

		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

Title of document:
Design Report: Quarry D Contact Water Pond 3 (CWP 3)

Client:
Agnico Eagle Mines Limited

Project:
Hope Bay Block 4 and 5 (6212-E-132-004 & 6212-E-132-005)

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		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.:	Rev.	Date:
		Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	R1	December 16, 2025

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			R1	December 16, 2025

List of Revisions

#	Prep.	Rev.	App.	Date	Pages	Remarks
RA	ALN, HB	RL	RL	Oct. 29, '25		Issued for internal review
RB	ALN, HB	RL	RL	Nov. 3, '25		Issued for client comments
R0	ALN, HB	RL	RL	Nov. 18, '25		Issued for design
R1	ALN, HB	RL	RL	Dec. 16, '25	All	Issued for design

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Table of contents

1.	Introduction	1
2.	Design Concept	1
3.	System Design	2
3.1	Design Criteria	2
3.2	Design	3
3.2.1	Pond Excavation	3
3.2.2	Diversion Berms	3
3.2.3	Thermistors	3
3.3	Survey Data	4
3.4	Foundation Conditions	4
4.	Construction Methodology	5
4.1	Construction Materials	5
4.2	Quality Control	6
4.3	Construction Equipment	6
4.4	Construction Schedule and Sequence	6
5.	Operational and Closure Considerations	7
6.	References	7

Appendix A: Drawings

List of Tables

Table 4-1 : List of Earthworks Materials	5
Table 4-2 : List of Tests for Earthworks	6

List of Figures

Figure 2-1: Contact Water Pond CWP 3	2
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		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

1. Introduction



The **Hope Bay property** is in the Kitikmeot region of Nunavut, Canada, approximately 685 km northeast of Yellowknife and 125 km southwest of Cambridge Bay. The property spans 191,342 hectares and includes portions of the Hope Bay and Elu greenstone belts. The 80-km-long Hope Bay greenstone belt hosts three main gold deposits: Doris, Madrid, and Boston.

Agnico Eagle Mines Ltd (AEM) acquired the Hope Bay property in February 2021. The project has significant infrastructure, including underground mine development at the Doris and Boston deposits, a processing plant, airstrips, a port, and an all-weather road network.

The site development involves new civil constructions such as pad ore storage, diversion berms, roads and others. Contact water from freshet, rain and underground mining dewatering is handled by storing water in sumps, ponds and pits which are then pumped to a treatment plant for re-use and discharge to the environment.

Part of the project involves the development of the Contact Water Pond 3 (CWP 3) to manage the contact water transferred from the Madrid and Patch 7 deposit areas.

2. Design Concept

Figure 2-1 presents an overview of the CWP 3. It is a pond used to collect contact water runoff transferred from the following areas:

- From the Madrid deposit area:
 - Sump 1: Contact water runoff from the south waste rock storage facility (WRSF)
 - Sump 2: Contact water runoff from the Madrid Portal infrastructure pad
 - Sump 3: Contact water runoff from the ore stockpile pad
 - Sump 5: Contact water runoff from the north WRSF
 - Sump 4: Contact water runoff from the overburden stockpile area
 - Naartok Pit Sump: Contact water runoff collected in the Naartok Pit sump
- From the Patch deposit area:
 - Patch 7 CWP 4: contact water runoff from the ore stockpile pad and WRSF

Water stored in this pond is then pumped to the Tailings Impoundment Area (TIA) Reclaim Pond for re-use in the process plant or for treatment prior to discharge to the receiving environment.

The drawings for CWP 3 can be found in **Appendix A** of this report.

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		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev.	Date:
			R1	December 16, 2025

The main design features of CWP 3 are the following:

- The pond will be excavated within bedrock.
- An access ramp will be built to allow access to the bottom of the pond.
- Diversion berms will be built as required to divert non-contact water away from it.
- Thermal berm will be placed if and wherever required.

