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Memo

To: Oliver Curran, VP Environmental Affairs Client: TMAC Resources Inc.

From: Ryan Williams, Senior Consultant, PEng Project No: 1CT022.055

Reviewed By: Cameron Hore, Senior Consultant, PEng Date: April 1, 2020

Subject: Hope Bay Project: Doris and Madrid 2019 Construction Summary Report

1 Introduction

This memo provides a summary of formal documentation related to the engineering and construction support provided by SRK Consulting (Canada) Inc. (SRK) during 2019 for the following facilities at TMAC Resources Inc's Hope Bay site:

- Madrid South All-Weather Road (from 0 to 1 km);
- Madrid North Contact Water Pond (CWP);
- Madrid North Waste Rock Pile (WRP);
- Madrid North Portal Pad; and
- Naartok East Overburden Stockpile.

For all construction in 2019, SRK provided Issued for Construction (IFC) drawings and construction quality assurance (CQA) where necessary. High-density polyethylene (HDPE) liner installation was performed by A&A Technical Services. Earthworks were completed by Nuna Logistics Ltd. and as-built surveys were conducted by Sub-Arctic Surveys Ltd. SRK has produced the as-built drawings included in this memo based on the survey data provided by Sub-Arctic.

Construction rock for all the components described in this report was sourced from either Quarry 2 at Doris, Quarry D located at approximately kilometre 7 along the Doris-Windy Road, or from approved construction waste rock from the Naartok East Crown Pillar Recovery (CPR). Quarry 2 and Quarry D are both designated construction quarries. Geochemical characterization of this material has been completed (SRK 2007, SRK 2008, SRK 2017) and the material has been confirmed to be non potentially acid generating and non neutral metal leaching. Geochemical characterization of outcrop from within the Naartok East CPR boundaries indicate that arsenic content is low, suggesting that waste rock from Naartok East has a potentially low risk for neutral pH leaching and could be geochemically suitable as construction rock (SRK 2019).

2 Summary of Attachments

The attachments to this memo are organised into separate infrastructure items with sub-level attachments for different forms of documentation.

Of the infrastructure items constructed in 2019, only the Madrid North CWP and the Madrid North Portal Pads involved the installation of liner materials. The complete Madrid North CWP and Madrid North Portal Pad as-built construction reports are provided separately (SRK 2020a, SRK 2020b). The A&A Liner Installation Report for the Madrid North CWP and the Madrid North Portal Pads is provided in Attachment 2.3 and 4.2, respectively.

The contents of all attachments are summarized in Table 1.

Table 1: Contents of Attachments

Attachment		Infrastructure Item	Contents	
1	1.1	Madrid South All-Weather Road	Issued for Construction Drawings	
'	1.2	(0 to 1 km)	As-built Drawings	
	2.1		Issued for Construction Drawings	
2	2.2	Madrid North CWP ⁽¹⁾	Madrid North CWP (1) As-built Drawir	As-built Drawings
	2.3		A&A Liner Installation Report	
3.1		Madrid North WRP	Issued for Construction Drawings	
3	3.2	Wadid Notiff WRF	Interim As-built Drawings (2)	
4	4.1	Madrid North	As-built Drawings	
4	4.2	Portal Pad ⁽³⁾	A&A Liner Installation Report	
5	5.1	Neombel: Foot Overhounder Steeler !!-	Issued for Construction Drawings	
5	5.2	Naartok East Overburden Stockpile	Interim As-built Drawings (4)	
6	6.1	Naartok East Overburden Stockpile Expansion (5)	Issued for Construction Drawings	

Notes:

- 1. The complete as-built report for the Madrid North CWP is provided separately (SRK 2020a).
- Construction of the Madrid North WRP is on-going as part of operations. The interim as-built drawings reflect construction activities completed in 2019.
- 3. Under the current water licence, IFC drawings are not required for the construction of pads. However, during construction of the Madrid North Portal Pad, a field decision was made to line one area of the pad to provide contact water management to allow temporary stockpiling of waste rock. Further details of the field decision are provided in Section 3.4.
- Construction of the Naartok East Overburden Stockpile is on-going as part of operations. The interim as-built drawings reflect construction activities completed in 2019.
- 5. IFC drawings for the expansion of the Naartok East Overburden Stockpile were submitted in December 2019. However, at the time of this report construction of the expansion had not commenced. Therefore, as-built drawings are provided for the construction of the original stockpile design.

3 Field Decisions and Design Changes

3.1 Madrid South All-Weather Road

As confirmed by the As-Built Drawings in Attachment 1.2, the Madrid South All-Weather Road was constructed in accordance with the IFC drawings. No design changes were made during construction.

3.2 Madrid North CWP

In general, the Madrid North CWP was built to the design lines, grades, and requirements as confirmed by the As-Built Drawings. Deviations from, and amendments to, the designs were generally completed to adapt the design to encountered field conditions. The main field decisions and design changes during construction of the CWP are summarized below. Further details are provided in the as-built construction summary report (SRK 2020a).

3.2.1 Overburden Excavation

During excavation of overburden to expose bedrock for the HDPE liner tie-in, a significant pocket of overburden material was encountered at the southern end of the CWP berm. This overburden pocket was found to be approximately 5 m deep. As a result, a decision was made in the field by TMAC and the SRK Site Engineer to backfill the excavated overburden pocket with ROQ and shift the liner tie-in point to higher ground upstream. Deeper than anticipated overburden was also encountered in the northern end of the CWP. Overburden was up to 4 m deep at its deepest. However, concrete plinths were able to be used and the liner to bedrock tie-in did not shift significantly from the original projected design.

