AtkinsRéalis	AGNICO EAGLE	**TECHNICAL NOTE** Design Report: Sump 5		
		Document No.: Agnico Eagle no.: 6212-695-132-REP-006 ATRL Ref.: 704148-2000-41ER-0004	**Rev.** R0	**Date:** June 10, 2026

Title of document: **Design Report: Sump 5**

Client: Agnico Eagle Mines Limited

Project: **Hope Bay Block 4 and 5 (6212-E-132-004 & 6212-E-132-005)**

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List of Revisions

#	Prep.	Rev.	App.	Date	Pages	Remarks
R0	ALN, HB	RL	RL	Jun. 10, ‘26		Issued for design

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AtkinsRéalis	AGNICO EAGLE	TECHNICAL NOTE Design Report: Sump 5		
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1. Introduction



The **Hope Bay property** is in the Kitikmeot region of Nunavut, Canada, approximately 685 km northeast of Yellowknife and 125 km southwest of Cambridge Bay. The property spans 191,342 hectares and includes portions of the Hope Bay and Elu greenstone belts. The 80-km-long Hope Bay greenstone belt hosts three main gold deposits: Doris, Madrid, and Boston.

Agnico Eagle Mines Limited (Agnico Eagle) acquired the Hope Bay property in February 2021. The project has significant infrastructure, including underground mine development at the Doris and Boston deposits, a processing plant, airstrips, a port, and an all-weather road network.

The site development involves new civil constructions such as pad for ore storage, diversion berms, roads and others. Contact water from freshet, rain and underground mining dewatering is handled by storing water in sumps, ponds and pits which are then pumped to a treatment plant for re-use and discharge to the environment.

Part of the project involves the development of Sump 5 to manage the contact water runoff from the Madrid North waste rock storage facility (WRSF) and the ore stockpile pad.

2. Design Concept

Figure 2-1 presents an overview of the Sump 5. It is a pond used to collect contact water runoff transferred from the Madrid North WRSF and ore stockpile pad. Water stored in this sump is then pumped to the Contact Water Pond 3 (CWP 3).

The drawings for Sump 5 can be found in **Appendix A** of this report.

The main design features of Sump 5 are the following:

- The pond will be excavated within bedrock.
- An access ramp will be built to allow access to the bottom of the pond.
- Diversion berms will be built as required to divert contact water from the WRSF and ore pad toward it.

