGC03-09, 03-10 and 05-05.

Drawing 8 illustrates the downward progression of the freezing front over time in the Surface Cell. The depth of the -0.5°C isotherm from various thermistors is plotted against the distance from the crest of the West Twin Dyke. As can be seen, the data suggests the -0.5°C isotherm has migrated between 4 and 16 m downwards since the cover system was completed in 2005. The data also illustrates how the thickness of frozen ground increases with proximity to the West Twin Dyke.

Drawing 8 also illustrates that the freeze-back of the Surface Cell talik is generally proceeding quicker than anticipated, compared to the results of the geothermal analysis documented in BGC (2004a). This is illustrated by comparing the observed depth of the -0.2°C isotherm with the freeze-back envelope predicted by the original geothermal modeling. The -0.2°C isotherm was used for this comparison as it was the freezing point depression assumed during the original geothermal modelling completed as part of the talik freeze-back assessment (BGC 2004a).

Drawing 9 provides data collected from the piezometers installed in the Surface Cell talik. The graphs indicate the following:

- All of the piezometers in the surface cell talik have frozen back in response to continued downward migration of the freezing front in the tailings. These piezometers were installed at depths ranging from 15 to 24 m bgs.
- The freezeback of the piezometer tips occurred at temperature ranging from -0.5°C to -1.3°C, and the freezeback temperature appears to decrease with depth.

Drawing 10 illustrates the relationship between piezometer tip depth and the freeze-back temperature of the piezometers in the Surface Cell cover system. The deeper the piezometer tip, the lower the temperature before freeze-back occurred. This relationship is likely related to the increasing metals concentration in the remnant talik as it becomes smaller and cryo-concentration of metals in the pore water occurs. The freezing point of the water becomes greater with a higher metals concentration.

Drawings 11 and 12 provide data collected from thermistors and frost gauges important to monitoring active layer thaw and interpreting overall cover performance. The graphs indicate the following:

- The depth of active layer thaw was generally confined within the cover materials or the very top of the tailings throughout 2014.
- The geothermal performance of the cover in 2014 was improved compared to previous years. While the tailings generally remain frozen throughout the year, the active layer thickness in 2014 was similar to, or slightly increased, compared to 2013. The active layer thickness continued to be thinner than the years prior to 2013 in the Surface Cell.

It should be noted that the frost gauges were installed to provide visual evidence of shallow ground temperature behaviour for monitoring personnel, inspectors, or community residents. As such, they should not be considered precision ground temperature monitoring instruments, as this task is accomplished by the thermistor monitoring network. It is important to note, the data from some of the frost gauges during the 2014 monitoring program was considered unreliable due to the low fluid levels within the frost gauges (as noted for FG-2) and are therefore not included within this report. When monitoring frost gauges, the depth to the boundary between the unfrozen and frozen fluid is recorded. The lowest depth of unfrozen fluid, which translates to soil conditions, recorded throughout the summer is termed the active thaw layer. When the fluid levels are reduced due to leakage true active thaw layer cannot be determined as the measured frozen fluid boundary will be lower than the frozen soil conditions. It is recommended that prior to the 2015 monitoring program, all the active frost gauges are re-filled and re-installed.

The results of water quality testing undertaken on samples collected at the inlet of the West Twin Dyke Spillway are also provided on Drawing 12. As can be seen, the total zinc concentration in the water draining off the Surface Cell continued to be low (less than 0.02 mg/L) in samples collected in 2014. Similar results have been observed since 2010. Additionally, similar reductions in the total sulphate concentrations have also been observed since 2010. The water quality monitoring data suggests that the improved geothermal performance of the Surface Cell cover system observed since 2007 has had beneficial effects on the quality of the surface water runoff.

No water samples were collected from the monitoring wells installed in the Surface Cell (BGC05-11 and BGC05-12) in 2014. These monitoring wells are no longer functioning and water sample collection from them is no longer possible. Considering that the piezometric monitoring results suggest that the talik pore water is hydraulically confined, the water quality in the Surface Cell talik is not expected to impact the water quality in the Reservoir. Thus replacement of the monitoring wells to monitor water quality in the Surface Cell talik is not considered necessary at this time.

6.3.2. West Twin Dyke

Construction Details

The West Twin Dyke is a frozen core, upstream constructed, earth fill dyke that separates the Surface Cell and the Reservoir (Drawing 3). The dyke is constructed of frozen, compacted shale and founded on frozen, settled tailings. The dyke is approximately 14 m high and the downstream face slopes at an angle of approximately 15°. The downstream face of the dyke was surfaced with a 0.25 m thick layer (minimum thickness) of sand, gravel and cobbles sourced from the Twin Lakes Delta.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-7). The main observations made during the inspection are summarized by the following:

- No settlement or cracking of the crest was noted
- The downstream face of the dyke shows no indications of erosion or settlement
- No seepage was noted on the face of the dyke or at the toe of the dyke.

In general, the dyke appeared to be in a satisfactory condition and no maintenance was recommended.

Monitoring Data

The West Twin Dyke is instrumented with five thermocouple cables installed within the dyke, four thermistors and one vibrating wire piezometer installed from the crest of the dyke. The location of each of these instruments is provided on Drawings 5 and 13. Select plots providing the results of the monitoring, for interpretation purposes, are provided on Drawings 14 and 15.

Drawing 14 provides data from Thermistors BGC03-33 and BGC03-34, which provide a continuous geothermal monitoring profile from 5 m below the crest of the dyke to approximately 24 m bgs, approximately 6 m below the base of the dyke. The data indicates the following:

- The entire depth profile continues to cool. The rate of cooling at depths between 7 and 18 m bgs appears to have accelerated in 2014. Although cooling continues below 18 m bgs, it is at a slower rate than observed in 2013.
- The small zone of tailings between 22 and 24 m bgs previously considered to be thawed is now frozen and cooler than -3.0ºC, continuing the cooling trend that has been observed in recent years.

Drawing 15 provides data from additional thermistors installed from the crest of the dyke, as well as one vibrating wire piezometer installed within a thawed zone at depth. The data indicates the following:

- The profile immediately upstream of the dyke is frozen (cooler than -0.5°C) to at least 23 m bgs (approximately 5 m below base of dyke).
- The geothermal profile continues to cool with time.
- At piezometer 05-17, the pore pressures within the small thawed zone at approximately 24.5 m bgs (approximately 6.5 m below the base of the dyke) are approximately 10 m artesian and have increased since 2013. The elevated pore pressures observed at depth remain well below the trigger levels established in BGC (2009c) and are not considered to negatively impact the stability of the West Twin Dyke.
- The piezometric tip temperature suggests a minimum freezing point depression of approximately 0.7°C.

No thermocouple data has been collected in 2014 due to the very erratic nature of the data collected from these instruments in previous years. These instruments are well over 10 years old and are no longer able to provide useful data. Given the current geothermal characterization of the dyke and dyke foundation, replacement of these instruments is not considered necessary. This is in-line with the monitoring schedule included within the new Water License.

6.3.3. Test Cell Area

Construction Details

The Test Cell area consists of the Test Cell, former Test Cell Dyke and Toe of Test Cell Dyke. These areas were discussed separately in previous annual inspection reports but have now been combined into one section for the sake of efficiency.

A thermal cover was constructed over the Test Cell area in 2004 and 2005. The thermal cover consists of a 1 m thick layer (minimum thickness) of granular shale overlain by a 0.25 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. It should be noted that, due to surface grade considerations, significantly increased shale thicknesses (greater than 2 m) were applied to approximately 20% of the Test Cell. Additionally, due to practical construction considerations related to the grain size distribution of the armour materials, the armour layer thickness regularly exceeded 0.25 m. The shale was sourced from the Mt. Fuji, East Twin and Area 14 borrow areas (Drawing 1). The armour material was sourced from the Twin Lakes Delta deposit (Drawing 1). The Test Cell tailings cover is drained by a main swale which conveys surface water directly into the Reservoir.

The Test Cell Dyke is an earth fill dyke that separates the Test Cell and the Reservoir (Drawing 3). The dyke is constructed of frozen, compacted shale and founded on frozen, settled tailings. The dyke was approximately 4 m high. During reclamation, the top 2 m of shale material in the dyke was removed and placed in the Test Cell as cover material. The remnant dyke was surfaced with a layer of armour material, approximately 0.25 m thick. As such, the dyke is now essentially integrated into the Test Cell cover.

A thermal cover was constructed over the tailings at the toe of the Test Cell Dyke in 2005. The thermal cover consists of a 1 m thick (minimal thickness) layer of granular shale overlain by a 0.25 m thick layer (minimal thickness) of armour material comprised of sand, gravel and cobbles. It should be noted that, due to surface grade considerations, shale fill thickness in excess of the 1 m minimum thickness was applied to the tailings beach at the toe of Test Cell Dyke, especially immediately adjacent to the remnant Test Cell dyke. The shale was sourced from the Mt. Fuji and Area 14 borrow areas (Drawing 1). The armour material was sourced from the Twin Lakes Delta deposit (Drawing 1). The riprap was sourced from the dolostone outcrop at the south end of the West Twin Dyke.

Inspection Conditions

Select photos from the inspection of the Test Cell and Test Cell Dyke are provided in Appendix I (Drawing I-8). The main observations are summarized by the following:

- No erosion of the cover was observed.
- No ponding on the cover was observed.

- Similar to previous inspection observations, the north south arm of the Test Cell dyke, and the Test Cell cover just inside of the dyke, remains undulating suggesting some amount of thaw settlement of the cover materials has occurred. The undulating nature of the cover in this area has not changed greatly in the last several years and the cover is overbuilt in this area due to grade requirements. The undulating nature has not negative impacts on cover performance and, as such, this area is not of concern.

Select photos from the inspection of the cover along the toe of the Test Cell Dyke are provided in Appendix I (Drawing I-9). The main observations are summarized by the following:

- No erosion of the cover was observed
- No erosion or disturbance due to ice plucking of the riprap was observed

In general, the tailings cover in the Test Cell area appears to be in satisfactory condition. As such, no maintenance was recommended.

Monitoring Data

The Test Cell area is instrumented with seven thermistors, five vibrating wire piezometers, four frost gauges and two monitoring wells. The location of each of these instruments is provided on Drawing 13. Select plots providing the results of the monitoring are provided on Drawings 16 through 18.

Drawing 16 provides geothermal and piezometric monitoring data collected from the Test Cell in 2014. The data indicates the following:

- The subsurface profile between 1.5 and 15 m depth in the Test Cell at the base of the West Twin Dyke (BGC05-04) is colder than -0.5ºC and is presumed to be frozen. The geothermal profile continues to cool with time.
- As suggested by Thermistor BGC05-29, the subsurface profile just inside the former location of the Test Cell dyke is frozen (colder than -0.5°C) to approximately 22 m bgs.
- The data collected from Thermistor BGC05-19, indicates cooling of the geothermal profile near the centre of the Test Cell talik, especially at depths between 5 and 10 m bgs.
- Thermistor 05-19 also suggests that the centre of the Test Cell talik has frozen back to approximately 12 m bgs.
- The minor amount of warming observed in 2013 in thermistor 05-29 between 20 and 26 m bgs was not observed in 2014. This may be due to movement of groundwater at depth between the Test Cell and the Reservoir.
- The pore water pressures measured at 20 to 25 m bgs in the Test Cell talik generally correspond to a piezometric surface between 1 and 2 m bgs. A small increase in the piezometric elevation (approximately 0.5 m) was observed in 2014, compared to 2013. This is a larger increase than has been seen in many years and may be a result of constraining of the hydraulic connection between the Test Cell talik and the Reservoir due to downward migration of the freezing front.

- The temperature of the unfrozen pore water is generally between -0.3˚C and -0.5˚C and is relatively stable. Minimal cooling of the recorded pore water temperature has been observed since installation in 2005.

Drawing 17 provides data collected from thermistors and frost gauges important to monitoring active layer thaw in the Test Cell and interpreting overall cover performance. The graphs indicate the following:

- The active layer thaw was generally confined within the cover materials throughout 2014.
- The geothermal performance of the cover in 2014 was improved compared to previous years, as indicated by near surface data collected from frost gauge FG8, as well as Thermistor BGC05-04.

Select plots providing the results of the monitoring of the Test Cell Dyke are provided on Drawing 18. The monitoring data indicates the following:

- Data from thermistor BGC03-22 suggests that the dyke and foundation beneath the dyke was frozen to approximately 21 m bgs throughout the year and the geothermal profile continues to cool with time.
- The vibrating wire piezometer (BC05-24) installed approximately 20 m bgs beneath the dyke has frozen back as indicated by the tip temperature below -3.5°C and the corresponding variability in pore pressures. The recorded pore pressure is considered to be localized and associated with the freeze-back of the piezometer tip and is not considered to be representative of pore water pressures throughout the Test Cell talik.

Select plots providing the results of the monitoring of the tailings cover at the toe of the Test Cell Dyke are provided on Drawing 19. The graphs indicate the following:

- The subsurface profile at the toe of the Test Cell Dyke (BGC05-27) between 3 and 20 m bgs is colder than -0.5˚C and is assumed to be frozen. The geothermal profile continues to cool with time. The monitoring data demonstrates that the upper 20 m of the subsurface profile is frozen, despite being along a shoreline which was periodically submerged in water during operations of the tailings disposal area. The fact that permafrost exists at this location, to the extent it does, is considered beneficial to the overall Test Cell talik freeze-back. This is because it exceeds expectations and assumptions made in the contaminant loading model (CanZinco 2004) which assumed a completely thawed tailings profile adjacent to the water.
- The monitoring data from Piezometer BGC05-28 indicates piezometric elevation of approximately 370 m. This is nominally the elevation of the water level in the Reservoir. This suggests that the hydraulic connectivity between the tailings at depth and the Reservoir remains intact. The pore water temperature measured at the piezometer tip has cooled to approximately -0.8°C in 2013. It is likely this piezometer will become frozen in over the next couple of years.
- No data from frost gauges FG7 and FG9 are provided on Drawing 19 due to either low fluid levels or the frost gauge being frozen into the pvc casing, respectively.

No water samples were collected from the monitoring wells in the Test Cell in 2014 due to operational difficulties with the heat trace contained within the wells. These wells are no longer functioning and water samples can no longer be collected from them. Given the positive water quality monitoring results from the WTDA, replacement of these instruments is not considered necessary at this time.

6.3.4. Toe of West Twin Dyke Tailings Cover

Construction Details

A thermal cover was constructed over the tailings at the toe of the West Twin Dyke in 2005. The thermal cover consists of a 1 m thick layer (minimum thickness) of granular shale overlain by a 0.25 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Mt. Fuji and Area 14 borrow areas (Drawing 1). The armour material was sourced from the Twin Lakes Delta deposit (Drawing 1). The riprap was sourced from the dolostone outcrop at the south end of the West Twin Dyke.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-10). The main observations are summarized by the following:

- No erosion of the cover was observed.
- As observed during previous inspections, some minor thermokarsting was observed, but was not seen to be negatively impacting the overall performance of the cover system. No visual changes in the appearance of these thermokarst features have been noted in recent years, suggesting the area is generally physically stable.
- No erosion or ice plucking of the riprap along the shoreline was observed.

No additional maintenance items were recommended.

Monitoring Data

The tailings cover at the toe of the West Twin Dyke is instrumented with two thermistors. The location of each of these instruments is provided on Drawing 13. Select plots providing the results of the monitoring are provided on Drawing 20. The graphs indicate the following:

- As illustrated by the data collected from Thermistor BGC05-26, the subsurface profile below 4 m is colder than -6°C and is presumed to be frozen to at least 24 m bgs, the depth of the bottom of the instrument. The bottom 15 m of the instrument is measuring ground temperatures in the bedrock, demonstrating the frozen nature of the bedrock near the base of the West Twin Dyke.
- As illustrated by the data collected from Thermistor BGC03-19, the subsurface profile below 3 m is colder than -3°C and is presumed to be frozen to at least 11 m bgs, the depth of the bottom of the instrument.

West Twin Disposal Area Water Quality

As required in the previous Water License used for site operations in 2014, water quality sampling and testing were undertaken at the West Twin Outlet Channel throughout the open water period in 2014. This channel is considered the final discharge point for water from the WTDA before entering the environment in Twin Lakes Creek. Samples were collected bi- weekly and subsequently forwarded to a laboratory for a variety of tests. Water samples were tested for pH, conductivity, Total Suspended Solids (TSS), sulphate, total metal concentrations, and ammonia (NH_3). The total zinc, total lead, total cadmium and sulphate concentrations observed at the West Twin Outlet Channel throughout 2014 are illustrated on Drawing 21. As can be seen, the total zinc, lead and cadmium concentrations observed throughout 2013 met discharge criteria, as they have since the covers were completed in 2005. Zinc concentrations recorded in 2014 are near the lowest, behind 2013, in terms of peak and average concentration since the covers were completed. Sulphate concentration were higher than in 2013 but lower than the years previous. The low and reducing metals and sulphate concentrations suggest that the thermal covers, and the water cover in the Reservoir, are effective in limiting metal loading to the water in the Reservoir.

It should be noted that the TSS values obtained from the samples collected throughout 2014 were slightly lower than 2013 and similar to what was observed in the three years prior. The last five years have all been well below discharge criteria and well within the historical range. These TSS values generally indicate an improvement over values recorded prior to breaching of the East Twin access road. Hence, the results suggest that excessive amounts of solids are not being carried in the outflow from the WTDA.

Stantec (2015) should be referenced for a detailed review of the results of the water quality monitoring program undertaken in 2014.

6.3.5. Landfill Cover

Construction Details

A thermal cover was constructed over the Nanisivik Landfill in 2005. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.25 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Townsite Shale borrow area. The armour material was sourced from the Twin Lakes Delta deposit. The face of the landfill is sloped at approximately 3(H):1(V).

It should be noted that the thermal cover constructed at the Landfill was built over an existing surficial shale cover that was continuously placed and maintained during operations as part of the on-going landfill operating procedures (for pest control and protection against wind transport). No attempts were made during construction of the thermal cover in 2005 to verify the existing cover thickness and thereby, reduce the fill requirements. Therefore, it can be assumed that the thermal cover is thicker than just the material placed in 2005.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-11). The main observations are summarized by the following:

- No erosion of the cover was observed, either on the upper flatter portion of the cover or on the sloping face of the Landfill.
- No seepage was observed at the toe of cover during the time of the inspection.
- Some exposed shale was observed on the lower portion of the west face. This is due to an insufficient amount of armour material available for covering and is not related to erosion. Previous attempts undertaken to spread the armour over the exposed shale were determined to be impractical from a construction standpoint. Considering the observed stability of the area since construction, additional maintenance is not considered necessary.
- Some minor cracking and thermokarsting was observed along the upslope water deflection berm. This has resulted in a minor amount of surface water running along the west edge of the cover system. This is not considered to be negatively affecting the performance of the adjacent landfill cover system and no maintenance is considered necessary at this time.
- No areas of settlement or thermokarst features were observed on the surface of the landfill cover system.

No maintenance items were recommended.

Monitoring Data

The Landfill cover is instrumented with one thermistor and one frost gauge. The location of each of these instruments is provided on Drawing 22. Select plots providing the results of the monitoring are provided on Drawing 23. The graphs indicate the following:

- The landfill debris underlying the cover remained frozen throughout the year.
- The geothermal profile within the underlying landfill debris continues to cool over time.
- The active layer thaw did not penetrate into the underlying waste material throughout 2014.
- The geothermal performance of the landfill cover in 2014 was observed to be slightly improved compared to previous years. This is inferred from both the frost gauge and thermistor data provided on Drawing 23. The frost gauge data indicates the active layer thickness in 2014 reached a maximum depth of approximately 1.4 m bgs. Additionally, the maximum temperature recorded at the thermistor node located at 2.3 m bgs, near the cover/landfill debris interface, was approximately -2.5°C.

6.3.6. West Open Pit Waste Rock Cover

Construction Details

A thermal cover was constructed over the West Open Pit in 2006. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Townsite Shale borrow area. The armour material was sourced from the local borrow source located along the road between the 09S portal and the 17 North Portal. The face of the West Open Pit cover is sloped at a maximum angle of approximately 3(H):1(V).

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-12). The main observations are summarized by the following:

- No erosion of the armour surface was noted.
- No areas of settlement or thermokarst features were observed during the 2014 inspection.
- No seepage water was observed at the toe of the cover.
- Similar to previous inspection observations, a small crack (1-3 cm wide) was observed in the crown pillar area. The size of the crack is consistent with previous years.

No additional maintenance items are recommended.

Monitoring Data

The West Open Pit cover is instrumented with one thermistor (BGC08-01). The purpose of the thermistor is to monitor the freeze-back of the underlying backfill materials as well as the active layer thaw within the cover materials. The location of this instrument is provided on Drawing 24. The results of the monitoring undertaken in 2014 are provided on Drawing 25. The graphs indicate the following:

- The waste rock used to backfill the open pit has frozen back.
- The active layer thaw was confined within the cover materials throughout 2014. This is improved from the cover performance observed in, and previous to, 2013.
- The geothermal profile within the waste rock observed in 2014 was similar to what was observed in 2013.

6.3.7. East Open Pit Waste Rock Cover

Construction Details

A thermal cover was constructed over the East Open Pit in 2005. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Shale Hill shale borrow area. The armour material was sourced from the local borrow source located along the road between the K-Baseline Portal and Kuhulu Lake. The face of the East Open Pit cover is sloped at a maximum angle of approximately 3(H):1(V). A remnant highwall exists along the south edge of the pit area. The remnant highwall ranges from 1 to 5 m high and is sloped back at an angle of approximately 60°. A bench exists between a portion of the remnant highwall and the main portion of the cover system.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-13). The main observations are summarized by the following:

- Some surface (rill) erosion was noted on the surface of the cover. The erosion appears to have progressed a minor amount over the past several years, especially on the lower portion of the slope.
- Some minor cracking was noted on the surface of the cover. This cracking is thought to be related to thermal expansion and contraction of the cover materials and settlement of the underlying backfill materials.
- No areas of settlement or thermokarst features were observed on the surface of the cover.

The minor erosion noted on the surface of the cover was anticipated. The armour material at this location contains a fine grained fraction that was expected to wash away with time leaving behind the coarse grained fraction, a process known as self-armouring. As such, the armour thickness at this location, and many others where alternative armour materials were used, was increased to 0.35 m. The coarse grained fraction of the armour material is expected to limit the amount of erosion that can occur, without negatively impacting the geothermal performance of the cover.

The cracking in the crown pillar at the East Open Pit was not observed to have progressed since 2013. As previously noted, the cracking coincides with the mined out rib pillar and the area of the 39 Portal. As in previous years, no opening to the underground was observed during the inspection. As such, the cracking was not seen to be of immediate concern especially given the lack of progression over the past several years. Accordingly, no maintenance was recommended, but the crown pillar should continue to be visually monitored for additional deformation.

Monitoring Data

The East Open Pit Waste Rock cover is instrumented with two thermistors and two frost gauges. The location of each of these instruments is provided on Drawing 26. Select plots providing the results of the monitoring are provided on Drawing 27. The graphs indicate the following:

- Data from Thermistor BGC05-02, installed in the area with a thin layer of waste rock backfill (approximately 3 m thick), indicates the waste rock has frozen back and continues to cool with time at a similar rate as observed in previous years.
- Data from Thermistor BGC05-03, installed in the area with a thick layer of waste rock backfill (approximately 9 m thick), indicates the waste rock, and hydrocarbon contaminated soil buried at depth, has frozen back. The geothermal profile continues to cool at a comparable rate to last year.
- The active layer monitoring data from Thermistor BGC05-03 and BGC05-02 indicates that the active layer was confined within the cover materials throughout 2014. The

thermal performance of the cover observed in 2014 was similar to the performance in 2012 and 2013, with approximately 1.2 m of active layer thaw.

No data was collected from the two frost gauges in 2014 due to the gauges being frozen into the pvc casings.

6.3.8. East Trench Waste Rock Cover

Construction Details

A thermal cover was constructed over the East Trench in 2005. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Shale Hill shale borrow area. The armour material was sourced from the local borrow source located along the road between the K-Baseline Portal and Kuhulu Lake. The face of the East Trench cover is sloped at an angle ranging between 4(H):1(V) and 6(H):1(V).

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-14). The main observations are summarized by the following:

- As noted in previous inspections, minor surface erosion was observed near the upslope edge of the cover where seepage water periodically discharges from natural rock outcrops onto the surface of the cover. No progression of this erosion was noted in 2014.
- No seepage was observed at the toe of the cover.

In general, the East Trench cover appears to be in satisfactory condition. As such, no maintenance was recommended.

6.3.9. Oceanview Open Pit Waste Rock Cover

Construction Details

A thermal cover was constructed over the Oceanview Open Pit in 2005. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Shale Hill borrow area (Drawing 1). The armour material was sourced from the Chris Creek “A” borrow area (Drawing 1). The face of the Oceanview Open Pit cover is sloped at a maximum angle of approximately 3(H):1(V). The surface of the cover in the bottom of the pit slopes to the north at a grade of approximately 3%.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-15). The main observations are summarized by the following:

- No ponded water was observed on the surface of the cover.
- The upslope water deflection berm appeared to be effective in directing surface water away from the cover. This was inferred from visual observations indicating surface water flow along the upslope edge of the berm.
- Minor surface erosion was noted along the backslope at the south end of the cover, as has been noted during previous inspections. The erosion appears to be slowly progressing, based on visual observations, but is not expected to impact the overall performance of the cover system. Self armouring of the erosion areas is occurring as the coarser particles within the armour layer are exposed and concentrated at surface, as anticipated.
- Some seepage was noted at the toe of the cover. Seepage has been noted in this area previously and its likely source is infiltration water that is running along the ice saturated base of the cover before existing at the toe.
- Some acid rock drainage staining was observed on the east edge of the cover. The stained area appeared to originate upslope of the extent of the cover and appeared to cover a similar extent to what has been observed in previous inspections.
- A minor sinkhole was observed in the middle of the cover surface.

In general, the Oceanview Open Pit cover appears to be in satisfactory condition. As such, no maintenance was recommended. The surficial erosion should continue to be visually monitored for additional progression.

As discussed previously, minor erosion of the surface of the cover was anticipated. Similar to the East Open Pit cover, the armour material at this location contains a fine grained fraction that was expected to wash away with time. As such, the armour thickness was increased to 0.35 m. The coarse grained fraction of the armour material, which can be observed in photos on Drawing I-15, is expected to limit the amount of erosion that can occur, without negatively affecting the geothermal performance of the cover.

Monitoring Data

The Oceanview Pit Waste Rock cover is instrumented with one thermistor and one frost gauge. The location of each of these instruments is provided on Drawing 28. Select plots providing the results of the monitoring are provided on Drawing 29. The graphs indicate the following:

- The active layer was confined within the cover materials throughout 2014.
- The waste rock underlying the cover has frozen back and remained completely frozen throughout 2014.
- The geothermal profile within the waste rock, between 5 and 12 m, shows increased rate of cooling compared to the past 3 years.
- Based on the thermistor data collected from BGC05-01, the geothermal performance of the cover in 2014 was slightly improved to the performance observed since 2010. The maximum active layer thaw depth indicated by Thermistor BGC05-01 was estimated to be approximately 1.4 m bgs, which is approximately 0.9 m above the base of the cover. Frost gauge FG16 indicates an active layer thaw depth of 1.1 m bgs.

6.3.10. Area 14 Waste Rock Cover

Construction Details

Area 14 is a satellite ore body situated on a west-facing slope, approximately 1 km to the east of East Twin Lake (Drawing 1). A waste rock pile was created outside the portal during mining operations. In 1988, the waste rock pile was flattened and a thermal cover was constructed over the top of the waste rock pile, but the face was left exposed.

In 2005, the thermal cover at the Area 14 waste rock pile was completed by constructing a cover over the exposed face and armouring the entire surface of the cover. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer (minimum thickness) of armour material comprised of sand, gravel and cobbles. All shale was sourced from the Area 14 shale borrow area (Drawing 1). All armouring material was sourced from the hill side immediately north of the Area 14 portal.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-16). The main observations are summarized by the following:

- No erosion of the cover materials was observed.
- No areas of settlement or thermokarst features were observed on the surface of the cover.
- No seepage was noted at the toe of the cover.

In general, the Area 14 waste rock cover appears to be in satisfactory condition. As such, no maintenance was recommended.

Monitoring Data

The Area 14 Waste Rock cover is instrumented with one thermocouple and one frost gauge; locations for each of these instruments are provided on Drawing 30. Select plots providing the results of the monitoring are provided on Drawing 31. The graphs indicate the following:

- The active layer thaw was limited to the upper 1.2 m of the cover and did not penetrate into the underlying waste rock in 2014.
- There is uncertainty in the geothermal performance of the cover in 2014 compared to 2013. The 2014 cover performance may be slightly reduced compared to 2013, but the larger active layer thaw readings may be caused by low fluid levels in the frost gauge. In either case, 2014 was still improved in comparison to the performance measured in the years prior to 2013.

No thermocouple data was collected in 2014 from the Area 14 Waste Rock cover instrument. Reading of this instrument has been discontinued due to the erratic nature of the results.

6.3.11. Upper Dump Pond Tailings Cover

Construction Details

The upper dump pond was an emergency tailings storage containment cell located between the water tank and the West Twin Disposal Area, as shown on Drawing 1. A thermal cover was constructed over the tailings in the Upper Dump Pond in 2005. The thermal cover consists of a 2 m thick (minimal thickness) layer of granular shale overlain by a 0.25 m thick layer (minimal thickness) of armour material comprised of sand, gravel and cobbles. The shale was sourced from the Townsite shale borrow area (Drawing 1). The armour material was sourced from the Twin Lakes Delta deposit (Drawing 1).

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-17). The main observations from the inspection are summarized by the following:

- No erosion of the surface of the cover was observed.
- No seepage was noted at the toe of the cover.
- No areas of settlement or thermokarst features were observed on the surface of the cover.

In general, the Upper Dump Pond tailings cover appears to be in a satisfactory state. As such, no maintenance was recommended.

Monitoring Data

The Upper Dump Pond tailings cover is instrumented with one frost gauge. The location of this instrument is provided on Drawing 32. Due to low fluid level in the Upper Dump Pond frost gauge in 2014, the data collected in 2014 is not representative of cover performance and has not been included. Maintenance of the frost gauge is recommended for 2015.

Since the performance of the cover is based on the maximum thaw depths observed, it is important that the frost gauge in Upper Dump Pond be topped up with fluid for accurate results.

6.3.12. Industrial Complex Foundation Cover

Construction Details

The Industrial Complex is located approximately 1 km north of the town site (Drawing 1). The Industrial Complex housed the concentrator, DMS circuit, power plant, maintenance shops, warehouse, administration and technical offices and associated facilities. The facility was built on bedrock and a reinforced concrete foundation.

The Industrial Complex was dismantled between 2005 and 2006 and the remnant foundation was backfilled with metals contaminated soils. In 2008, a thermal cover was constructed over the backfilled foundation. The thermal cover consists of a 2 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer (minimum thickness) of armour material. The shale was sourced from the Mill Area deposit and the armour materials were locally sourced. The sloping face of the cover is approximately 3(H):1(V).

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-18). The main observations made during the inspection are summarized by the following:

- No erosion of the surface of the cover was observed.
- No settlement or thermokarst features were observed on the surface of the cover.
- No seepage was observed at the toe of the cover.

In general, the Industrial Complex cover appears to be in satisfactory condition. As such, no maintenance was recommended.

Monitoring Data

The Industrial Complex Foundation cover is instrumented with one thermistor. The location of this instrument is provided on Drawing 33. Select plots providing the results of the monitoring are provided on Drawing 34. The graph indicates the following:

- The metals contaminated soils used to backfill the foundation footprint have frozen back.
- The active layer thaw (approximately 2 m) was confined within the cover materials throughout 2014 and the cover performance appears to have improved over time.
- The geothermal profile beneath the cover appears to be steady, or slightly warmer, compared to 2013. In comparison to the years prior to 2013, the geothermal profile continues to be cooler.

6.4. Mine Openings

6.4.1. 00/01 Portals and Crown Pillar

Construction Details

00 Portal

The 00 Portal is located at the edge of the West Open Pit at the western end of the mine, as illustrated on Drawing 1. The 00 Portal was the principal access at the western extremity of the mine. The portal measured approximately 5 by 5 m in cross section. The brow immediately above the portal was approximately 4 to 5 m high.

In 2005, the portal was backfilled with waste rock. The backfill was placed approximately to within 1 m of the top of the portal and into the portal approximately 5 m from the entrance. The backfill extended outside the portal face and was sloped at an angle of approximately 3(H):1(V) and graded into the overall backfill of the West Open Pit. A thermal cover was subsequently constructed over the waste rock as part of the West Open Pit thermal cover.

01 Portal

The 01 Portal was located at the western end of the mine, as shown on Drawing 1. The 01 Portal housed the main ventilation fans during mining operations which were mounted in a plate steel bulkhead. The portal opening was approximately 22 m wide and 4 m high. The brow immediately above the portal was approximately 4 to 5 m high.

In 2005, the portal was backfilled with waste rock. The backfill was placed approximately to within 1 m of the top of the portal and into the portal up to the face of the bulkhead. The backfill extended outside the portal face and was sloped at an angle of approximately 3(H):1(V) and graded into the overall backfill of the West Open Pit. A thermal cover was subsequently constructed over the waste rock as part of the West Open Pit thermal cover.

00/01 Crown Pillar

A stope connecting the 00 and 01 portals was located approximately 5 m behind the West Open Pit highwall. A portion of the rock between the 00 and 01 portals, known as the 00/01 rib pillar, was removed during later stages of mine development as part of the pillar recovery program. After the recovery of a portion of the rib pillar, a crack developed in the crown pillar above the area that had been mined. In 2005, a fill pillar was constructed beneath the cracked portion of the crown pillar to provide additional support and prevent development of an opening into the mine workings if the pillar were to collapse. The pillar was constructed out of waste rock and was constructed approximately to within 1 m of the top of the opening.

Inspection Conditions

The area of the West Open Pit cover where the portals had existed was inspected in July 2014. The observations are summarized by the following:

- No indications of surface deformation were observed.
- No indications of seepage from the mine workings were observed.

No maintenance of the portal plugs or the cover over the portal plugs was recommended.

The crown pillar was also inspected from surface. The observations are summarized by the following:

- No additional cracking was observed.
- Based on visual observations, the existing cracking did not seem to dilate further since 2013.
- No visually distinguishable deformation was observed in the crown pillar.
- Overall, no significant changes have been observed in the West Open Pit crown pillar since the fill pillar was constructed in 2005.

In general, the portal plugs constructed for the 00 and 01 portals and the 00/01 crown pillar appear to be in satisfactory condition. It is recommended that the crown pillar area continue to be visually monitored to check for further propagation of the cracking.

6.4.2. 17 North Portal

Construction Details

The 17 North Portal was a culverted portal giving access to the Main Ore Zone. The location of the portal is illustrated on Drawing 1. The 17 North Decline was approximately 5 by 5 m in cross section and the culvert was half round with a diameter of 5 m and a length of 28 m. The culvert was supported by a 0.25 m thick by 2 m high concrete wall on either side and it extended 5 m inside the dolostone bedrock of the drift.

In 2005, the culvert was removed and the portal was backfilled with granular shale derived from the Shale Hill borrow area. The backfill was placed to within 1 m of the top of the portal and extended into the portal for approximately 4 m. The backfill extended outside the portal face and was sloped at an angle ranging between 4(H):1(V) and 7H:1V and graded into the surrounding topography. A 0.35 m thick layer of armour material, derived from the Kuhulu Lake borrow area, was then applied to the surface of the shale backfill.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-19). The main observations are summarized by the following:

- No erosion of the surface of the cover was observed.
- No seepage was observed at the toe of the cover.

In general, the 17 North Portal cover appears to be physically stable and in satisfactory condition. As such, no maintenance was recommended.

6.4.3. Oceanview Portal

Construction Details

The Oceanview Portal was a bare rock entrance into the north side of the Oceanview underground workings. The location of the portal is illustrated on Drawing 1. The Oceanview decline had a cross section of approximately 5 by 5 m. The brow of the portal was approximately 5 m in height.

Prior to 2004, the portal had been backfilled with waste rock and covered over with locally derived overburden materials. In 2005, a thermal cover was constructed over the existing portal plug. The cover consisted of a 2.0 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer of armour material consisting of sand, gravel and cobbles. The shale was derived from the Shale Hill borrow area. The armour material was derived from the Chris Creek “A” deposit. The thermal cover was extended to the east and north of the portal entrance in an old ore stockpile area. Additionally, a water deflection berm was constructed upslope of the cover extension to prevent concentrated surface water from flowing over the cover area.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-20). The main observations from the inspection are summarized by the following:

- A small area of surficial settlement was observed near the southwest corner of the portal cover. The settlement area appears to have stabilized as it has not changed significantly since it was first observed in 2006.
- Some minor cracking was noted along the east edge of the portal cover, as has been noticed in previous inspections. The condition of the cracking does not appear to have changed in several years.
- No seepage was observed at the toe of the cover.
- The surface water deflection berm appeared to be effective in diverting water around the cover extension.

In general, the Oceanview Portal cover appears to be physically stable and in satisfactory condition. As such, no maintenance was recommended.

6.4.4. K-Baseline Portal

Construction Details

The K-Baseline portal was a culverted entry used to access the K-Baseline ore body. The location of the portal is illustrated on Drawing 1. The K-Baseline decline was approximately 5 m by 5 m in cross section and the culvert was half round with a diameter of 5 m and a length of 28 m. The culvert was supported by two concrete pony walls, 1 m wide by 2.4 m high, on both sides. The concrete pony walls extended 3 m inside the dolostone bedrock of the drift.

In 2004, the culvert was removed and the portal was backfilled with waste rock. In 2005, a thermal cover was constructed over the waste rock portal plug. The cover consisted of a 2.0 m thick layer (minimum thickness) of granular shale overlain by a 0.35 m thick layer of armour material consisting of sand, gravel and cobbles. The shale was derived from the Shale Hill borrow area. The armour material was derived from the Chris Creek “A” deposit. An additional thermal cover was constructed over the area below the road immediately outside the portal where mineralized soils and additional waste rock were located. A surficial cover of shale was also constructed adjacent to the thermal cover to improve drainage conditions.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-21). The main observations are summarized by the following:

- No erosion of the cover was observed.
- No seepage was noted at the toe of the cover.
- No areas of significant settlement were observed in the cover.
- Some minor cracking was noted on the surface of the cover, likely related to thermal expansion and contraction of the cover materials.

In general, the K-Baseline portal appears to be in satisfactory condition. As such, no maintenance was recommended.

6.4.5. Area 14 Portal

Construction Details

The Area 14 Portal was a bare rock portal that provided access to the Area 14 underground workings. The location of the portal is illustrated on Drawing 1. Mining ceased in this area around 1987 and the portal was backfilled with waste. The waste was covered and contoured with shale in 1987 and 1988.

In 2005, locally derived armour material was stockpiled adjacent to the portal cover for construction of the final armour layer. In late 2006, the stockpiled armour materials were spread over the portal area.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-22). The main observations are summarized by the following:

- No erosion of the cover was observed.
- No seepage was noted at the toe of the cover.

In general, the Area 14 Portal cover appears to be physically stable and in satisfactory condition. As such, no maintenance was recommended.

6.4.6. 09 South Portal

Construction Details

The 09 South Portal is located at the western end of the mine, as shown on Drawing 1. The 09 South Portal is a culverted entry giving access to the Main Ore Zone. The 09 South drift is approximately 5 m by 5 m in cross section. The culvert is round with a diameter of 5 m and a length of 28 m. The bottom of the culvert is filled with rockfill to provide a smooth floor. The culvert extends approximately 13 m inside the shale bedrock of 09 south drift, leaving 15 m exposed on surface, a portion of which is covered with talus from the slope above.

The 09 South Portal remained open until the fall of 2008 to provide access to the underground workings during the underground waste disposal program. Once the underground waste disposal program was complete in September 2008, work on permanent closure of the 09 South Portal began.