These two deeper than anticipated overburden areas resulted in two main design deviations. Firstly, in the southern section, it resulted in the overburden excavation area being backfilled and moving the alignment of the liner tie-in further north into the CWP basin. Secondly in the northern section, it resulted in taller than anticipated concrete plinths. Both changes, while noteworthy, do not impact the overall design intent of the CWP.

3.2.2 Berm Alignment Modification

After filling in the southern overburden pocket with ROQ, a field decision was made by TMAC and the SRK Site Engineer to modify the CWP berm alignment. This was done in order to simplify the liner to bedrock tie-in. The modification involved adding a new berm, approximately 20 m long and perpendicular to the existing berm, at chainage 0+040. The liner then was tied into a bedrock outcrop to the northwest. This enabled the length of the liner to bedrock tie-in to be shortened and allowed the temporary access ramp (built into the excavation backfill area) to be used as part of the berm fill. The implications of the berm re-alignment meant that the CWP basin area, and therefore pond capacity, was slightly reduced. However, the reduction in storage area was minimal and most of the area that was cut-off by the new berm was above the full supply level (elev. 69.7 m). The as-built CWP storage capacity, with the berm alignment modification, exceeds the design storage capacity and therefore meets the design intent.

3.2.3 Concrete Plinths

The CWP design included an allowance for concrete plinths, if required, once the overburden was excavated and the bedrock exposed. Concrete plinths were designed to be a minimum of 0.3 m high, with a maximum height of 0.5 m.

During excavation of overburden, several overburden pockets were encountered, as well as many undulations in the bedrock. Some areas ended up having near vertical faces up to 2 m high. Except for the one large overburden pocket that was backfilled, concrete plinths were used to bridge across the excavated pockets and/ or bedrock undulations to facilitate a smooth liner tie-in. As a result, some of the concrete plinths were required to be constructed taller than the maximum design plinth height. Therefore, backfill was specified on both sides of the plinth for additional lateral support.

The GSE polylock embedment strip, as recommended in the original design to allow the liner to be welded to the top of concrete, was not available on site. Instead, the liner was bolted directly into the top of concrete using Sopramastic and a stainless-steel plate. This method achieves the design intent.

3.2.4 Berm Downstream Slope

After completion of earthworks and liner installation, the final CWP layout was surveyed by Sub-Arctic. SRK reviewed the as-built survey and identified that the downstream slopes were steeper than design slope of 2H:1V. These over-steepened slopes were also noted at various times during the ongoing QA checks on site. The outside as-built slopes were typically between 1.7H:1V to 1.9H:1V, but some sections were as steep as 1.65H:1V. SRK instructed TMAC to place additional fill or regrade the slopes to achieve the design slope of 2H:1V. Instead, TMAC requested SRK to complete a stability analysis to determine if the steeper than design slopes were stable in the current arrangement to meet the minimum required factors of safety.

SRK completed a stability analysis to assess the risk of geotechnical instability of the CWP berm in its as-built condition. The results of the analysis indicate that the berm is stable and meets minimum factors of safety (FoS), provided the overburden in the foundation remains frozen, or as a minimum, doesn't thaw beyond the average active layer thickness in this area (typically around 1 m). If the overburden completely thaws, the FoS may reduce below the criteria of 1.5 but this is not an indication that immediate failure is expected. Therefore, the risk of failure is low. Additionally, if a failure were to occur it would be slow and progressive rather than rapid, i.e. there would be enough warning provided adequate monitoring is in place.

SRK is therefore satisfied that the berm's downstream slope does not currently need to be regraded to meet a slope of 2H:1V. Further details of the analysis are provided in SRK (2020a).

3.3 Madrid North WRP

Based on the as-built survey data, the Madrid North WRP perimeter berm and access road was built according to design, though the WRP pad was constructed higher than originally intended. However, the pad still achieves the design intent of creating a graded base towards the CWP,

and therefore the increased height of the pad does not affect the design. TMAC operations were in regular communication with SRK during construction of the pad and SRK is satisfied that the constructed pad meets the design objectives.

SRK notes that based on the as-built survey, the toe of the WRP at its southern extent has exceeded the design toe location and is encroaching on the full supply level of the Madrid North CWP. SRK recommends that during freshet and summer months, the WRP toe be monitored to ensure the pond level in the CWP does not reach the toe of the WRP.

TMAC are currently free dumping the first bench of waste rock on the constructed WRP pad and are meeting the overall design slopes of 2.5H:1V based on the as-built survey. At the time of this report, the WRP sumps had not been constructed, though SRK understands TMAC is planning to construct the sumps prior to the 2020 freshet. SRK will continue to provide on-going construction support and will inspect the constructed WRP during the 2020 annual geotechnical inspection (typically scheduled for the months of July or August). The construction process and as-built drawings for the sumps will be reported in the 2020 after they are constructed.

3.4 Madrid North Portal Pad

The Madrid North Portal Pad was originally designed as a pad laydown area to support development of the Madrid North Portal. It was designed in accordance with other pads on site, consisting of a minimum 1 m (for thermal protection of permafrost) of geochemically suitable run-of-quarry (ROQ) or run-of-mine (ROM) rock.