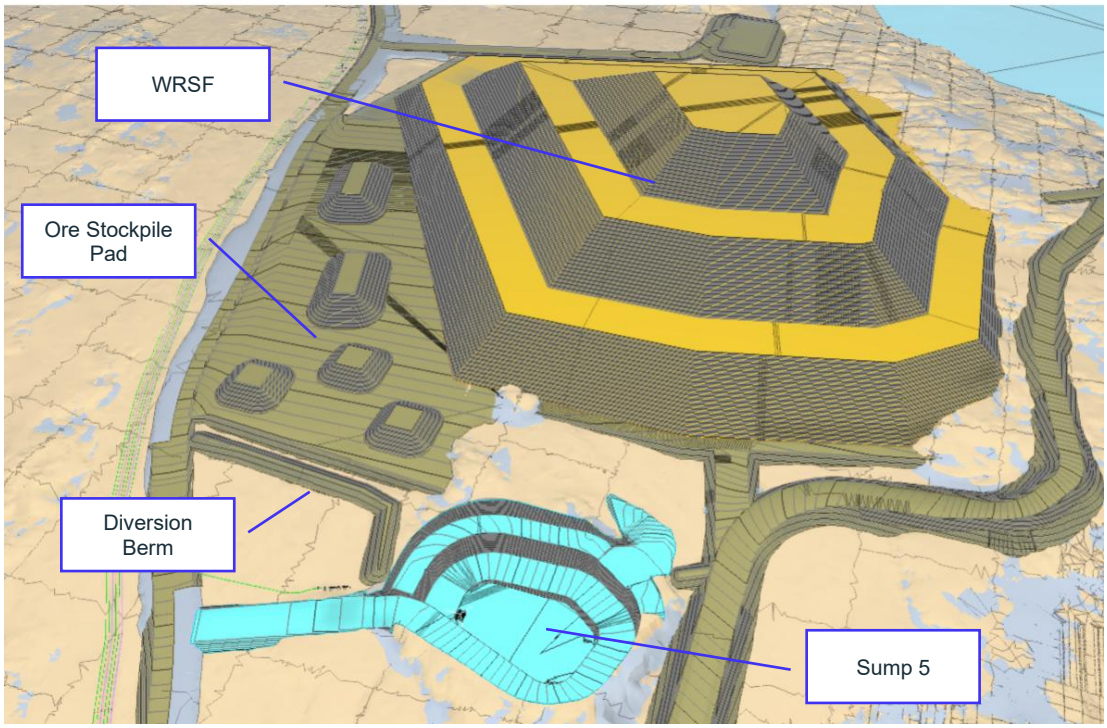


Figure 2-1: Sump 5

3. System Design

3.1 Design Criteria

The following design criteria are based on the general design criteria developed for this project (AtkinsRéalis 2025).

The design criteria for Sump 5 are as follows:

- Pond capacity to contain contact water from a 100-year, 24 hr rain event (57.2 mm) and an average year (2.33 yr) 30-day snowmelt (190.0 mm)
- Runoff coefficient: 1
- Catchment area draining to pond: 125,200 m^2
- Time to drain down the design runoff volume: 30 days
- Minimum live volume required: approx. 31,000 m^3
- Freeboard: minimum 0.5 m

- Dead storage: 0.5 m
- Sump 5 is designed for temporary storage of water and is not designed as a permanent holding pond.

3.2 Design

The following section provides an overview of the design of Sump 5. Further details can be found in the drawings in **Appendix A**.

3.2.1 Pond Excavation

Overall, the Sump 5 footprint is located entirely on the bedrock outcrop.

Sump 5 will be blasted and excavated in bedrock using benches up to 10 m deep with 1H:10V slope side wall to ensure its stability. The Sump 5 sizes are approximately as:

- Length: about 60 m; width: about 30 m; depth: about 10 m.

3.2.2 Diversion Berms

The diversion berms will consist of an earthwork structure with a HDPE geomembrane liner anchored into the overburden.

3.3 Survey Data

The design of these water management infrastructures is based on topographic contour maps produced from aerial LiDAR survey taking in September 2024 supplied by Agnico Eagle.

3.4 Foundation Conditions

The project site is in a continuous permafrost that has been estimated to extend to depths in excess of 500 m, with an active layer of about 1 m to 2 m.

Based on the review of satellite imagery and survey topography data of the area, Sump 5 is located predominantly on outcrop with limited footprint extending over tundra.

Overall, the geomorphology of the project site consists of rock outcrop ridges, typically trending North-South and parallel ranges. The general slope trend is towards south-east, and the predominant ground elevation at Sump 5 are 54 to 62 m.

The project geotechnical database comprises of a series of geotechnical drilling campaigns, many mine exploration drill holes, and numerous geological and geotechnical studies commissioned during various phases of this project. The project geomorphology is described as typical coastal lowland with numerous lakes and ponds separated by glacial landforms. These geological features run parallel to the geological intrusions of diabase dykes and sills. The shape of the drainage basins is generally long and narrow and is predominantly orientated along the north-south axis. Lakes are usually present along the bottom of those elongated valleys.

Surface conditions are either rock outcrop or tundra or surface runoff and water features and lakes. Patterned ground, usually consisting of surface drainage rills, mask the underlying soils. Small, frost-heaved clay-silt polygons are very common. Tundra is typically shallow to deep overburden over bedrock.