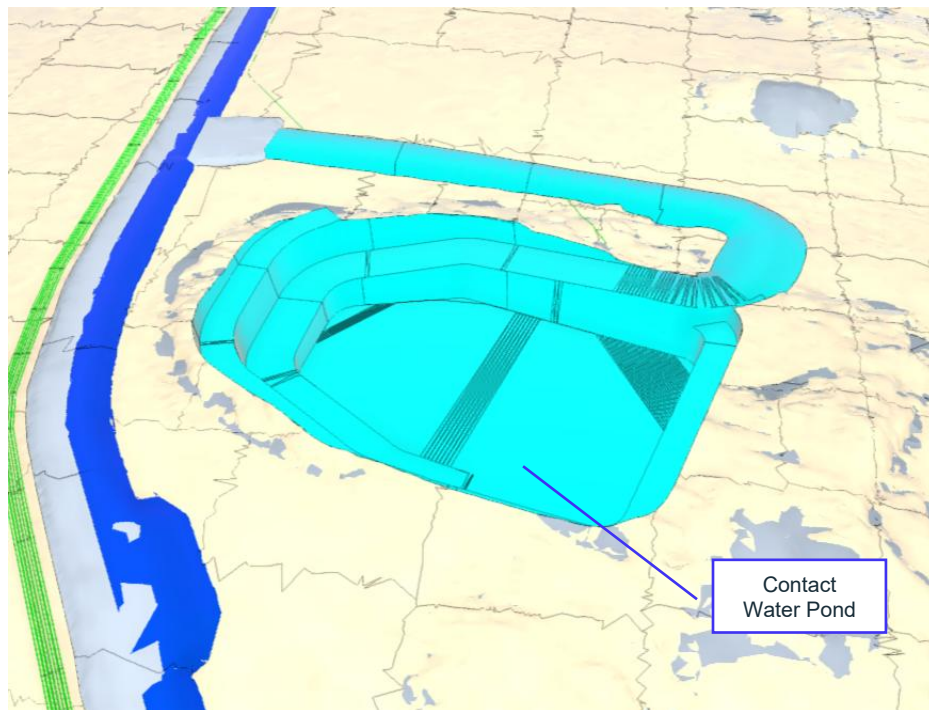


Figure 2-1: Contact Water Pond CWP 3

3. System Design

3.1 Design Criteria

The following design criteria are based on the general design criteria developed for this project (AtkinsRéal 2025).

The design criteria for CWP 3 are as follows:

- Pond capacity to contain contact water from a 100-year, 24 hr rain event (57.2 mm) and an average year (2.33 yr) 30-day snowmelt (190.0 mm)

		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

- Runoff coefficient: 1
- Catchment area draining to pond: 17,000 m²
- Additional volume to store:
 - From Sump 1: 22,495 m³
 - From Sump 2: 7,665 m³
 - From Sump 3: 4,200 m³
 - From Sump 4: 17,800 m³
 - From Sump 5: 28,180 m³
 - From Naartok Pit sump: 11,370 m³
- Time to drain pond down: 30 days
- Live volume required: approx. 95, 915 m³
- Freeboard: minimum 1.0 m
- Dead storage: 0.5 m
- The CWP3 Pond is designed for temporary storage of water and is not designed as a permanent holding pond.

3.2 Design

The following section provides an overview of the design of CWP 3. Further details can be found in the drawings in **Appendix A**.

3.2.1 Pond Excavation

CWP 3 will be blasted and excavated in bedrock using benches up to 10 m deep with 1H:10V slope side wall to ensure its stability. The CWP 3 pond sizes are approximately as:

- CWP 3: Length: 170 m x Width from 85 to 130 m x Depth from 12 to 20 m.

3.2.2 Diversion Berms

The diversion berms will consist of an earthwork structure with a HDPE geomembrane liner anchored into the overburden.

3.2.3 Thermistors

To ensure that the overburden / bedrock is frozen to the east of the CWP 3, a thermistor string will be installed along its eastern edge. The size of thermistor string will be defined during the detailed engineering phase of the facility. The focus would be to monitor the ground thermal regime in the bedrock and any changes to it.

		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

3.3 Survey Data

The design of these water management infrastructures is based on topographic contour maps produced from aerial LiDAR survey taking in September 2024 supplied by AEM.

3.4 Foundation Conditions

The project site is in a continuous permafrost that has been estimated to extend to depths in excess of 500 m, with an active layer of about 1 m to 2 m.

Based on the review of satellite imagery and survey topography data of the area, the CWP 3 Pond is located predominantly on outcrop with limited footprint extending over tundra.

Overall, the geomorphology of the project site consists of rock outcrop ridges, typically trending North-South and parallel ranges. The general slope trend is towards north-east, and the predominant ground elevation in the CWP 3 Pond are 60 to 65m.

The project geotechnical database comprises of a series of geotechnical drilling campaigns, many mine exploration drill holes, and numerous geological and geotechnical studies commissioned during various phases of this project. The project geomorphology is described as typical coastal lowland with numerous lakes and ponds separated by glacial landforms. These geological features run parallel to the geological intrusions of diabase dykes and sills. The shape of the drainage basins is generally long and narrow and is predominantly orientated along the north-south axis. Lakes are usually present along the bottom of those elongated valleys.

Surface conditions are either rock outcrop or tundra or surface runoff and water features and lakes. Patterned ground, usually consisting of surface drainage rills, mask the underlying soils. Small, frost-heaved clay-silt polygons are very common. Tundra is typically shallow to deep overburden over bedrock.