During construction, TMAC decided that it would be preferable for underground trucks delivering waste from the portal development to temporarily place waste rock on the pad and instead allow larger haul trucks designed for surface operations to collect the waste and haul it to the waste rock pile. Based on this decision, the dedicated area of the portal pad where the transfers would occur required contact water management. A field decision was made to line one area of the pad with geomembrane to create a waste rock re-handling area that could manage potential contact water during daily operations.

SRK produced a design for the lined temporary waste storage area to facilitate construction in the field. The design included standard material specifications and thicknesses for transition and bedding material to protect the liner, as well as lined berms to create sufficient storage capacity for the maximum daily snowmelt. The lined cell area was graded to the northeast corner to create a sump area for collection of potential contact water. The size of the lined cell was designed to accommodate underground waste from the portal development for at least one day of operations.

The As-Built Drawings for the pad and lined cell are provided in Attachment 4.1. A&A's Liner Installation Report is also included in Attachment 4.2. Based on the as-built data, the lined cell area meets the design storage capacity and the design grade. Further construction details are provided in SRK (2020b).

3.5 Naartok Overburden Stockpile

As confirmed by the As-Built Drawings in Attachment 5.2, the Naartok East Overburden Stockpile was constructed in general accordance with the IFC drawings. No design changes were made during construction. Additional ROQ/ROM access roads were built within the footprint of the stockpile to facilitate haul truck access. This is considered an operational requirement and does not impact the design or function of the stockpile.

The as-built data (collected January 2020) indicates that the downstream slope of the stockpile is slightly steeper than the design slope of 5H:1V. However, TMAC are continuing to shape and regrade the stockpile to meet the design grades.

In December 2019, TMAC requested SRK to evaluate expansion options for the overburden stockpile. SRK completed the design to expand the overburden stockpile height and footprint and IFC drawings were submitted in December 2019. At the time of receipt of as-built survey data (January 2020) TMAC had not commenced the stockpile expansion, therefore the as-built drawings only include comparison of as-built surfaces to the original stockpile design.

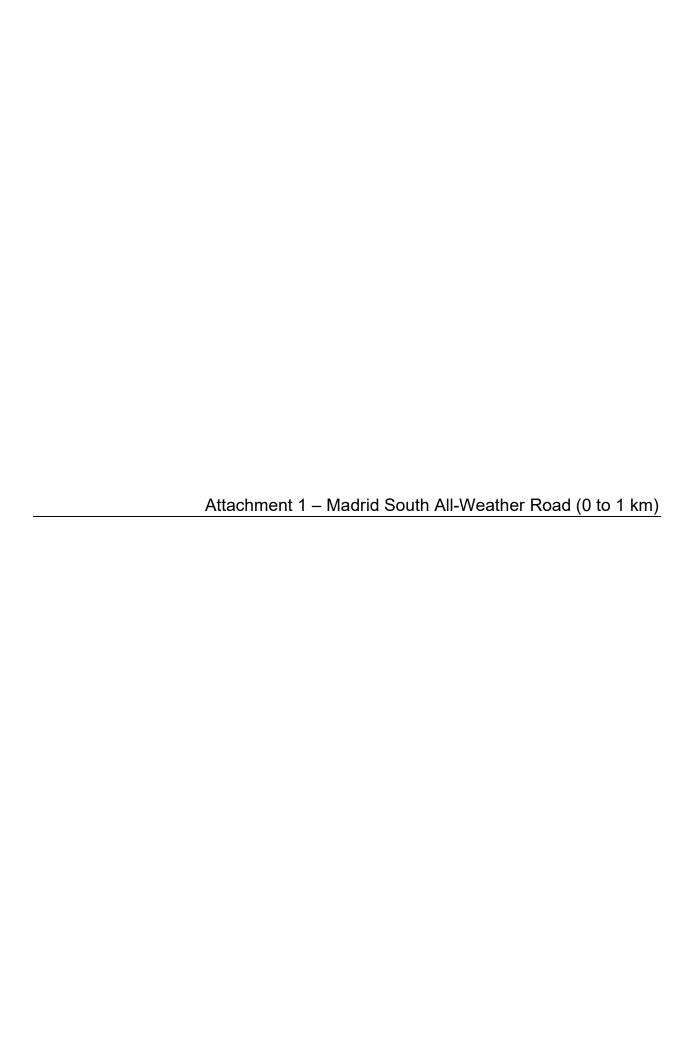
Disclaimer—SRK Consulting (Canada) Inc. has prepared this document for TMAC Resources Inc. Any use or decisions by which a third party makes of this document are the responsibility of such third parties. In no circumstance does SRK accept any consequential liability arising from commercial decisions or actions resulting from the use of this report by a third party.