Bedrock contour maps have been developed for the project site based on outcrop mapping and large database of drill holes (mine exploration and geotechnical). Bedrock is predominantly outcrop in the Sump 5 area. The bedrock is mainly comprised of mafic metavolcanic (mainly meta-basalts) and meta-sedimentary rocks that are bound by Archean granite intrusives and gneisses.

The overburden soils are mostly of marine origin and include clay, silt and some sand. The overburden soils are typically ice rich and thaw sensitive.

The pond layout may be slightly adjusted to ensure the pond water containment is located within the bedrock excavation. Further detailed geotechnical investigation specific to the Sump 5 area is on-going.

It was determined that slope stability and thermal analyses are not required for this facility, given the facility is entirely located within competent rock formation.

4. Construction Methodology

4.1 Construction Materials

The most common material available at the site is the Run of Quarry (ROQ). The low permeable till is almost absent at the site; therefore, HDPE geomembrane is used as liner for the pond and surface runoff diversion berms. The Liner system typically is a liner sandwiched between geotextile and crushed rock bedding material.

All geosynthetics materials and liner construction shall be in accordance with the requirements developed by the Geosynthetic Research Institute (GRI).

Natural sand or sand and gravel borrows are also not available at the site, therefore, crushed rock is used as transition, bedding and surfacing materials.

To protect the liner from crushed stone damages, non-woven geotextile installed on one or both sides of the liner is used, depending on the use of bedding layer.

All materials used for earthworks are natural and from local excavations, free from acid generation/metal leaching potential (i.e. non-PAG). Materials selection, placement, compaction and quality control requirements are as per the project's earthworks specifications.

A summary description of the earthworks materials (per SRK 2022) is presented in the following Table 4-1.

Table 4-1 : List of Earthworks Materials

Type	Description
ROQ	Run-off-Quarry Material, (Ø 0-600 mm); minimum thickness 900 mm; compacted to method specification (specified roller passes)
Transition Material	Crushed stone (Ø 0-200 mm); minimum thickness 300 mm; compacted to method specification (specified roller passes)
Bedding Material	Crushed stone (Ø 0-25 mm); minimum thickness 100 mm; compacted to 95% Standard Proctor Maximum Dry Density (S.P.M.D)
Surfacing Material	Crushed stone (Ø 0-38 mm); minimum thickness 100 mm; compacted to method specification (specified roller passes)
Geotextile	Needle punched Non-woven Geotextile Fabric
Geomembrane	High Density Polyethylene liner, 1.5 mm thickness (60 mil)

4.2 Quality Control

The project's earthworks construction Quality Control and Assurance Program (QA/QC) testing standard procedures listed in Table 4-2 shall be followed.

Table 4-2 : List of Tests for Earthworks

Test Standard	Details
ASTM D2487	Classification of Soils for Engineering Purposes
ASTM D2216	Water (Moisture) Content in Soil and Rock
ASTM C136	Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates
ASTM D854	Specific Gravity of Soils
ASTM D698	Laboratory Compaction Characteristics of Soil Using Standard Effort
ASTM D2922	ASTM D2922 Density of Soil in Place by Nuclear Methods

4.3 Construction Equipment

The following construction equipment will be used in the construction of Sump 5:

- CAT 374 Excavator
- John Deere 470 Excavator or similar sized equipment
- CAT 745 Rock Trucks or similar sized equipment
- CAT D8 Dozer

- CAT 14H Grader
- Sandvik D800 Drill.