In 2008, the culvert was removed and the portal was backfilled with granular shale derived from the Mill Area deposit. The backfill was placed to within 0.5 m of the top of the portal and extended into the portal for approximately 5 m. The backfill extended outside the portal face and was sloped at an angle of approximately 4(H):1(V) and graded into the surrounding topography. A 0.25 m (minimum thickness) layer of locally derived armour material was then applied to the surface of the shale backfill.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-23). The main observations are summarized by the following:

- No erosion of the cover was observed.
- No seepage was noted at the toe of the cover.
- Some minor cracking and deformation of the cover was noted, in a similar condition as was observed in 2012 and 2013. The area appears to have stabilized since the cracking was first noted in 2009.
- Some shale from rock outcrops upslope was deposited on the surface of the portal cover.

In general, the 09 South Portal cover appears to be physically stable and in satisfactory condition. As such, no maintenance was recommended.

6.4.7. Lower Adit

Construction Details

The Lower Adit is located at the western end of the mine near the Industrial Complex, as illustrated on Drawing 1. The Lower Adit provided the main access into the underground crusher and fine ore bin, as well as secondary access to the Main Ore Zone.

The Lower Adit Portal remained open until the fall of 2008 to provide ventilation to the underground workings during the underground waste disposal program. Once the underground waste disposal program was complete in September 2008, work on permanent closure of the Lower Adit Portal began.

In 2008, the portal was backfilled with granular shale derived from the Mill Area deposit. The backfill was placed to within 0.5 m of the top of the portal and extended into the portal for approximately 5 m. The backfill extended outside the portal face and was sloped at an angle of approximately 4(H):1(V) and graded into the surrounding topography. A 0.25 m (minimum thickness) layer of locally derived armour material was then applied to the surface of the shale backfill.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-24). The main observations are summarized by the following:

- No erosion of the cover was observed.
- No seepage was noted at the toe of the cover.
- No cracking or deformation of the cover was noted.

In general, the Lower Adit portal plug appears to be in satisfactory condition. As such, no maintenance was recommended.

6.4.8. Portal to Mill Foundation

Construction Details

A portal was driven beneath the Mill building in 1980 as part of emergency repair operations for the Mill. The portal is located approximately 70 m south of the Mill, adjacent to Twin Lakes Creek. The portal provided access to an ice lens which was present beneath the mill building. As part of the repair operations, the ice lens was mined out and concrete pillars were constructed to provide additional support for the Mill. After the repair operations were complete, the portal was plugged with shale rock fill.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-25). The main inspection observations are summarized by the following:

- No erosion of the armoured portal plug was observed.
- No seepage was noted at the toe of the portal plug.

In general, the portal plug appears to be physically stable. No additional maintenance was recommended.

6.4.9. Shale Hill Raise

Construction Details

The Shale Hill Raise provided ventilation for the underground workings in the Shale Hill area. The location of the raise (E582524, N8107427, UTM NAD 83 Zone 16) is illustrated on Drawing 1. The 3 m diameter raise was approximately 47 m deep. During mining operations, the raise was sealed with a 3 m diameter steel tank with the bottom cut out and with two adaptors in the top for 36-inch ventilation fans. The tank was fixed to a cemented collar at the top of the raise.

Prior to 2005, the surface structure was removed and, according to mine site records, the Shale Hill raise was backfilled with waste rock. A mound of shale was constructed at surface with side slopes of approximately 3(H):1(V). In 2005, a 0.35 m thick layer of locally derived armour material was applied to the surface of the mound to complete the remediation of this raise.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-26). During the inspection, the armour surface appeared to be stable with no visual indications of erosion or surface deformation. As such, no maintenance was recommended.

6.4.10. Oceanview East Raise

Construction Details

The Oceanview East Raise was situated at the extreme east end of the Oceanview underground workings. The location of the raise (E585123, N8107506, UTM NAD 83 Zone 16) is illustrated on Drawing 1. The 4 by 4 m raise was approximately 10 m deep and provided ventilation for the underground workings in the Oceanview area. During mining operations, the raise was covered with a wooden wind deflector with a locked door.

In 2002, the wooden deflector was removed and, according to mine site records, the raise was backfilled with waste rock. During backfilling, it was noted that an ice plug was present in the raise at a depth of approximately 1.5 m below ground surface. As such, waste rock was placed only to this depth. A 3 m high mound was placed on top of the raise to accommodate for possible future settlement of the ice plug. The mound was constructed of shale and surfaced with coarse rock.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-26). During the inspection, the armour surface appeared to be stable with no visual indications of erosion or surface deformation. As such, no maintenance was recommended.

It was noted that acid rock drainage water emanating from groundwater seepage upslope of the raise has caused staining of the ground surface around the raise (Photo 74?). The source of the ARD is not known, but is likely related to near surface exposure of sulphidic soils and/or bedrock. This water is collecting in a disturbed area downslope of the raise. This low spot could be backfilled to prevent collection of this water.

6.4.11. Oceanview West Raise

Construction Details

The Oceanview West raise was located near the west end of the Oceanview underground workings (E584851, N8107466, UTM NAD 83 Zone 16), as shown on Drawing 1. The 3 m diameter raise is approximately 26 m deep and provided ventilation for the underground workings. The raise was covered by a steel enclosure with a locked wooden cover.

In 2002, the steel enclosure was removed and, according to mine site records, the raise was backfilled with waste rock as part of the progressive reclamation of the mine site. During backfilling, it was noted that an ice plug was present in the raise at a depth of approximately 1.5 m below ground surface. As such, waste rock was placed only to this depth. A 3 m mound was placed on top of the raise to accommodate for possible future settlement of the ice plug. The mound was constructed of shale and surfaced with coarse rock.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-26). During the inspection, the armour surface appeared to be stable with no visual indications of erosion or surface deformation. As such, no maintenance was recommended.

6.4.12. Area 14 Raise

Construction Details

The location of the Area 14 Raise (E584187, N8105663, UTM NAD 83 Zone 16) is illustrated on Drawing 1. The raise had a cross section of 5 m by 5 m and an approximate depth of 8 m. Mining ceased in this area around 1987 and the raise was completely backfilled to the floor of the underground workings. Backfilling was completed with waste rock and the surface was then covered and contoured with shale in the summer of 1987 and 1988.

Inspection Conditions

Select photos from the inspection are provided in Appendix I (Drawing I-26). During the inspection, no settlement of the area surrounding the former raise was observed. As such, no maintenance was recommended.

6.5. Shale and Armour Borrow Areas

6.5.1. Shale Borrow Areas

Several shale borrow areas were developed during the reclamation process to provide material for construction of the thermal covers. The borrow areas include the following:

- Mt. Fuji
- Area 14
- West Twin
- East Twin
- Shale Hill
- Townsite

The borrow areas were reclaimed once no longer required. The reclamation efforts included re-grading of slopes for stability and sufficient grading of the floor of each borrow area to provide for positive drainage of surface water.

Select photos from the inspection are provided in Appendix I (Drawing I-27). The main observations are summarized by the following:

- Mt. Fuji
    - The benches remained partially covered in snow at the time of the inspection.
    - Based on areas not covered in snow, the benches are continuing to slowly fill in from the raveling of the remaining bench faces and are expected to eventually form a stable slope at the natural angle of repose.

    - The floor had no significant areas of ponded water and is considered generally well drained.
    - No issues requiring maintenance were observed.
- Area 14
    - The area was partially covered in snow at the time of the inspection.
    - In general, the re-graded pit walls appear to be stable.
    - One area of erosion has occurred at the north end of the borrow area where natural surface water periodically discharge occurs into the pit, typically during freshet. At this location, the overburden material has been eroded down to the top of the bedrock and has been deposited into the floor of the pit. This area was observed to have stabilized during recent inspections as down-cutting of these materials no longer appears to be occurring.
    - No ponding of water was observed at the time of the inspection, but there has been significant thermokarsting at the entrance to the pit. As such, it is likely that this impedes drainage at some point in the year. The material is sufficiently fractured that any ponded water likely drains when the ground thaws.
- West Twin
    - In general, the re-graded pit walls appear to be stable.
    - No ponding was observed in the floor of the pit.
    - No issues requiring maintenance were observed.
- East Twin
    - In general, the re-graded pit walls appear to be stable.
    - No ponding was observed in the floor of the pit.
    - Some thermokarsting was noted on some of the remnant benches within the interior of the pit. This thermokarsting has resulted in some ponding within the interior benches of the pit.
    - No issues requiring maintenance were observed.
- Shale Hill
    - In general, the re-graded pit walls appear to be stable.
    - No ponding was observed in the floor of the pit.
    - No issues requiring maintenance were observed.
- Townsite
    - In general, the re-graded pit walls appear to be stable.
    - No ponding was observed in the floor of the pit.
    - No issues requiring maintenance were observed.

6.5.2. Armour Borrow Areas

Several armour borrow areas were developed during the reclamation process to provide material for construction of the thermal covers. The borrow areas include the following:

- Twin Lakes Delta deposit
- Chris Creek “A” and “B” deposits
- Kuhulu Lake Road deposit
- 09S/17N Road deposit
- Area 14

The borrow areas were reclaimed once no longer required. The reclamation efforts included sufficient grading of the floor of each borrow area to provide for positive drainage of surface water.

Select photos from the inspection are provided in Appendix I (Drawing I-28). The main observations are summarized by the following:

- Twin Lakes Delta deposit
  - The floor of the quarry was, in general, well drained. The surface water flows to the north edge of the borrow area and then proceeds to flow west to the Reservoir.
  - Only isolated thermokarsting was observed in the floor of the quarry.
  - No issues requiring maintenance were observed.
- Chris Creek “A” and “B” deposits
  - The floor of the quarry was observed to be well drained.
  - Some minor thermokarsting was observed in the floor of the quarry.
  - No issues requiring maintenance were observed.
- Kuhulu Lake Road deposit
  - The floor of the borrow area was graded in late 2006 in response to thermokarsting identified during the 2006 inspection. Since the grading in 2006, some additional thermokarsting has occurred, but the area appears to be well drained with only limited ponding occurring in the bottom of the pit. No additional maintenance was recommended.
- 09S/17N deposit
  - The face of the borrow area did not exhibit any indications of erosion or thermokarsting.
  - No issues requiring maintenance were observed.
- Area 14 deposit
  - Some minor thermokarsting was observed in the borrow area.
  - Given the stability exhibited by the area over the past few years, additional grading is no longer considered necessary but the area should continue to be monitored.

6.6. Other Areas

6.6.1. Concentrate Storage Shed

Construction Details

Dismantling of the Concentrate Storage Shed was completed in 2007. The concrete floor slab was left in-place and covered with a surficial cover of locally derived materials. According to mine site staff, the cover is approximately 30 cm thick, although this was not confirmed during the site inspection. The area adjacent to the Concentrate Storage Shed foundation, where metals contaminated soils had been excavated, was also backfilled and re-sloped to prevent ponding.

Inspection Conditions

The main observations made during the inspection are summarized by the following:

- Most of the surficial cover over the remnant concrete pad was removed previously by the Coast Guard.
- A building has been erected on the surface of the cleared concrete pad by Department of National Defence.
- A portion of the cleared concrete pad was being used to stockpile screened contaminated soil from the main tank farm foundation. This material was being stockpiled here prior to placement within the treatment cells. The soil stockpile was surrounded by a soil berm to limit interaction of the stockpile with any surface water running along the surface of the concrete pad.
- No erosion of the remaining cover was observed.
- No seepage from the cover area was noted.
- No ponding was noted on the surface of the cover or any areas immediately adjacent to the cover. Though the remaining cover over the lower portion of the pad has the potential to impede drainage and create ponding during spring melt or precipitation events.

No maintenance of the Concentrate Storage Shed concrete pad was recommended.

6.7. Summary of 2014 Maintenance Recommendations

The maintenance items recommended throughout Section 6 are summarized in Table 4.

Table 4. Recommended 2014 Maintenance and Action Items.

Inspection Item	Recommended Maintenance and Action Items
West Twin Dyke Spillway	Review additional enhancements to armouring along left bank of spillway. The area should continue to be visually monitored for any signs of erosion or permafrost degradation induced deformation of the side slopes.
West Twin Outlet Channel	Monitor the water elevation in the Reservoir. Consider seepage control measures such as backfill polishing pond upstream of the wall.
East Twin Creek Diversion Channel	Additional armouring of the left bank should be considered to enhance long term performance of the channel. Monitor seepage discharge point at toe of dyke.
Surface Cell Tailings Cover	No maintenance required. Continue to monitor thermokarst areas for additional deformation.
East Open Pit/ East Trench Waste Rock Cover	No maintenance required. Continue to visually monitor cracking in EOP crown pillar and minor surficial erosion of EOP and East Trench covers.
Oceanview Pit Waste Rock Cover	No maintenance required. Continue to visually monitor surficial erosion along backslope.
00/01 Portals and crown pillar	No maintenance required. Continue to visually monitor cracking in WOP crown pillar.
Instrumentation/ Monitoring	Re-install water level logger in Reservoir during 2015 inspection. Most frost gauges require additional methyl blue solution. This should be done prior to installation in 2015.

No maintenance was recommended at other areas not specifically noted in Table 4.

6.8. 2015 – 2019 Monitoring Schedule

As part of the Water License renewal process, BGC undertook a review of the geotechnical monitoring requirements for the Nanisivik Mine site. The intent of the review was to develop a revised monitoring schedule for the term of the next Water License. The results of this review are documented in a project memorandum (BGC 2013). Based on the results of the monitoring program undertaken to date, it was considered appropriate to reduce the monitoring schedule for the term of the next Water License. In summary, geotechnical instrumentation (thermistors, piezometers, frost gauges) will be undertaken as per the proposed schedule between July 1 and September 1. Additionally, the reclamation measures will continue to be inspected on an annual basis throughout the term of the next Water License by a qualified Geotechnical Engineer.

The proposed monitoring schedule was generally approved during the Water License renewal. The monitoring schedule incorporated within the new Water License is provided in Appendix III.

7.0 CONCLUSIONS AND RECOMMENDATIONS

The following paragraphs provide a summary of the significant observations, conclusions and recommendations based on the results of the 2014 Geotechnical Inspection Program undertaken at the Nanisivik Mine site.

Embankments

The breaches in the remnant dykes at the East Adit Treatment Facility continue to permit natural drainage of the area with only limited ponding in the Retention Pond. The size of this ponding is gradually decreasing with time as the low spots in the pond area backfill with natural sediment transported from upslope areas. The Main Tank Farm berm was nearly completely removed at the time of the inspection and no ponding was occurring in the area. The West Twin Dyke was physically stable with no signs of erosion or seepage.

Hydraulic Structures

The West Twin Dyke Spillway continues to effectively drain surface water from the Surface Cell cover into the Reservoir and is generally functioning as intended. Additional erosion of the left bank was noted in 2014, likely in response to elevated flows during freshet. Due to the nature of the flows causing the erosion, it is likely that this erosion will continue without additional enhancement of the left bank armour. If not addressed, this may lead to decline in the effectiveness of the spillway over time.

As observed during previous inspections, a small head pond developed on the Surface Cell cover at the spillway inlet. The presence of the head pond is not considered to negatively impact the overall cover performance; hence the elimination of the head pond is not considered necessary. Reduction in pond size may continue over time as the intact rock in the base of the spillway inlet becomes more permeable due to freeze thaw effects.

Seepage losses continued at the West Twin Outlet wall in 2014. Despite these seepage losses, the water level in the remnant polishing pond upstream of the wall remained near the invert of the outlet wall throughout the open water season in 2014. The stable water level in the Reservoir is thought to reflect the generally wetter than normal conditions at site in 2014 as opposed to being indicative of reduction in seepage losses. Increased instability in the shoreline of the polishing pond was also noted in 2014. Based on the water quality measurements collected since the East Twin Lake access road was breached in 2008, the seepage losses from the polishing pond do not appear to be affecting the overall performance of the West Twin Area reclamation measures. The seepage losses should continue to be monitored and additional seepage control measures should be considered.

Some erosion at the East Twin Diversion Dyke was noted during the inspection. The erosion was likely due to elevated flows during freshet in combination with snow blockage in the channel. The erosion appeared to be similar to what was observed in 2013. In general the dyke remains in stable condition. However, it is recommended that additional re-sloping and armouring of select areas of the channel be undertaken to enhance long term performance.

As has been observed during inspections over the past couple of years, seepage was observed at the toe of the East Twin Diversion Dyke in 2014 upstream of the convergence of flow from West Twin. This seepage is likely originating in the remnant polishing pond. There is no concern with respect to the integrity of the dyke due to the presence of this seepage. However, it is recommended that this seepage point be monitored for increased flows in 2015.

Thermal Covers

The thermal covers were generally performing well in 2014 similar to the performance observed in 2013. Only minor erosion, cracking and thermokarsting/settlement were observed, which were not seen to negatively impact the thermal performance nor the water quality in the surrounding areas. Many of these features have been noted previously and have been observed to be physically stable for the past several years. For the most part, the active layer thaw was contained within the thermal cover at most locations. Continued improvement has also been observed in the quality of the surface water runoff from the Surface Cell cover system. This is an indication of the beneficial impacts related to improved geothermal performance of the cover system. Additionally, the water quality observed at the final discharge point for the West Twin Disposal Area has also been observed to remain well below the discharge criteria.

It is anticipated that, over time, the ice saturation will continue to progress within the base of the cover materials. As this occurs, the geothermal performance of the covers will continue to improve, further confining the active layer thaw within the cover. No maintenance is required for the thermal covers at this time; however, visual monitoring is recommended to check for additional surface deformation.

Talik Freeze-back

Freeze-back of the talik in the Surface Cell and Test Cell appears to be proceeding at anticipated rates. Cooling of the entire geothermal profile was observed in most instruments at similar or accelerated rates of cooling compared to recent years.

Most piezometers within the surface cell have frozen back due to continued downward migration of the freezing front. Where the piezometers remain in thawed tailings, the pore pressures continue to increase. The increasing pore pressures are related to continued freeze-back of the tailings. The pore pressures are lowest near the dyke and highest in the centre of the talik. The piezometers in the Test Cell talik have shown pore pressures increasing at a greater rate than in the past. This may be due to constraining of the hydraulic connection between the Test Cell talik and Reservoir due to downward migration of the freezing front.

Mine Openings, Crown Pillars and Raises

The covers constructed over the mine openings generally appear to be physically stable. Cracking at the East Open Pit crown pillar does not appear to have progressed since 2011. No maintenance is recommended, but continued visual inspection is warranted.

Shale and Armour Borrow Areas

In general, the shale and armour borrow areas appear to be physically stable and are not causing any significant ponding to occur. No maintenance was recommended at any of the borrow areas.

8.0 CLOSURE

This report provides a performance assessment of numerous structures at the Nanisivik Mine, based on visual observations recorded during the site inspection and a review of monitoring data collected from various instruments throughout 2014.

We trust the information provided herein meets your present requirements. Thank you for allowing BGC to be of service, once again, to Nanisivik Mine. If you have any questions or require additional details, please contact the undersigned.

Respectfully submitted,

BGC ENGINEERING INC.
per:

REGISTERED PROFESSIONAL ENGINEER
G.K. CLAYPOOL
LICENSEE
28-Feb-15
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Mitchell Latimer, B.Sc., E.I.T
Geoenvironmental Engineer

Reviewed by:

James W. Cassie, M.Sc., P.Eng.
Principal Geotechnical Engineer

GC/ML/JWC/cs

PERMIT TO PRACTICE
BGC ENGINEERING INC.
Signature
Date 28-Feb-15
PERMIT NUMBER: P 285
The Association of Professional Engineers, Geologists and Geophysicists of the Northwest Territories

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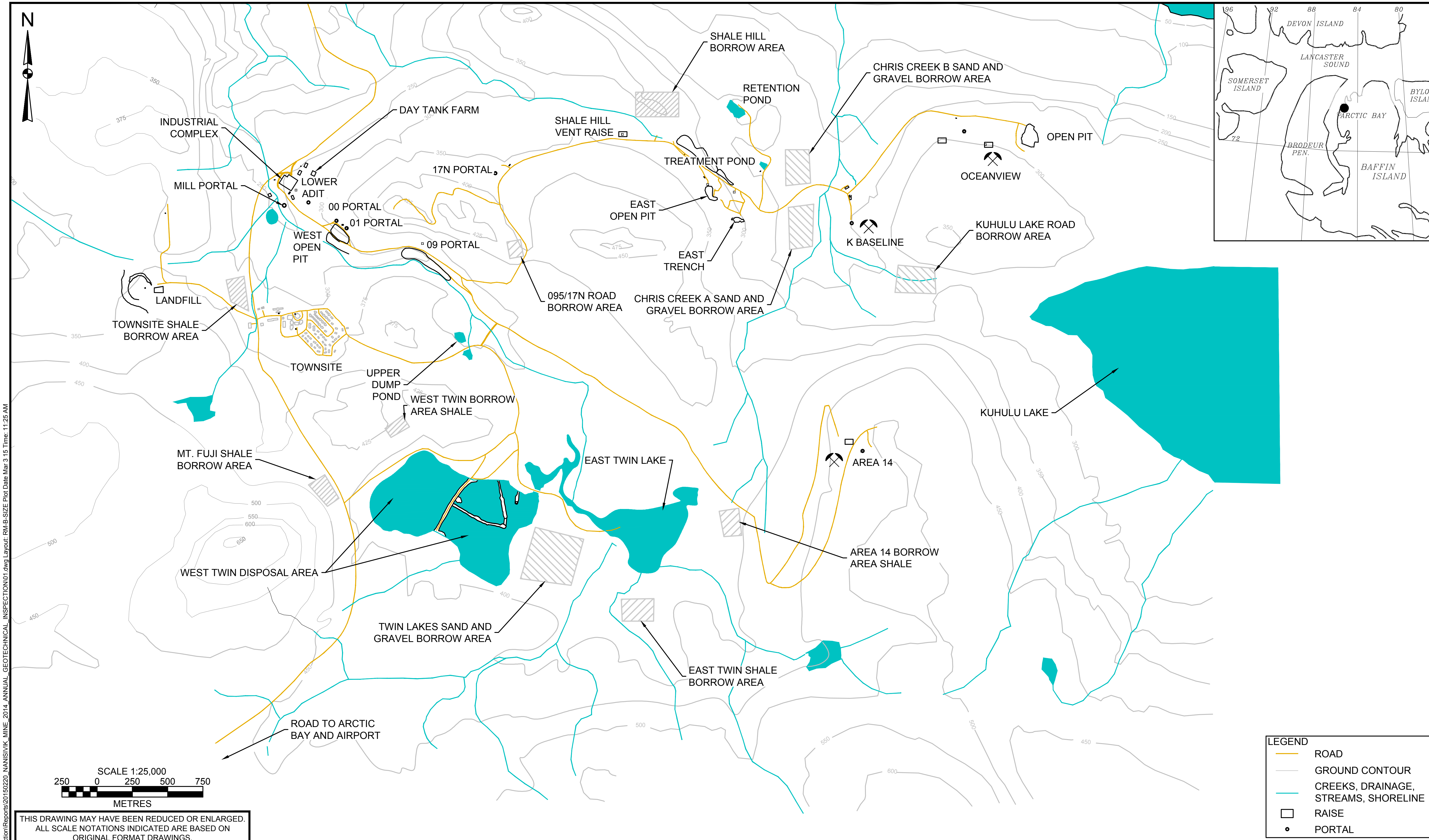
DRAWINGS



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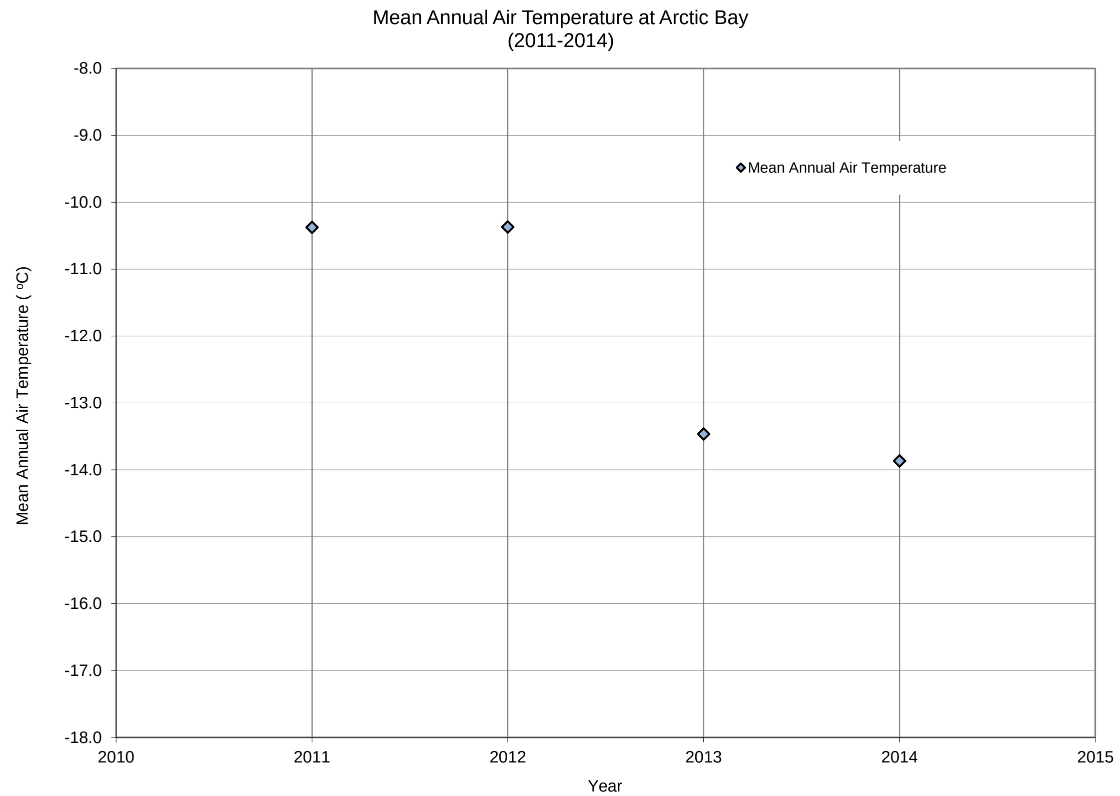
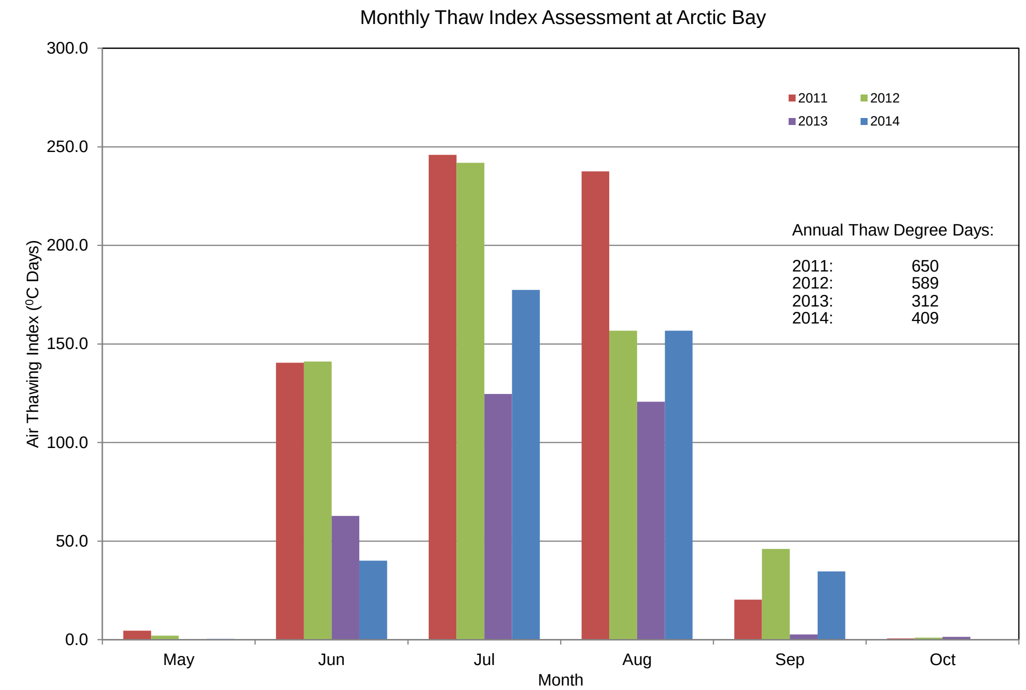
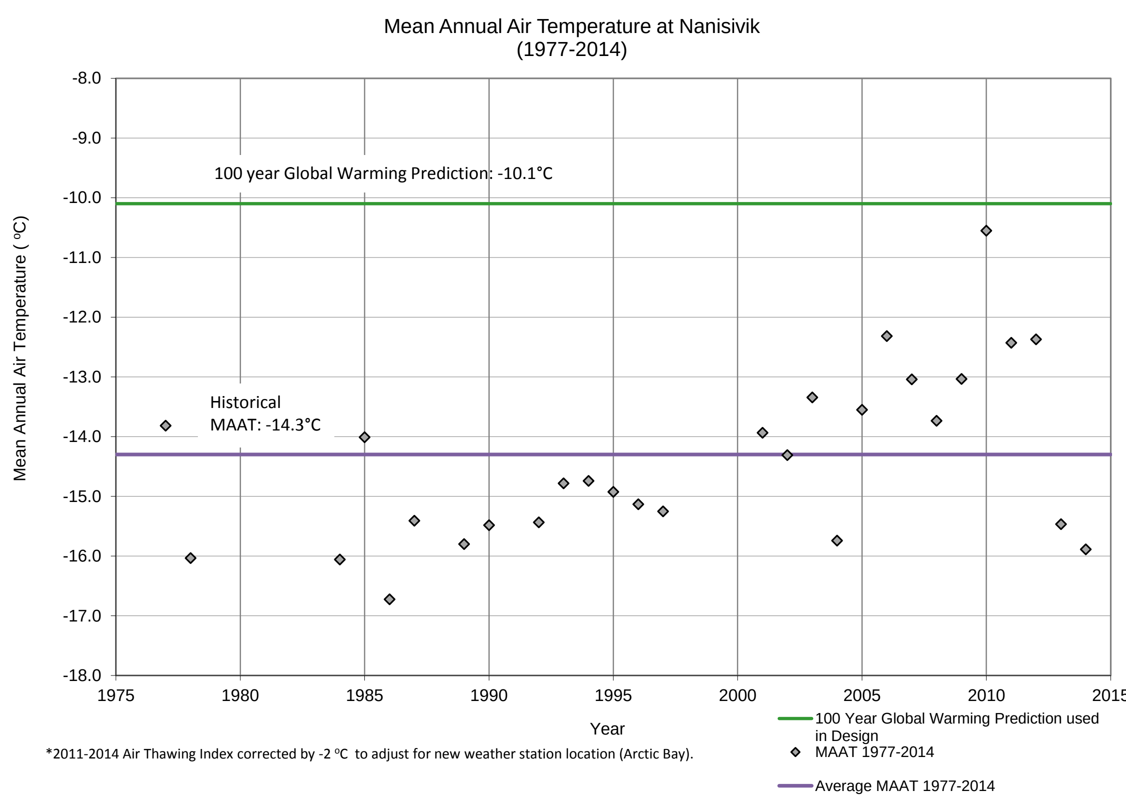
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: SITE LOCATION
PLAN

PROJECT No.: 0255-024-03
DWG No.: 01







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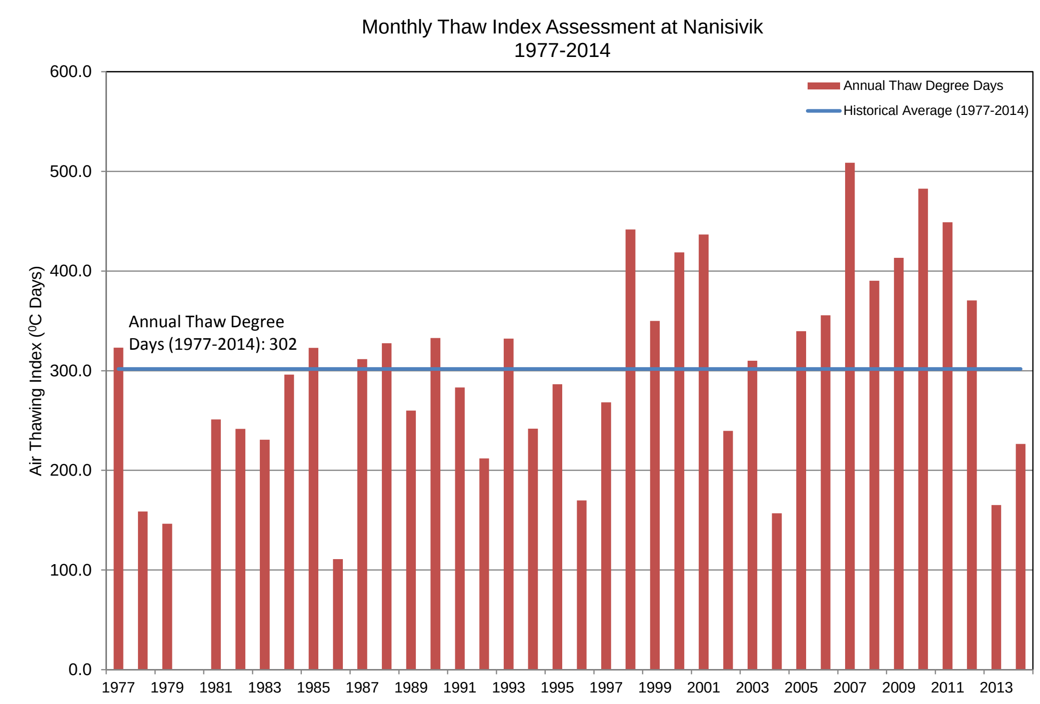

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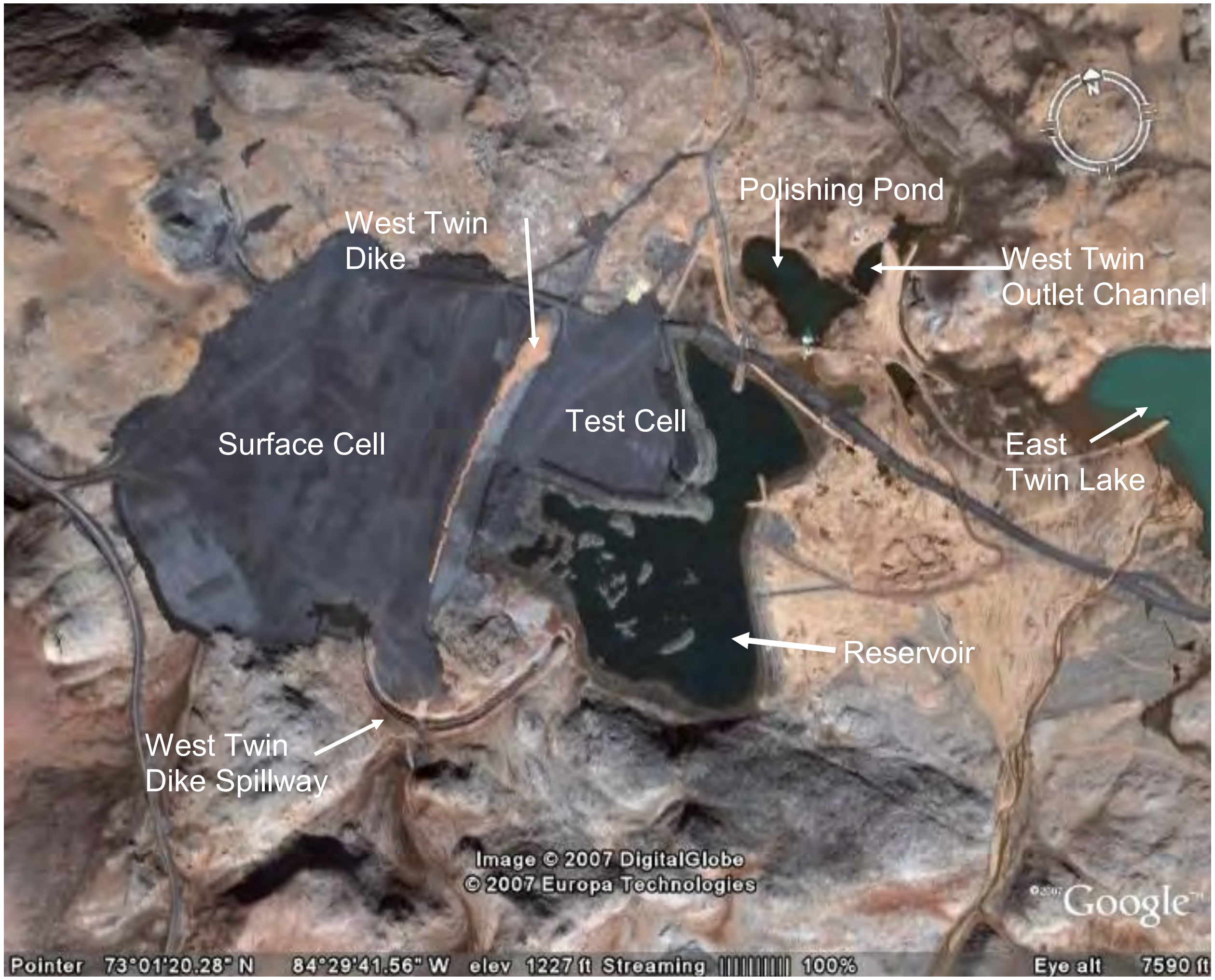

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Note:
1.Photo derived from Google Earth January 10, 2008
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2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: COMPONENTS OF WEST TWIN DISPOSAL AREA

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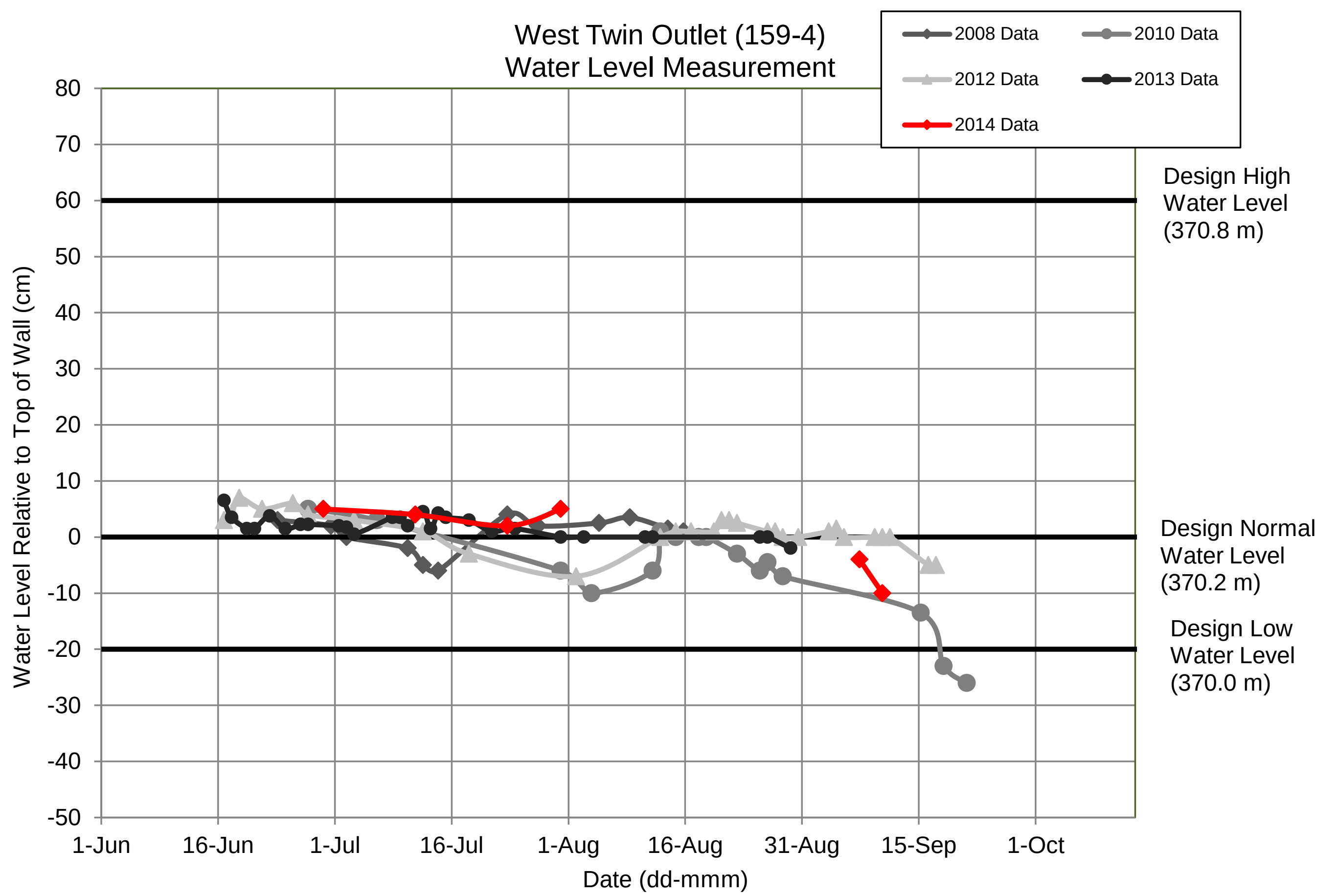