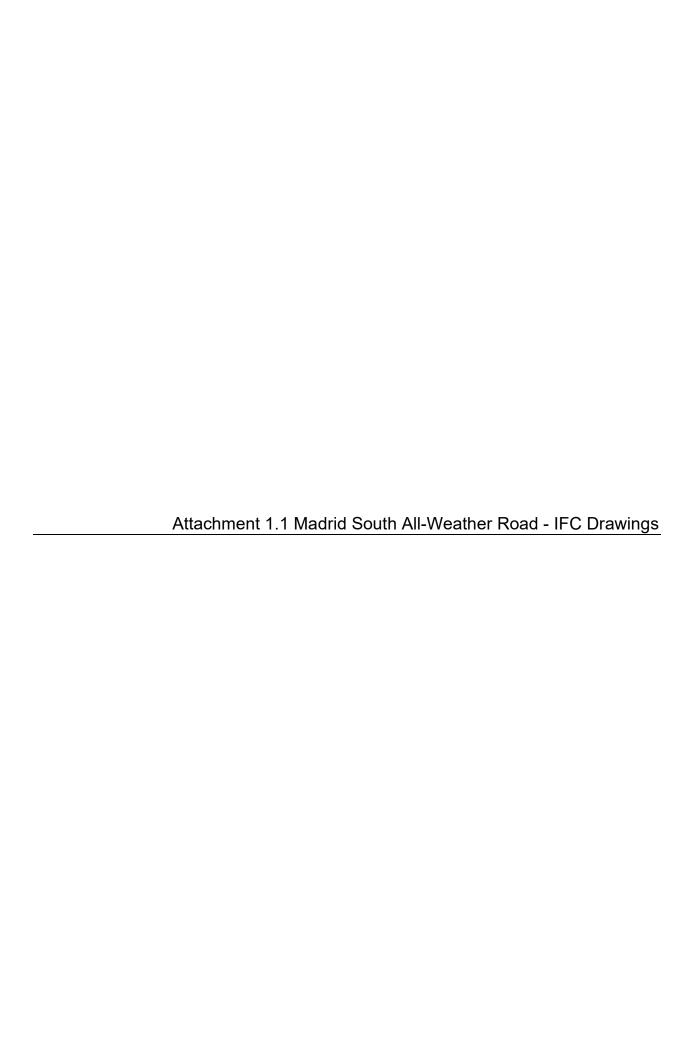
The opinions expressed in this report have been based on the information available to SRK at the time of preparation. SRK has exercised all due care in reviewing information supplied by others for use on this project. Whilst SRK has compared key supplied data with expected values, the accuracy of the results and conclusions from the review are entirely reliant on the accuracy and completeness of the supplied data. SRK does not accept responsibility for any errors or omissions in the supplied information, except to the extent that SRK was hired to verify the data.

4 References

SRK Consulting (Canada) Inc., 2007. Geochemical Characterization of Quarry Materials, Doris-North Project, Hope Bay, Nunavut, Canada (Revised March 2007). Report Prepared for Miramar Hope Bay Ltd., April 2007.

- SRK Consulting (Canada) Inc., 2008. Geochemical Characterization of Quarry Materials for the Doris-Windy All-Weather Road, Hope Bay Project. Report Prepared for Hope Bay Mining Ltd., 1CH008.000. August 2008.
- SRK Consulting (Canada) Inc., 2017. Geochemical Characterization of Madrid-Boston Project Quarries Report, Hope Bay Project. Report Prepared for TMAC Resources Inc., 1CT022.013. November 2017.
- SRK Consulting (Canada) Inc. 2019. Classification of Waste Rock in Support of Segregating Construction Rock from Naartok East Crown Pillar Recovery, Madrid North, Hope Bay Project. Report prepared for TMAC Resources Inc. Project No.: 1CT022.037. August 2019.
- SRK Consulting (Canada) Inc., 2020a. Madrid North Contact Water Pond As-Built Report, Hope Bay Project. Report Prepared for TMAC Resources Inc., 1CT022.043. March 2020.
- SRK Consulting (Canada) Inc., 2020b. Madrid North Portal Pad As-Built Report, Hope Bay Project. Memo Prepared for TMAC Resources Inc., 1CT022.051. April 2020.





Engineering Drawings for the Madrid South All-Weather Road, Hope Bay Project, Nunavut, Canada