4.4 Construction Schedule and Sequence

The sump excavation being predominantly in the rock, therefore the timing is of less concern from constructability considerations. This is unlike tundra excavation which might require excavation in winter for workability concerns. The construction of Sump 5 is planned for Q4 2026 / Q1 2027. The planned construction sequence is as follows:

1. The access road from Windy Road to Sump 5 will be constructed with ROQ and 38mm crushed rock material (i.e. road surfacing).
2. Overburden material within the sump footprint will be ripped, stripped and hauled away from the area to the designated overburden stockpile.
3. Sandvik drills and explosives will be used to pioneer blast the area down to a flat grade. Drilling and blasting will continue following the blast engineering master plan and sequencing to develop the pond and internal ramp.
4. As material is blasted to produce ROQ, excavators will load out the material, and it will be hauled away to other project locations for placement.
5. Excavators and dozers will support the blast program to scale the walls and manage the produced ROQ material.
6. The diversion berm(s) will be constructed during construction of the pond to manage water and prevent ingress during construction activities.

4.5 Construction Monitoring

As per Part D, Item 11 and Schedule D, Item 2, a construction summary report (including monitoring) will be provided following the terms of the Licence. Construction monitoring may include the following activities:

- Erosion and Sediment control
    - Installation of silt fence or silt curtain location dependent monitored during construction
    - Inspections after precipitation events or freshet monitoring
    - Inspection of dewatering equipment, discharge locations, and sampling
    - Ensuring compliance with water quality monitoring requirements
- Site tidiness and proper waste management ;
- Inspections of equipment for leaks or spills, spill kits and secondary containment of any hazardous materials ;
- Adherence to licence conditions;
- Construction QA/QC of geotechnical and liner installation.

5. Operational and Closure Considerations

The principal operational considerations for Sump 5 shall include as a minimum the following:

- Monitoring of operational water levels.

Closure activities for Sump 5 will be carried out in accordance with the Interim Closure and Reclamation Plan.

6. References

AtkinsRéalis 2025 — General Design Criteria for Water Management MPEI Infrastructure, 6212-695-132-DGC-001, revision RC, 2025-09-22.

SRK 2022 — Technical Specifications Earthworks and Geotechnical Engineering Hope Bay Project, Nunavut Canada Revision I – Issue for Construction. Prepared for Agnico Eagle Mines Limited.