Bedrock contour maps have been developed for the project site based on outcrop mapping and large database of drill holes (mine exploration and geotechnical). Bedrock is predominantly outcrop in the CWP 3 pond area. The bedrock is mainly comprised of mafic metavolcanic (mainly meta-basalts) and meta-sedimentary rocks that are bound by Archean granite intrusives and gneisses.

The overburden soils are mostly of marine origin and include clay, silt and some sand. The overburden soils are typically ice rich and thaw sensitive.

The pond layout may be slightly adjusted to ensure the pond water containment is located within the bedrock excavation. Further detailed geotechnical investigation specific to the CWP 3 pond area is on-going.

		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

4. Construction Methodology

4.1 Construction Materials

The most common material available at the site is the Run of Quarry (ROQ). The low permeable till is almost absent at the site; therefore, HDPE geomembrane is used as liner for the pond and surface runoff diversion berms. The Liner system typically is a liner sandwiched between geotextile and crushed rock bedding material.

All geosynthetic materials and liner construction shall be in accordance with the requirements developed by the Geosynthetic Research Institute (GRI).

Natural sand or sand and gravel borrows are also not available at the site, therefore, crushed rock is used as transition, bedding and surfacing materials.

To protect the liner from crushed stone damages, non-woven geotextile installed on one or both sides of the liner is used, depending on the use of bedding layer.

All materials used for earthworks are natural and from local excavations, free from acid generation/metal leaching potential (i.e. non-PAG). Materials selection, placement, compaction and quality control requirements are as per the project's earthworks specifications.

A summary description of the earthworks materials (per SRK 2022) is presented in the following Table 4-1.

Table 4-1 : List of Earthworks Materials

Type	Description
ROQ	Run-off-Quarry Material, (Ø 0-600 mm); minimum thickness 900 mm; compacted to method specification (specified roller passes)
Transition Material	Crushed stone (Ø 0-200 mm); minimum thickness 300 mm; compacted to method specification (specified roller passes)
Bedding Material	Crushed stone (Ø 0-25 mm); minimum thickness 100 mm; compacted to 95% Standard Proctor Maximum Dry Density (S.P.M.D)
Surfacing Material	Crushed stone (Ø 0-38 mm); minimum thickness 100 mm; compacted to method specification (specified roller passes)
Geotextile	Needle punched Non-woven Geotextile Fabric
Geomembrane	High Density Polyethylene liner, 1.5 mm thickness (60 mil)

		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

4.2 Quality Control

The project's earthworks construction Quality Control and Assurance Program (QA/QC) testing standard procedures listed in Table 4-2 shall be followed.

Table 4-2 : List of Tests for Earthworks

Test Standard	Details
ASTM D2487	Classification of Soils for Engineering Purposes
ASTM D2216	Water (Moisture) Content in Soil and Rock
ASTM C136	Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates
ASTM D854	Specific Gravity of Soils
ASTM D698	Laboratory Compaction Characteristics of Soil Using Standard Effort
ASTM D2922	ASTM D2922 Density of Soil in Place by Nuclear Methods

4.3 Construction Equipment

The following construction equipment will be used in the construction of CWP 3:

- CAT 374 Excavator
- John Deere 470 Excavator or similar sized equipment
- CAT 745 Rock Trucks or similar sized equipment
- CAT D8 Dozer
- CAT 14H Grader
- Sandvik D800 Drill.

4.4 Construction Schedule and Sequence

The pond excavation being predominantly in the rock, the timing is of less concern from constructability considerations. This is unlike tundra excavation which might require excavation in winter for workability concerns. The planned construction sequence is as follows:

1. The access road from Windy Road into CWP 3 will be constructed with ROQ and 38mm crush (i.e. road surfacing).
2. Overburden material within the pond footprint will be ripped, stripped and hauled away from the area to the designated overburden stockpile.
3. Sandvik drills and explosives will be used to pioneer blast the area down to a flat grade. Drilling and blasting will continue following the blast engineering master plan and sequencing to develop the pond and internal ramp.

		TECHNICAL NOTE		
		Design Report: Contact Water Pond CWP 3		
		Document No.: Agnico Eagle Mines no.: 6212-695-132-REP-002 ATRL Ref.: 704148-2000-41ER-0002	Rev. R1	Date: December 16, 2025

4. As material is blasted to produce ROQ, excavators will load out the material and it will be hauled away to other project locations for placement.
5. Excavators and dozers will support the blast program to scale the walls and manage the produced ROQ material.
6. The diversion berm(s) will be constructed during construction of the pond to manage water and prevent ingress during construction activities.

5. Operational and Closure Considerations

The principal operational considerations for the CWP 3 shall include as a minimum the following:

- Monitoring of operational water levels.
- Monitoring of temperature profile within the foundation influence depth at CWP 3 at a critical and strategic location during operation to ensure design assumptions are met to verify the stability and functioning of the pond.

Closure activities for CWP 3 will be carried out in accordance with the Interim Closure and Reclamation Plan.