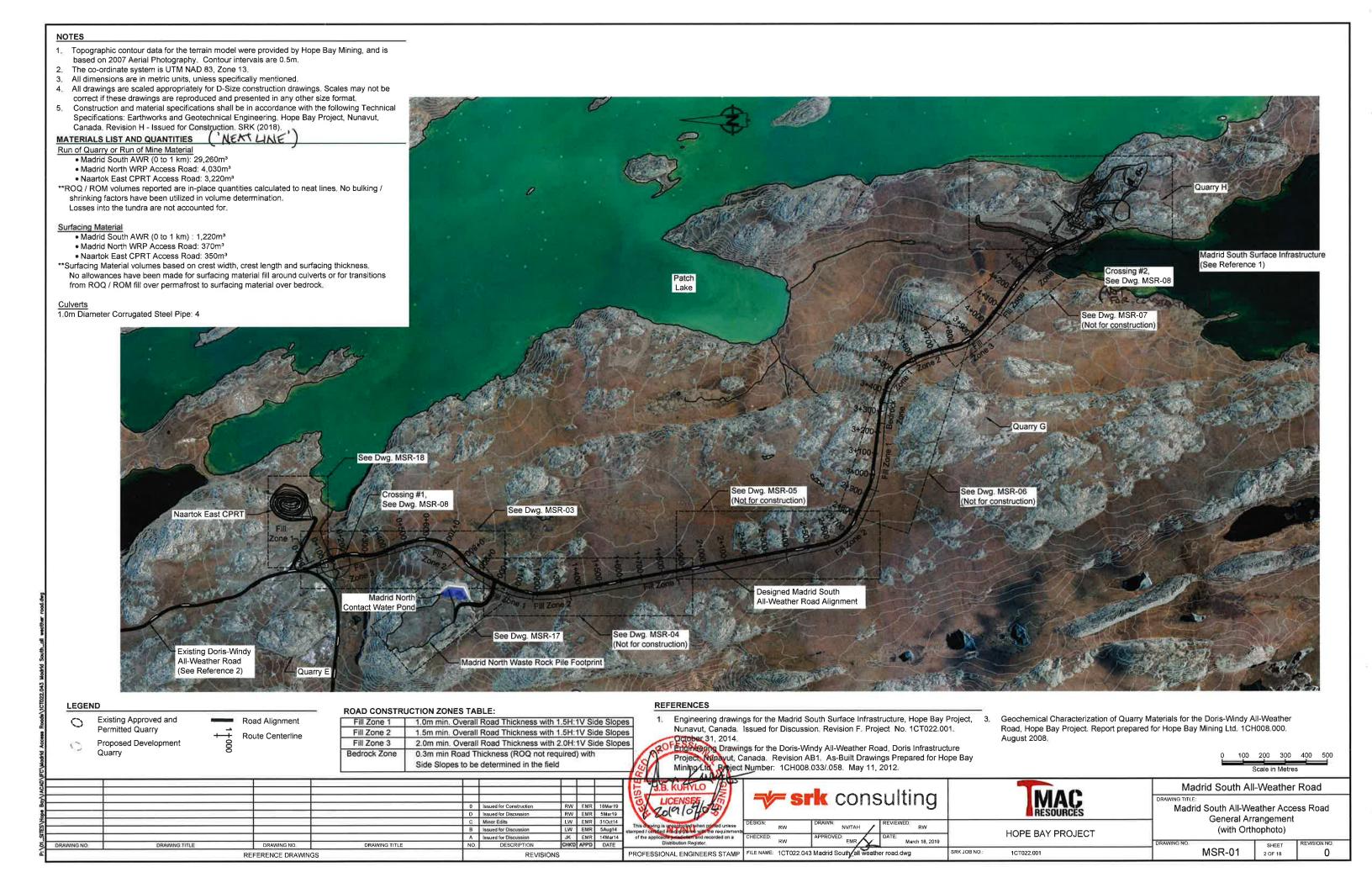
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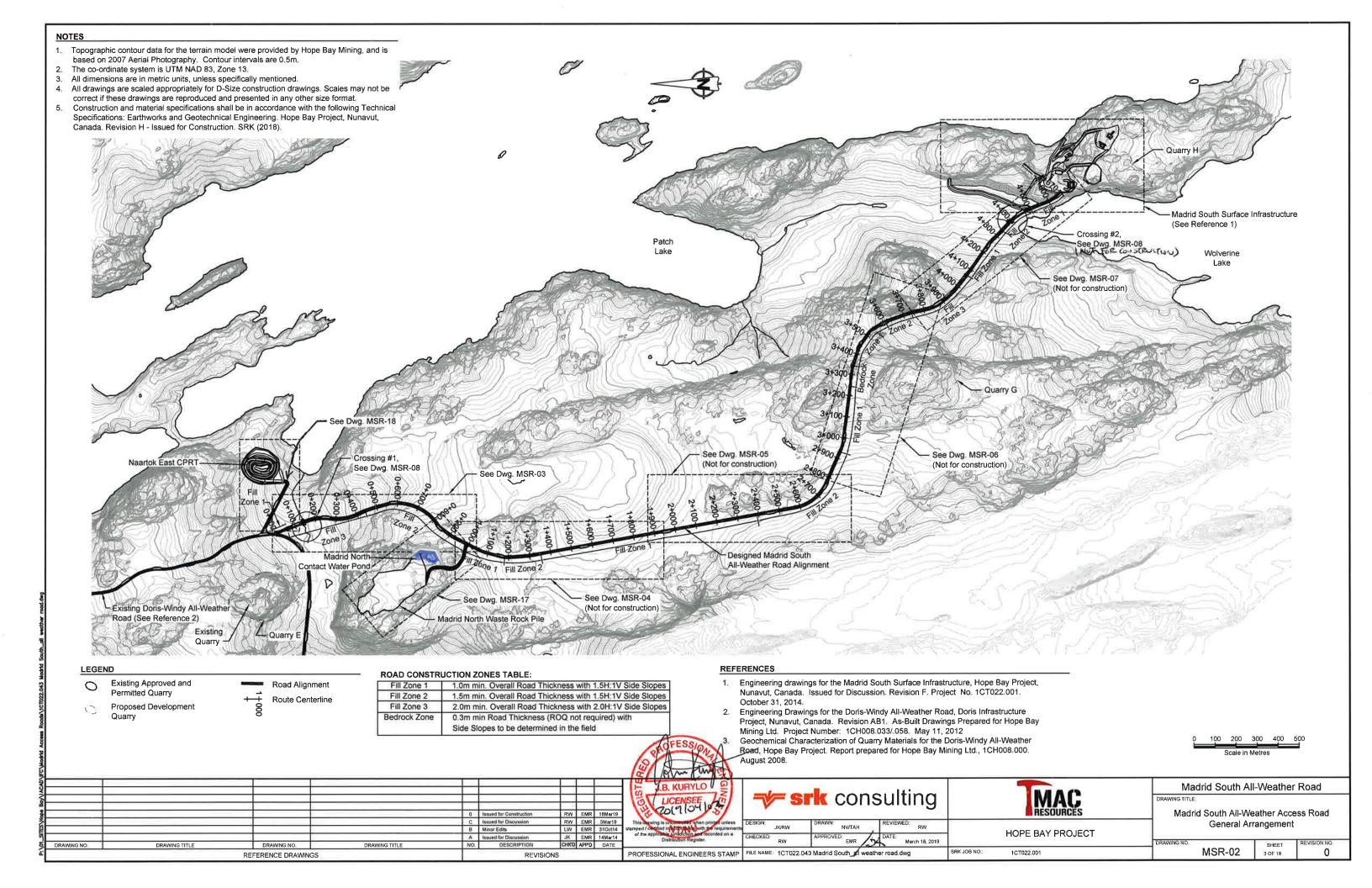
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MSR-00	Engineering Drawings for the Madrid South All-Weather Road, Hope Bay Project, Nunavut, Canada	0	Mar. 18, 2019	Issued for Construction
MSR-01	General Arrangement (with Orthophoto)	0	Mar. 18, 2019	Issued for Construction
MSR-02	General Arrangement	0	Mar. 18, 2019	Issued for Construction
MSR-03	Road Alignment Plan and Profile (1 of 5)	0	Mar. 18, 2019	Issued for Construction
MSR-04	Road Alignment Plan and Profile (2 of 5)	A	Mar. 14, 2014	Issued For Discussion
MSR-05	Road Alignment Plan and Profile (3 of 5)	А	Mar. 14, 2014	Issued For Discussion
MSR-06	Road Alignment Plan and Profile (4 of 5)	А	Mar. 14, 2014	Issued For Discussion
MSR:07	Road Alignment Plan and Profile (5 of 5)	D	Mar. 14, 2014	Issued For Discussion
MSR-08	Typical Road and Crossing Details	0	Mar. 18, 2019	Issued for Construction
