



MEMORANDUM

TO John Stecyk, Regional Engineer (Small Craft Harbours - Central & Arctic Region)

DATE October 22, 2009

CC Alan Kathan

FROM Gordon Chrystal

PROJECT No. 09-1345-1003

PANGNIRTUNG SMALL CRAFT HARBOUR – ANCILLARY PROJECT REQUIREMENT
 PERMIT APPLICATION FOR THE DUVAL RIVER CROSSING (TEMPORARY – FOR DURATION OF PROJECT ONLY)

This memorandum is in support of the provisional submission on the proposed Duval River crossing identified as being required by the Hamlet of Pangnirtung for the dredging contract for the small craft harbour.

Preliminary hydrology has been carried out on the Duval River. The catchment area has been mapped at 95.5 km². In addition to applying a simplified run-off methodology to the catchment (assuming a nominal run-off factor, average slopes and a generic storm distribution), the catchment has been compared to the existing data available for the Sylvia Grinnell River catchment, for which measured stream flow data is available along with the corresponding rainfall events.

The two catchments can be considered hydrologically comparable (with respect to run-off factors and climatic conditions) and the relative return period flows for the Duval River are considered acceptable. The results are as follows for the Duval River:

Return Period	River flow (m ³ /second)
1 : 10	17.7
1 : 100	23.8
1 : 1000	30.1

The 1:100 year flow is selected as the design flow.

At an installed slope of 2%, and a culvert length of 26m, the configuration of 3 culverts (each at 1.8m diameter) accommodates the design flow as follows:

Q_{peak} (m ³ /s), Tailwater Depth (m) & Velocity (m/s)	24	1.5	4.02
Pipe discharge Q_p (m ³ /s) – per pipe	8.000		
Calculated inlet head $H(m)$	2.877	(a 1m surcharge at the headwall)	
Control type	Inlet		
Critical flow depth H_c (m)	1.406		
Normal depth H_n (m)	1.299		
Culvert flow depth at entrance (m)	1.406		
Culvert exit velocity (m/s)	4.063	(velocity will scour river channel)	

There was a gauging station in the Duval River that was discontinued in 1983. Limited and sporadic data is available but the reliability of the data is questionable.

This gauging station recorded maximum daily discharges at 22.3 m³/second and lower (less than the 1:100 year return flows) and a single maximum instantaneous discharge measured at 39.7 m³/second (in 1982). This is not considered a reasonable flow condition as it was a single reading taken from a total of 4 years of flow data. This



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higher flow of $39.7 \text{ m}^3/\text{sec}$ has been checked against the proposed design and the culvert configuration will accommodate this flow with a 2.6m head at the inlet (total water depth = 4.4m at this flow on the headwall).

The minimum height of the berm crossing is required to be 6m (from channel floor to berm top) to ensure no overtopping under these circumstances.

It should be noted that the flood conditions of 2007 resulted from a break in an ice dam in the upper reaches of the Duval River catchment and were not only attributed to the run-off from a rainfall event. This condition would be considered a catastrophic event for which culverts are not normally designed. Under normal operating conditions a dam break would be expected to cause the failure of smaller infrastructure.

Trust this information is sufficient to support the application for the Duval River (Temporary) River crossing.

Please contact me at (403) 509-1873 should you have any questions regarding this assessment.

Regards

Gordon Chrystal P.Eng

Senior Project Manager.