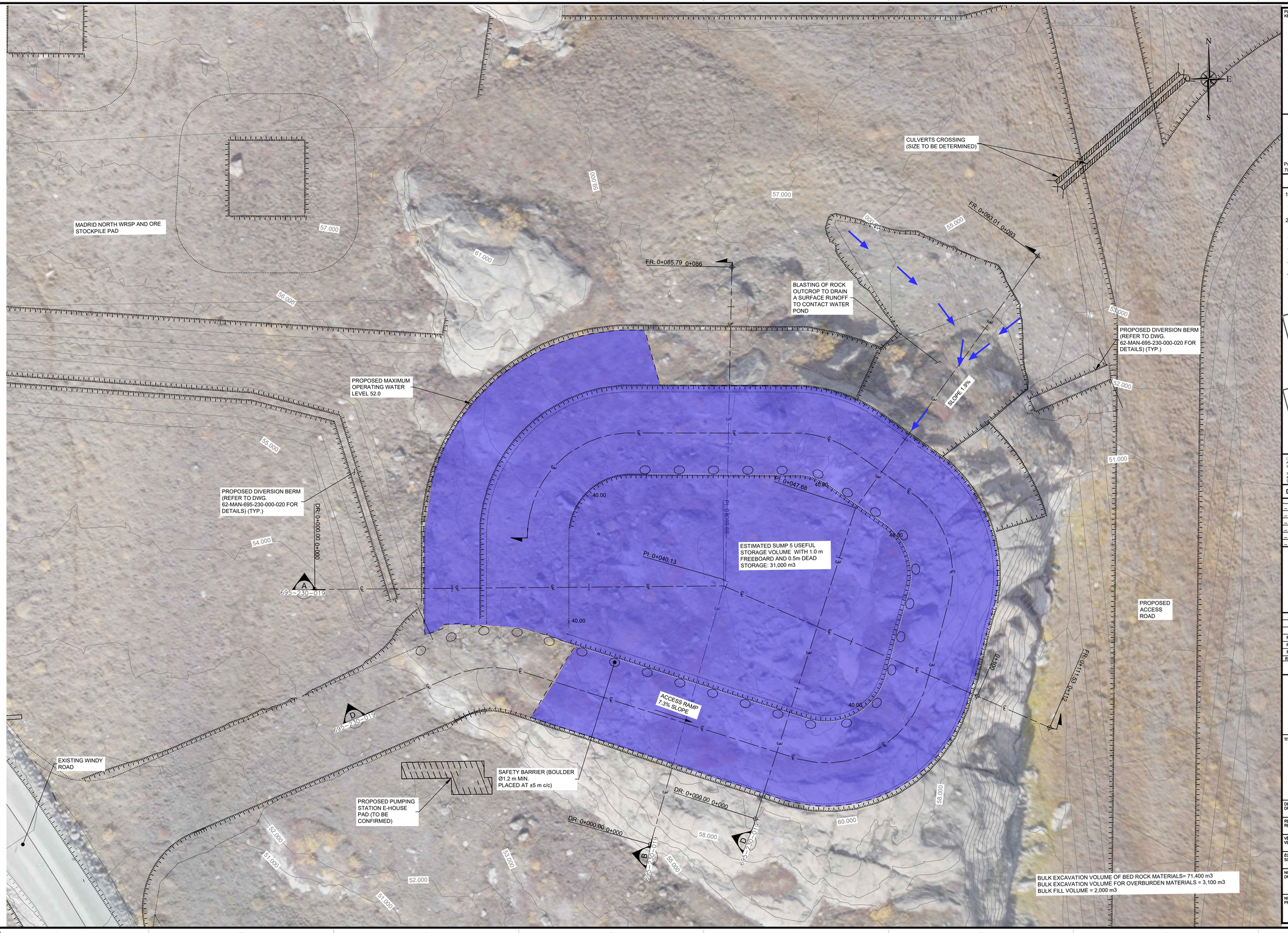
APPENDIX A. DRAWINGS

Drawing List

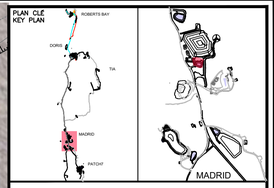
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62-MAN-695-230-000-019	RC	230 – General Earth Works Sump 5 Madrid Layout Profiles
62-MAN-695-230-000-020	RC	230 – General Earth Works Sump 5 Madrid Layout Details

NO. DESSIN
DRAWING NO. 62-MAN-695-230-000-018_RC

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Phone : 514 393-1000

Project # : 704148

DWG#: 704148-2000-41DP03-0062_PC

NOTES GÉNÉRALES / GENERAL NOTES

1. CONTOURS, ELEVATIONS, COORDINATES AND DIMENSIONS ARE IN METERS UNLESS NOTED OTHERWISE.
2. COORDINATE SYSTEM IS UTM NAD83 ZONE 13.

EXISTING TOP OF SLOPE
EXISTING BOTTOM OF SLOPE
PROPOSED TOP OF SLOPE
PROPOSED BOTTOM OF SLOPE

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TITRE / TITLE
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695 – WATER MANAG.
230 – GENERAL EARTH WORKS
SUMP 5 – WRSF
MADRID LAYOUT
GENERAL PLAN VIEW