MSR-09	Animal Crossings Plan and Sections	0	Mar. 18, 2019	Issued for Construction
MSR-10	Crossing #1 - Bridge Option - Plan, Section and Detail (1 of 2)	А	Mar. 14 <u>.</u> 2014	Discontinued (Removed)
MSR-11	Crossing #1 - Bridge Option - Plan, Section and Detail (2 of 2)	А	Mar. 14, 2014	Discontinued (Removed)
MSR-12	Crossing #1 - Arch Culvert Option	А	Mar. 14, 2014	Discontinued (Removed)
MSR-13	Crossing #2 - Arch Culvert	А	Mar. 14, 2014	Discontinued (Removed)
MSR-14	Typical Arch Culvert Plan and Profile (1 of 2)	А	Mar. 14, 2014	Discontinued (Removed)
MSR-15	Typical Arch Culvert Details (2 of 2)	А	Mar. 14, 2014	Discontinued (Removed)
MSR-16	Material List and Quantity Estimates	В	Aug. 5, 2014	Issued For Discussion
MSR-17	Madrid North Waste Rock Access Road Plan and Profile	0	Mar. 18, 2019	Issued for Construction
MSR-18	Naartok East CPRT Access Road Plan and Profile	0	Mar. 18, 2019	Issued for Construction