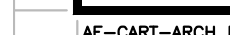
6. References

AtkinsRéalis 2025	General Design Criteria for Water Management MPEI Infrastructure, 6212-695-132-DGC-001, revision RC, 2025-09-22.
SRK 2022	Technical Specifications Earthworks and Geotechnical Engineering Hope Bay Project, Nunavut Canada Revision I – Issue for Construction. Prepared for Agnico Eagle Mines Limited.

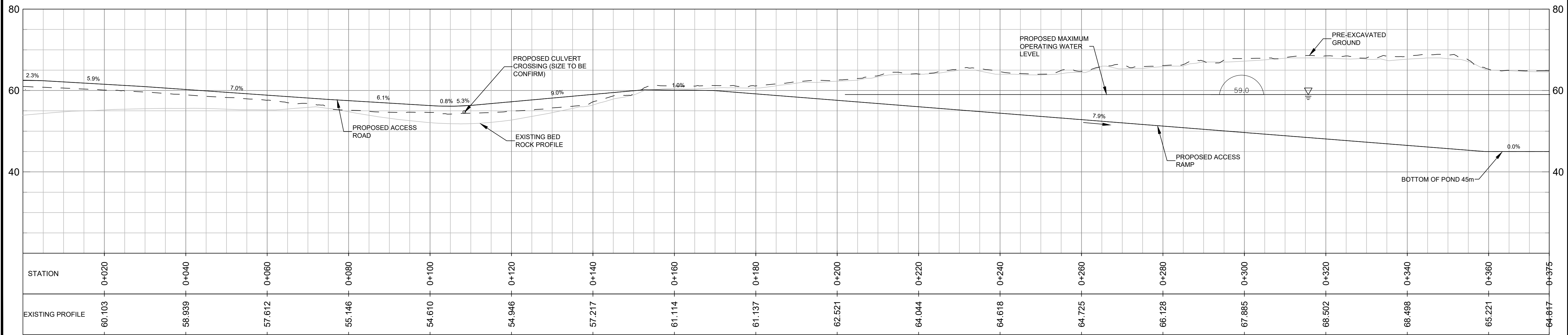