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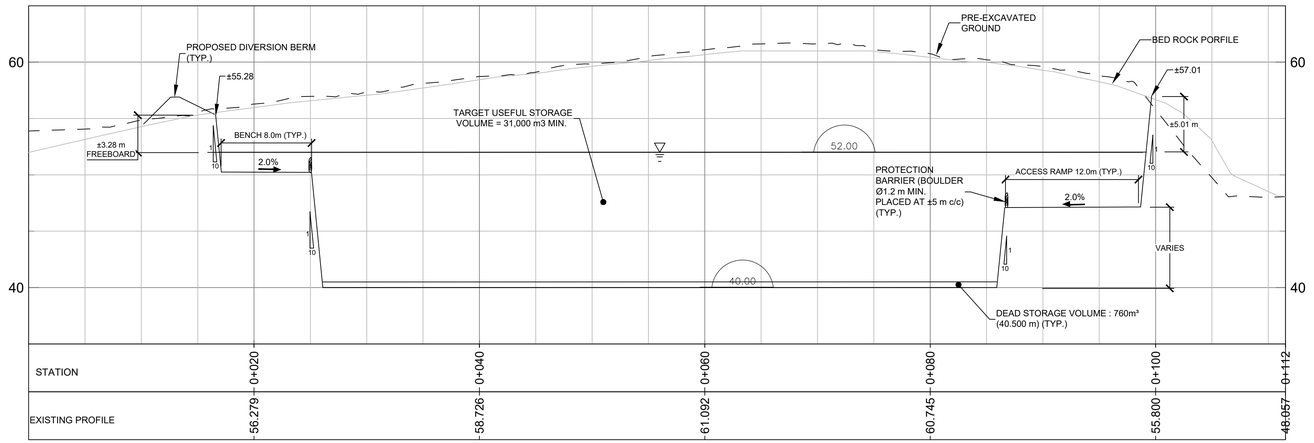
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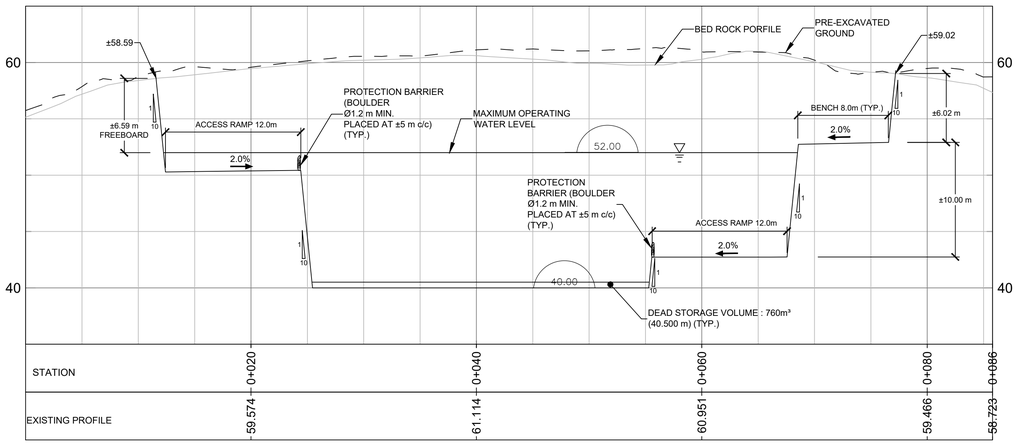
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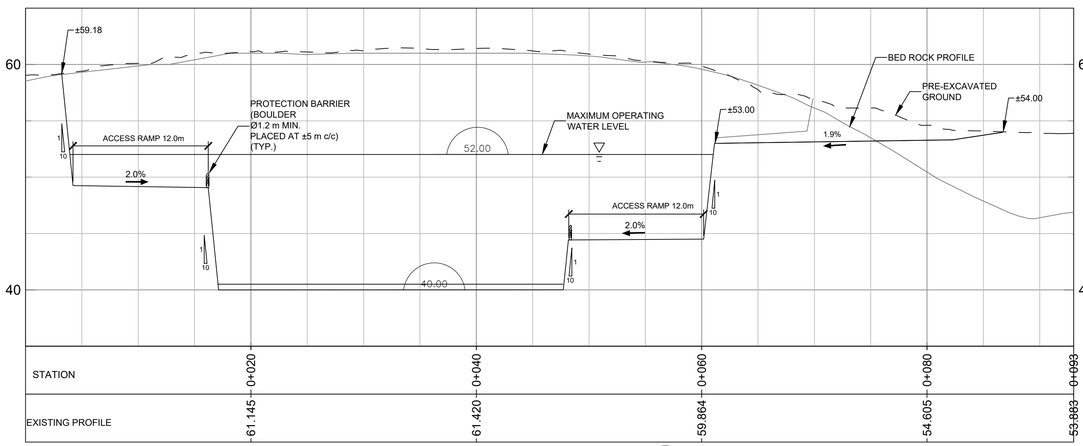
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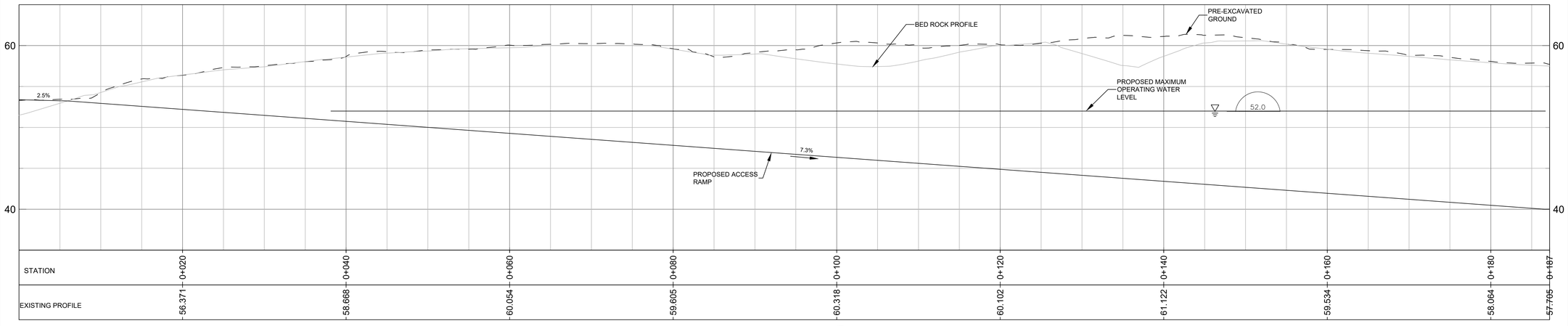
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695-230-018