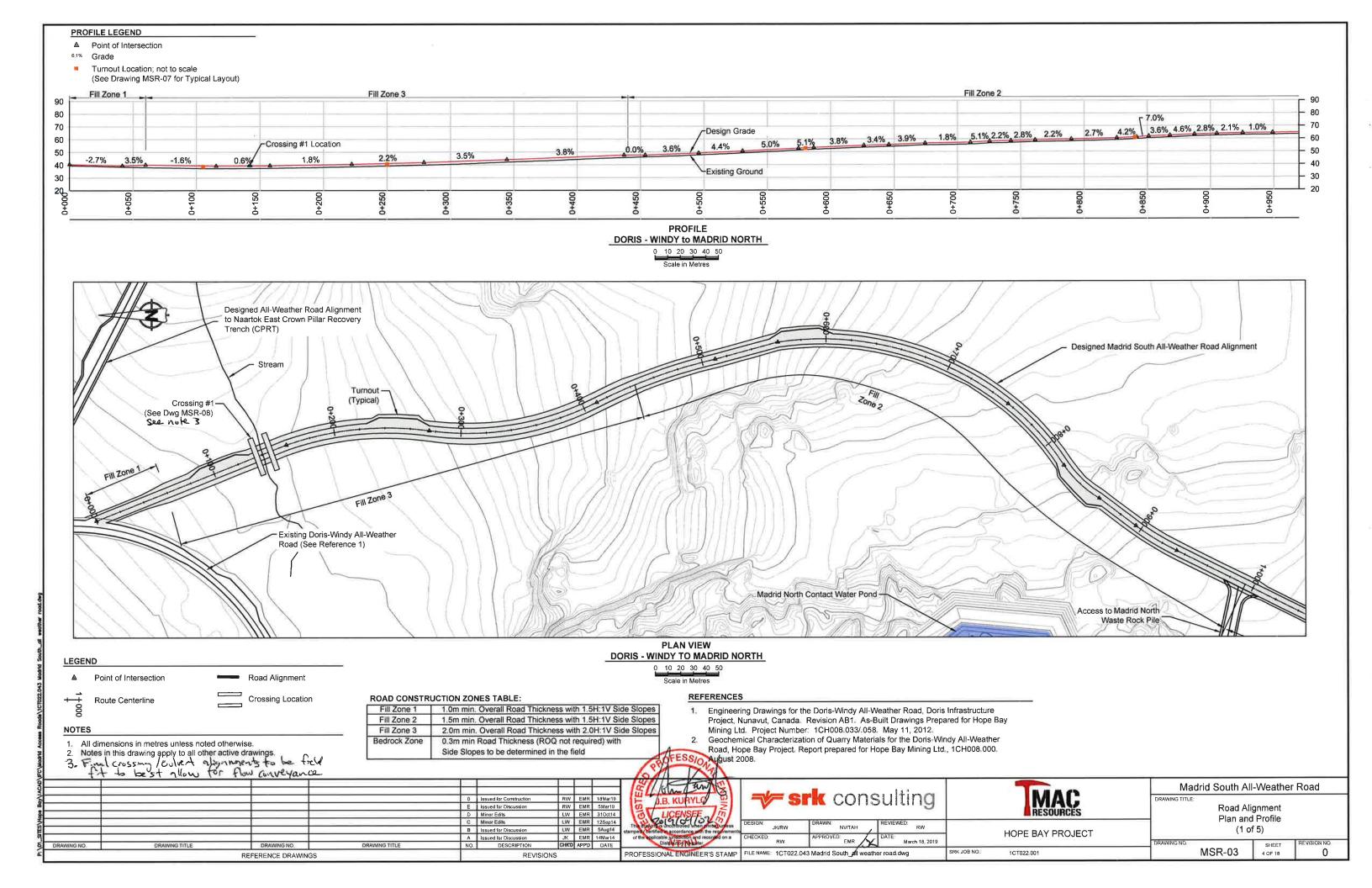


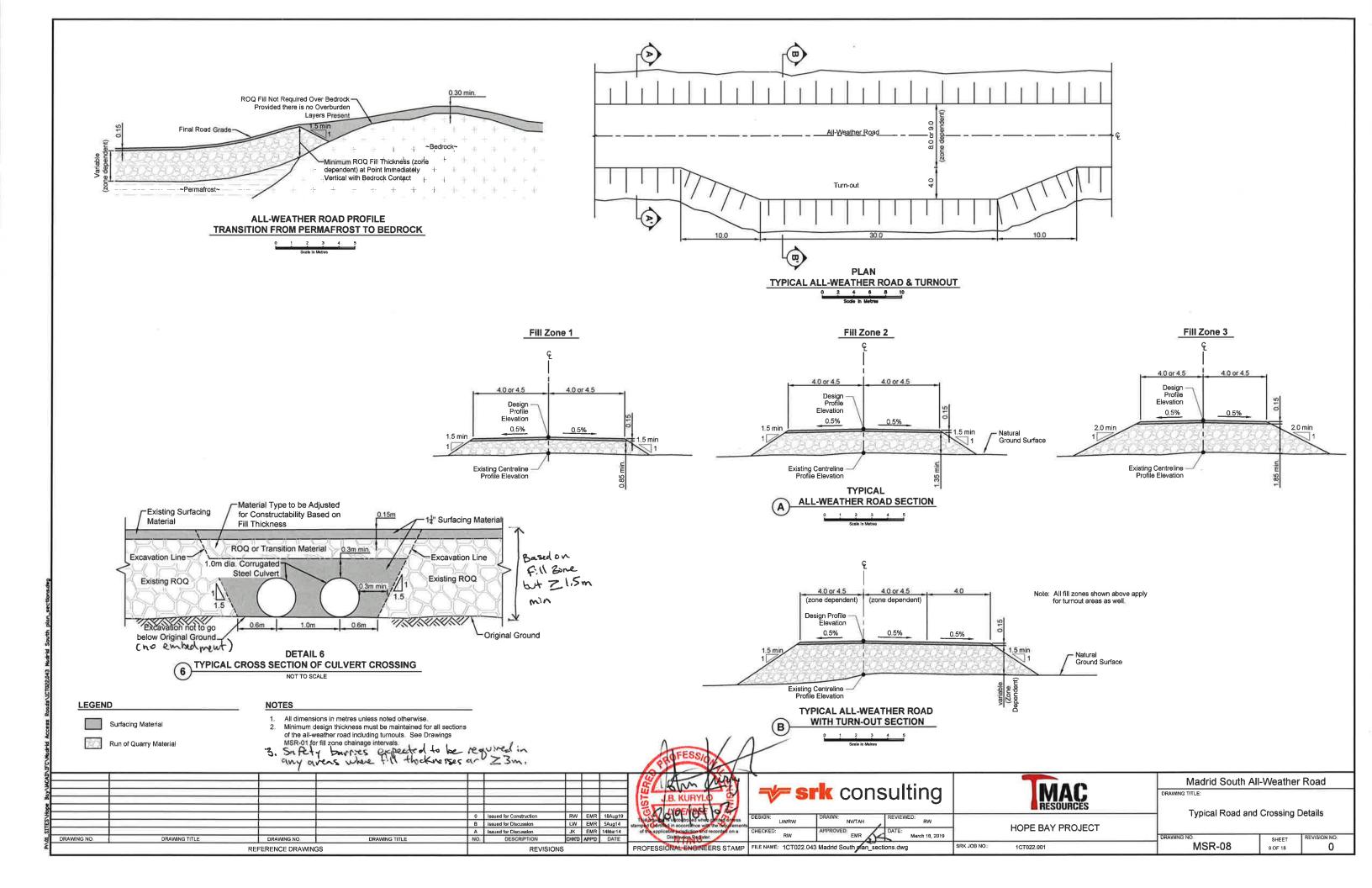
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2019/04/02
STK consulting