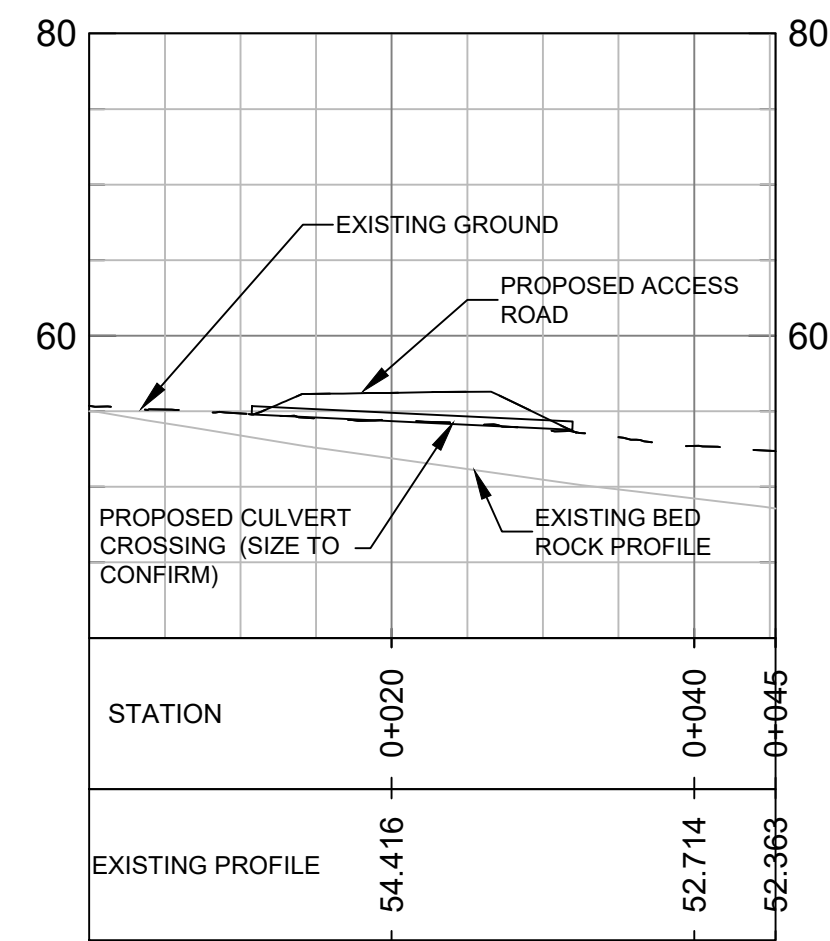
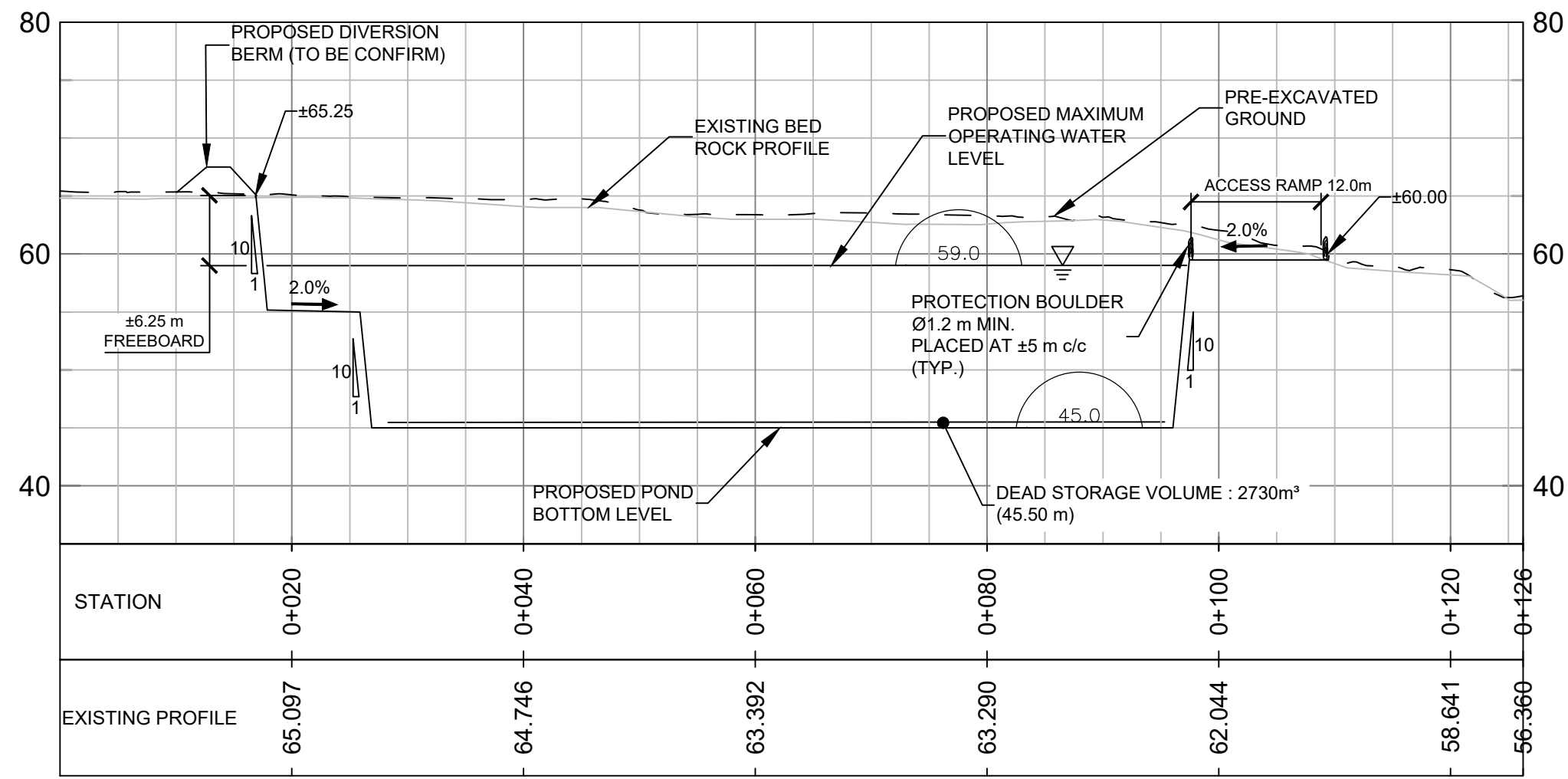
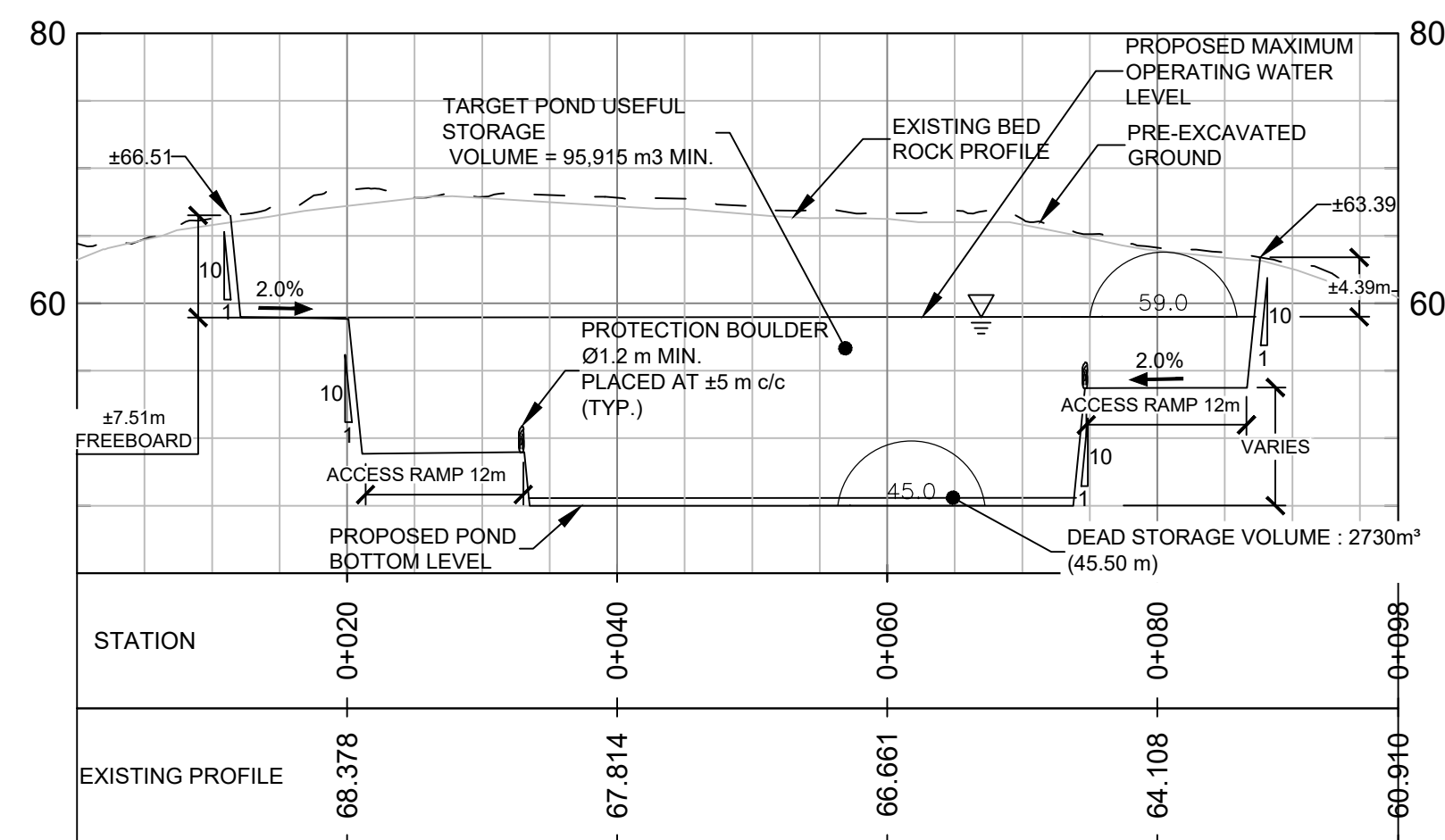
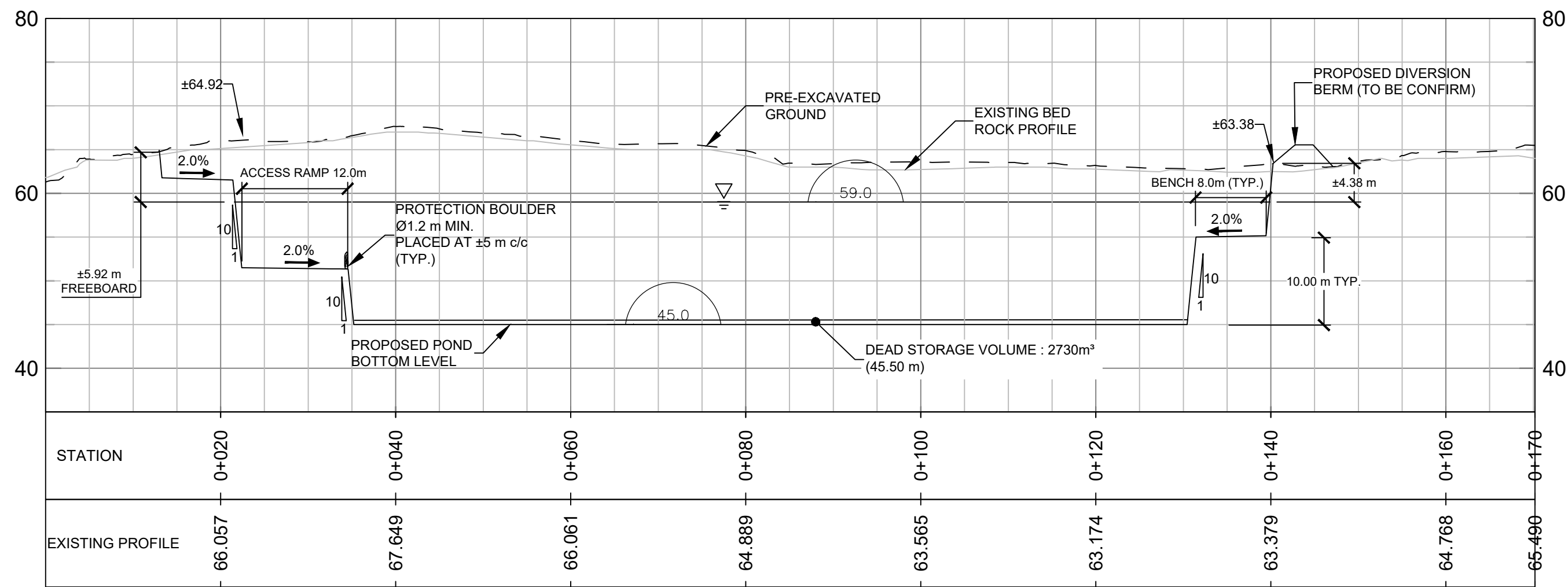
APPENDIX A. DRAWINGS