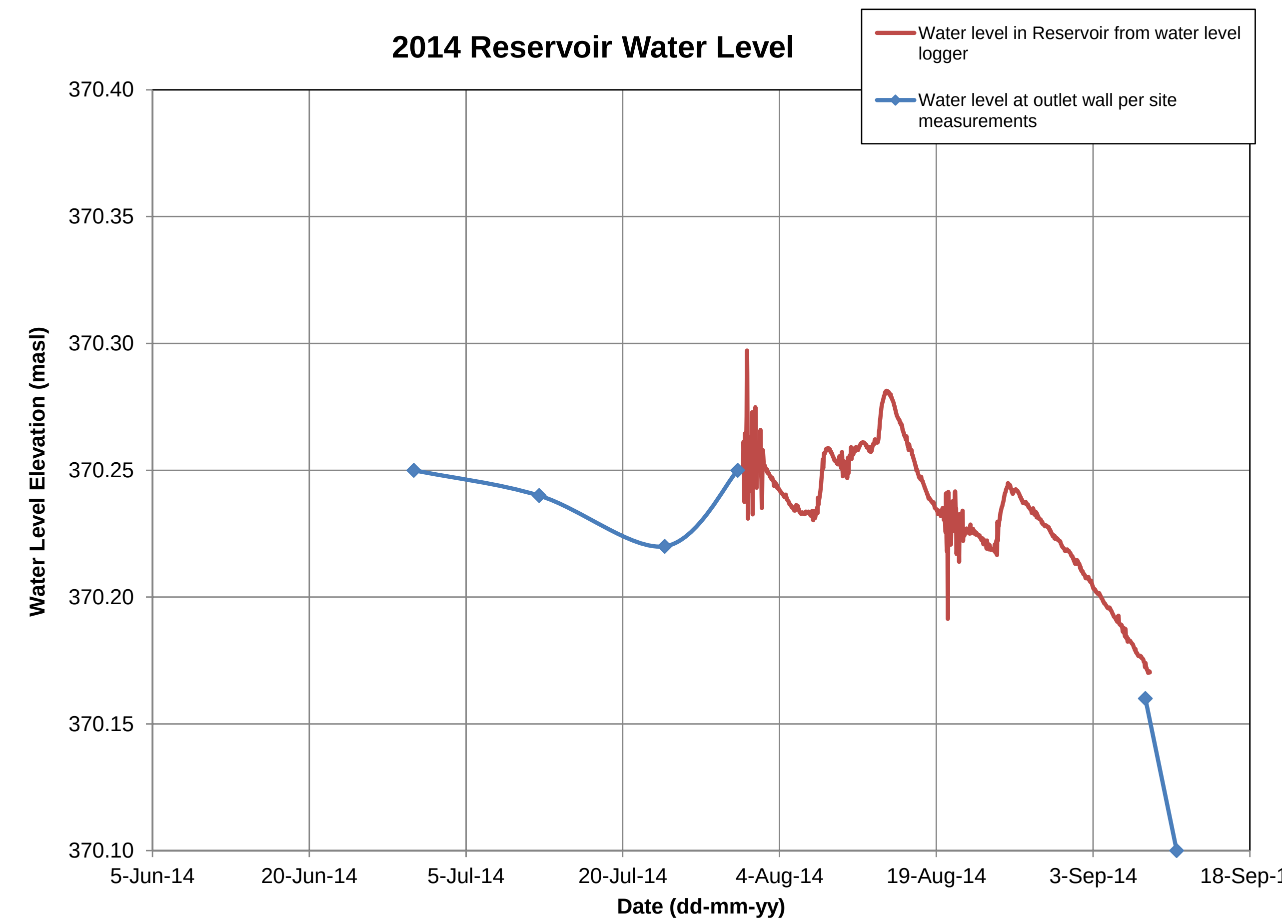

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TITLE: WATER LEVELS IN RESERVOIR AND AT WEST TWIN OUTLET WALL

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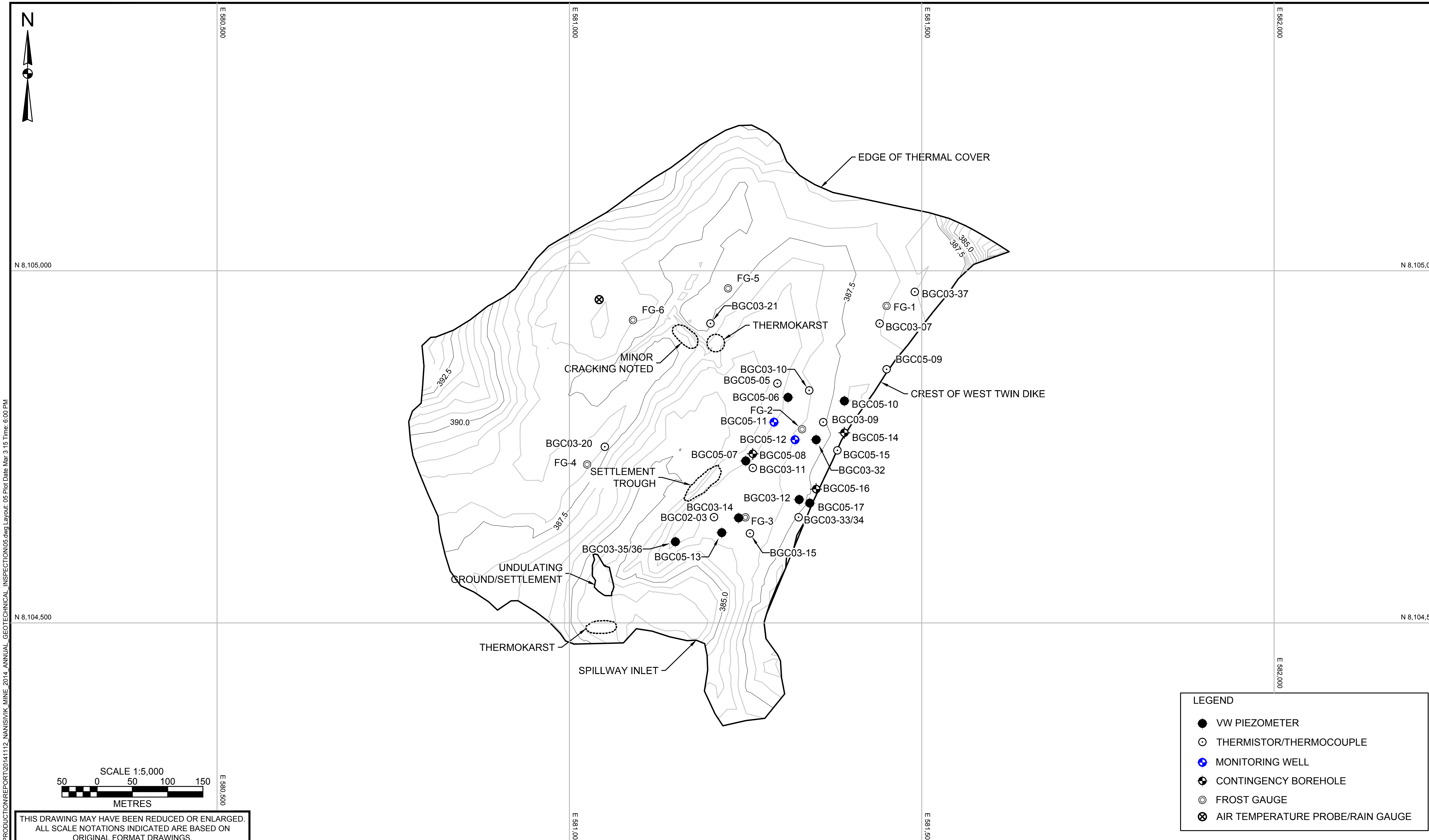

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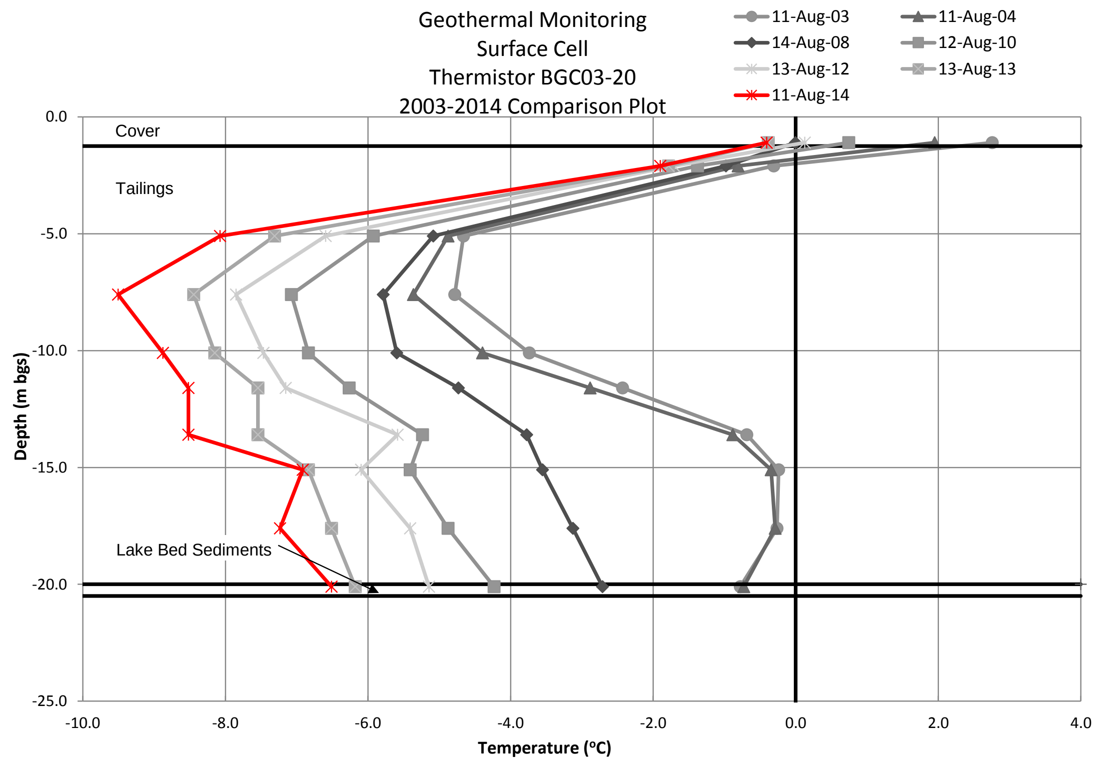

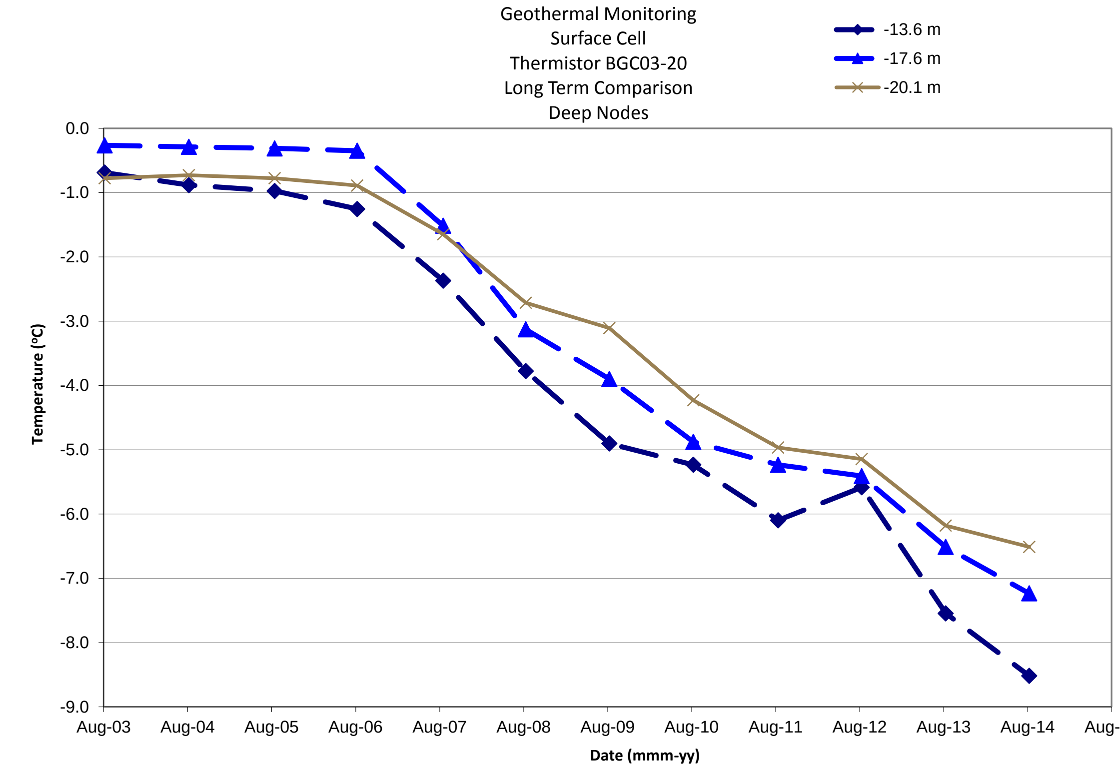

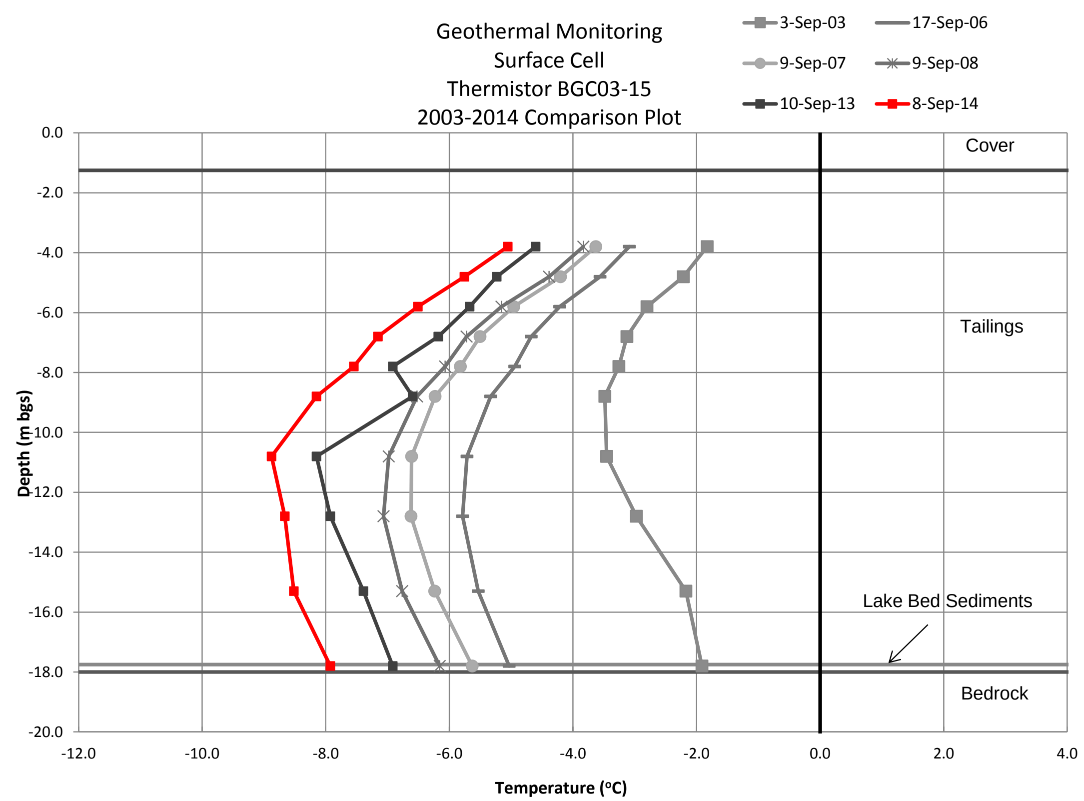

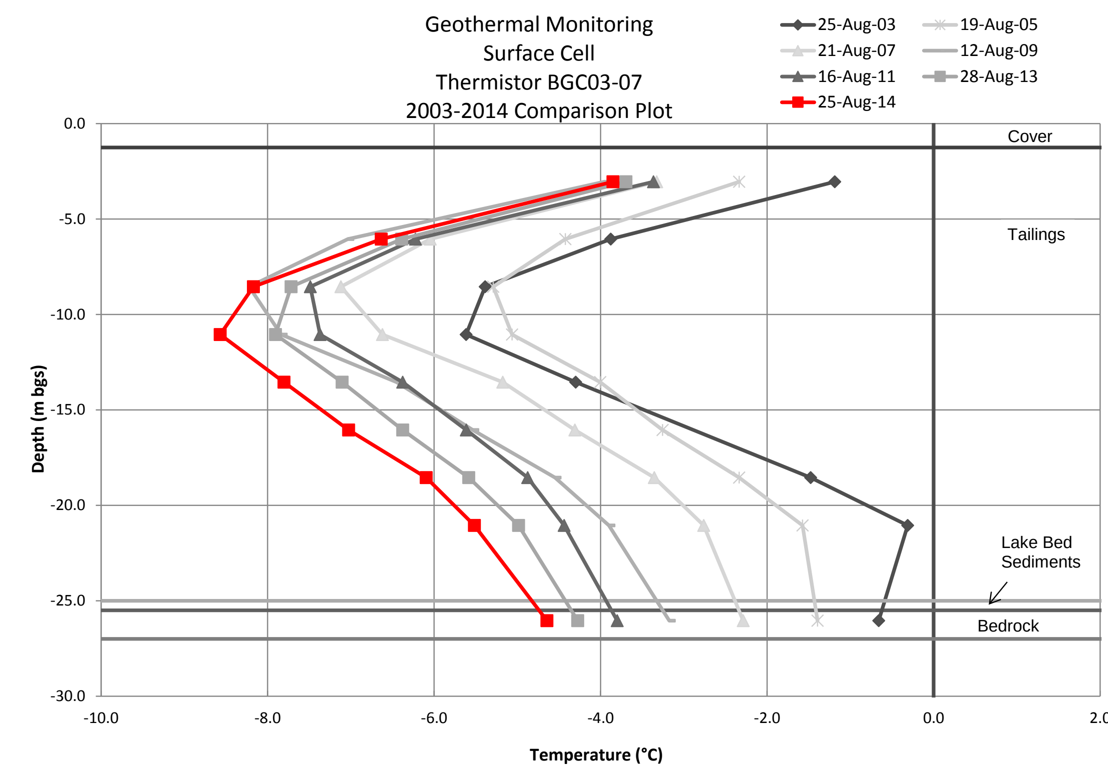

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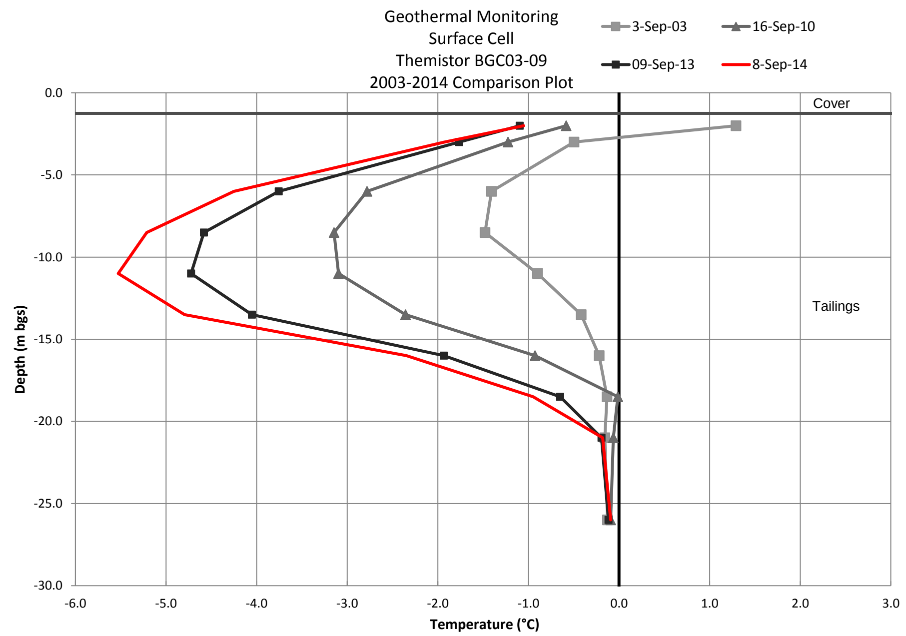
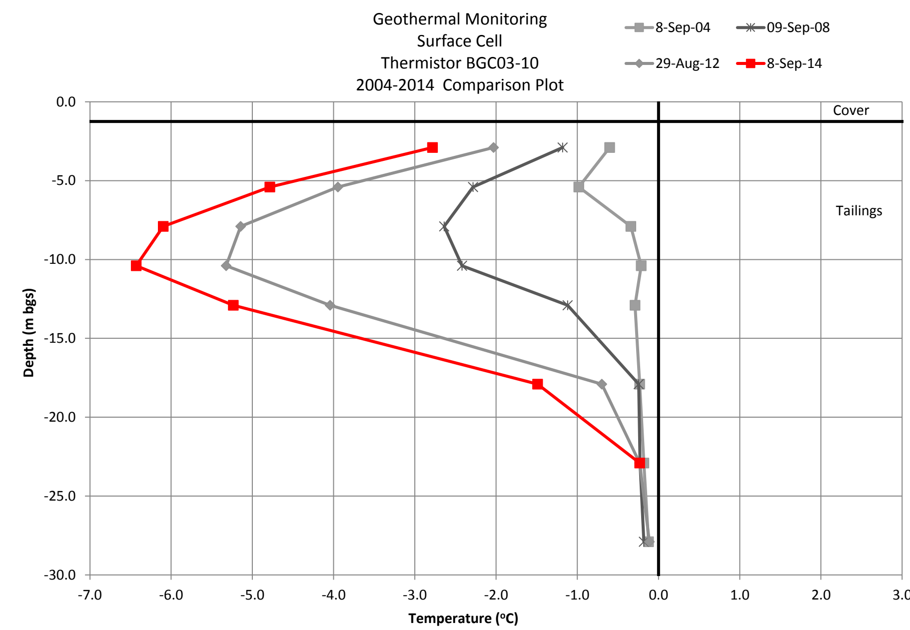
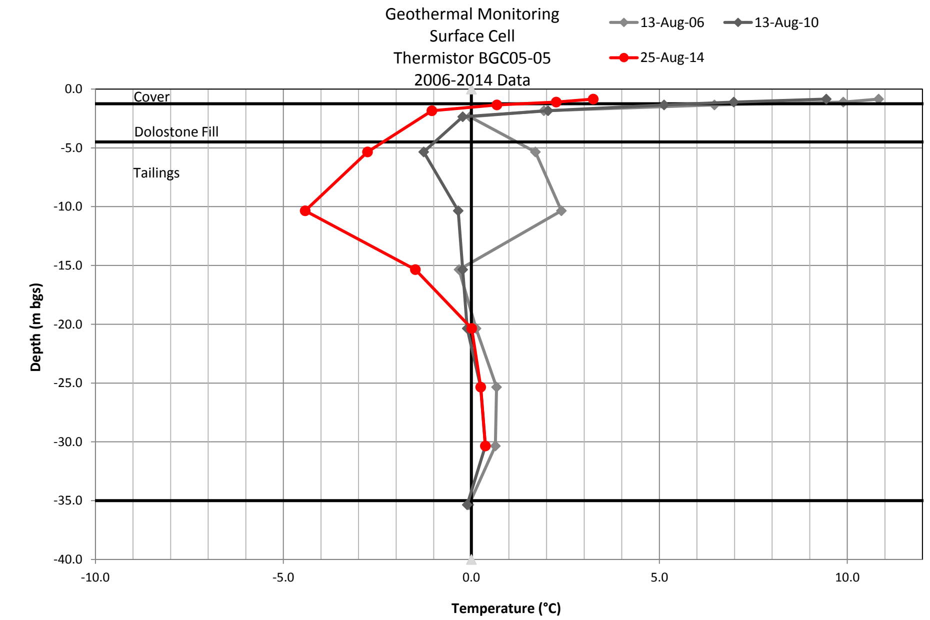
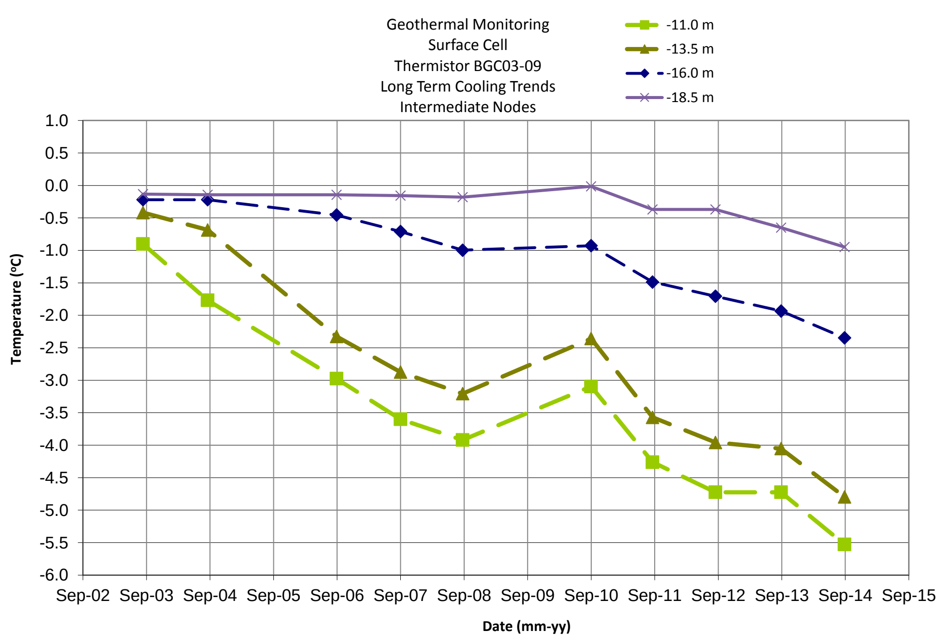
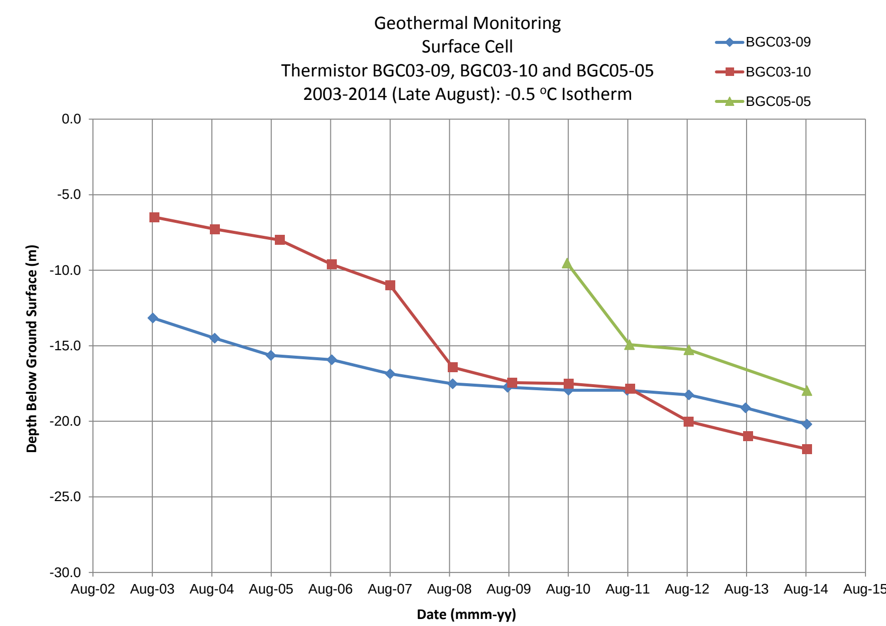
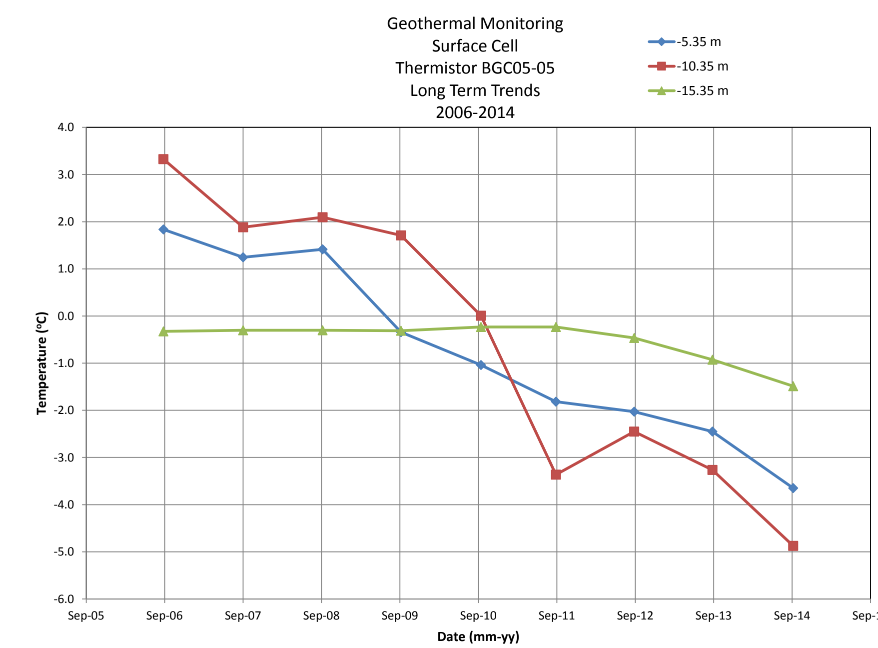












DWG TO BE READ WITH BGC REPORT TITLED: "NANISVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

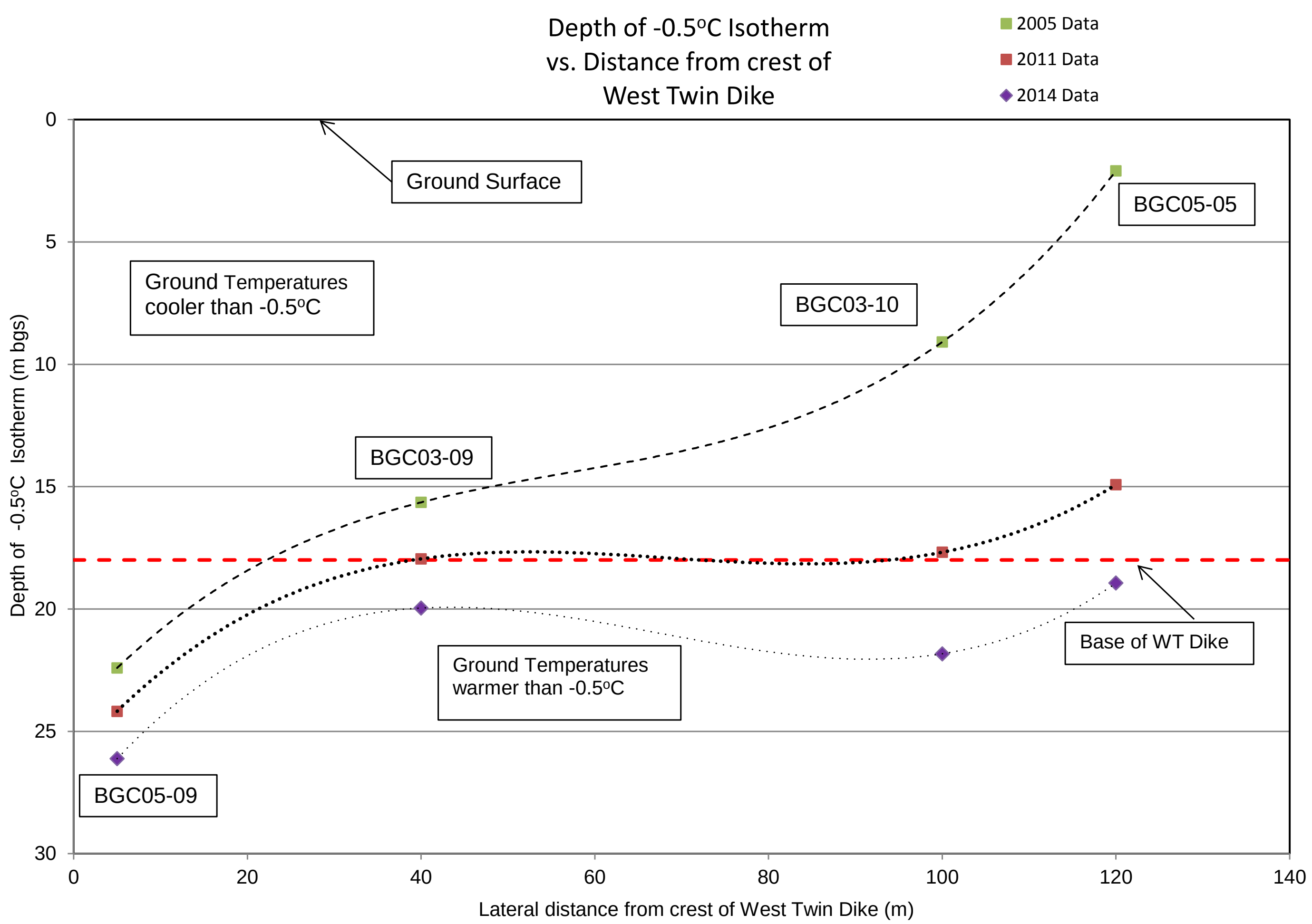
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

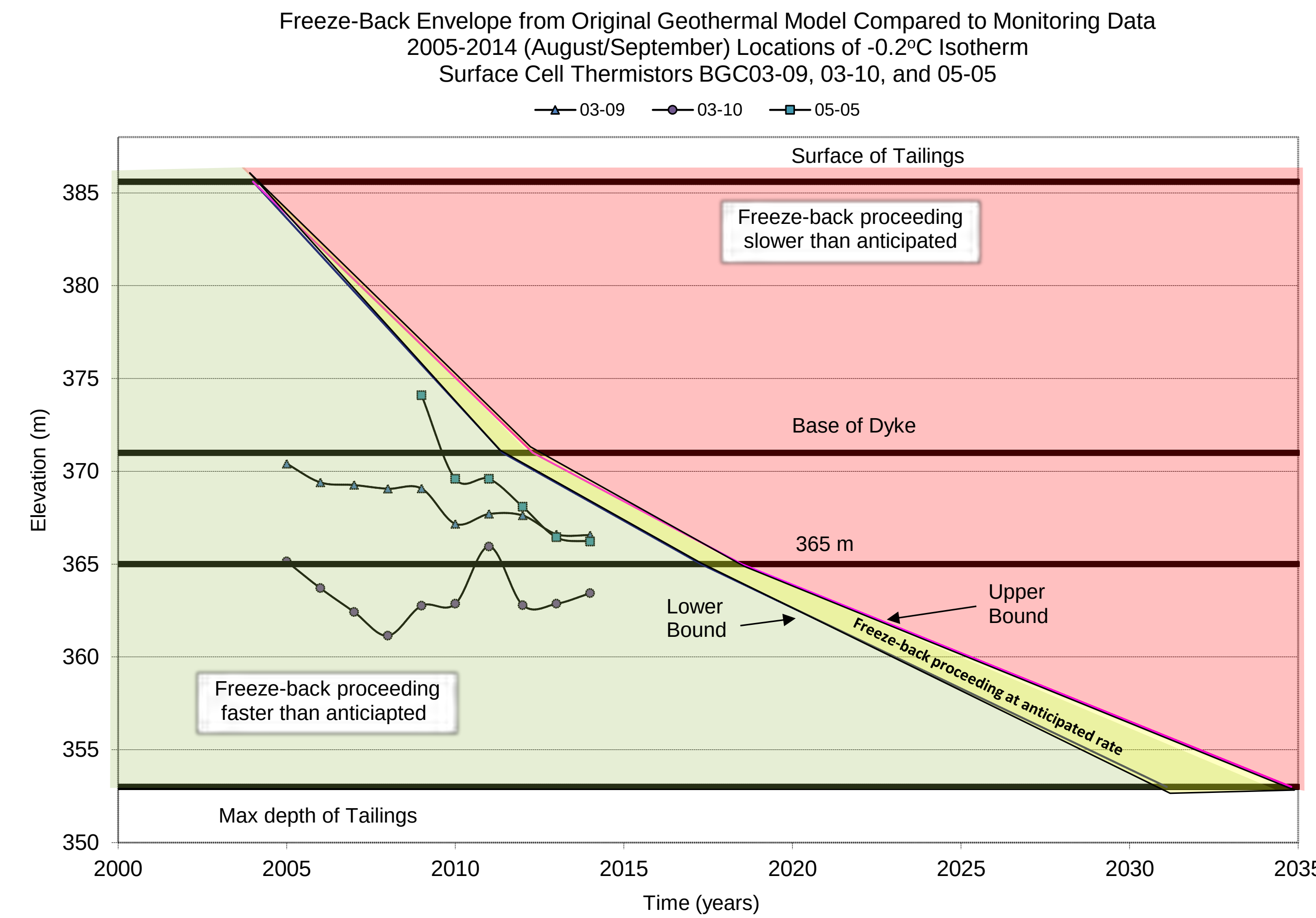
CLIENT: nyrstar

PROJECT: NANISVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: SURFACE CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 2

DWG No.: 7
PROJ No.: 0255-024-03
REV:





DWG TO BE READ WITH BGC REPORT TITLED: "NANISVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

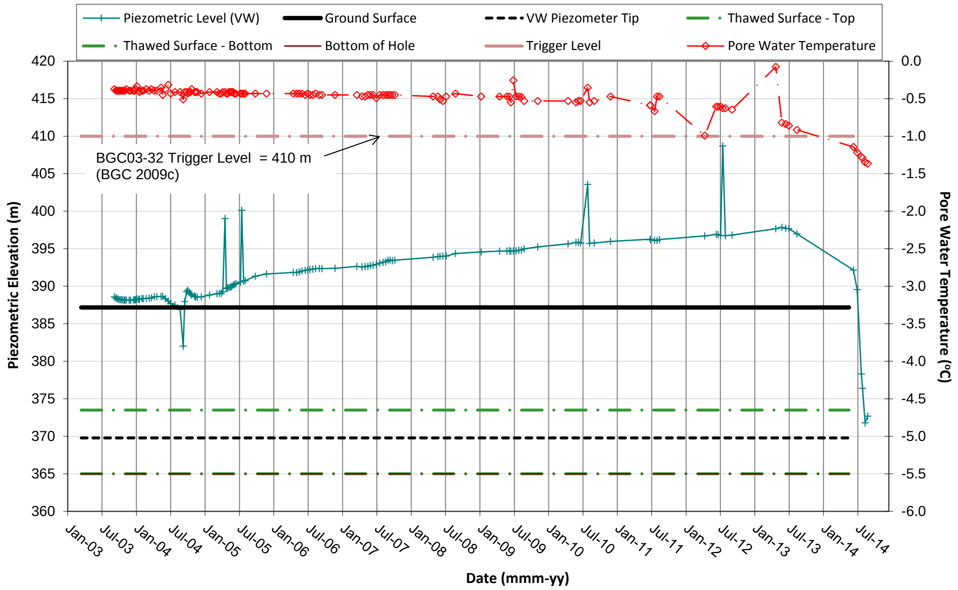
CLIENT: nyrstar

PROJECT: NANISVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

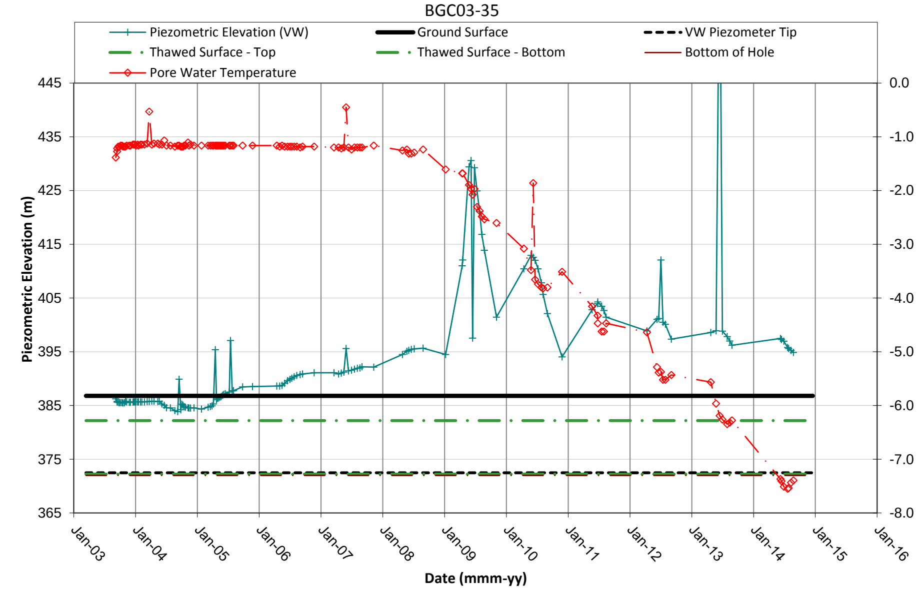
TITLE: SURFACE CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 3

DWG No.:	PROJ No.:	REV:
8	0255-024-03	

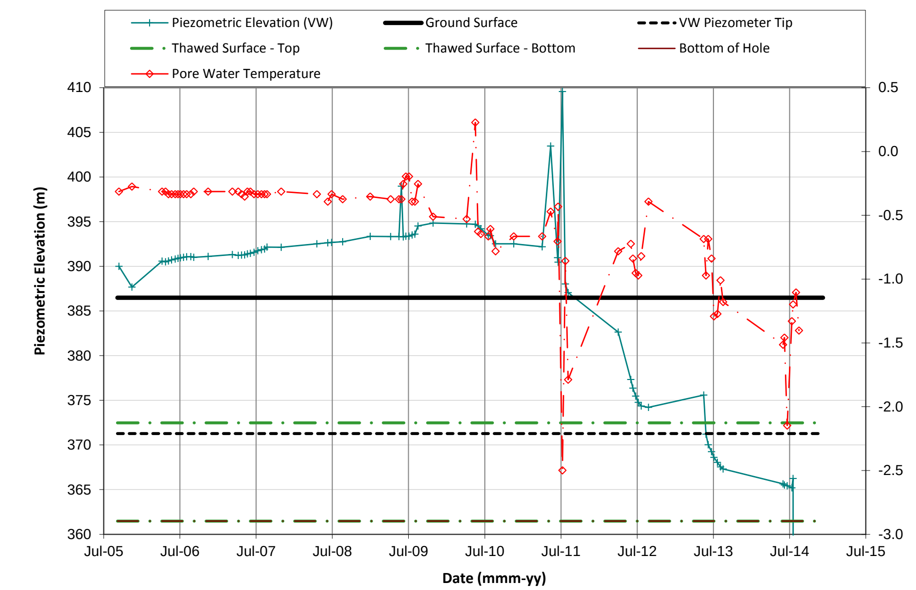
Piezometer Monitoring
Surface Cell Talik
BGC03-32



Piezometer Monitoring
Surface Cell Talik
BGC03-35



Piezometer Monitoring
Surface Cell Talik
BGC05-07



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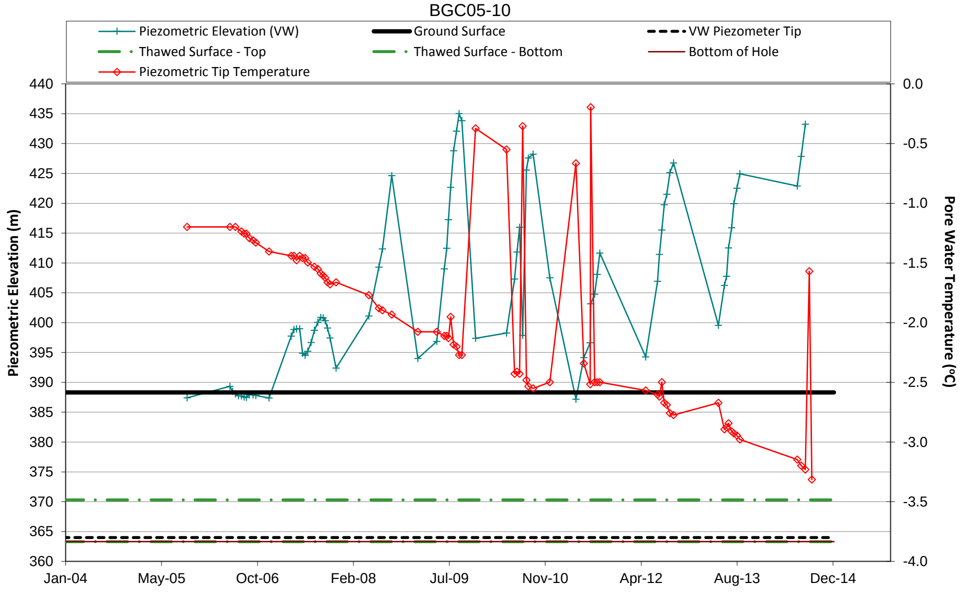
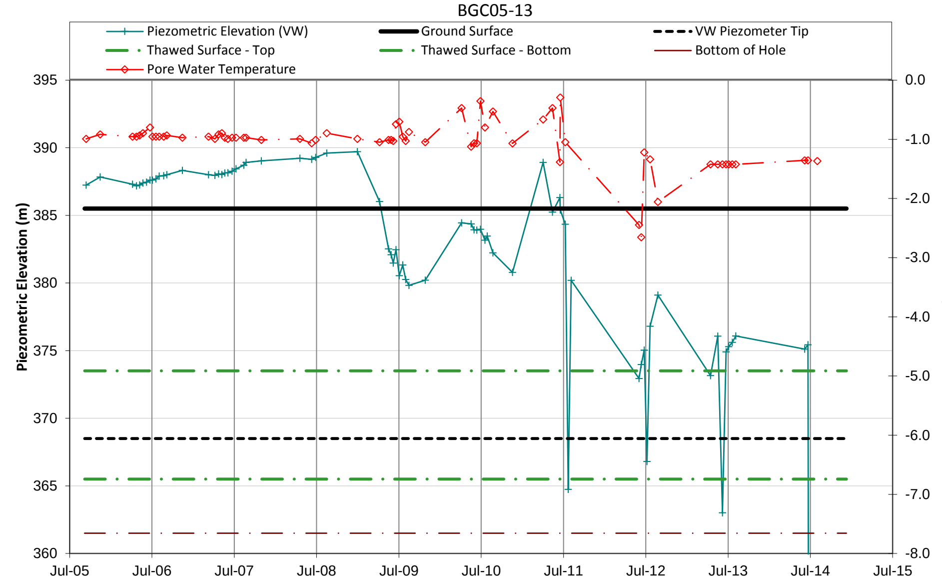
Surface Cell Talik
BGC05-10



Piezometer Monitoring
Surface Cell Talik
BGC05-13



DWG TO BE READ WITH BGC REPORT TITLED: "NANISVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: SURFACE CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 4

DWG No.: 9
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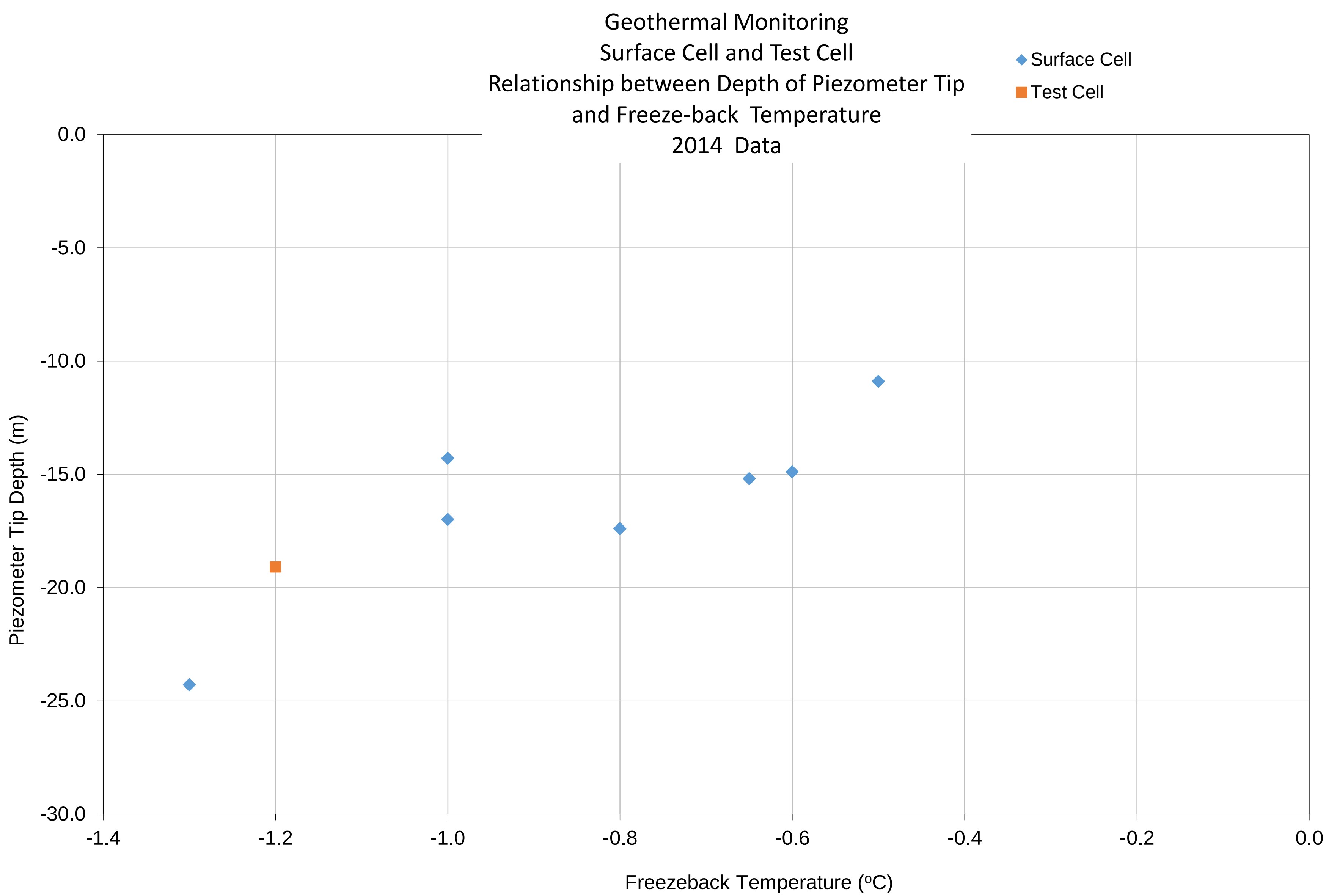

DWG TO BE READ WITH BGC REPORT TITLED: "NANISVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	AS SHOWN
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	JWC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: SURFACE CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 5

DWG No.:	PROJ No.:	REV:
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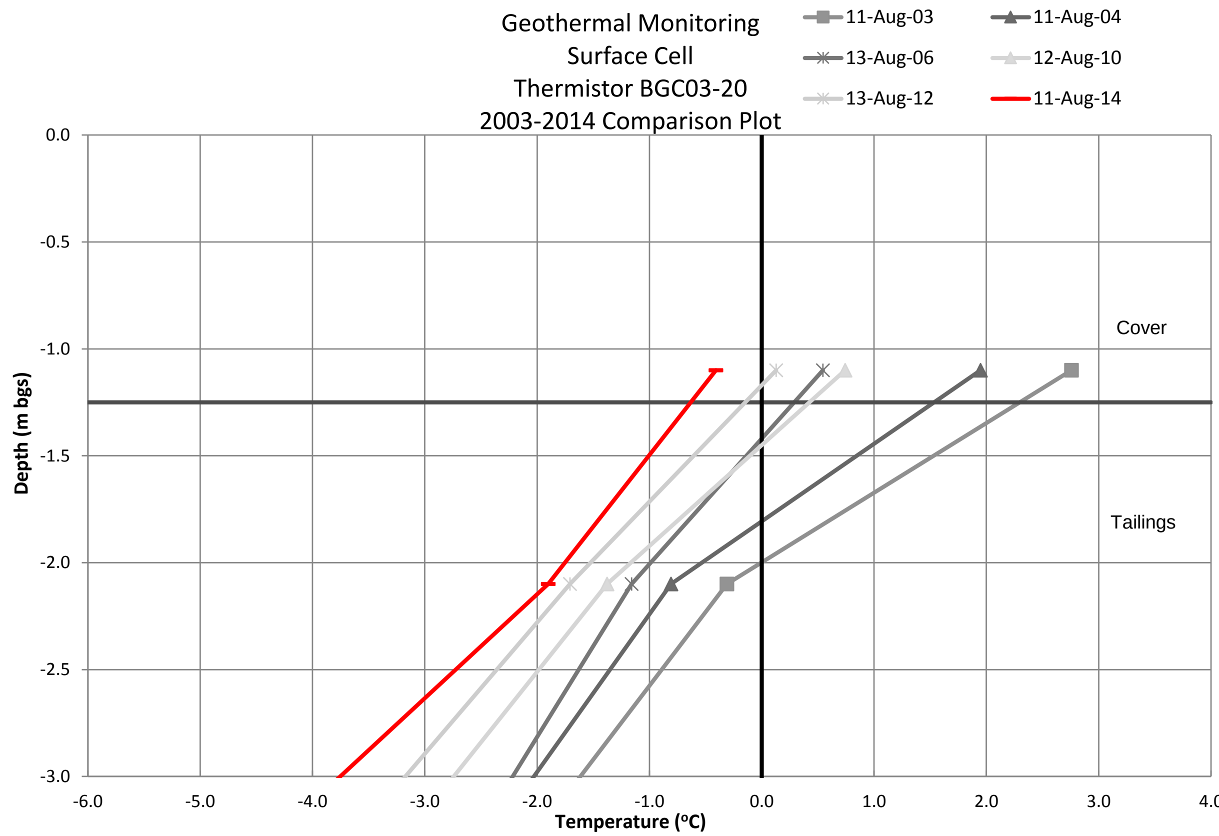
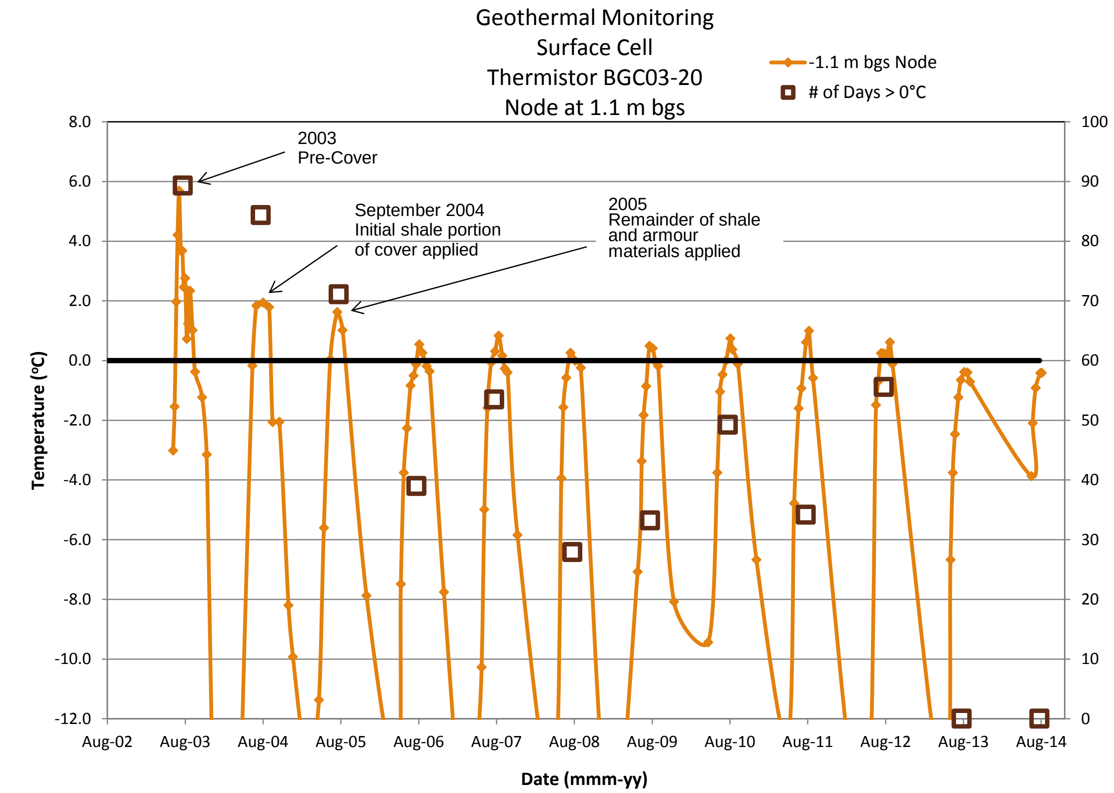
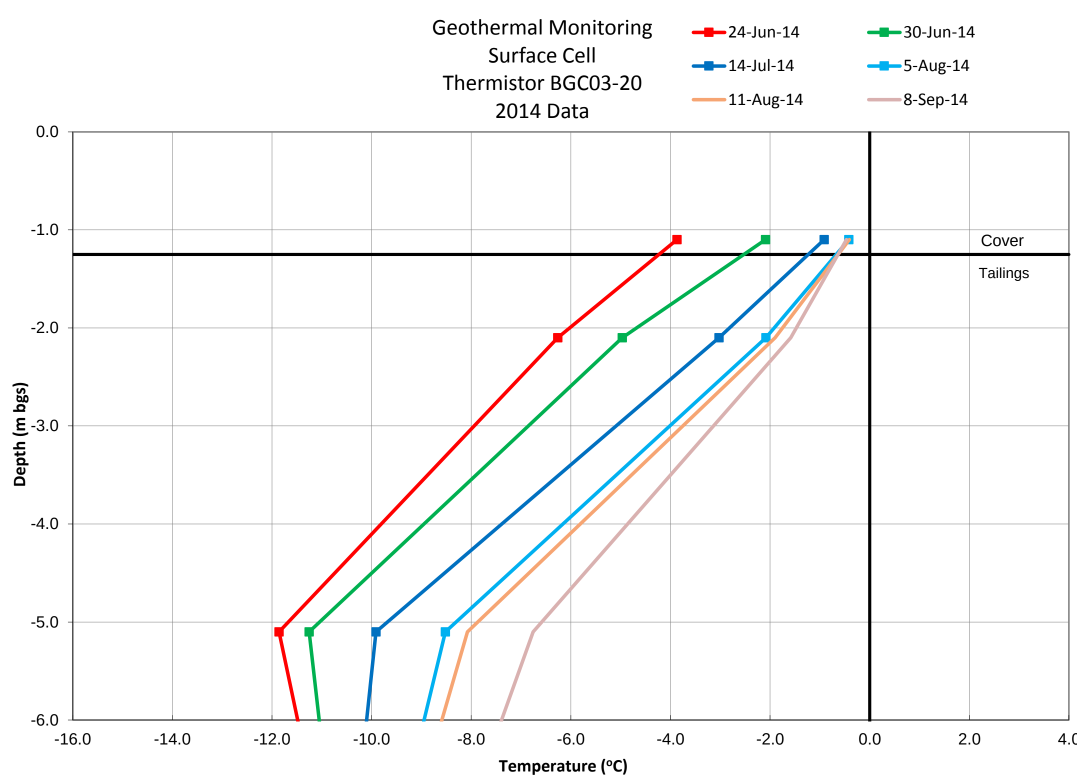
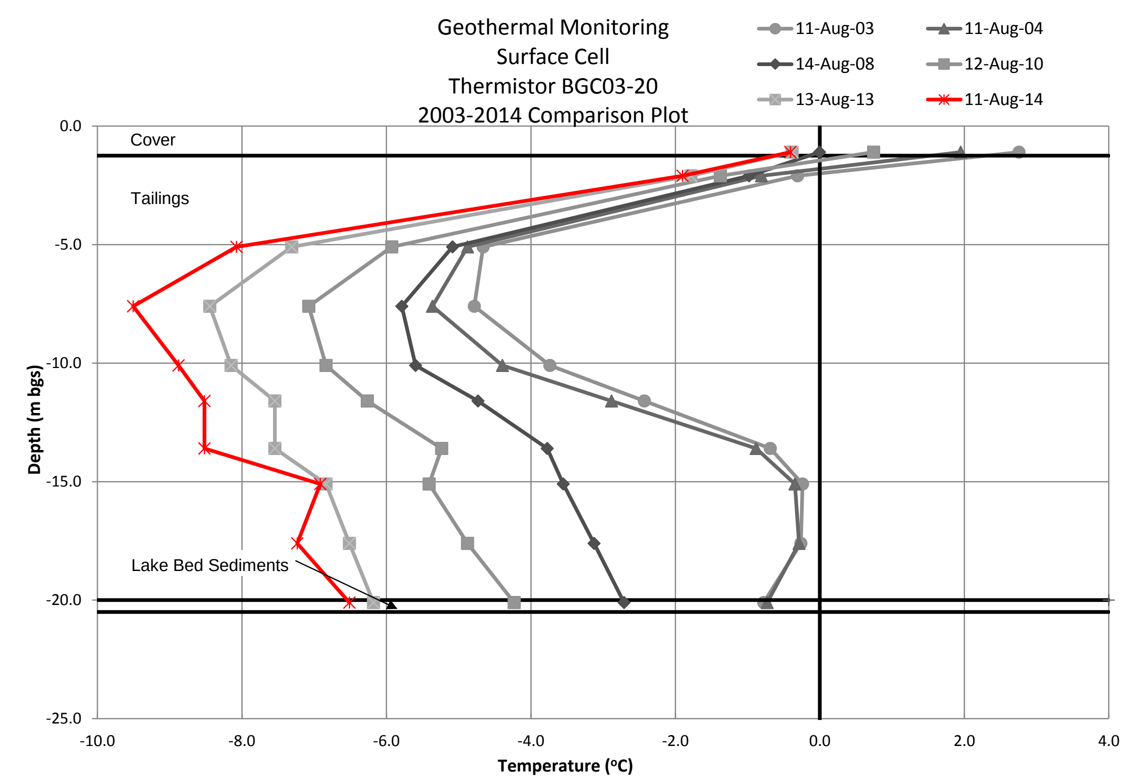








DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

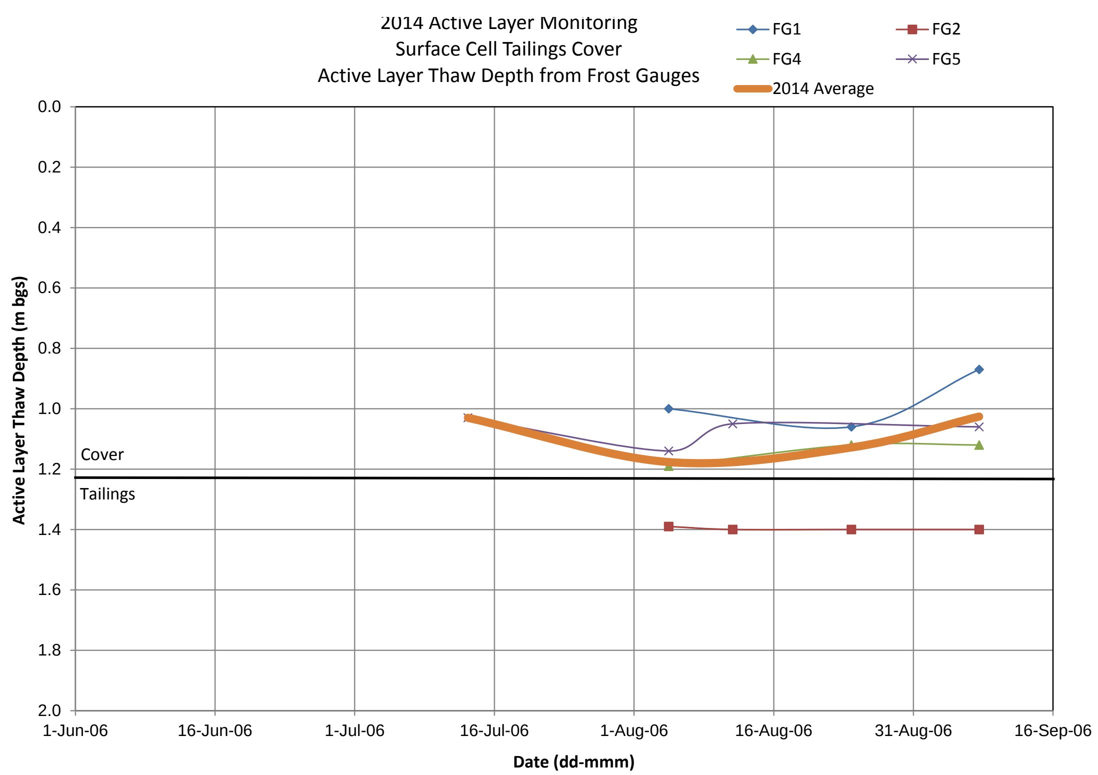
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

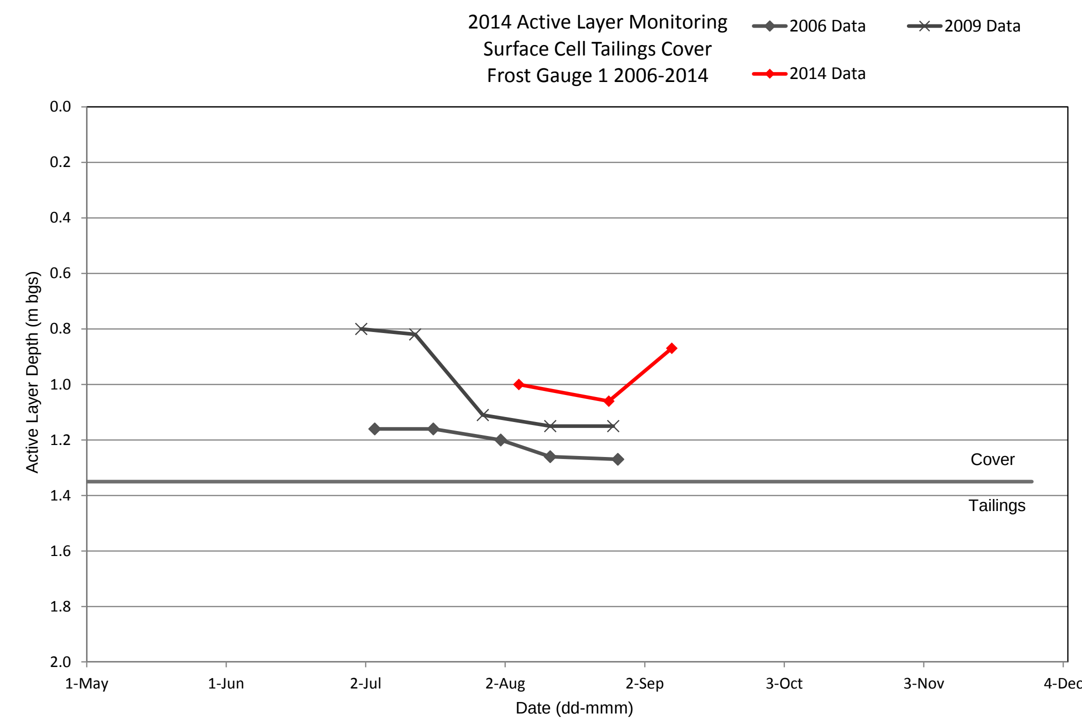
CLIENT: nyrstar

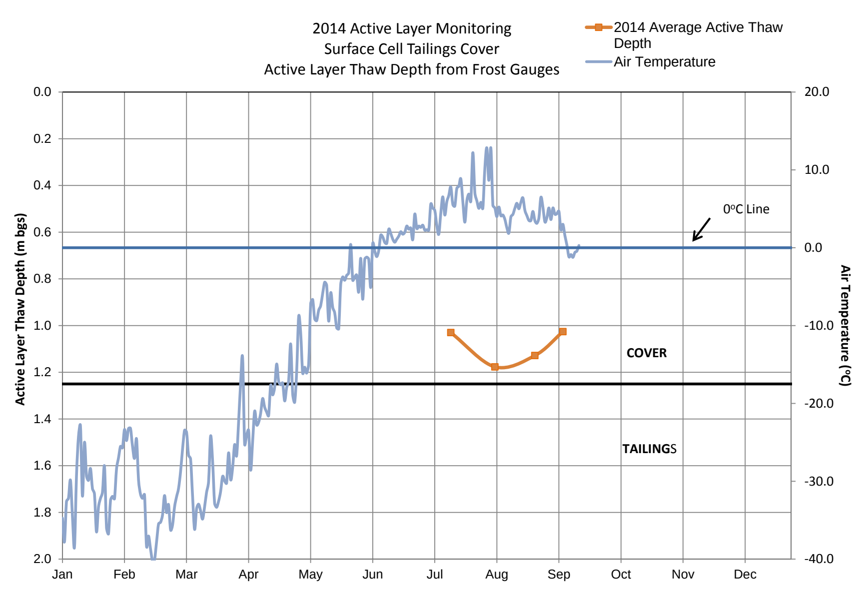
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: SURFACE CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 6

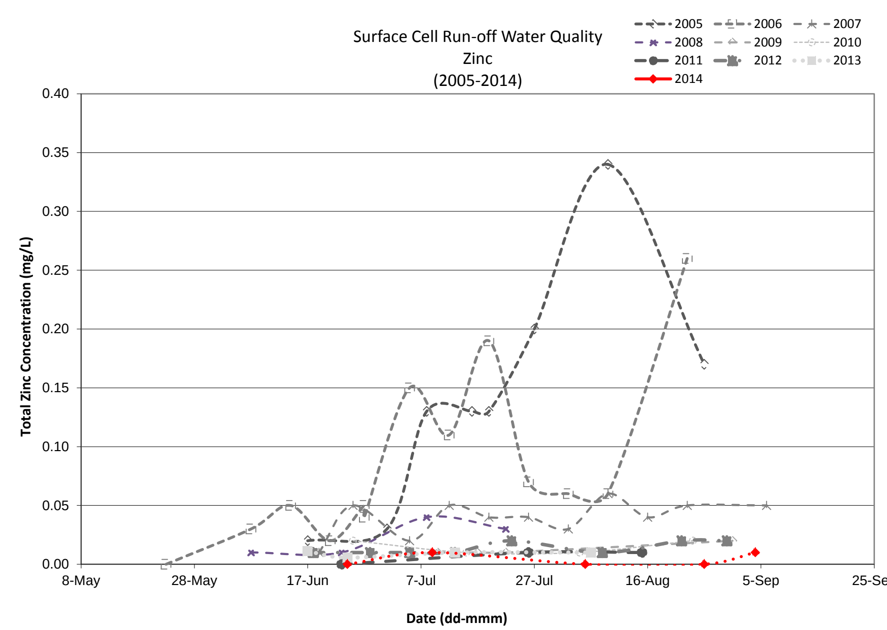
DWG No.:	PROJ No.:	REV:
11	0255-024-03	

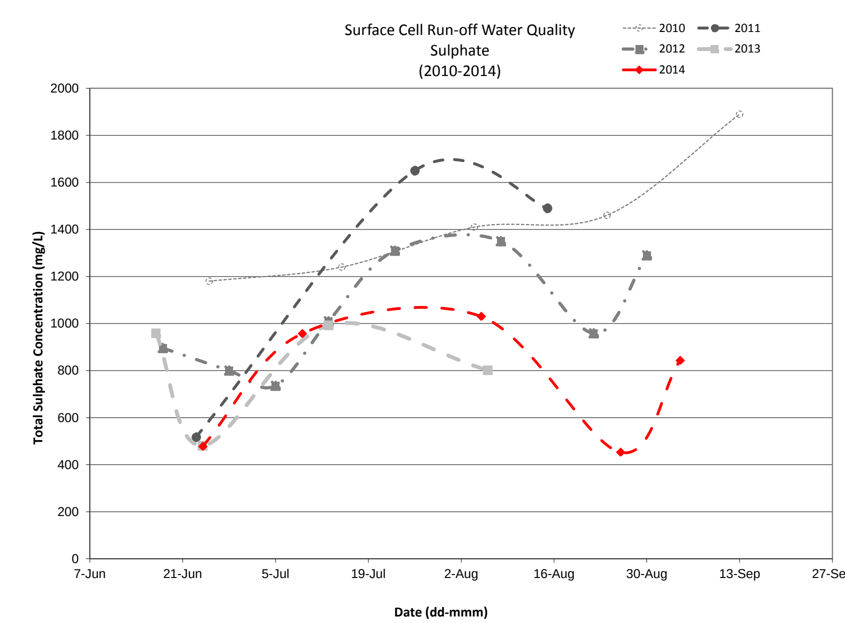






*2012-2013 Mean Daily Air Temperatures taken from the Arctic Bay weather station





DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: SURFACE CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 7

DWG No.:	PROJ No.:	REV:
12	0255-024-03	

NOTES:
1. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE NOTED.
2. THIS DRAWING MUST BE READ IN CONJUNCTION WITH BGC'S REPORT TITLED "NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION" AND DATED FEBRUARY 2015.
3. CONTOUR INTERVAL IS 1 m.
4. PROJECTION IS UTM NAD 83, Z16N.
5. UNLESS BGC AGREES OTHERWISE IN WRITING, THIS DRAWING SHALL NOT BE MODIFIED OR USED FOR ANY PURPOSE OTHER THAN THE PURPOSE FOR WHICH BGC GENERATED IT. BGC SHALL HAVE NO LIABILITY FOR ANY DAMAGES OR LOSS ARISING IN ANY WAY FROM ANY USE OR MODIFICATION OF THIS DOCUMENT NOT AUTHORIZED BY BGC. ANY USE OF OR RELIANCE UPON THIS DOCUMENT OR ITS CONTENT BY THIRD PARTIES SHALL BE AT SUCH THIRD PARTIES' SOLE RISK.