PROFILES C
SCALE : 1:250
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PROFILES D
SCALE : 1:250
695-230-018

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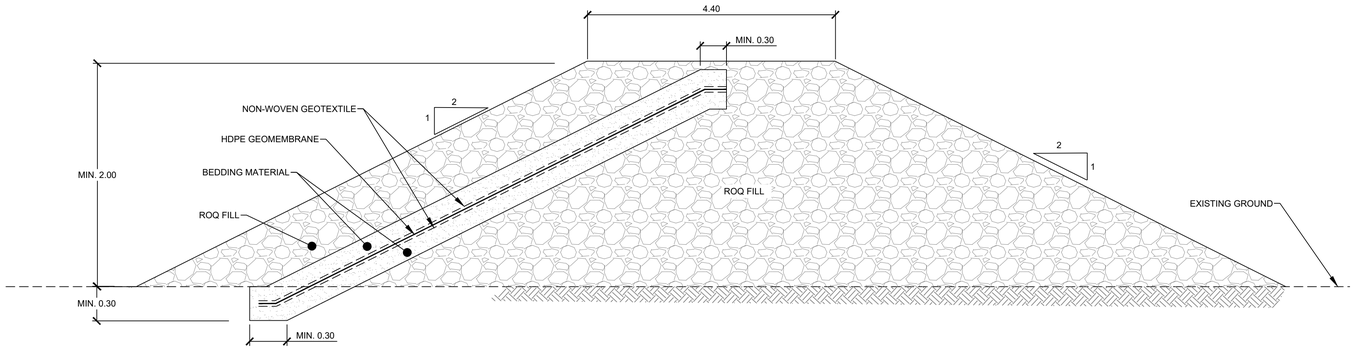
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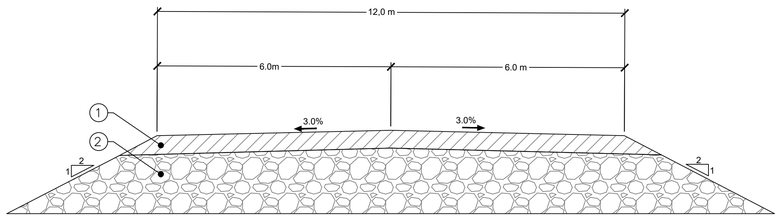
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DIVERSION BERM TYPICAL CROSS SECTION
SCALE: NONE