Drawing List

Drawing No.	Revision	Title
62-695-230-003	RC	230 – General Earth Works Quarry D CWP 3 Madrid Layout General Plan View
62-695-230-004	RC	230 – General Earth Works Quarry D CWP 3 Madrid Layout Profiles
62-695-230-005	RB	230 – General Earth Works Quarry D CWP 3 Madrid Layout Details



BULK EXCAVATION VOLUME OF BED ROCK MATERIALS= 154,660 m3
BULK EXCAVATION VOLUME FOR OVERBURDEN MATERIALS = 5,290 m3
BULK FILL VOLUME = 10,000 m3



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KEY PLAN

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Project # :
704148

DWG#: 704148-2000-41DP03-060_PC

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RA	2025-10-10	INTERNAL REVIEW	A.L.N.	R.L.	M.G.

REVISIONS

TITRE / TITLE
AGNICO EAGLE - HOPE BAY DIVISION
695 - WATER MANAG.
230 - GENERAL EARTH WORKS
QUARRY D CWP3
MADRID LAYOUT
PROFILES

DESSINÉ PAR
DRAWN BY BP BISHAL PAUDEL, E.I.T.

DATE
2025-10-31

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REVIEWED BY A.L.N. ANH-LONG NGUYEN P.Eng.

2025-10-31

VERIFIÉ PAR
VERIFIED BY R.L. RENÉ LAVIOLETTE, P.Eng.

2025-10-31

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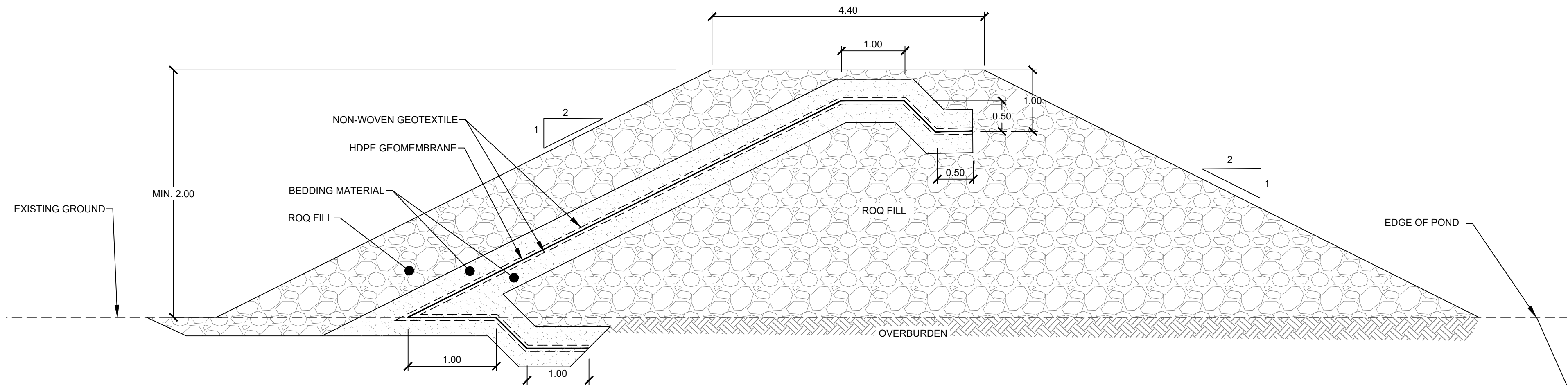
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DRAWING NO. 62-695-230-000-004