SCALE:	1:2,500
DATE:	FEB 2015
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CHECKED:	MCL
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: TEST CELL TAILINGS COVER

PROJECT No.: 0255-024-03
DWG No.: 13

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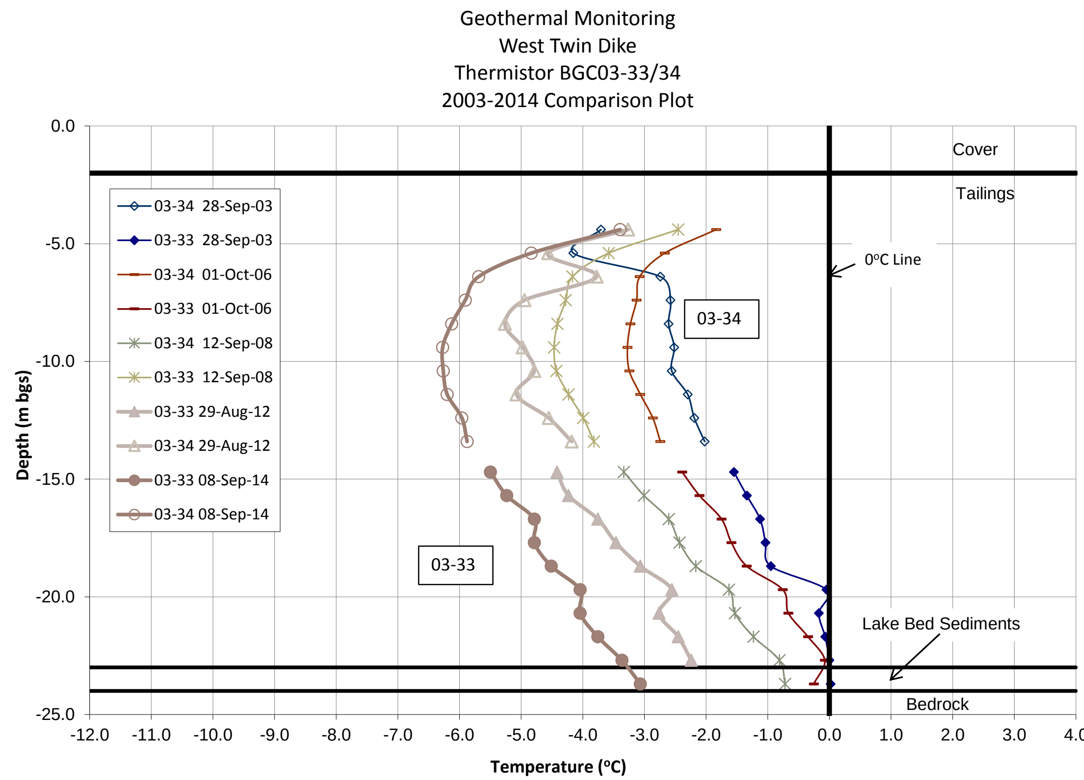
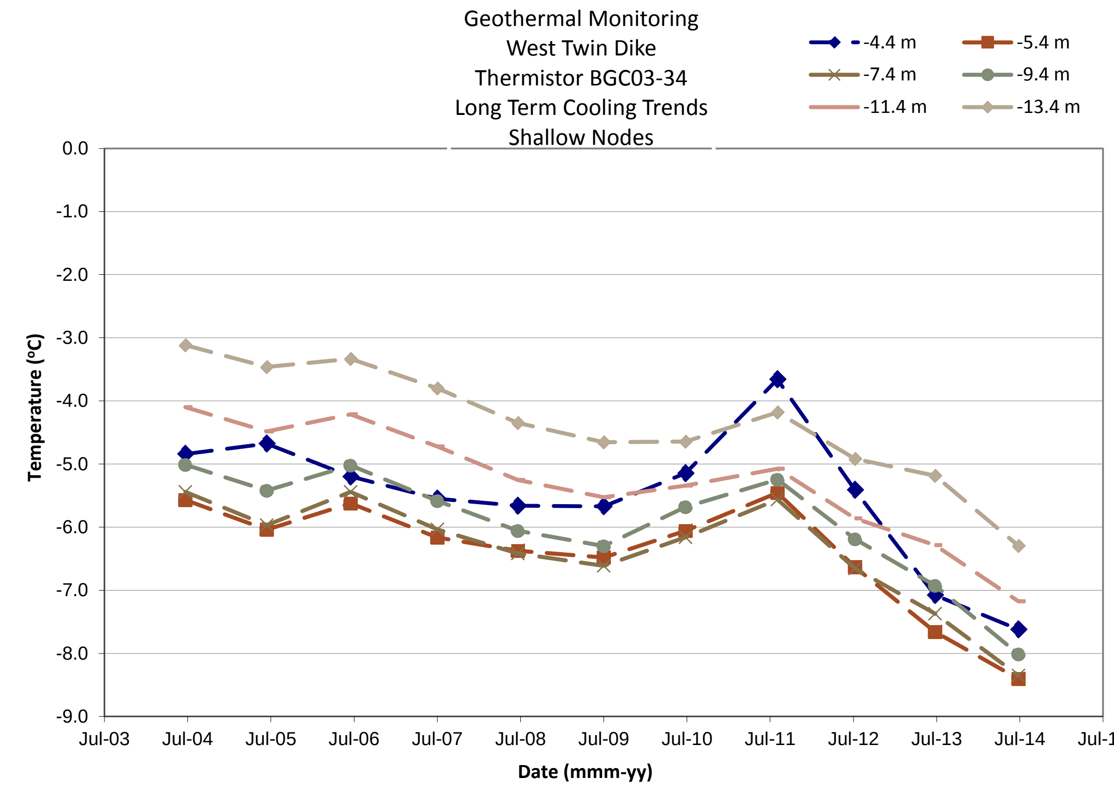
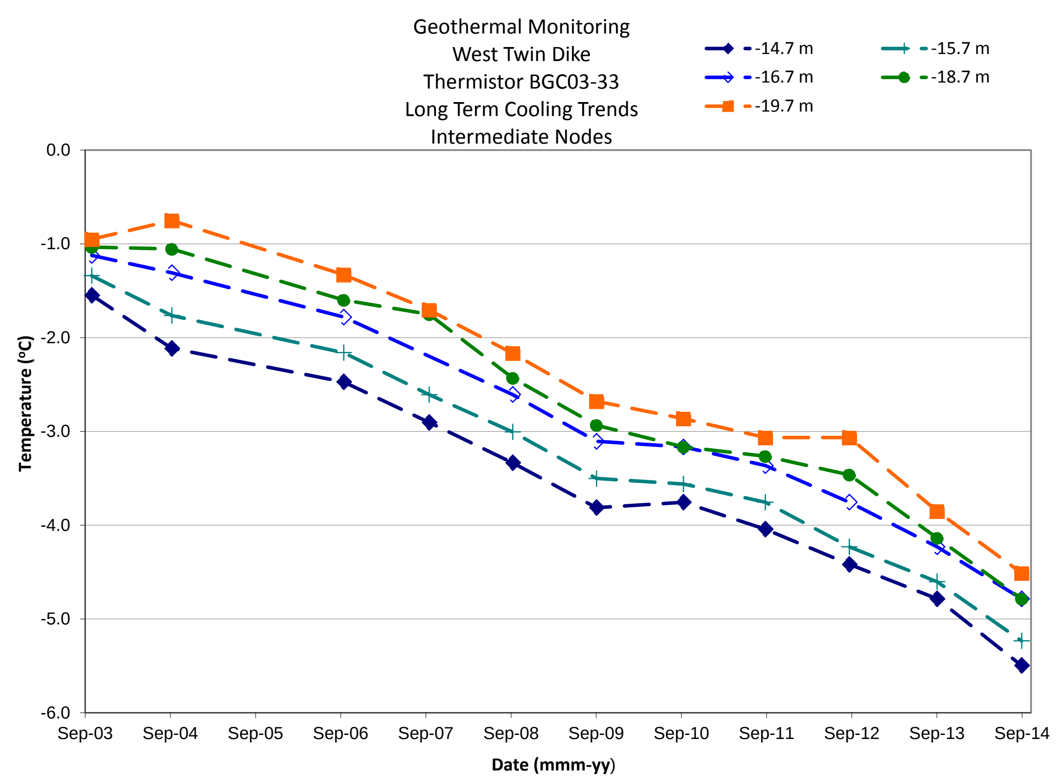
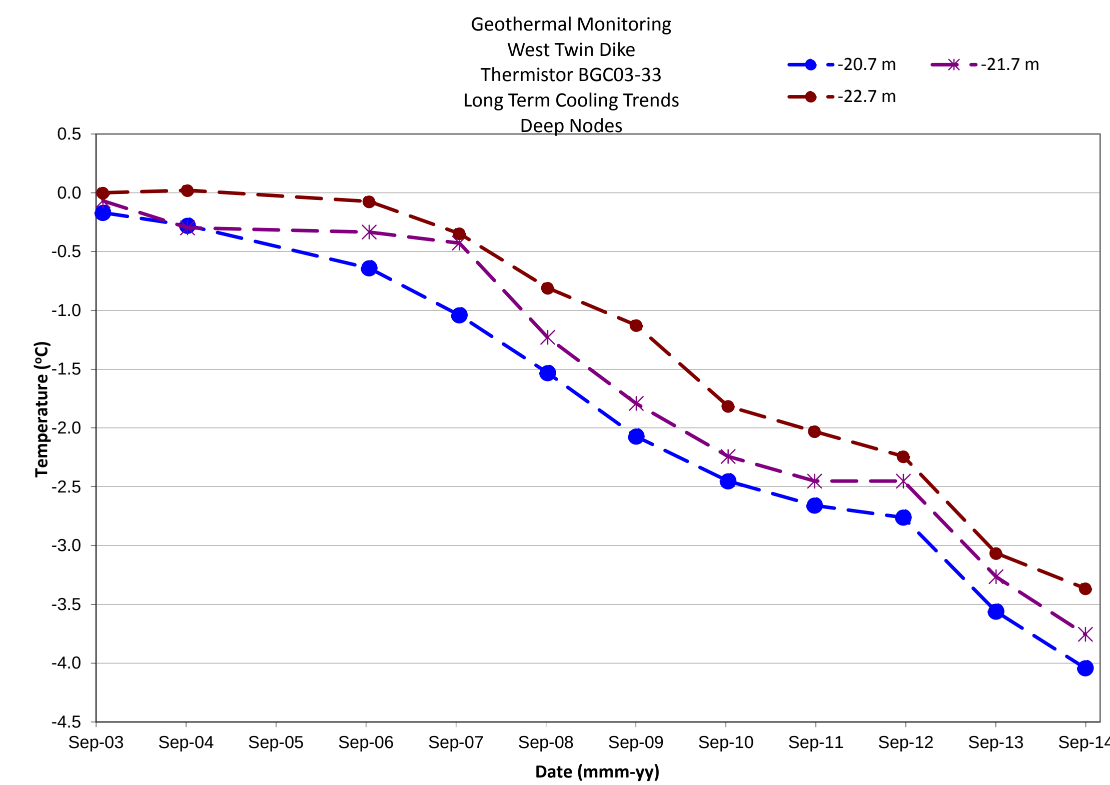








DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

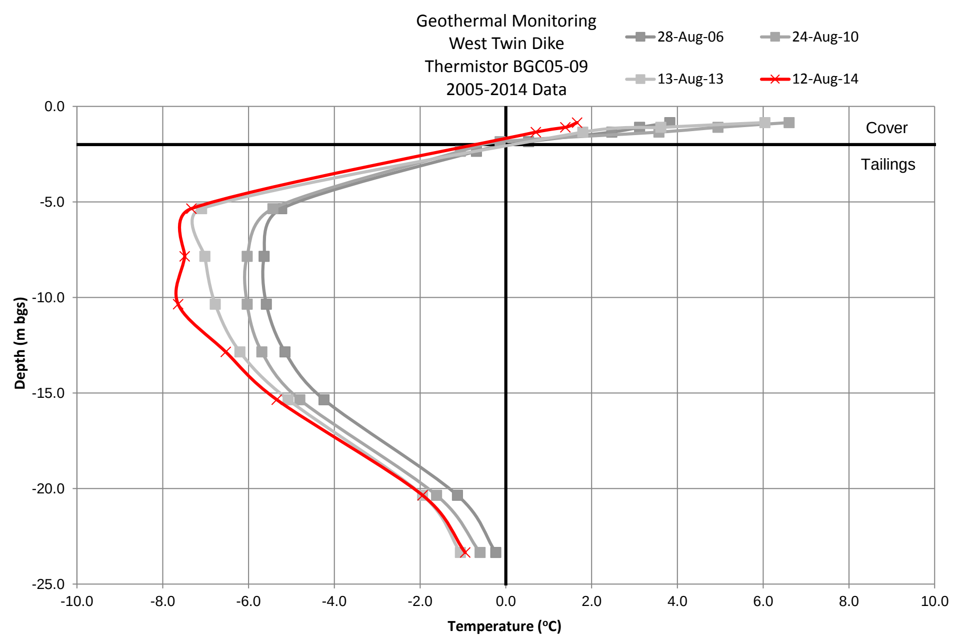
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

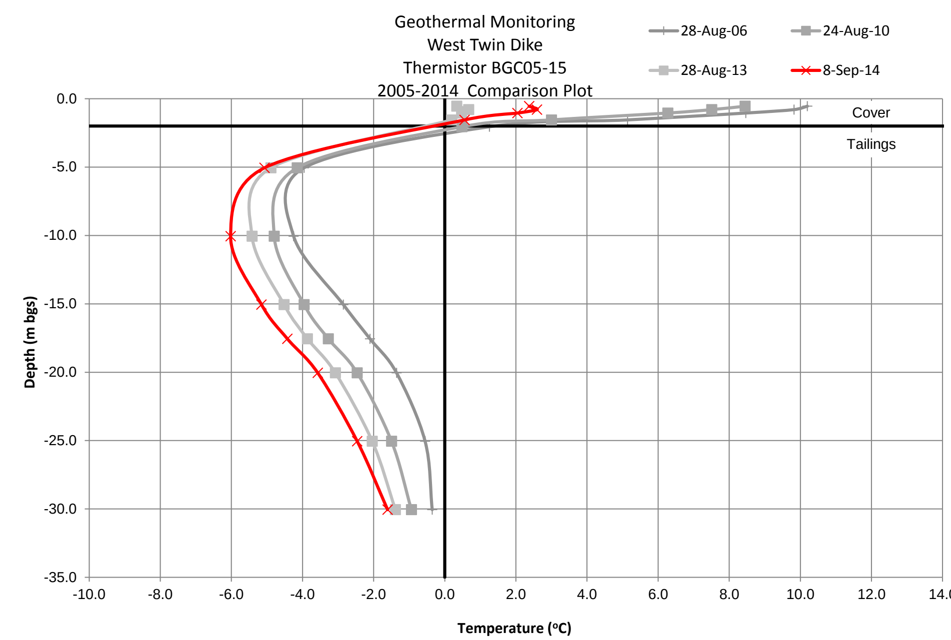
TITLE: WEST TWIN DIKE
GEOTECHNICAL MONITORING DATA 1

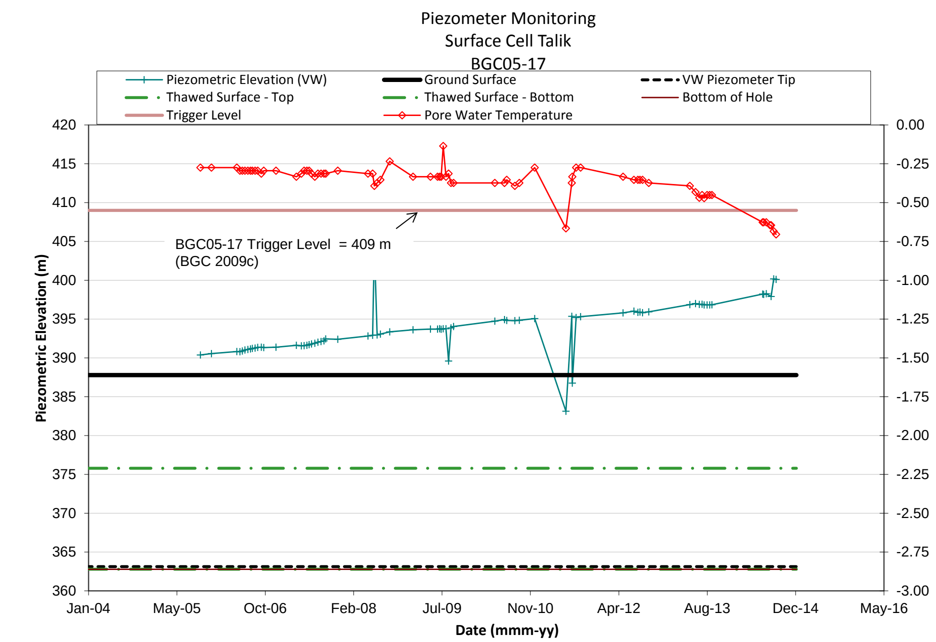
DWG No.: 14

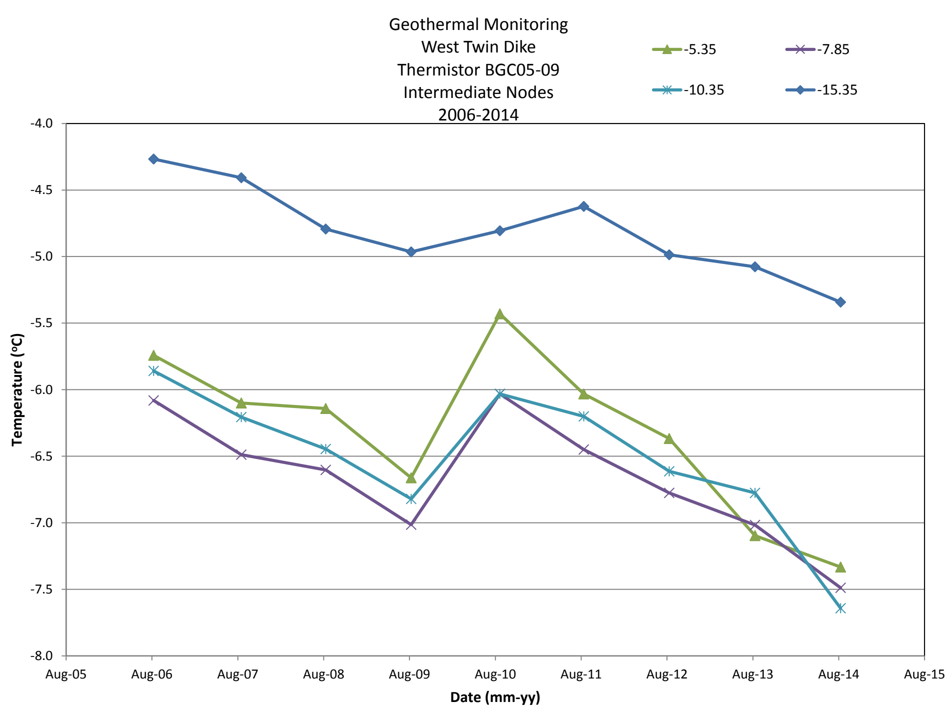
PROJ No.: 0255-024-03

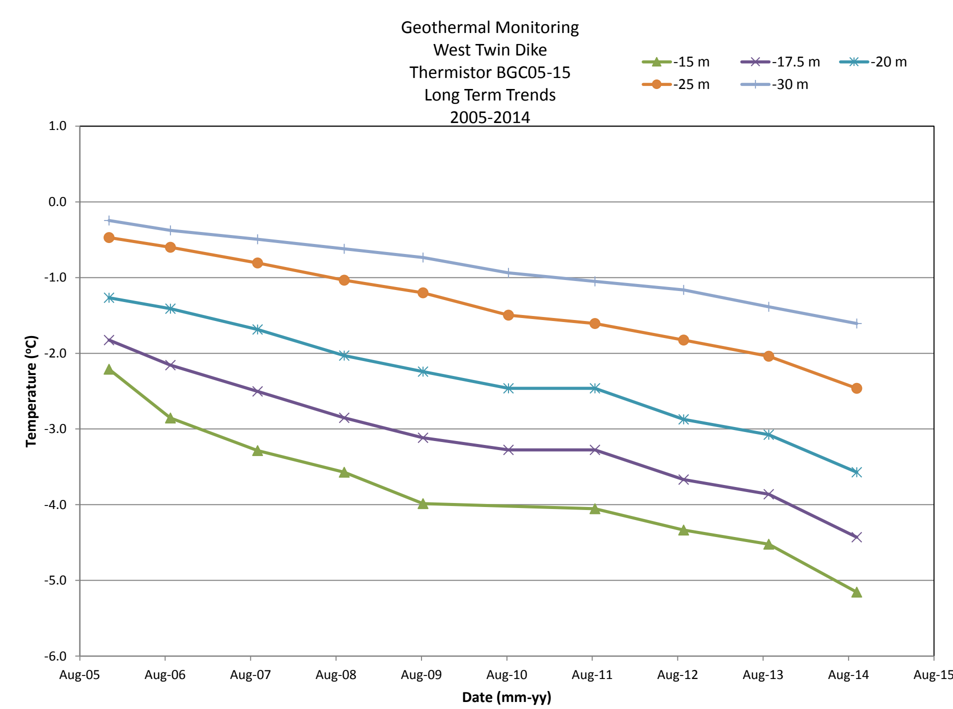
REV:











DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

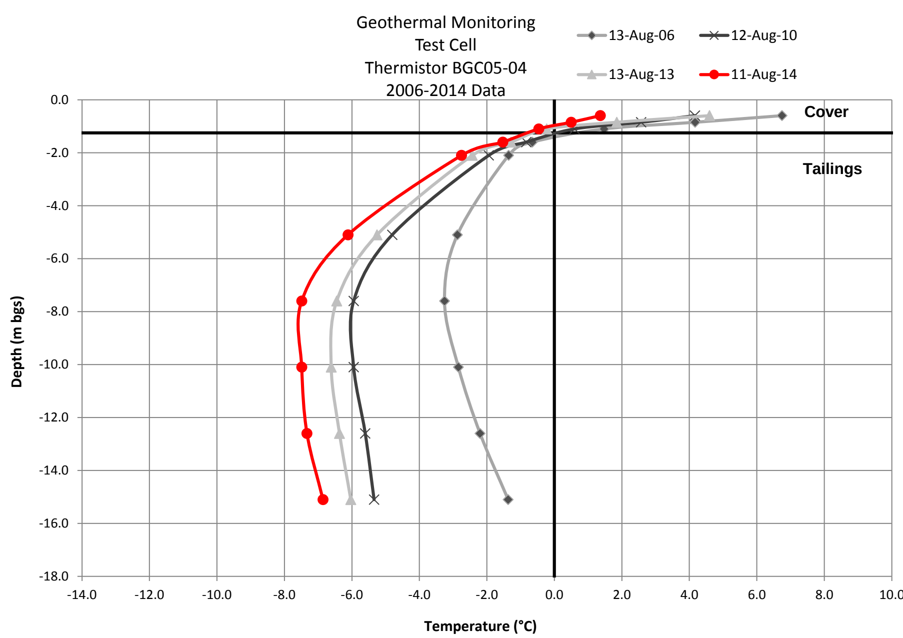
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

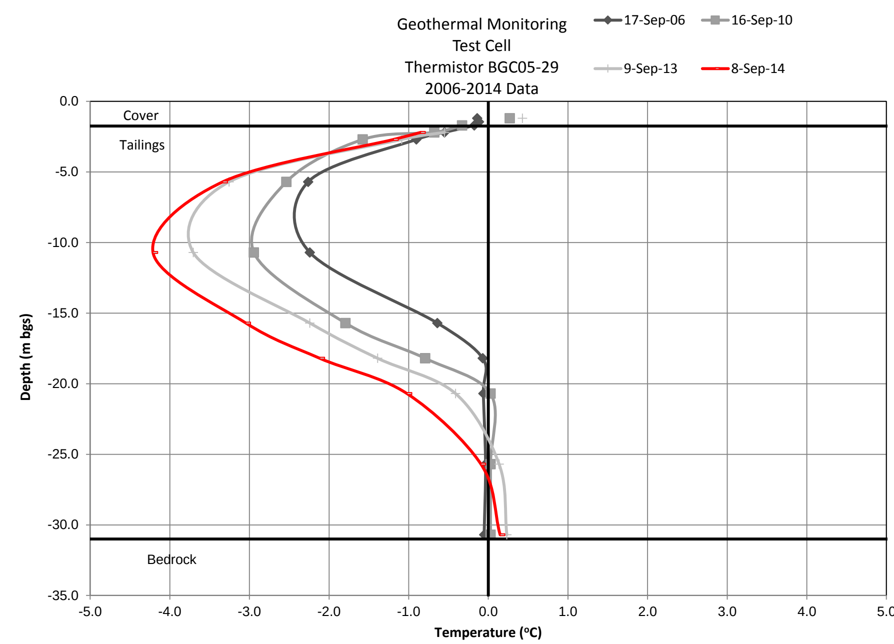
TITLE: WEST TWIN DIKE
GEOTECHNICAL MONITORING DATA 2

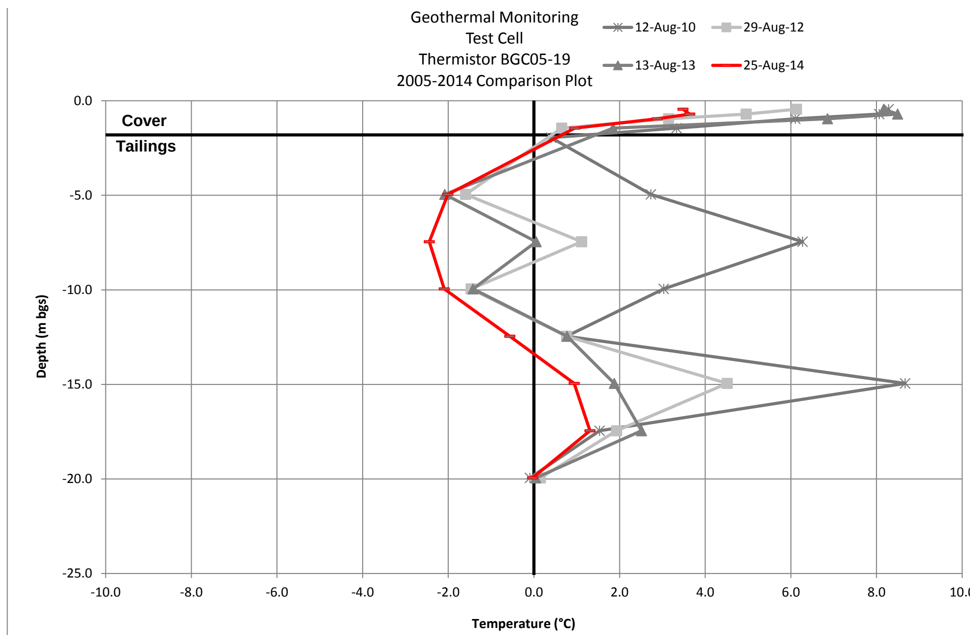
DWG No.: 15

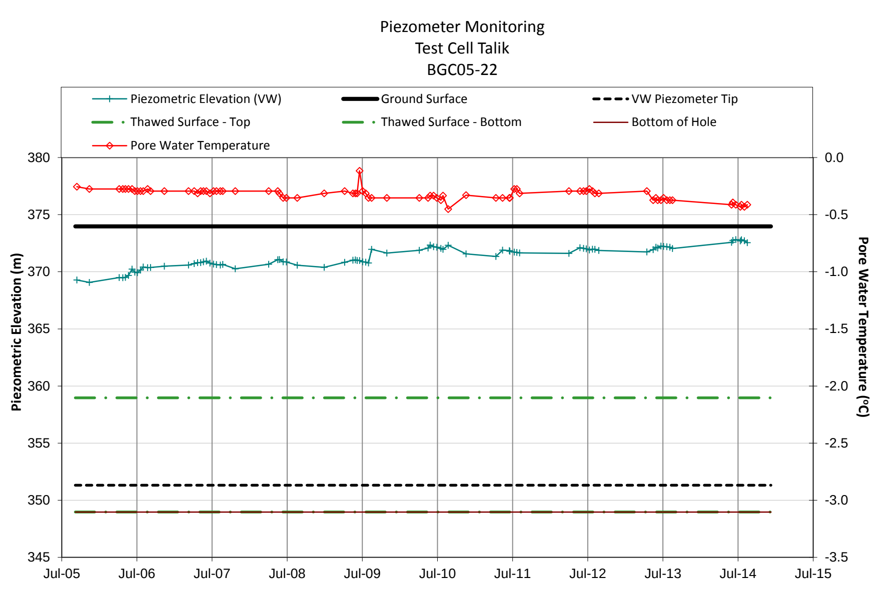
PROJ No.: 0255-024-03

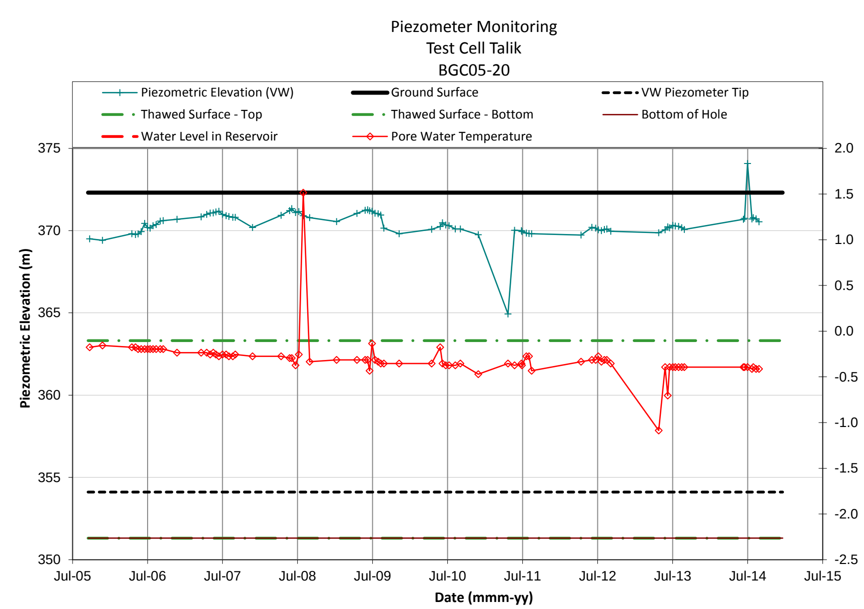
REV:

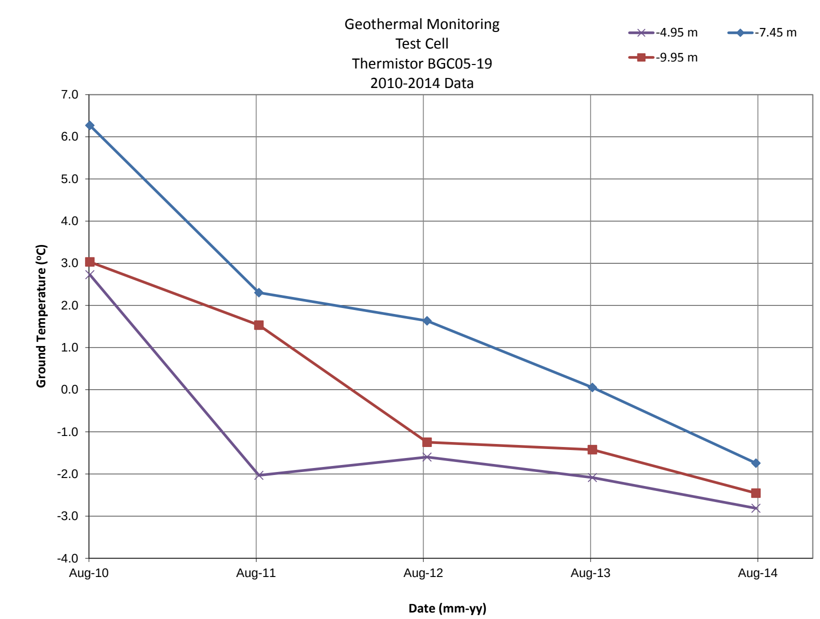












DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

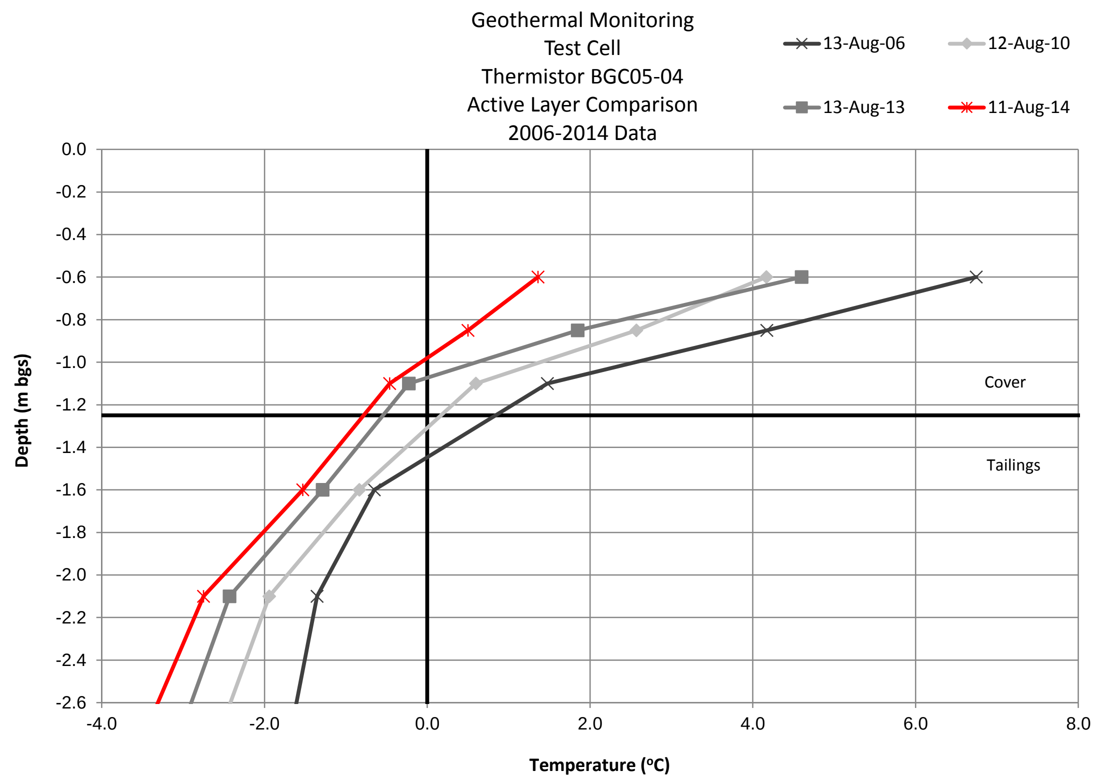
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

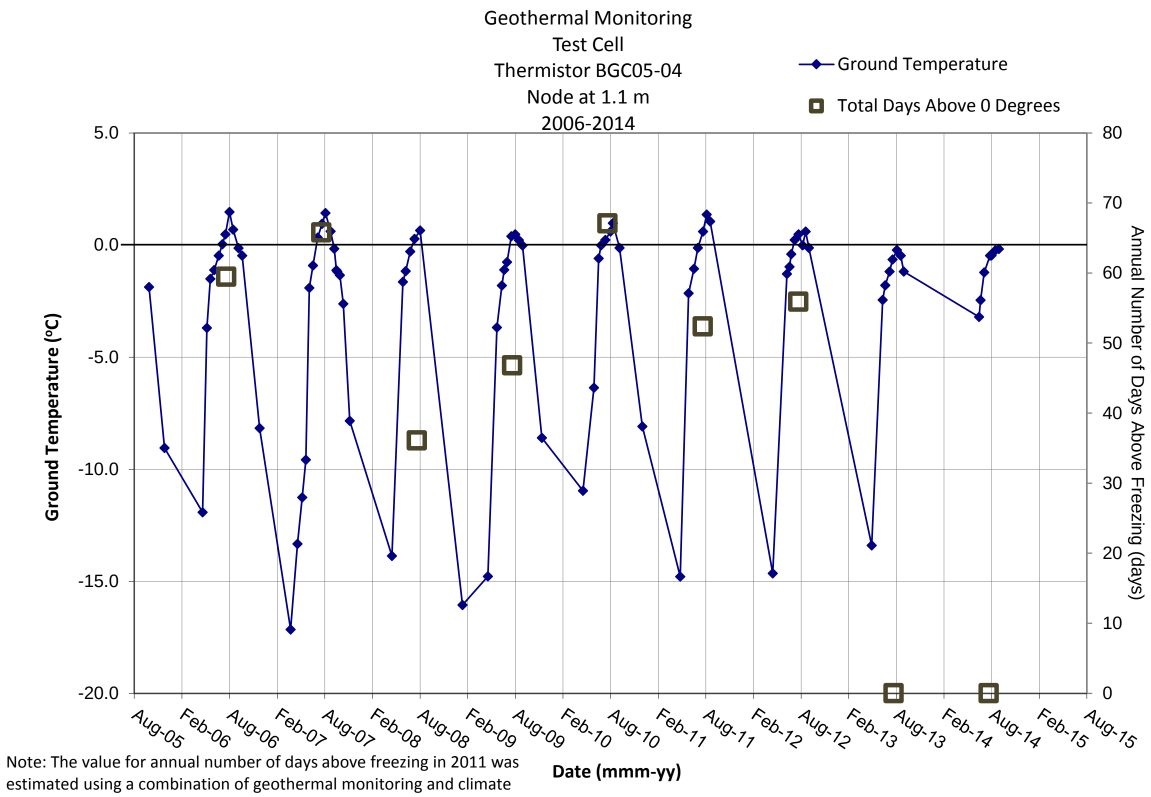
CLIENT: nyrstar

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

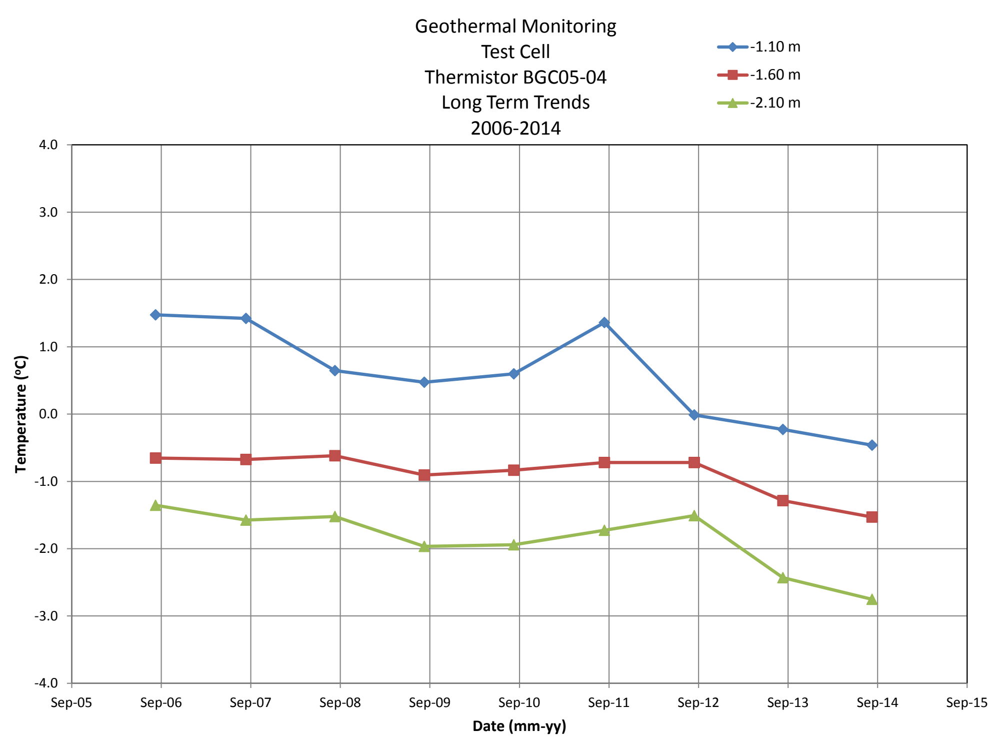
TITLE: TEST CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 1

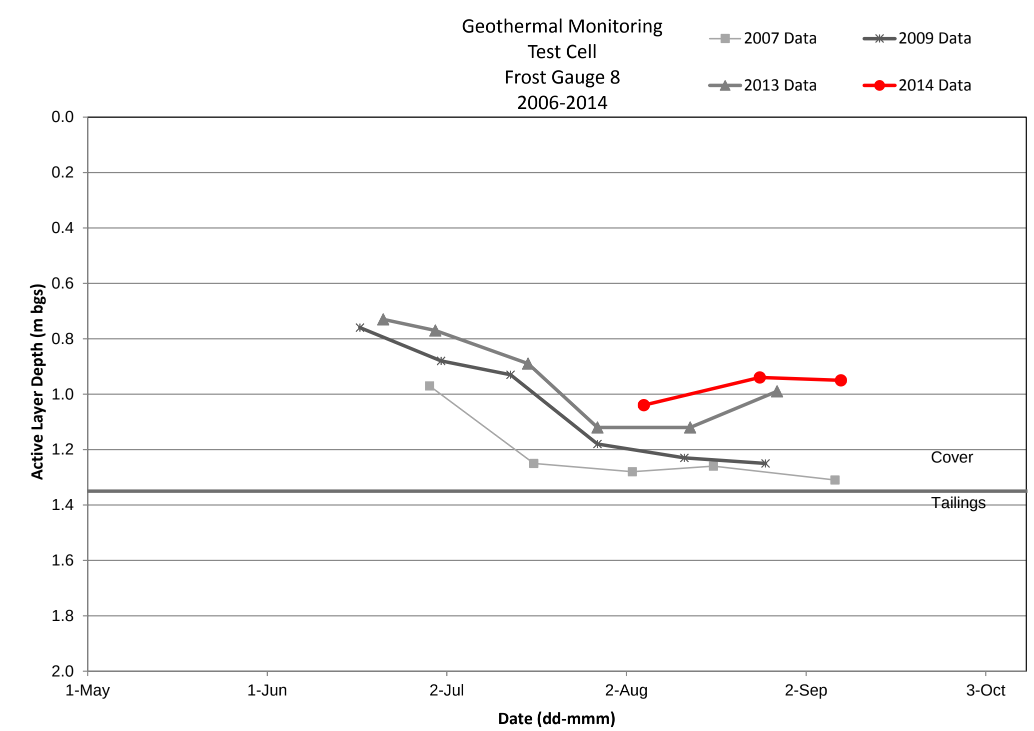
DWG No.:	PROJ No.:	REV:
16	0255-024-03	





Note: The value for annual number of days above freezing in 2011 was estimated using a combination of geothermal monitoring and climate data.





DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

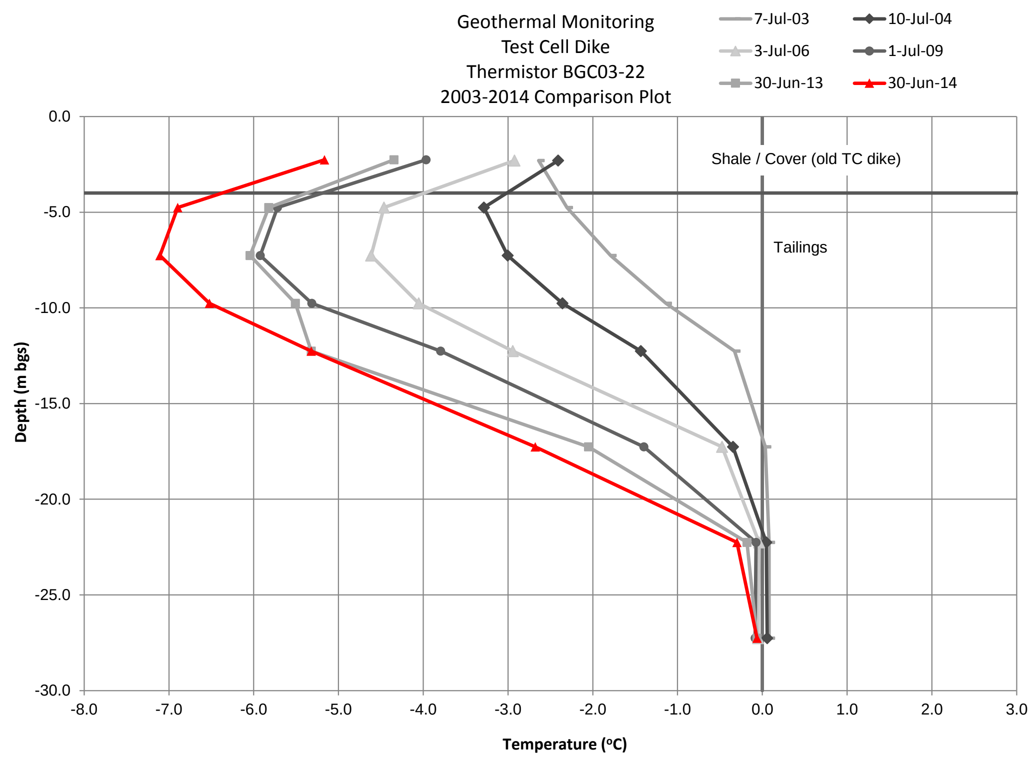
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

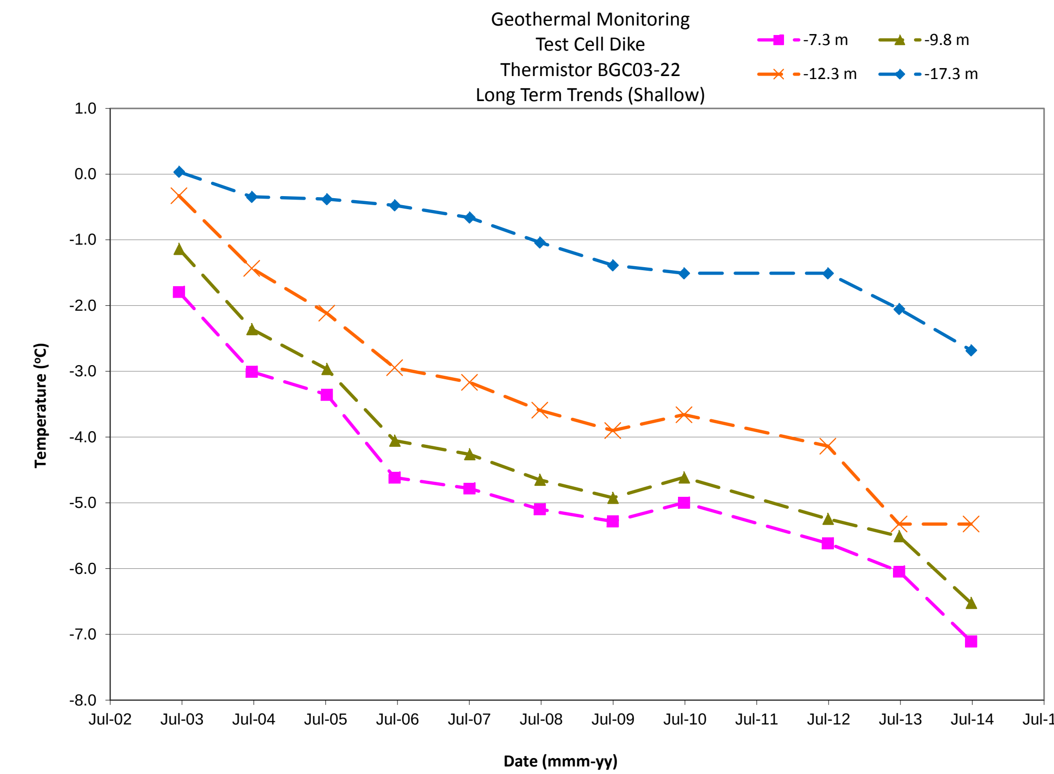
CLIENT: nyrstar

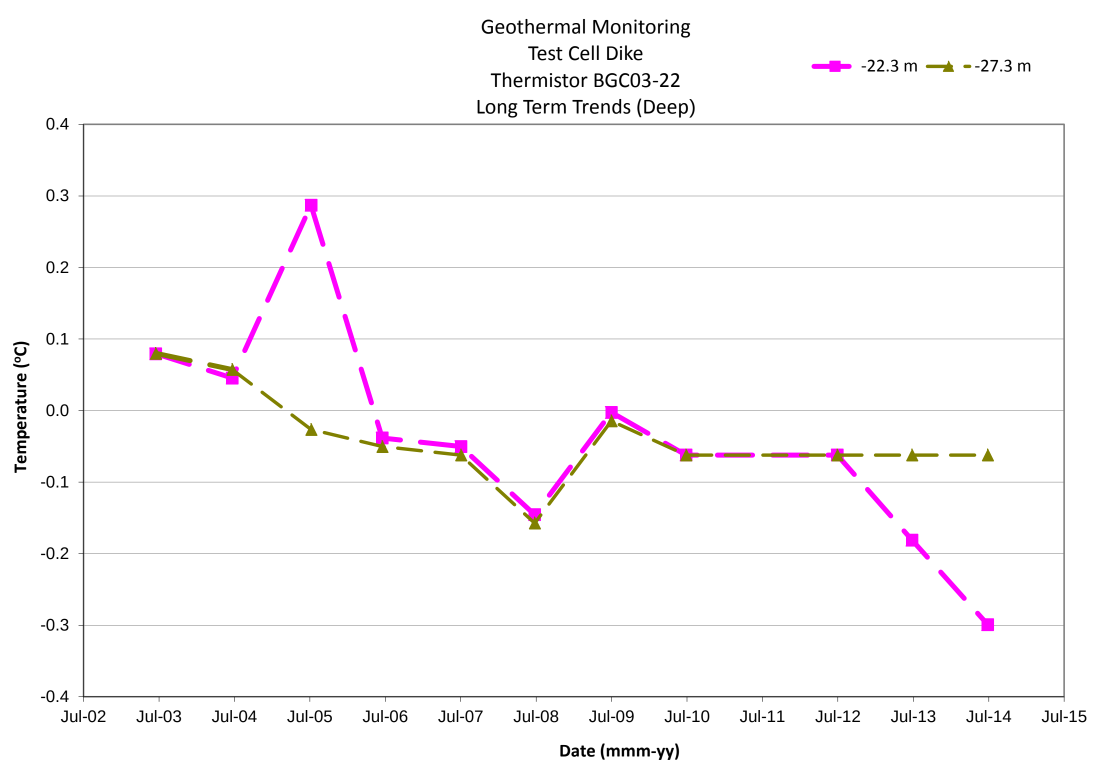
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

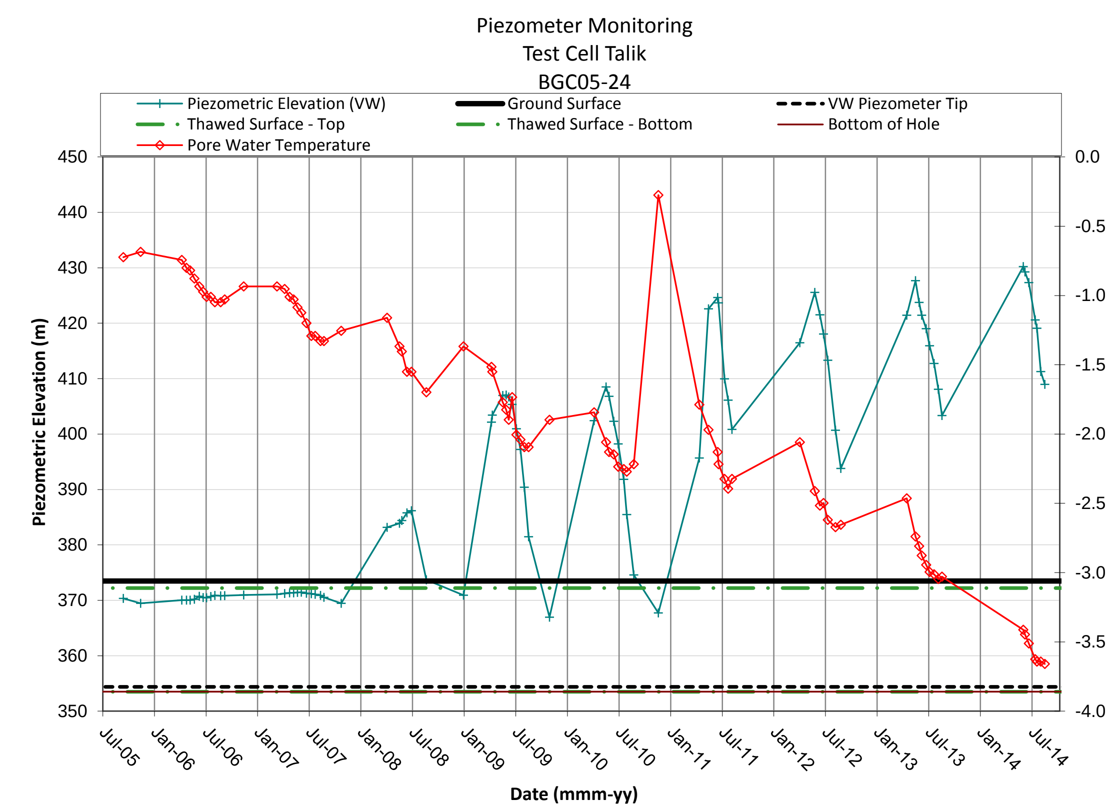
TITLE: TEST CELL TAILINGS COVER
GEOTECHNICAL MONITORING DATA 2

DWG No.:	PROJ No.:	REV:
17	0255-024-03	









DWG TO BE READ WITH BGC REPORT TITLED: "NANISVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
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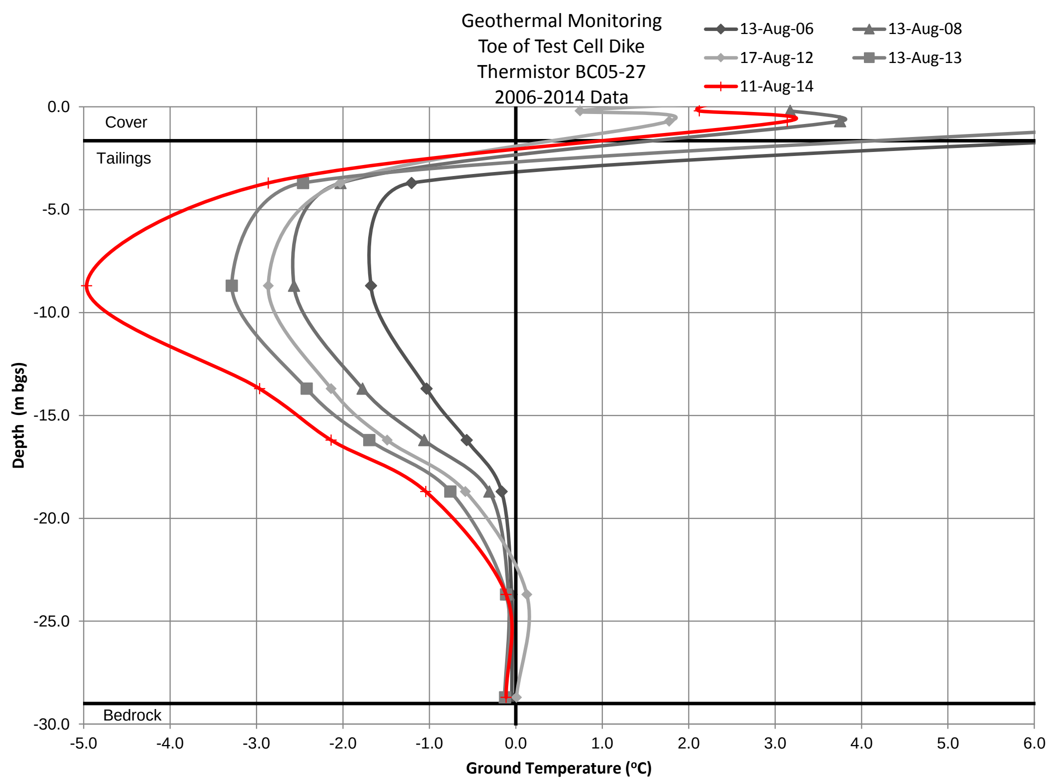
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

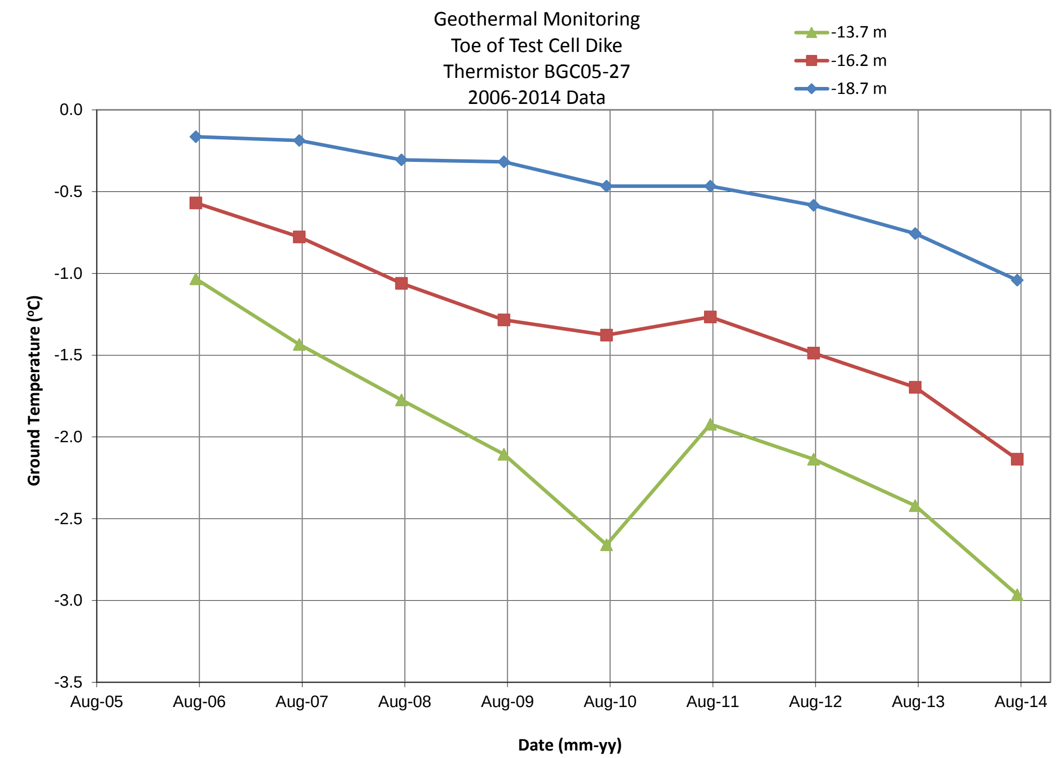
CLIENT: nyrstar

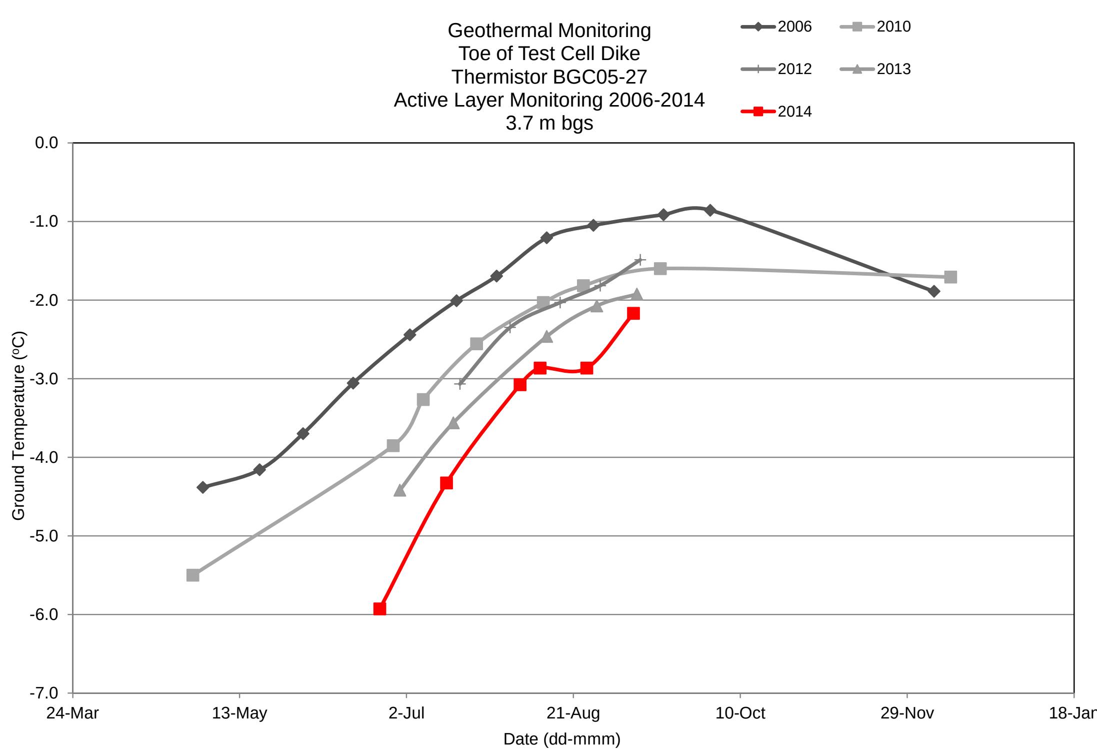
PROJECT: NANISVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

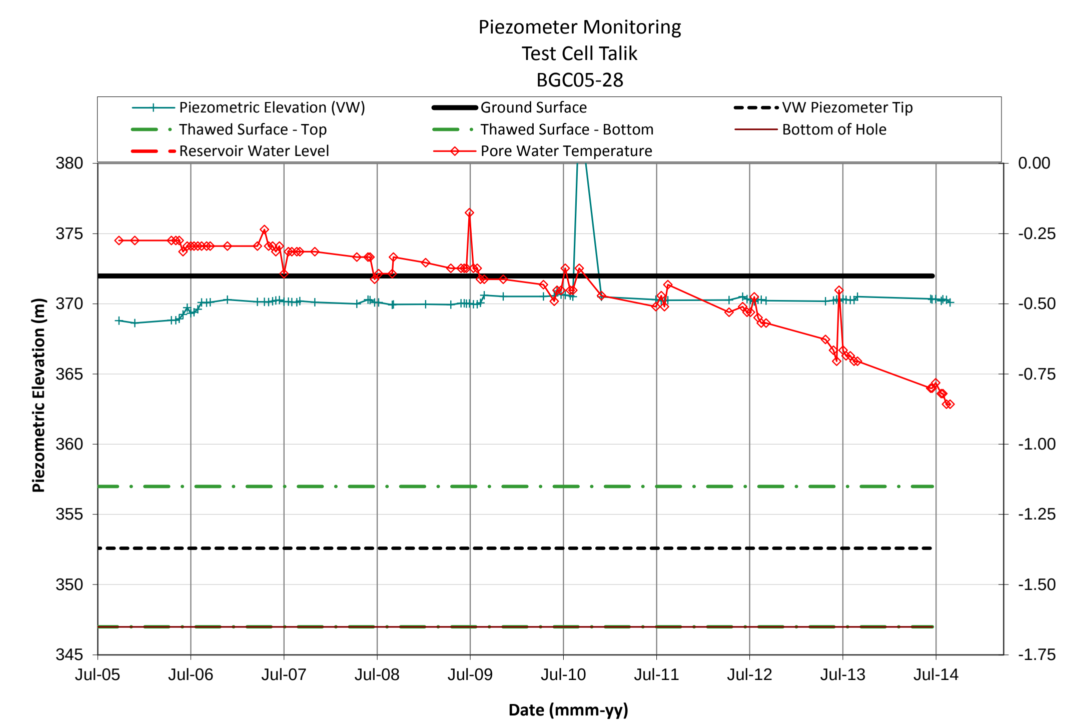
TITLE: TEST CELL DIKE
GEOTECHNICAL MONITORING DATA

DWG No.:	PROJ No.:	REV:
18	0255-024-03	









DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
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CLIENT: nyrstar

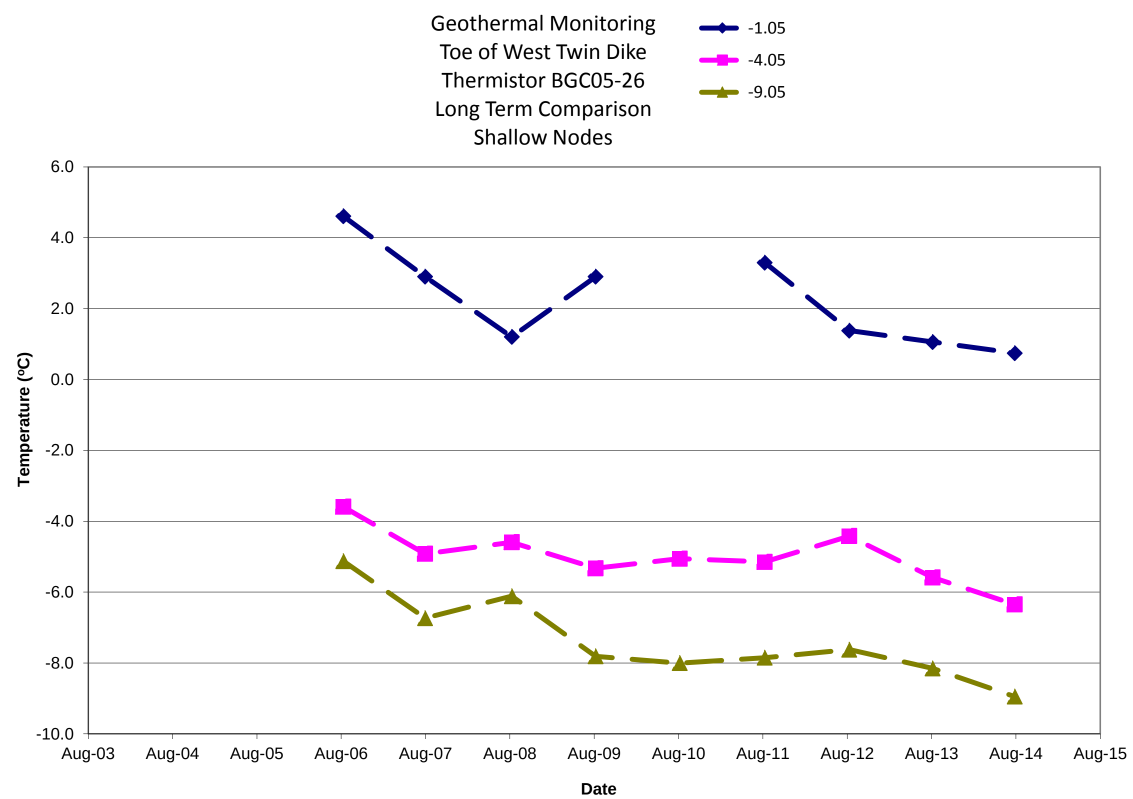
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

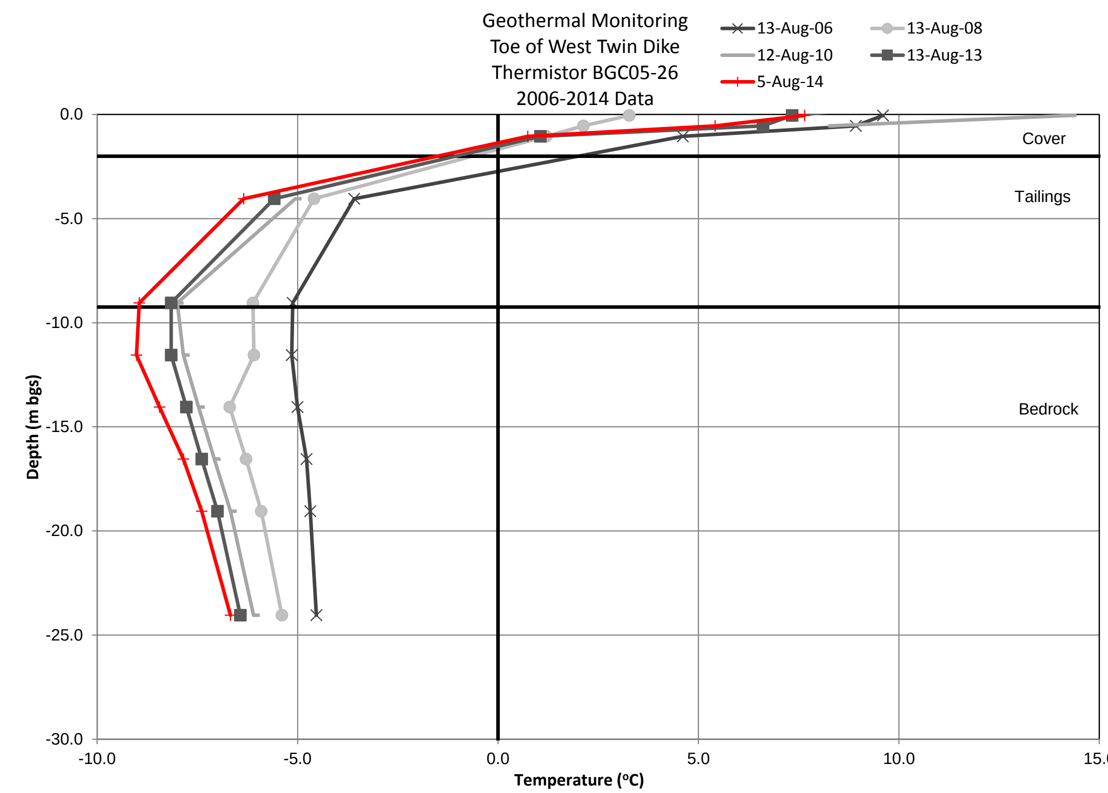
TITLE: TOE OF TEST CELL DIKE TAILINGS COVER
GEOTECHNICAL MONITORING DATA

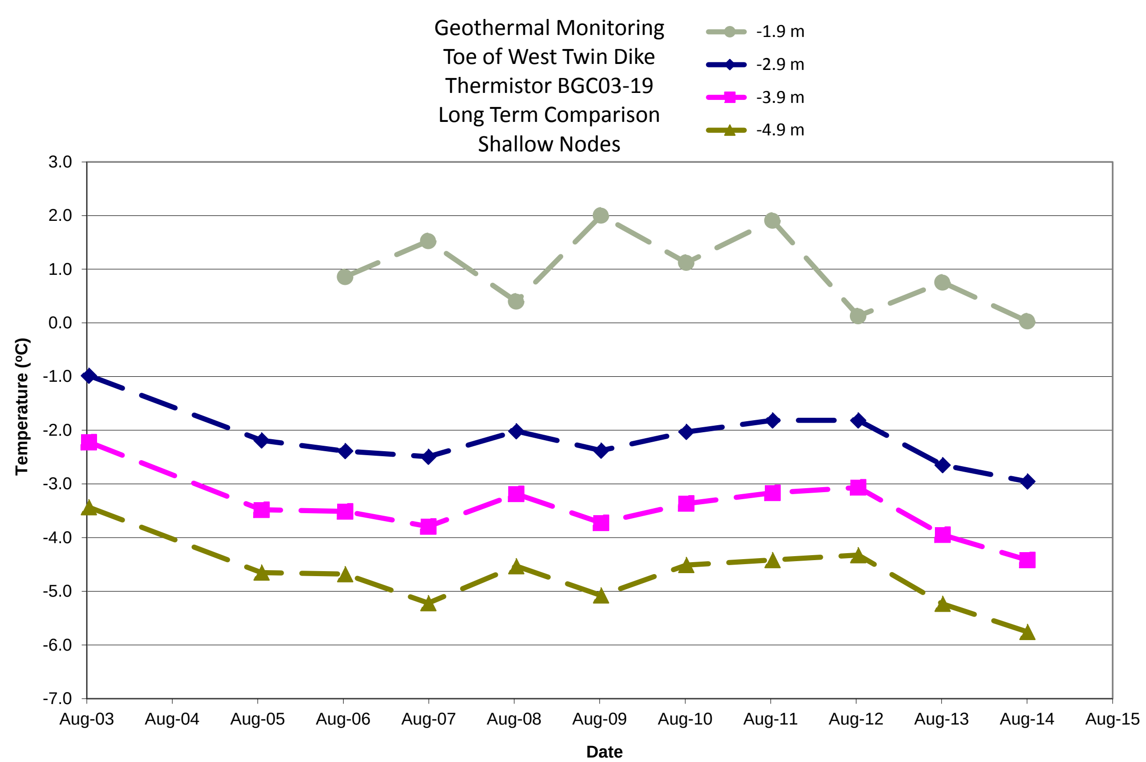
DWG No.: 19

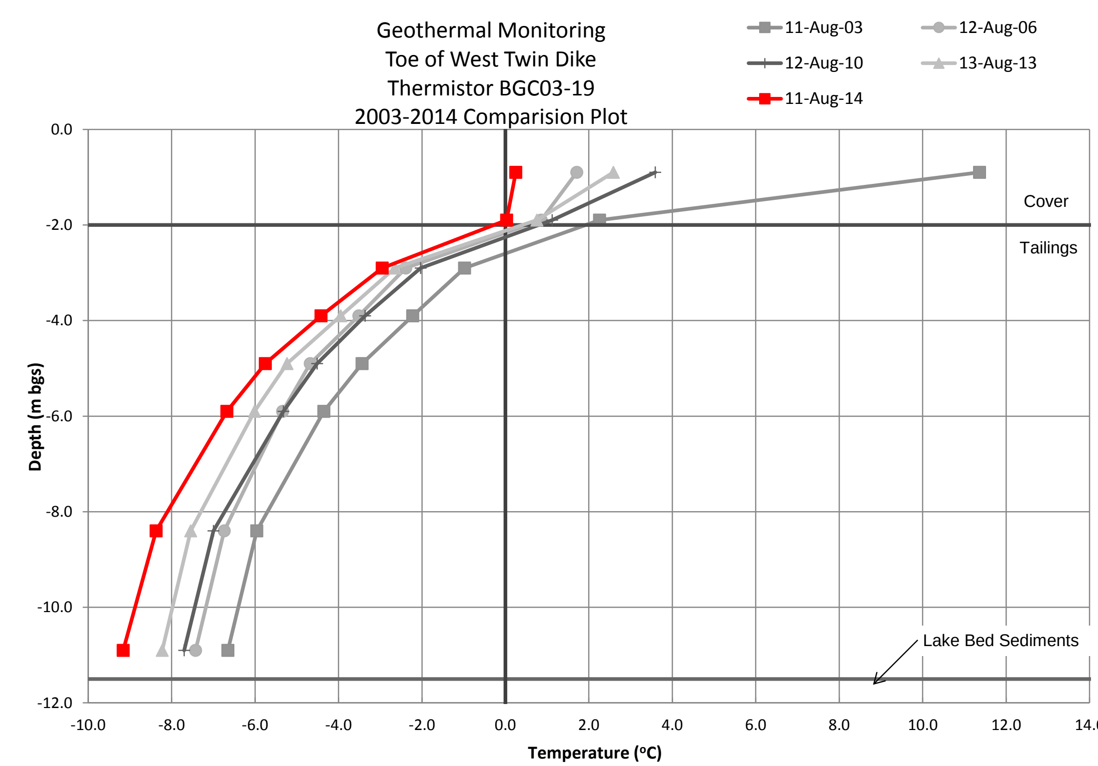
PROJ No.: 0255-024-03

REV:









DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

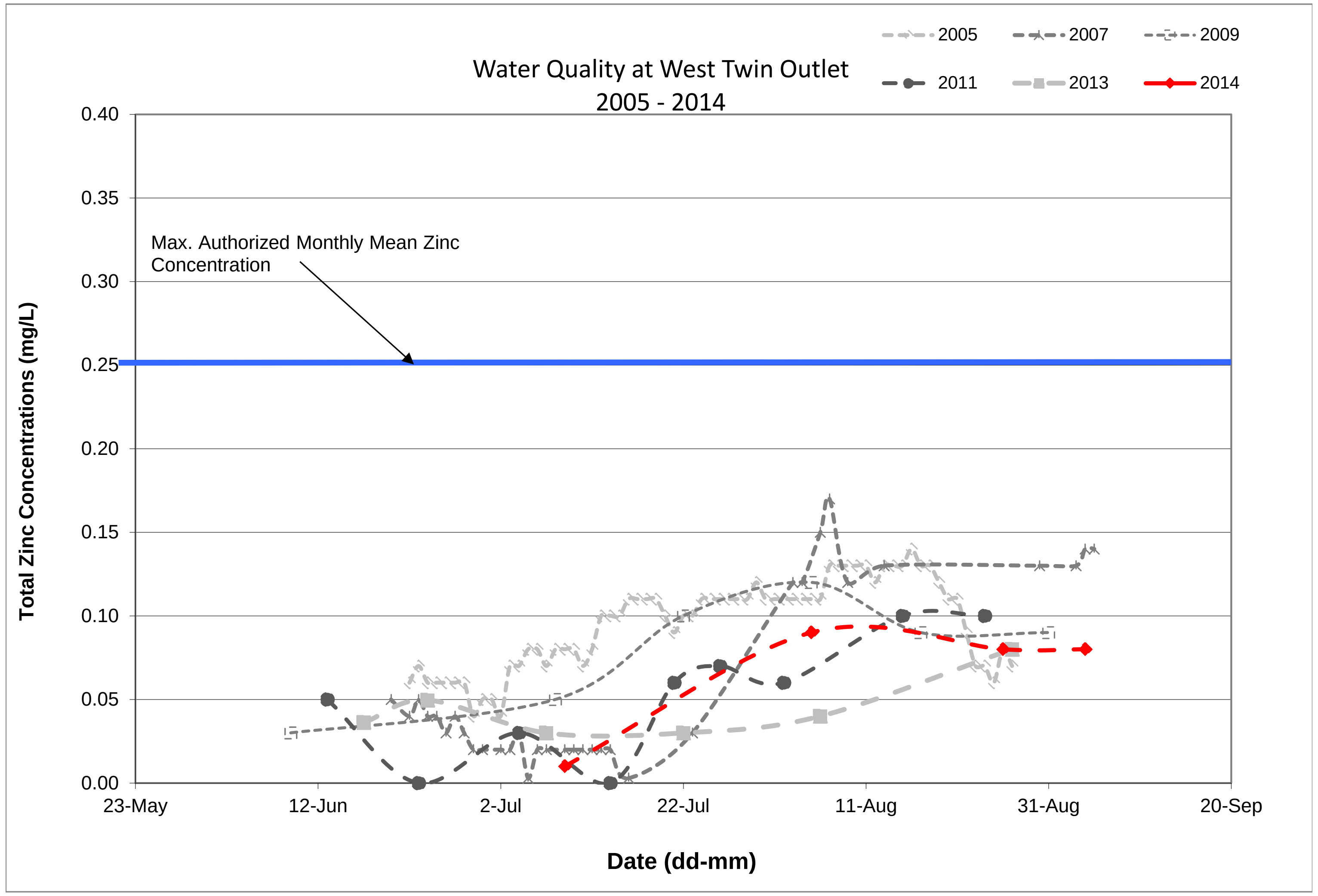
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

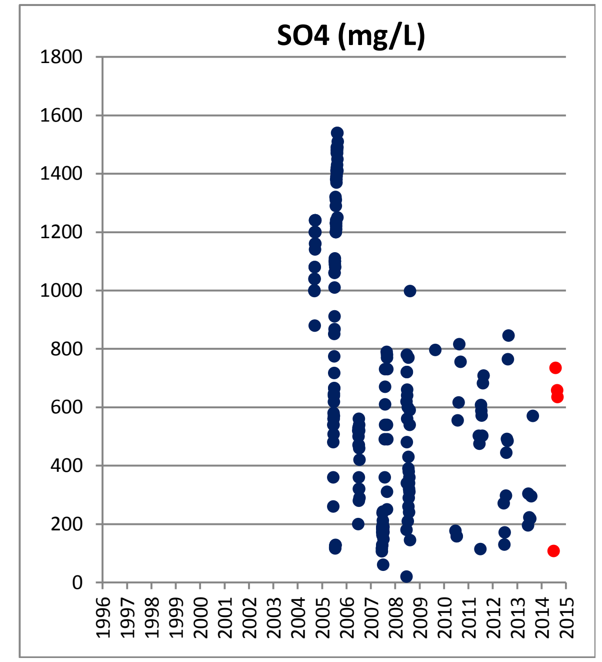
CLIENT: nyrstar

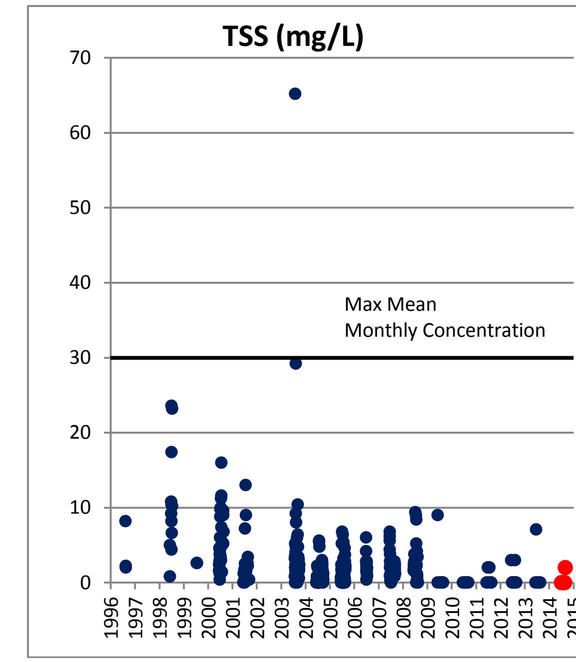
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

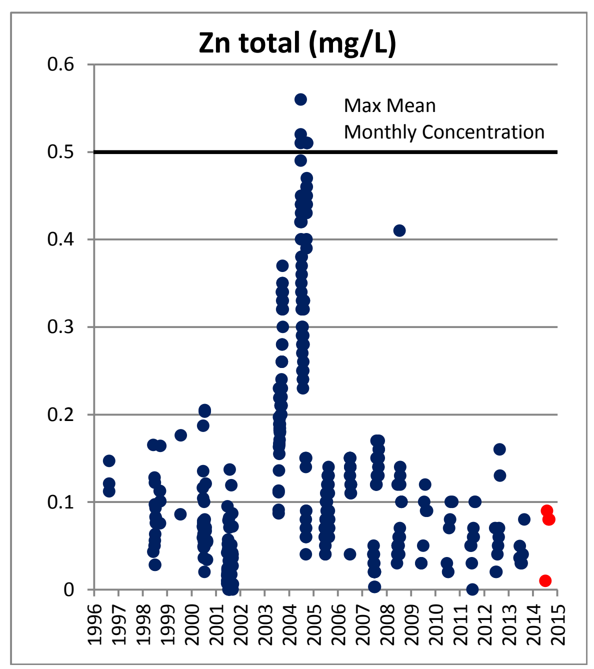
TITLE: TOE OF WEST TWIN DIKE TAILINGS COVER GEOTECHNICAL MONITORING DATA

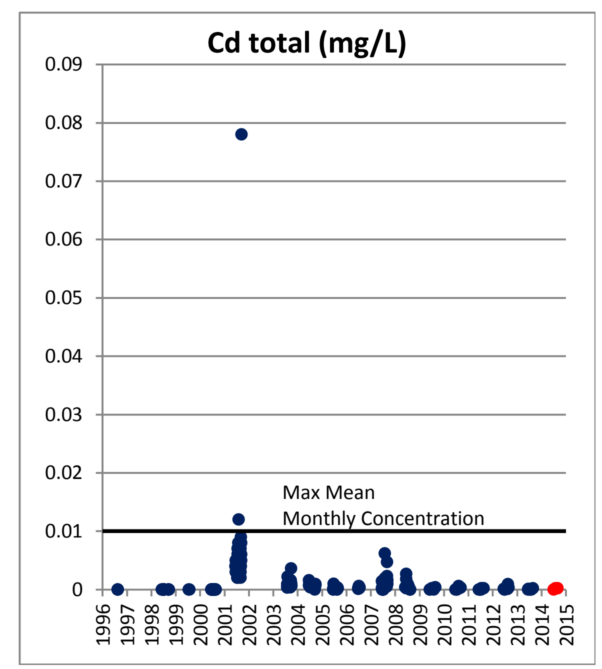
DWG No.:	PROJ No.:	REV:
20	0255-024-03	

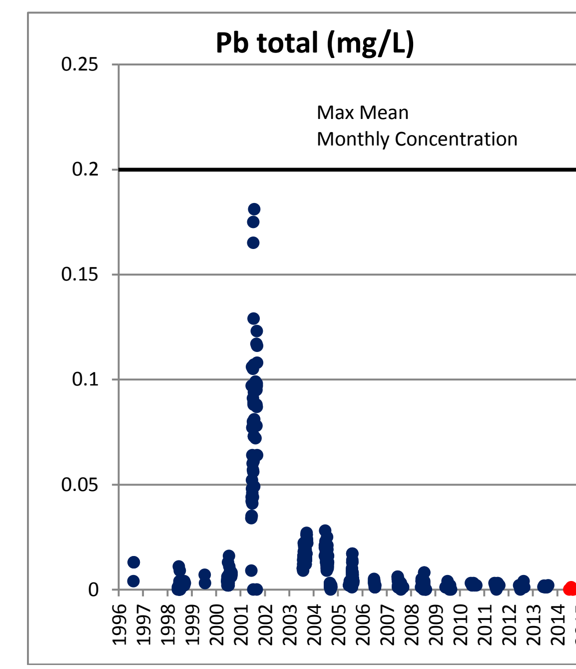












Water Quality data obtained from the Stantec annual water quality report (Stantec 2015) [currently in progress]

DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

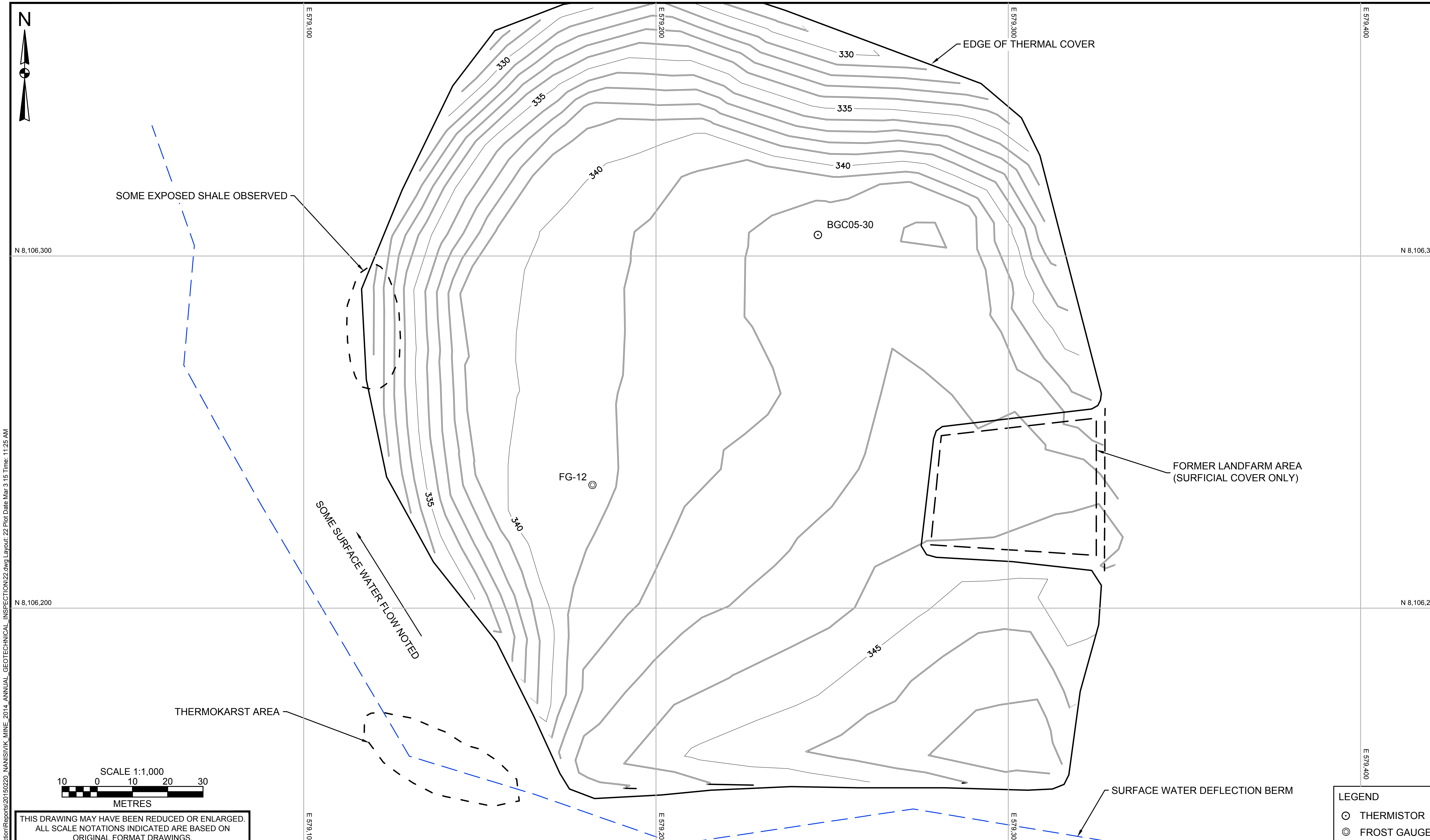
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: WEST TWIN DISPOSAL AREA
WATER QUALITY DATA

DWG No.:	PROJ No.:	REV:
21	0255-024-03	



NOTES:

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2. THIS DRAWING MUST BE READ IN CONJUNCTION WITH BGC'S REPORT TITLED "NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION", AND DATED FEBRUARY 2015.
3. CONTOUR INTERVAL IS 1 m.
4. PROJECTION IS UTM NAD 83 ZONE 16N.
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DATE:	FEB 2015
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BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

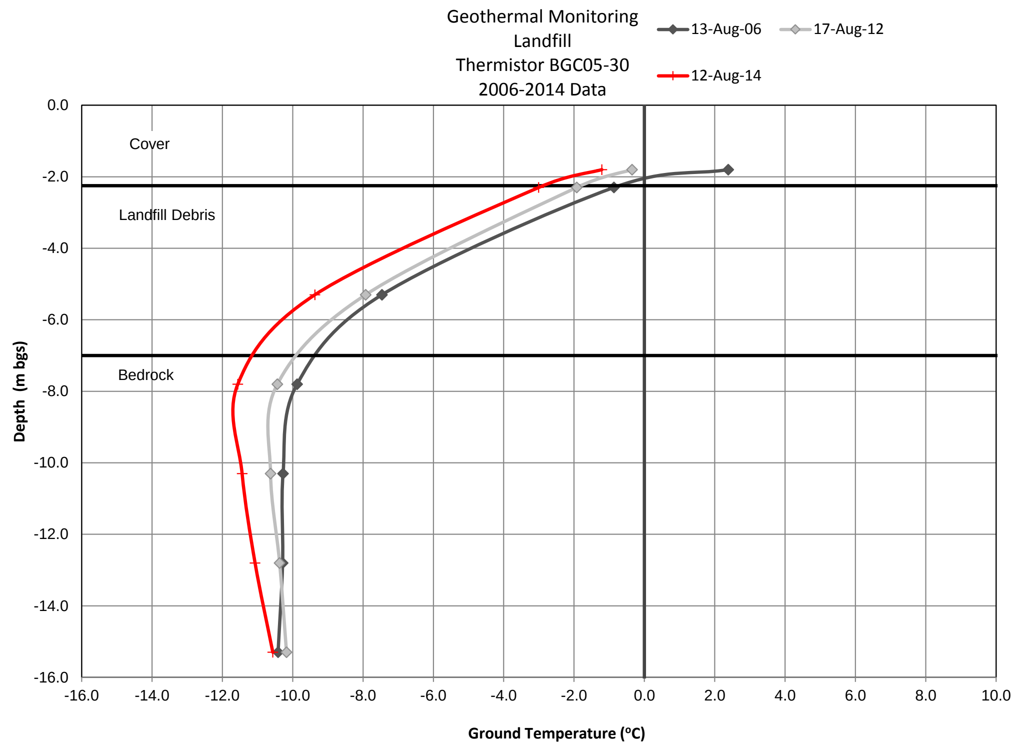
CLIENT: nyrstar

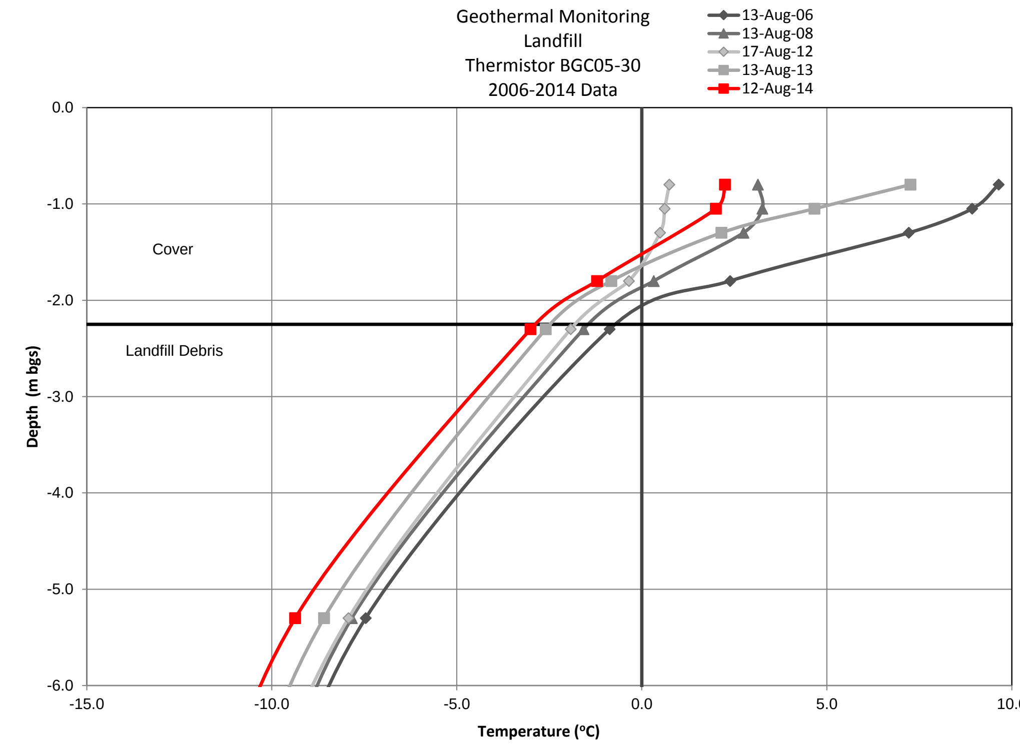
PROJECT: NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION

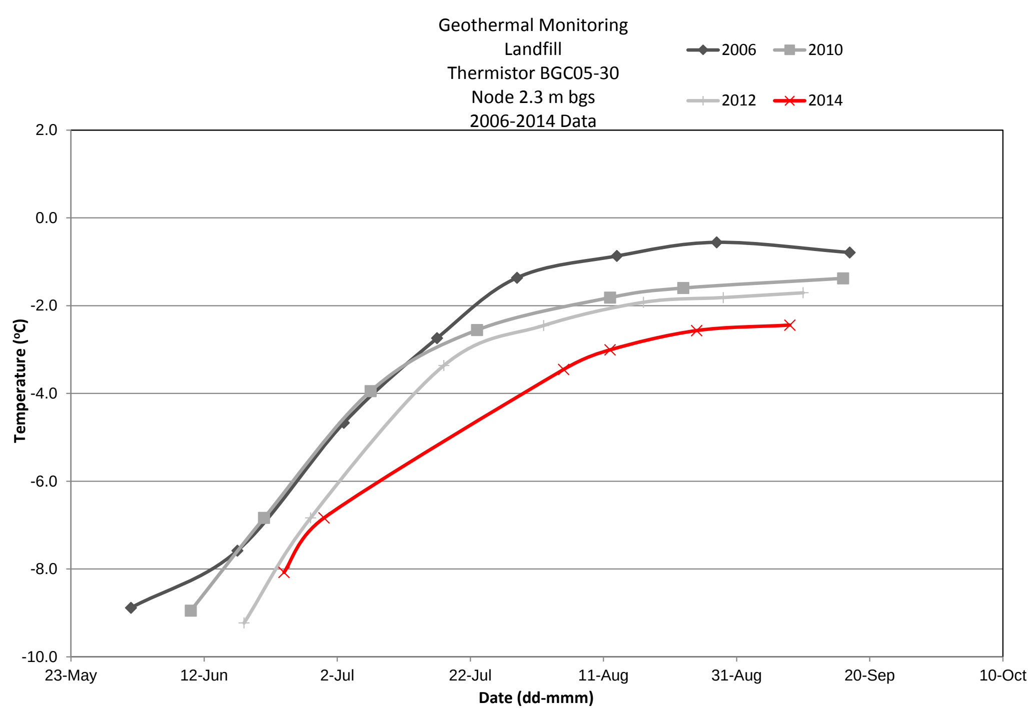
TITLE: LANDFILL COVER

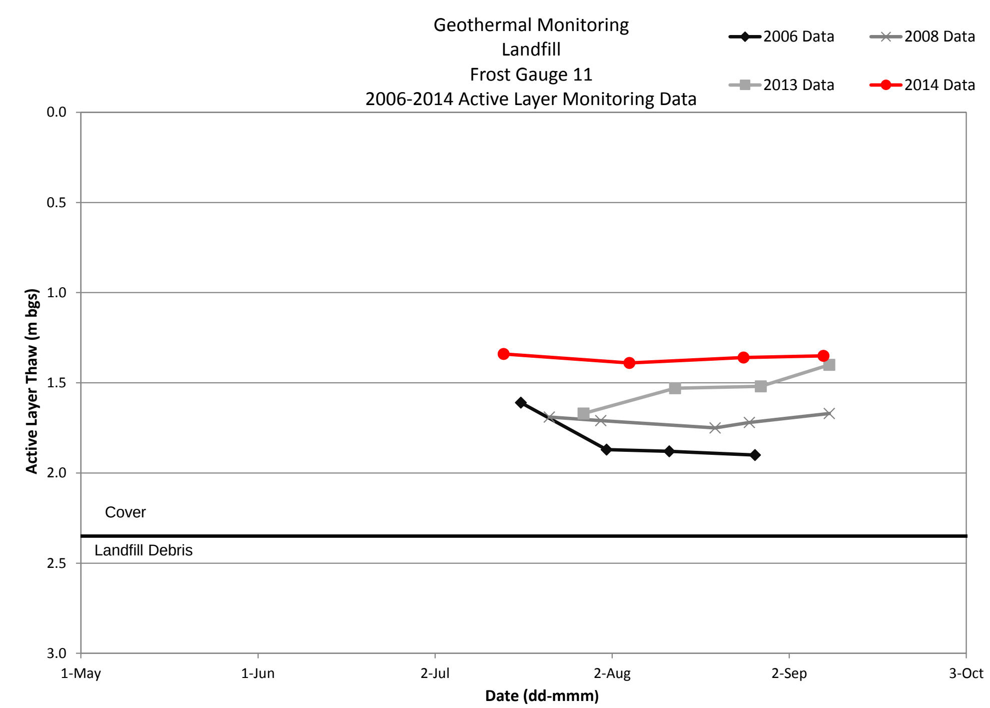
PROJECT No.: 0255-024-03

DWG No.: 22









DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT:

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: LANDFILL COVER
GEOTECHNICAL MONITORING DATA

DWG No.:	PROJ No.:	REV:
23	0255-024-03	

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NOTES:

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3. CONTOUR INTERVAL IS 1 m.
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SCALE:	1:1,000
DATE:	FEB 2015
DRAWN:	COC
CHECKED:	MCL
APPROVED:	GKC

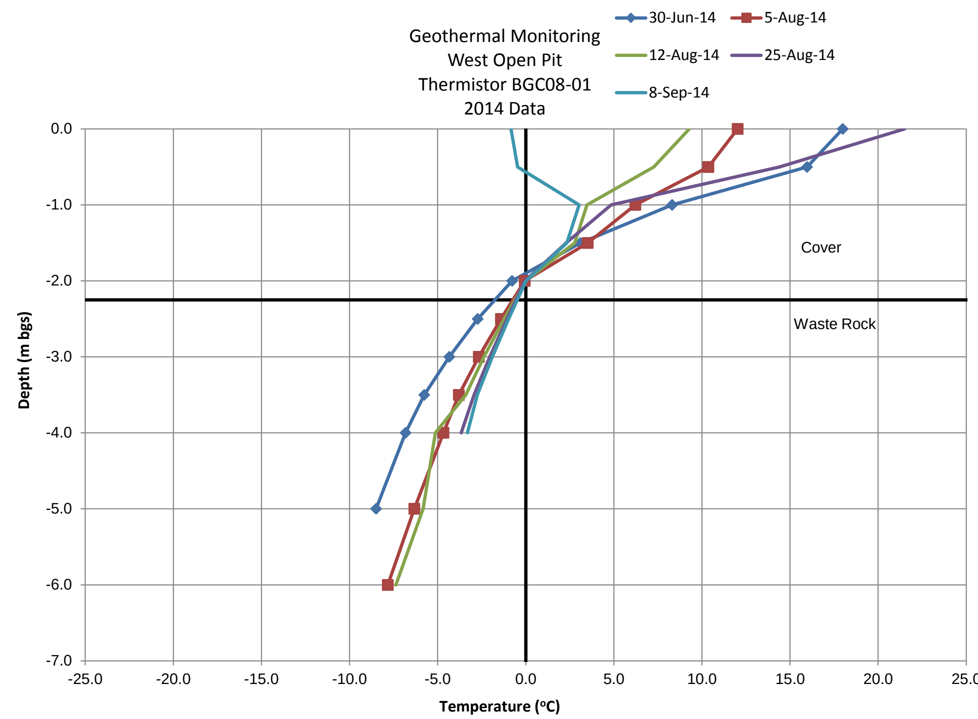
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

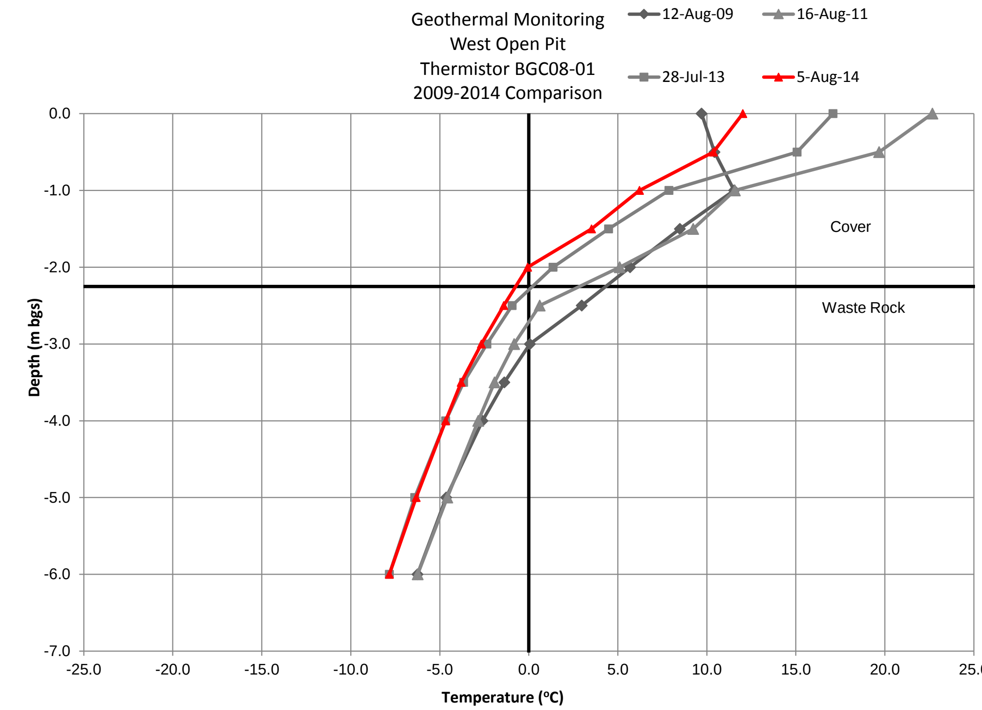
CLIENT: nyrstar

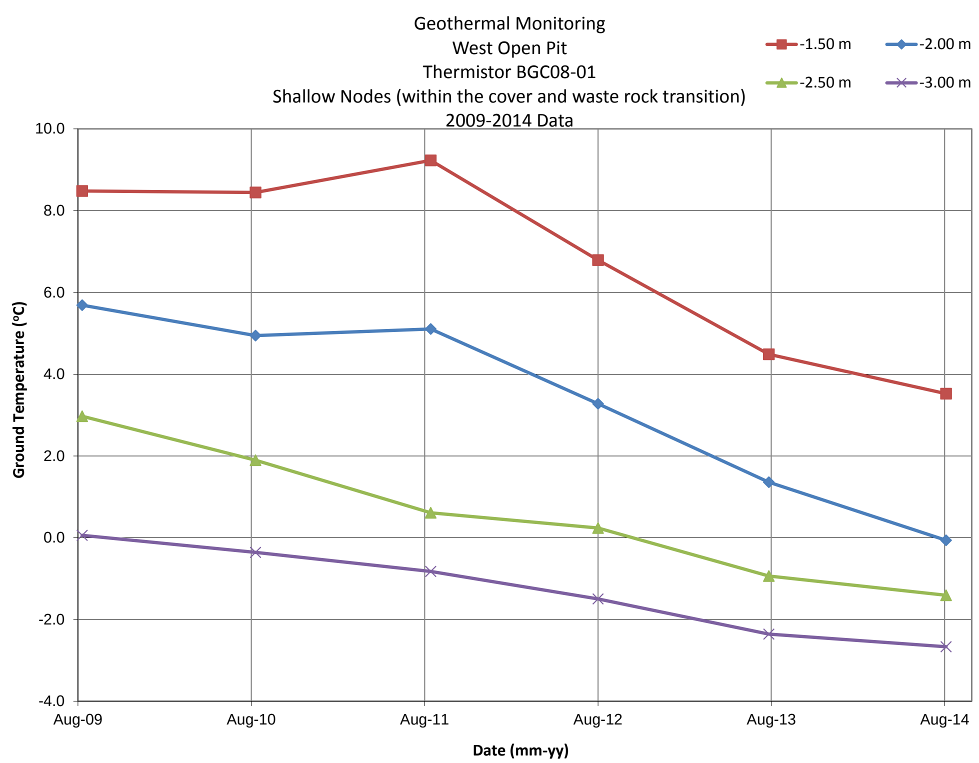
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

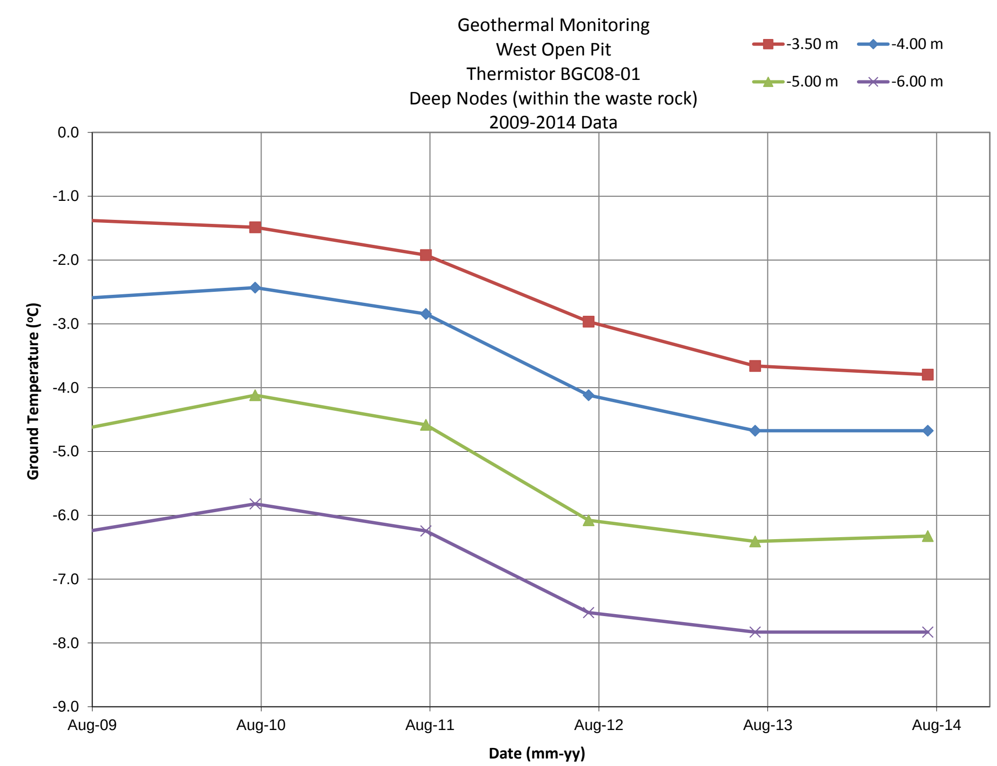
TITLE: WEST OPEN PIT
WASTE ROCK COVER

PROJECT No.:	DWG No.:
0255-024-03	24



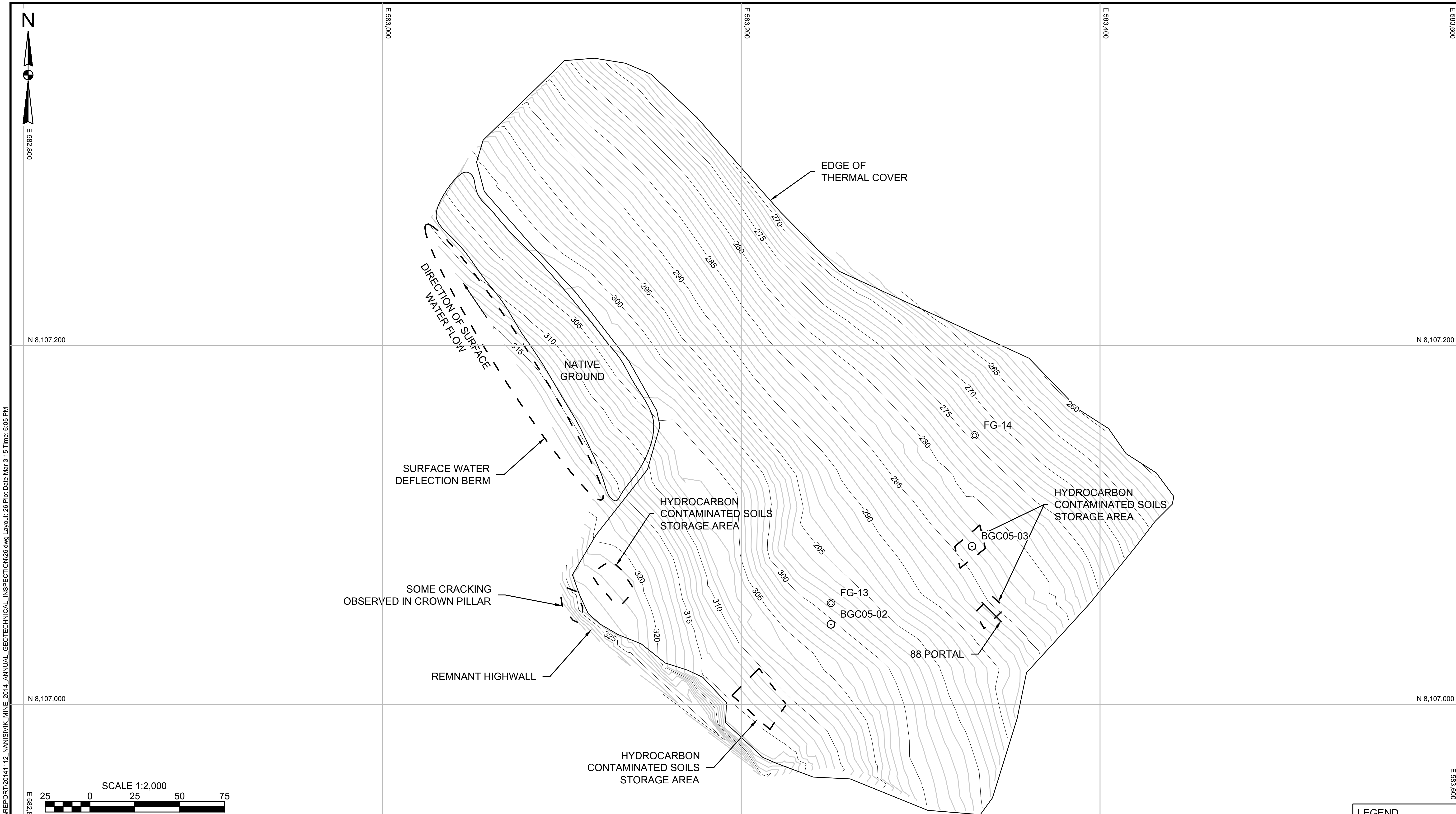






SCALE:	N/A	PROJECT: NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION
DATE:	FEB 2015	TITLE: WEST OPEN PIT WASTE ROCK COVER GEOTECHNICAL MONITORING DATA
DESIGNED:	GKC	BGC ENGINEERING INC. AN APPLIED EARTH SCIENCES COMPANY
DRAWN:	MCL	CLIENT: nyrstar
CHECKED:	GKC	DWG No.: 25
APPROVED:	GKC	PROJ No.: 0255-024-03 REV:

DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015



THIS DRAWING MAY HAVE BEEN REDUCED OR ENLARGED. ALL SCALE NOTATIONS INDICATED ARE BASED ON ORIGINAL FORMAT DRAWINGS.

NOTES:
1. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE NOTED.
2. THIS DRAWING MUST BE READ IN CONJUNCTION WITH BGC'S REPORT TITLED "NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION" AND DATED FEBRUARY 2015.
3. CONTOUR INTERVAL IS 1 m.
4. PROJECTION IS UTM NAD 83, Z16N.
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DATE:	FEB 2015
DRAWN:	COC
CHECKED:	MCL
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

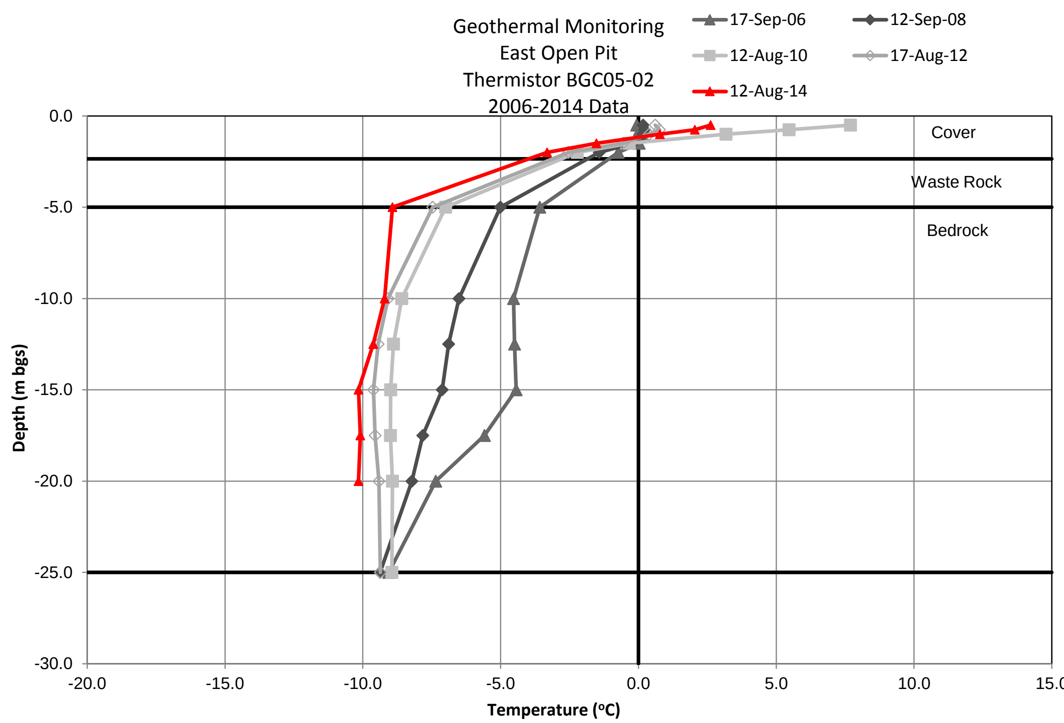
CLIENT: nyrstar

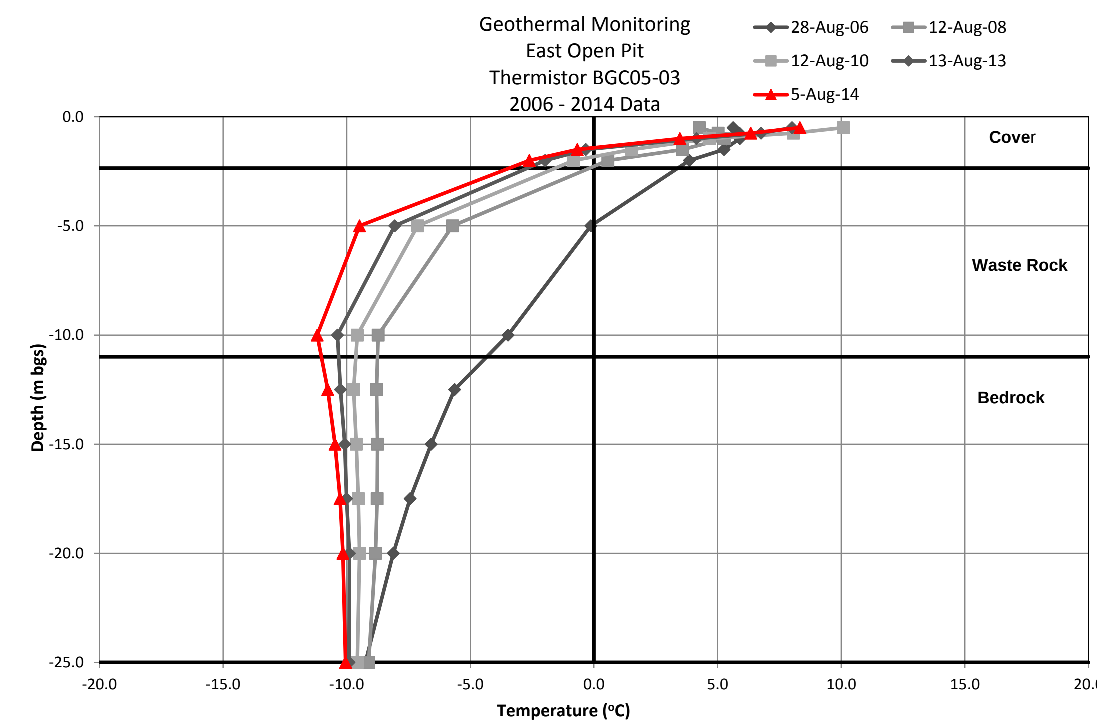
PROJECT: NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION

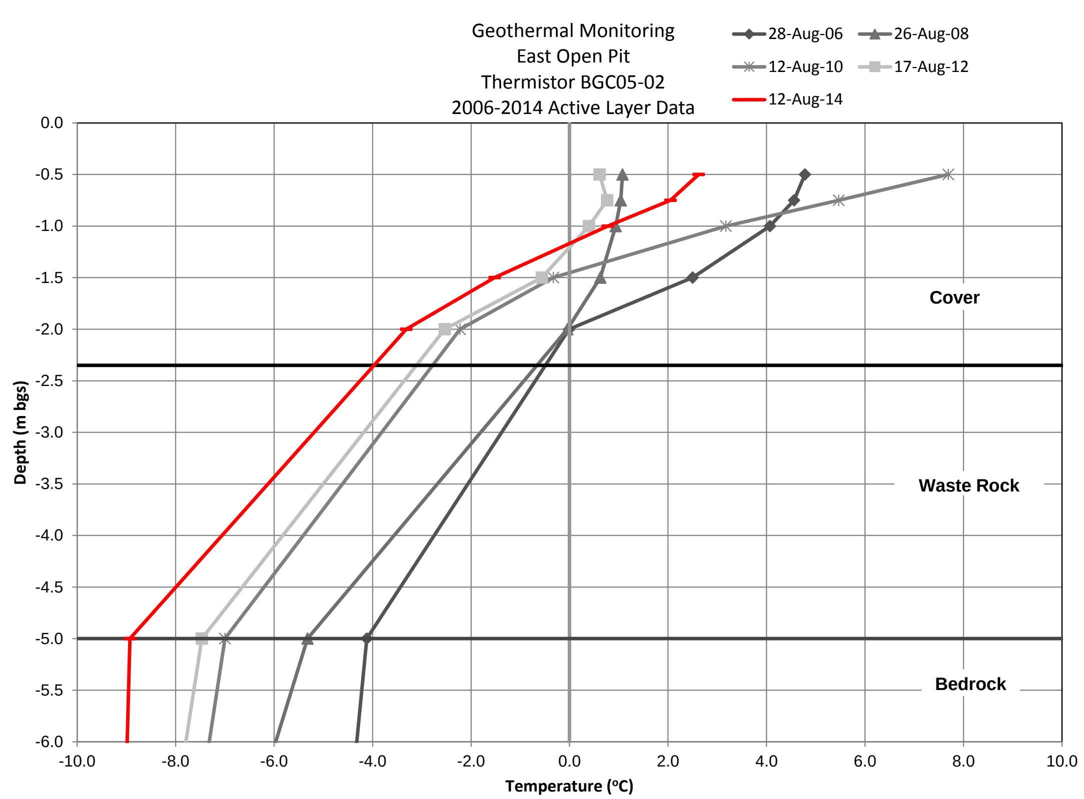
TITLE: EAST OPEN PIT WASTE ROCK COVER

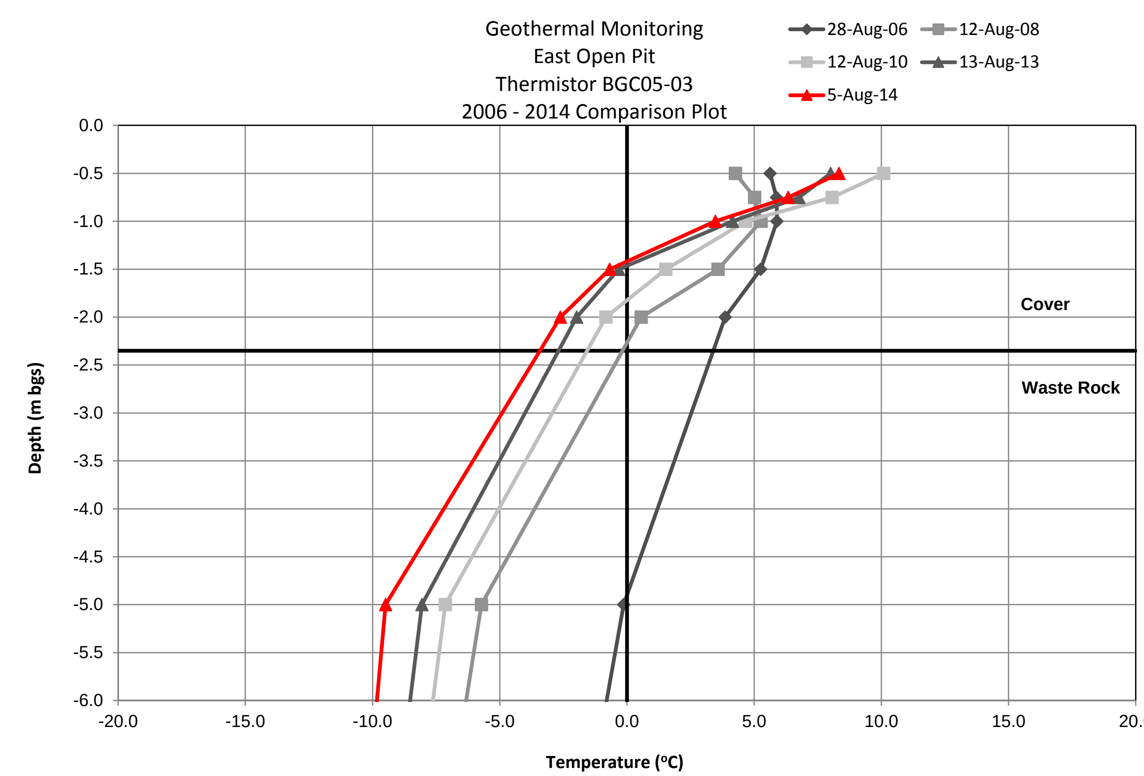
PROJECT No.: 0255-024-03

DWG No.: 26









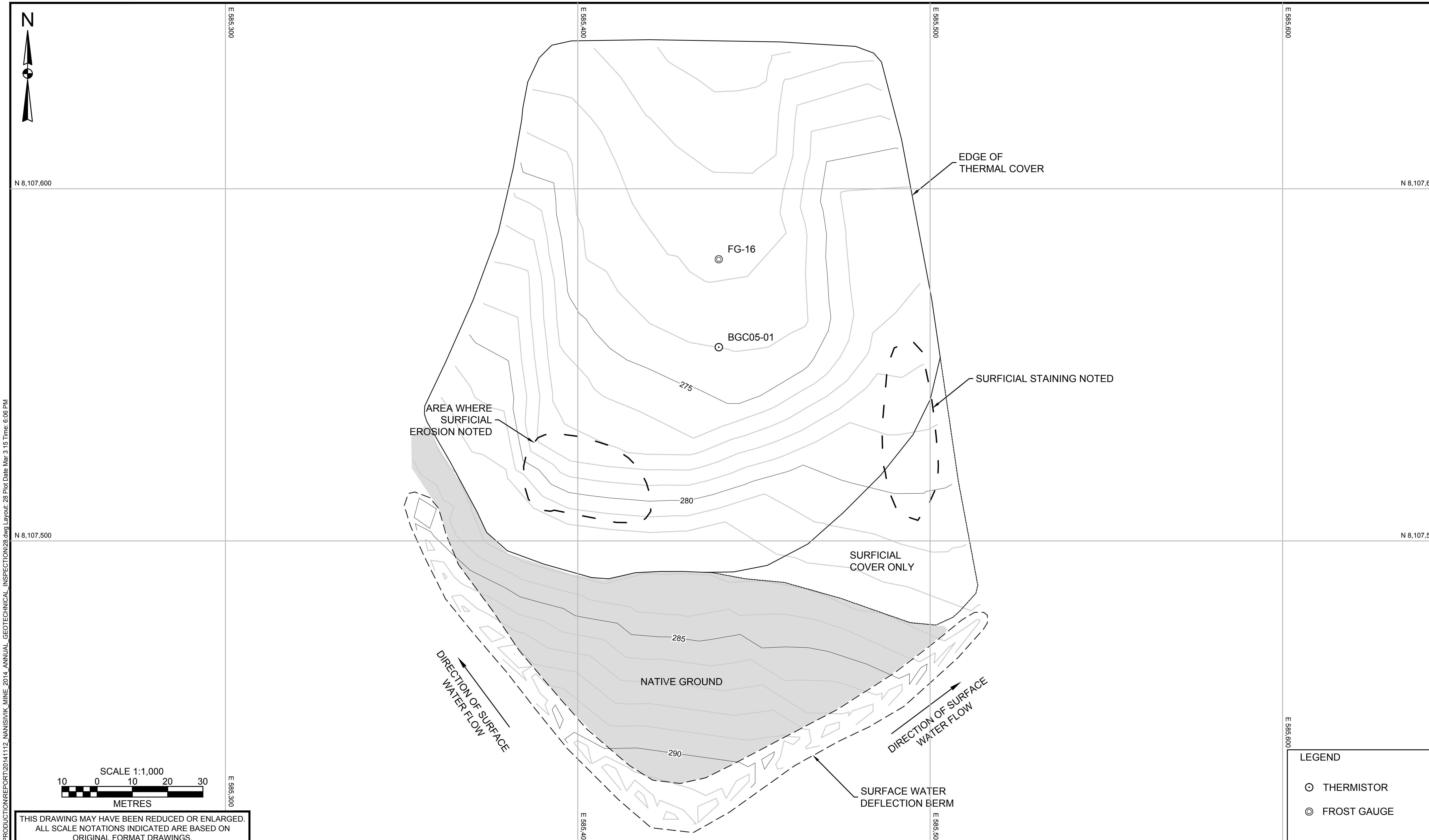
DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT:	NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION
TITLE:	EAST OPEN PIT WASTE ROCK COVER GEOTECHNICAL MONITORING DATA
DWG No.:	27
PROJ No.:	0255-024-03
REV:	