BEDDING MATERIAL PARTICLE SIZE DISTRIBUTION LIMITS	
PARTICLE SIZE (mm)	% PASSING
25.0	100
20.0	90-100
12.5	50-100
10.0	30-100
5.0	10-80
0.63	2-35
0.08	1-15

DESCRIPTION	
ROQ FILL	NPAG CRUSHED STONE FOUNDATION (Ø0-600 mm), MIN. THICKNESS 900mm, COMPACTED TO METHOD SPEC.
MEMBRANE BEDDING	NPAG CRUSHED STONE FOUNDATION (Ø0-25 mm) THICKNESS VARIABLE MIN 300 mm COMPACTED TO 95% S.P.M.D.D.
GEOTEXTILE	NEEDLE PUNCHED NONWOVEN, SHORT STAPLE POLYPROPYLENE FIBER, TENSILE STRENGTH MINIMUM 650 N
HDPE MEMBRANE	HIGH DENSITY POLYETHYLENE RESIN MEMBRANE (HDPE), 1.5 mm THICKNESS (60 MILS)



ROAD TYPICAL CROSS SECTION FOR OVERBURDEN AREA

DESCRIPTION :

1. SURFACING MATERIAL : CRUSHED STONES 0-38 mm, COMPACTED TO 95% METHOD SPEC. , 150 mm THICKNESS
2. RUN-OF-QUARRY (ROQ) : NPAG CRUSHED STONE (Ø0-600 mm). VARIABLE THICKNESS FROM 1 050 mm TO 1 350 mm*

NOTE :
ALL MATERIAL MUST BE COMPETENT, WELL GRADED AND COMPACTABLE, NON-ACID-GENERATING ROCK, FREE OF ORGANIC MATTER, FROZEN SOIL, SNOW AND ICE.

*COMBINED WITH A 150 mm SURFACE LAYER, RESULTING IN A TOTAL THICKNESS OF 1 200 TO 1 500 mm TO PROTECT THE PERMAFROST

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RC	2026-03-05	ISSUED FOR COMMENTS	A.L.N.	R.L.	M.G.
RB	2025-11-12	FOR COMMENTS	A.L.N.	R.L.	M.G.
RA	2025-10-27	INTERNAL REVIEW	A.L.N.	R.L.	M.G.

REVISIONS

TITRE / TITLE
AGNICO EAGLE – HOPE BAY DIVISION
695 – WATER MANAG.
230 – GENERAL EARTH WORKS
SUMP 5 – WRSF
MADRID LAYOUT
DETAILS

DESSINÉ PAR / DRAWN BY	BP BISHAL PAUDEL, E.I.T.	DATE 2026-03-05
REVU PAR / REVIEWED BY	ALN ANH-LONG NGUYEN, P.ENG.	2026-03-05
VÉRIFIÉ PAR / VERIFIED BY	RL RENÉ LAVIOLETTE, P.ENG.	2026-03-05
ÉCHELLE / SCALE	AS SHOWN	DATE 2026-03-05

NO. DESSIN / DRAWING NO. 62-MAN-695-230-000-020

NO. PROJET / PROJECT NO.	REVISION	FEUILLE / SHT
6212/704148	RC	1 / 1

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