NO. PROJET
PROJECT NO. 6212/704148

REVISION
RC

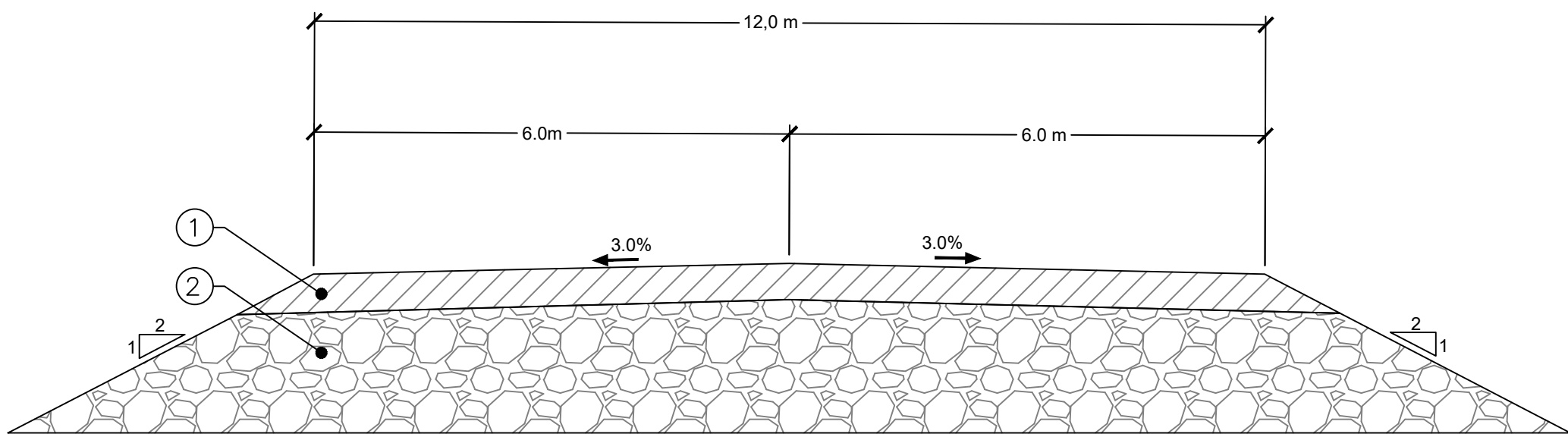
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DIVERSION BERM TYPICAL CROSS SECTION
SCALE: NONE

BEDDING MATERIAL PARTICLE SIZE DISTRIBUTION LIMITS	
PARTICLE SIZE (mm)	% PASSING
25.0	100
20.0	90-100
12.5	50-100
10.0	30-100
5.0	10-80
0.63	2-35
0.08	1-15

DESCRIPTION	
ROQ FILL	NPAG CRUSHED STONE FOUNDATION (Ø0-600 mm), MIN. THICKNESS 900mm, COMPACTED TO METHOD SPEC.
MEMBRANE BEDDING	NPAG CRUSHED STONE FOUNDATION (Ø0-25 mm) THICKNESS VARIABLE MIN 300 mm COMPACTED TO 95% S.P.M.D.D.
GEOTEXTILE	NEEDLE PUNCHED NONWOVEN, SHORT STAPLE POLYPROPYLENE FIBER, TENSILE STRENGTH MINIMUM 650 N
HDPE MEMBRANE	HIGH DENSITY POLYETHYLENE RESIN MEMBRANE (HDPE), 1.5 mm THICKNESS (60 MILS)



ROAD TYPICAL CROSS SECTION FOR OVERBURDEN AREA

DESCRIPTION :	
①	SURFACING MATERIAL - CRUSHED STONES Ø-38 mm, COMPACTED TO 95% METHOD SPEC. , 150 mm THICKNESS
②	RUN-OF-QUARRY (ROQ) : NPAG CRUSHED STONE (Ø0-600 mm), VARIABLE THICKNESS FROM 1 050 mm TO 1 350 mm*
NOTE : ALL MATERIAL MUST BE COMPETENT, WELL GRADED AND COMPACTABLE, NON-ACID-GENERATING ROCK, FREE OF ORGANIC MATTER, FROZEN SOIL, SNOW AND ICE.	
*COMBINED WITH A 150 mm SURFACE LAYER, RESULTING IN A TOTAL THICKNESS OF 1 200 TO 1 500 mm TO PROTECT THE PERMAFROST	

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Project # : 704148
DWG#: 704148-2000-41DP03-0075_PB

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REVISIONS

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QUARRY D - CWP3
MADRID LAYOUT
DETAILS

DESSINÉ PAR B.P. BISHAL PAUDEL, E.I.T. DATE 2025-11-17

REVU PAR ALN LONG NGUYEN, P.ENG. 2025-11-17

VÉRIFIÉ PAR R.L. RENE LAVIOLETTE, P.ENG. 2025-11-17

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