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NOTES:

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3. CONTOUR INTERVAL IS 1 m.
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DATE:	FEB 2015
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CHECKED:	MCL
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BGC ENGINEERING INC.
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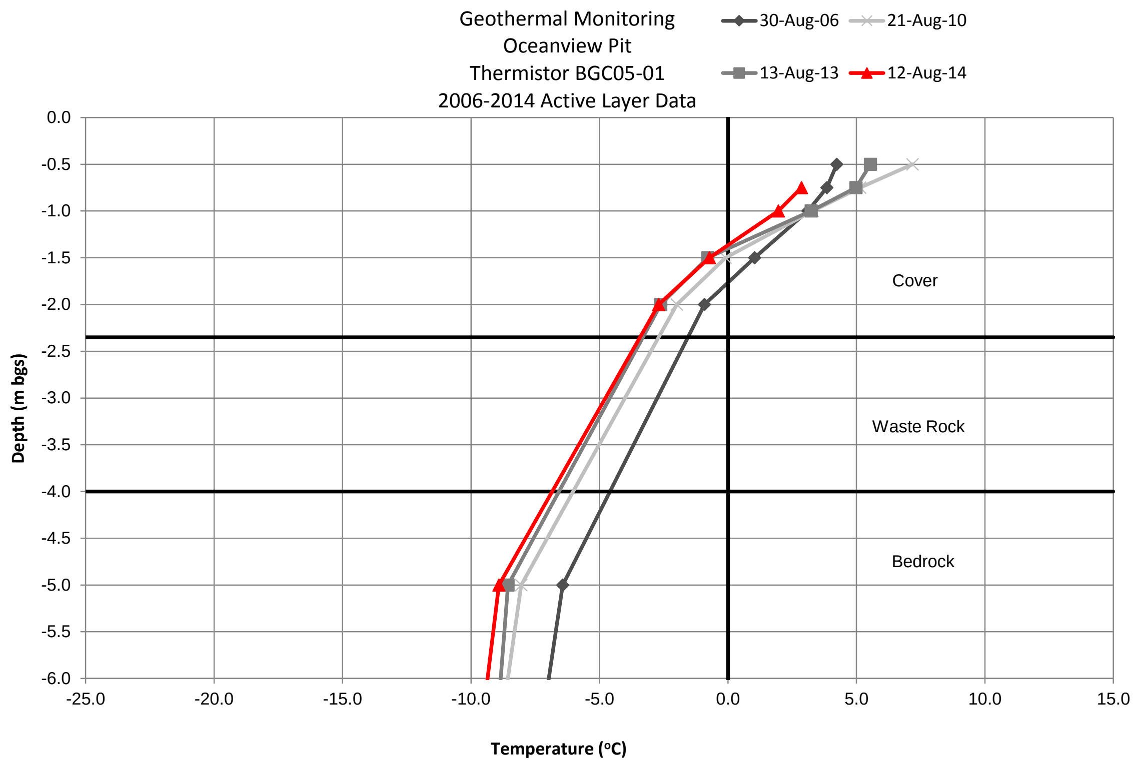
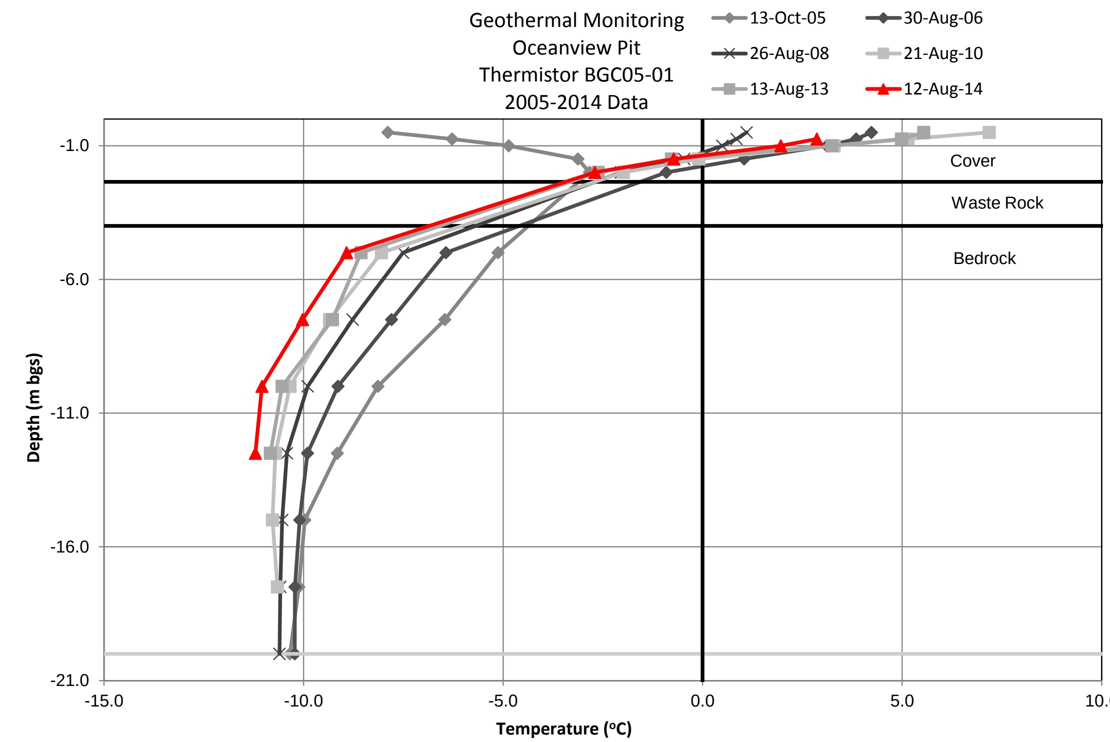
CLIENT: nyrstar

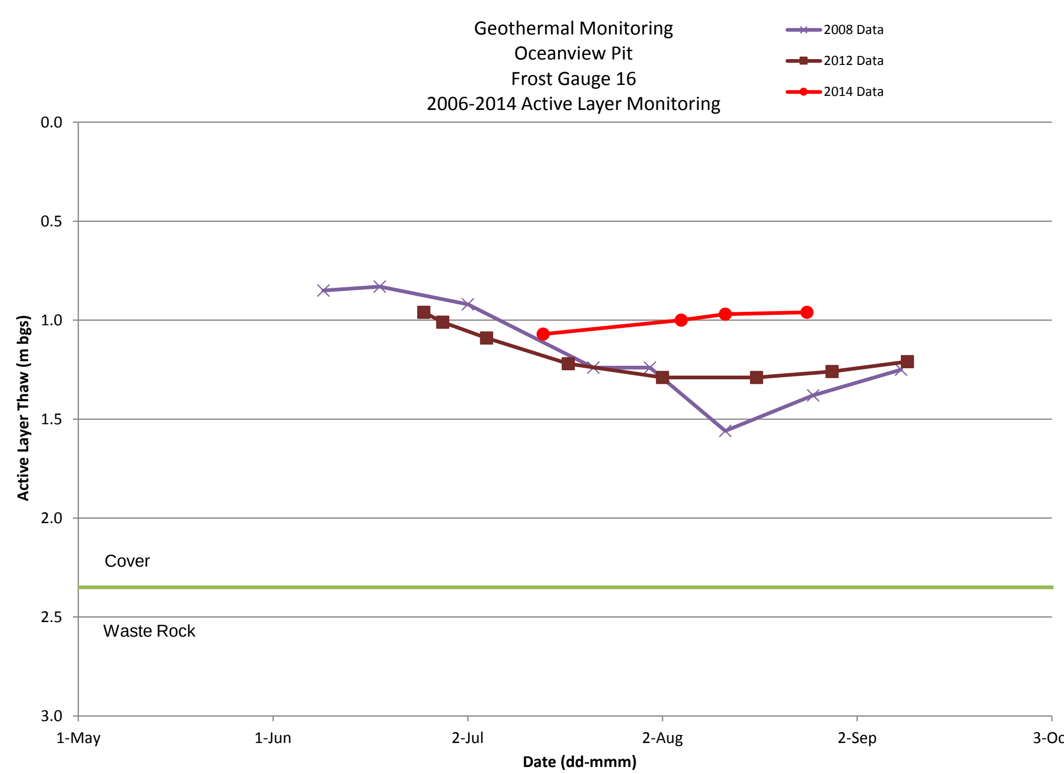
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

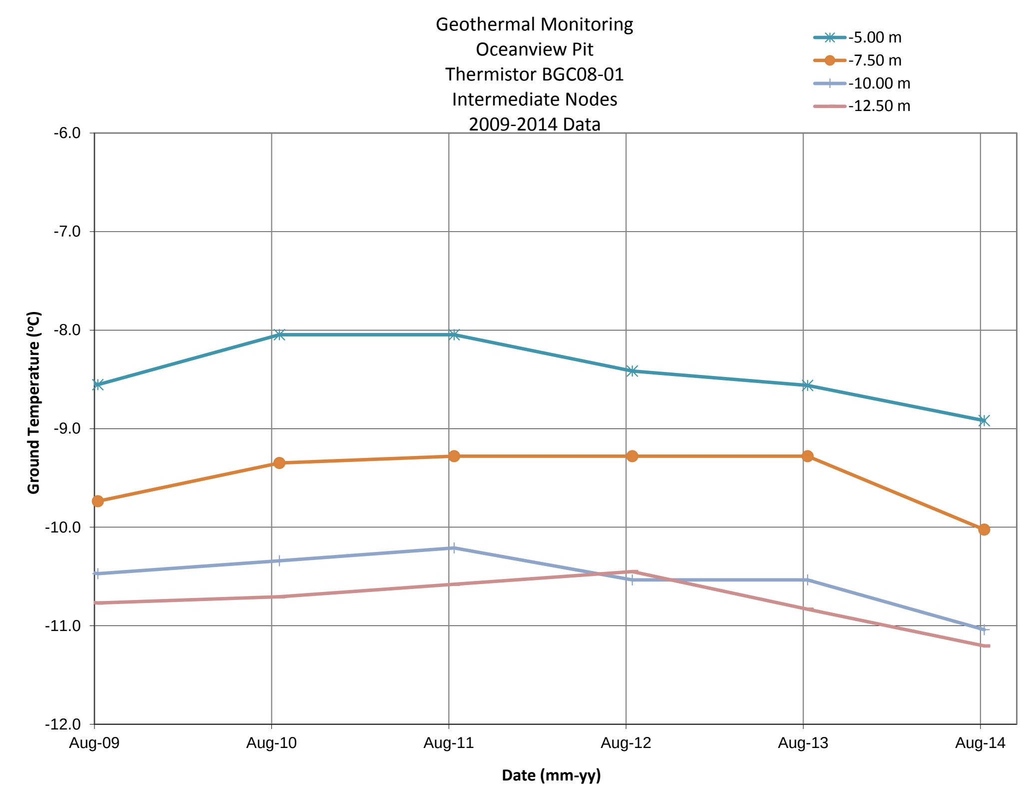
TITLE: OCEANVIEW OPEN PIT WASTE ROCK COVER

PROJECT No.:	DWG No.:
0255-024-03	28









DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
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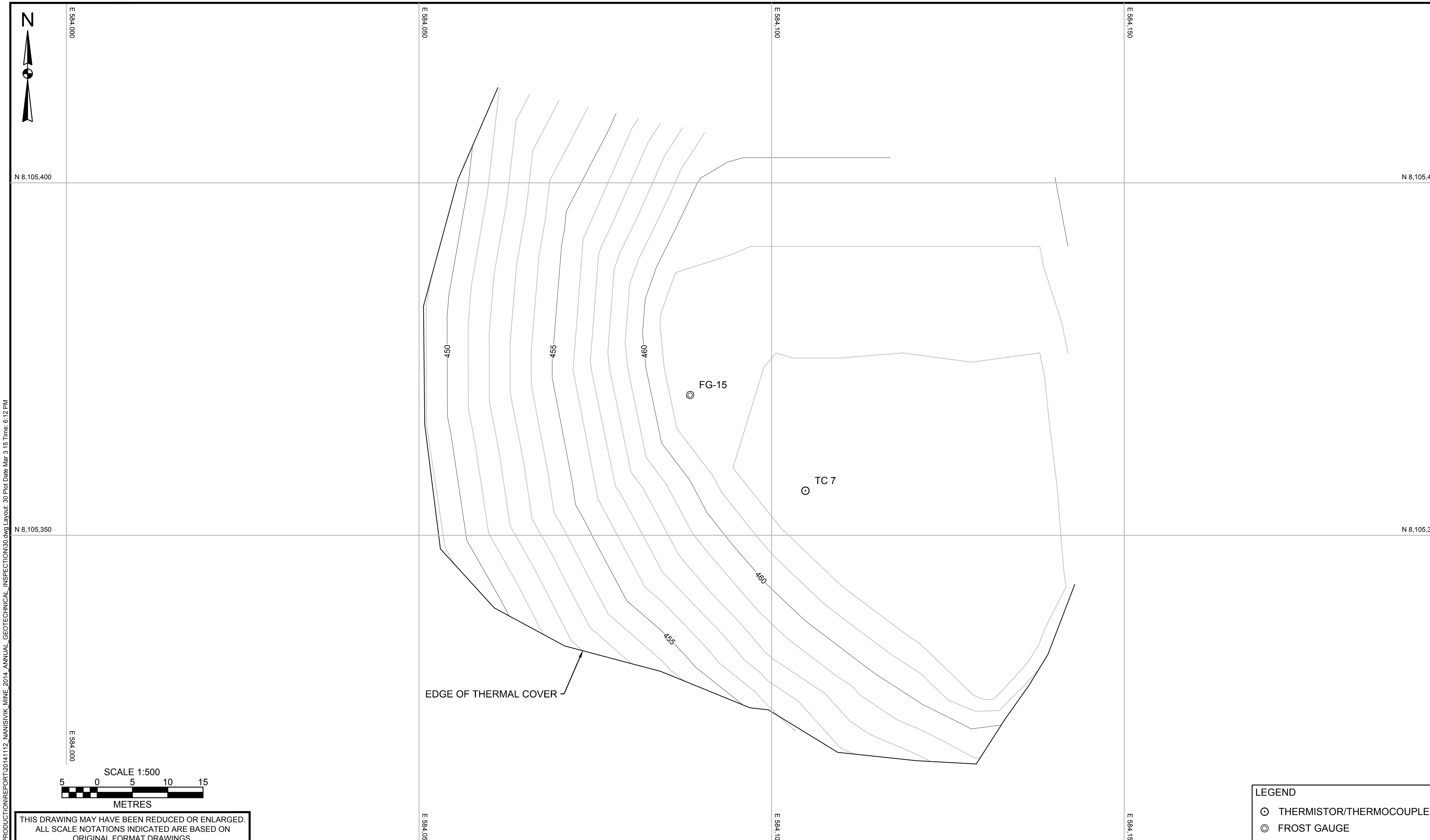
BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: OCEANVIEW PIT WASTE ROCK COVER
GEOTECHNICAL MONITORING DATA

DWG No.:	PROJ No.:	REV:
29	0255-024-03	



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NOTES:

1. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE NOTED.
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PROJECT: NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: AREA 14 WASTE ROCK COVER

PROJECT No.:	DWG No.:
0255-024-03	30

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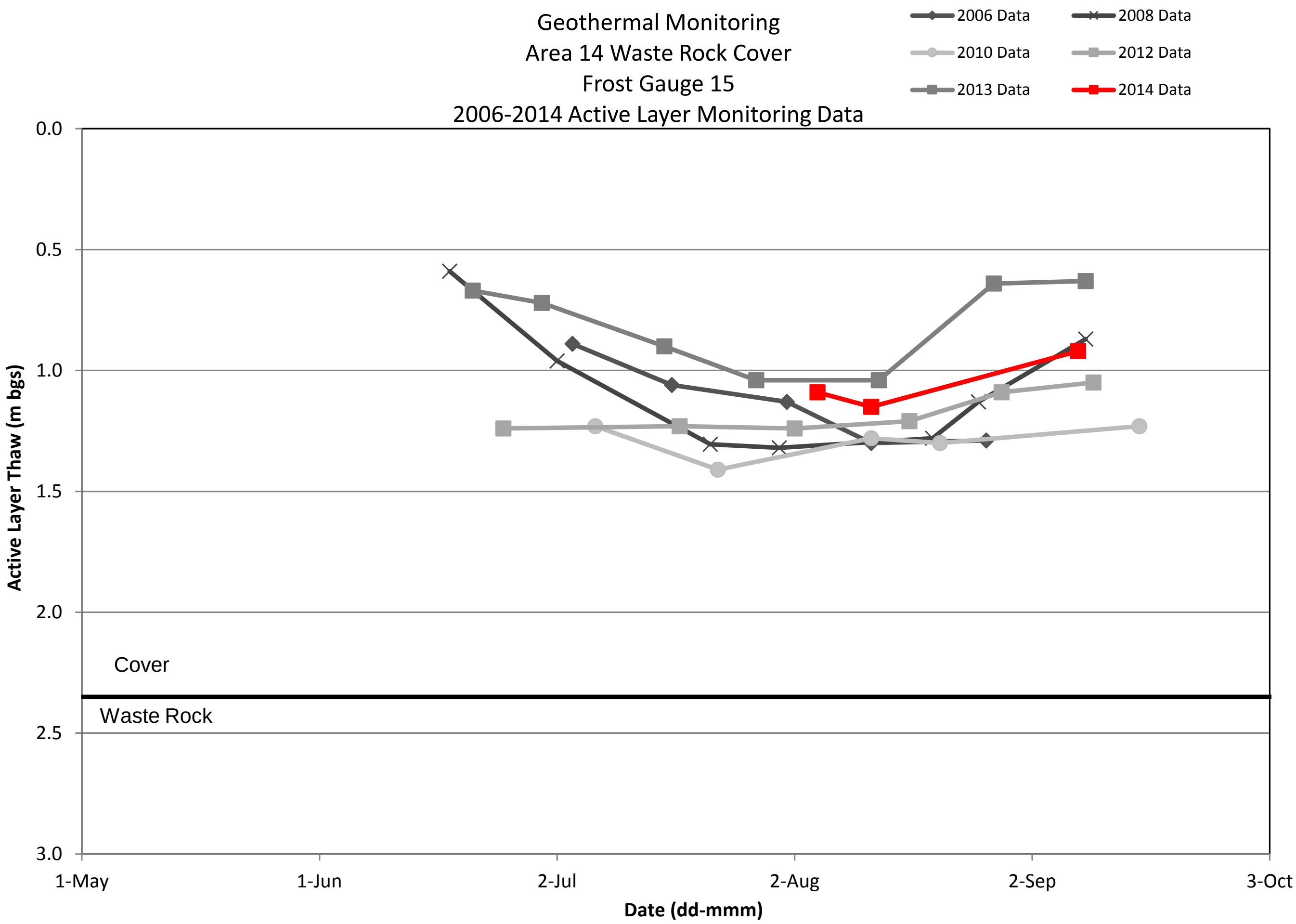

DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2015 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	AS SHOWN	DRAWN:	MCL
DATE:	FEB 2015	CHECKED:	JWC
DESIGNED:	GKC	APPROVED:	GKC

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PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: AREA 14 WASTE ROCK COVER
GEOTECHNICAL MONITORING DATA

CLIENT: nyrstar

DWG No.:	PROJ No.:	REV:
31	0255-024-03	

NOTES:

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3. CONTOUR INTERVAL IS 0.2 m.
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DATE:	FEB 2015
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APPROVED:	GKC

BGC ENGINEERING INC.
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CLIENT: nyrstar

PROJECT: NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: UPPER DUMP POND TAILINGS COVER

PROJECT No.: 0255-024-03

DWG No.: 32

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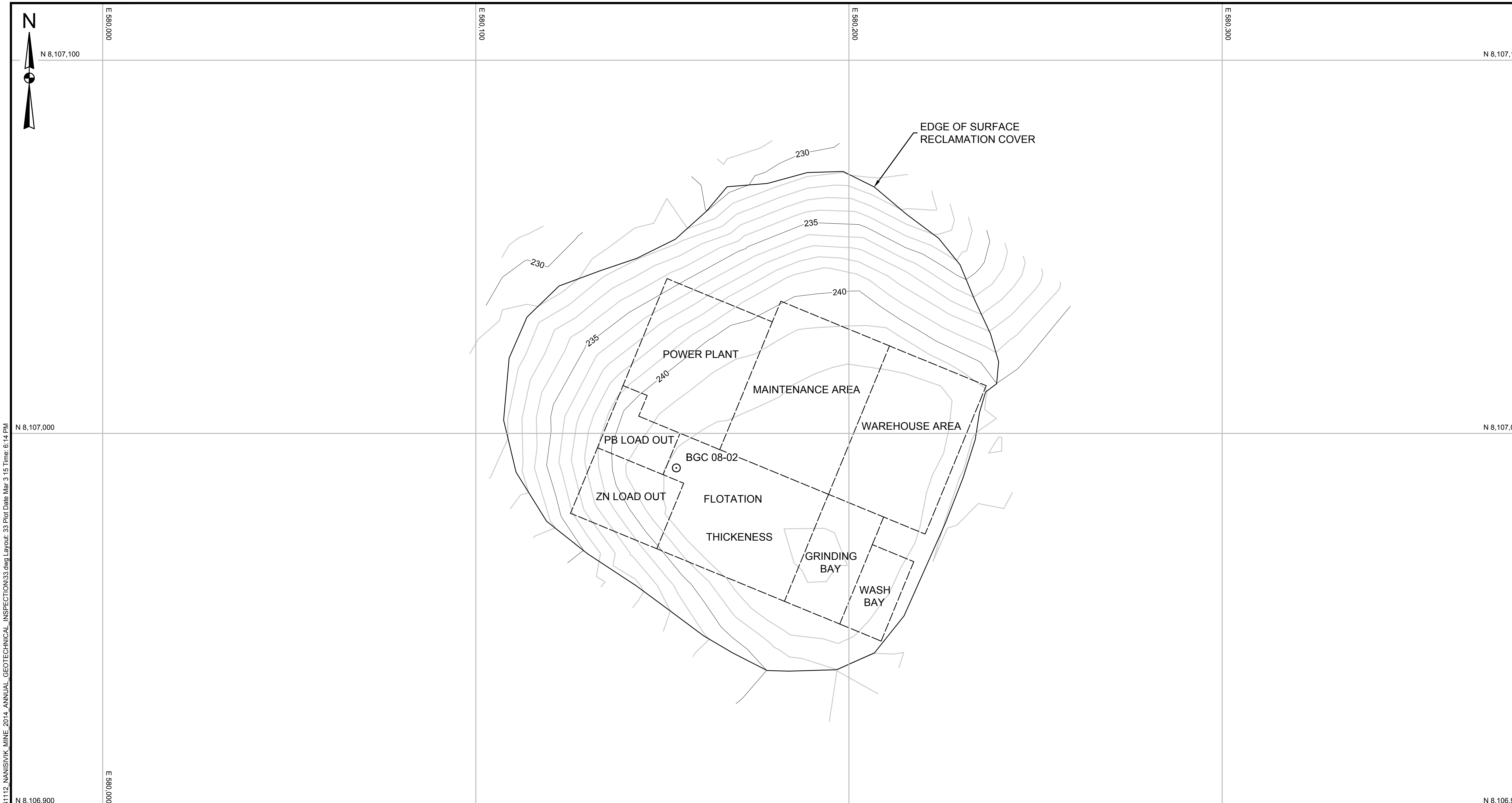

SCALE 1:1,000

10 0 10 20 30

METRES

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LEGEND

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NOTES:

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2. THIS DRAWING MUST BE READ IN CONJUNCTION WITH BGC'S REPORT TITLED "NANISIVIK MINE 2014 ANNUAL GEOTECHNICAL INSPECTION" AND DATED FEBRUARY 2015.
3. CONTOUR INTERVAL IS 1 m.
4. PROJECTION IS UTM NAD 83, Z16N.
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DATE:	FEB 2015
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CHECKED:	MCL
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

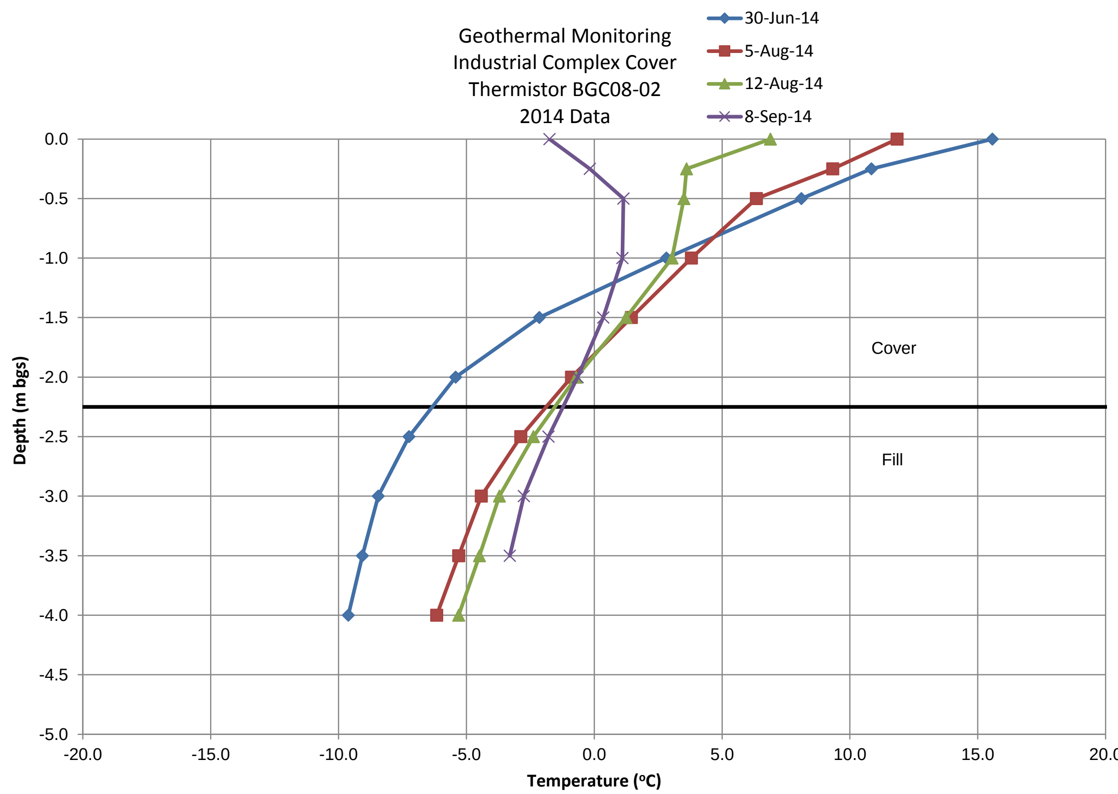
CLIENT: nyrstar

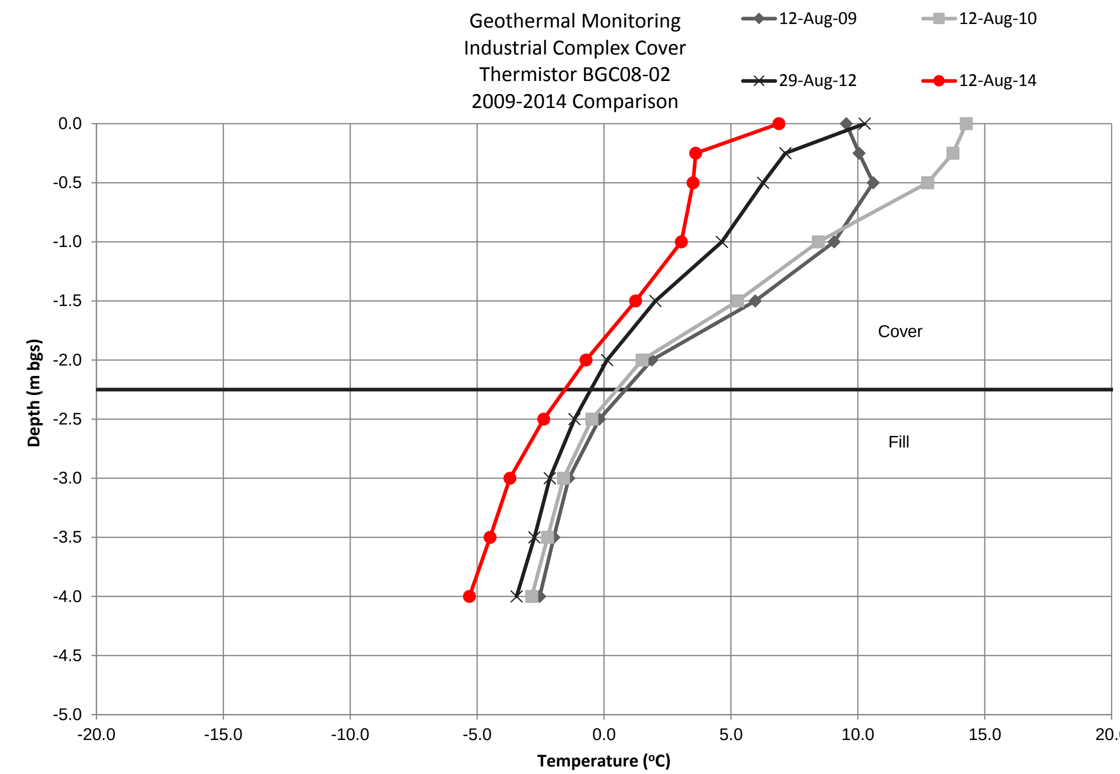
PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

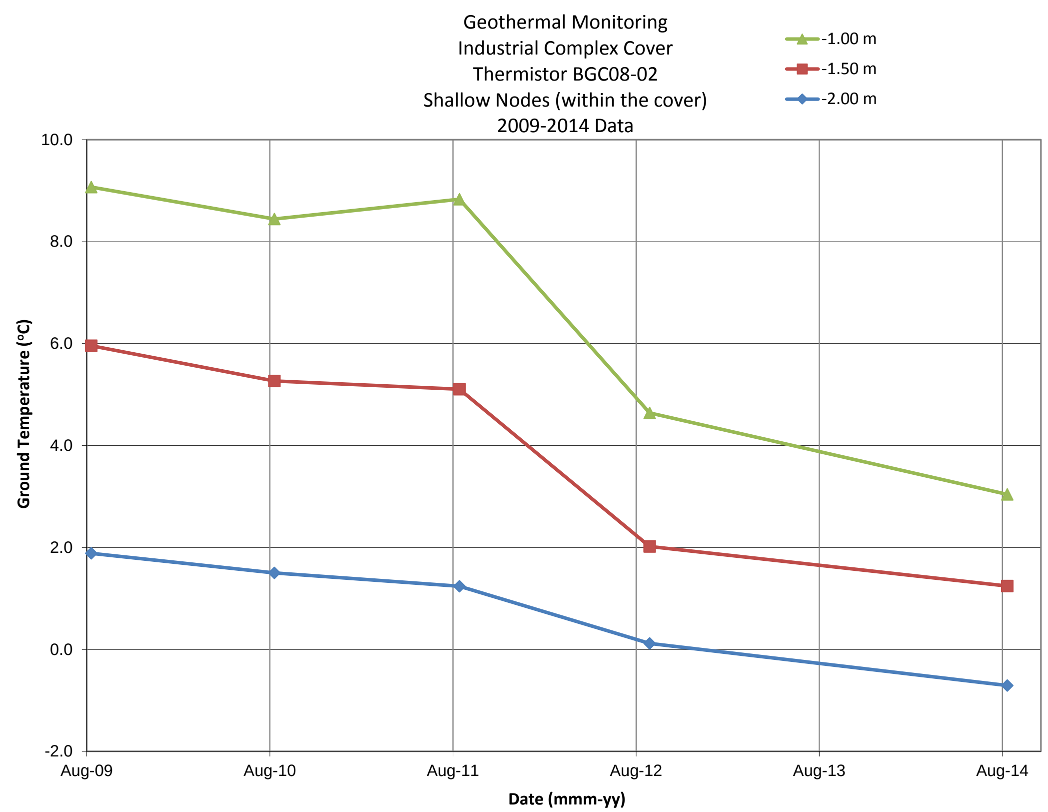
TITLE: INDUSTRIAL COMPLEX COVER

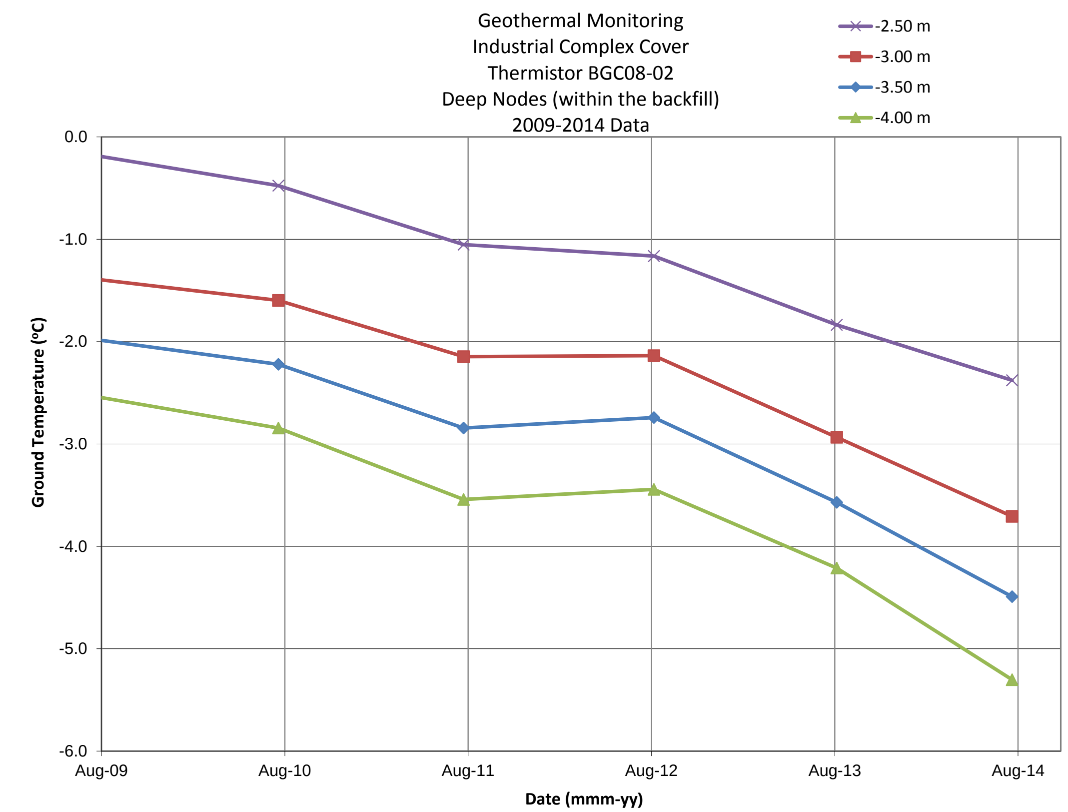
PROJECT No.: 0255-024-03

DWG No.: 33









DWG TO BE READ WITH BGC REPORT TITLED: "NANISIVIK MINE 2014 GEOTECHNICAL INSPECTION", FEB 2015

SCALE:	N/A
DATE:	FEB 2015
DESIGNED:	GKC
DRAWN:	MCL
CHECKED:	GKC
APPROVED:	GKC

BGC ENGINEERING INC.
AN APPLIED EARTH SCIENCES COMPANY

CLIENT: nyrstar

PROJECT: NANISIVIK MINE
2014 ANNUAL GEOTECHNICAL INSPECTION

TITLE: INDUSTRIAL COMPLEX COVER
GEOTECHNICAL MONITORING DATA

DWG No.:	PROJ No.:	REV:
34	0255-024-03	