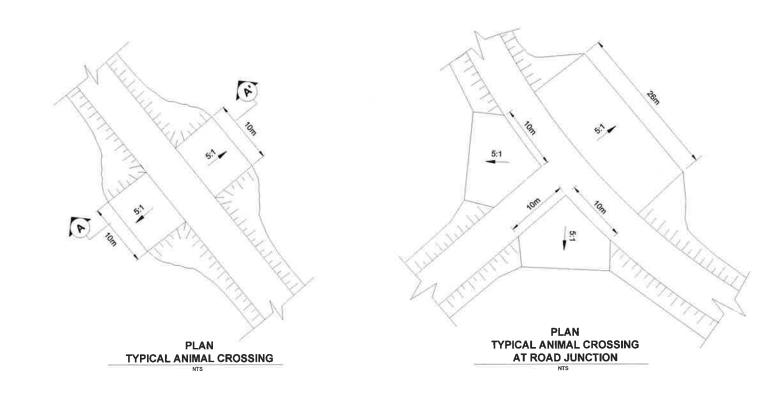
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Revision 0
March 18, 2019
Drawing MSR-00

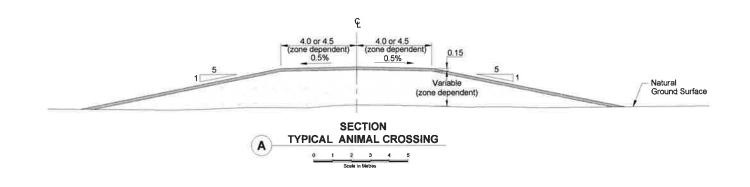




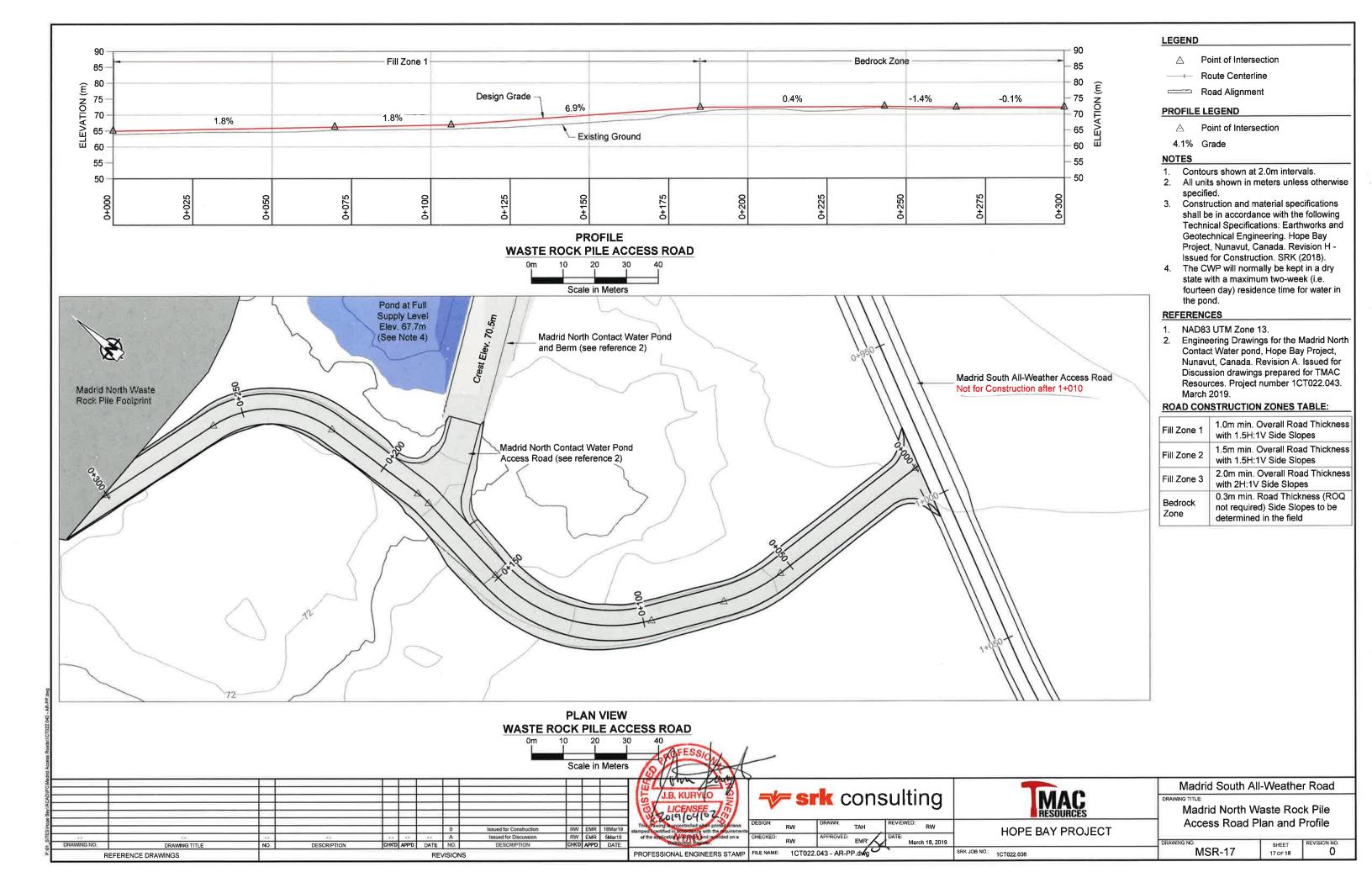


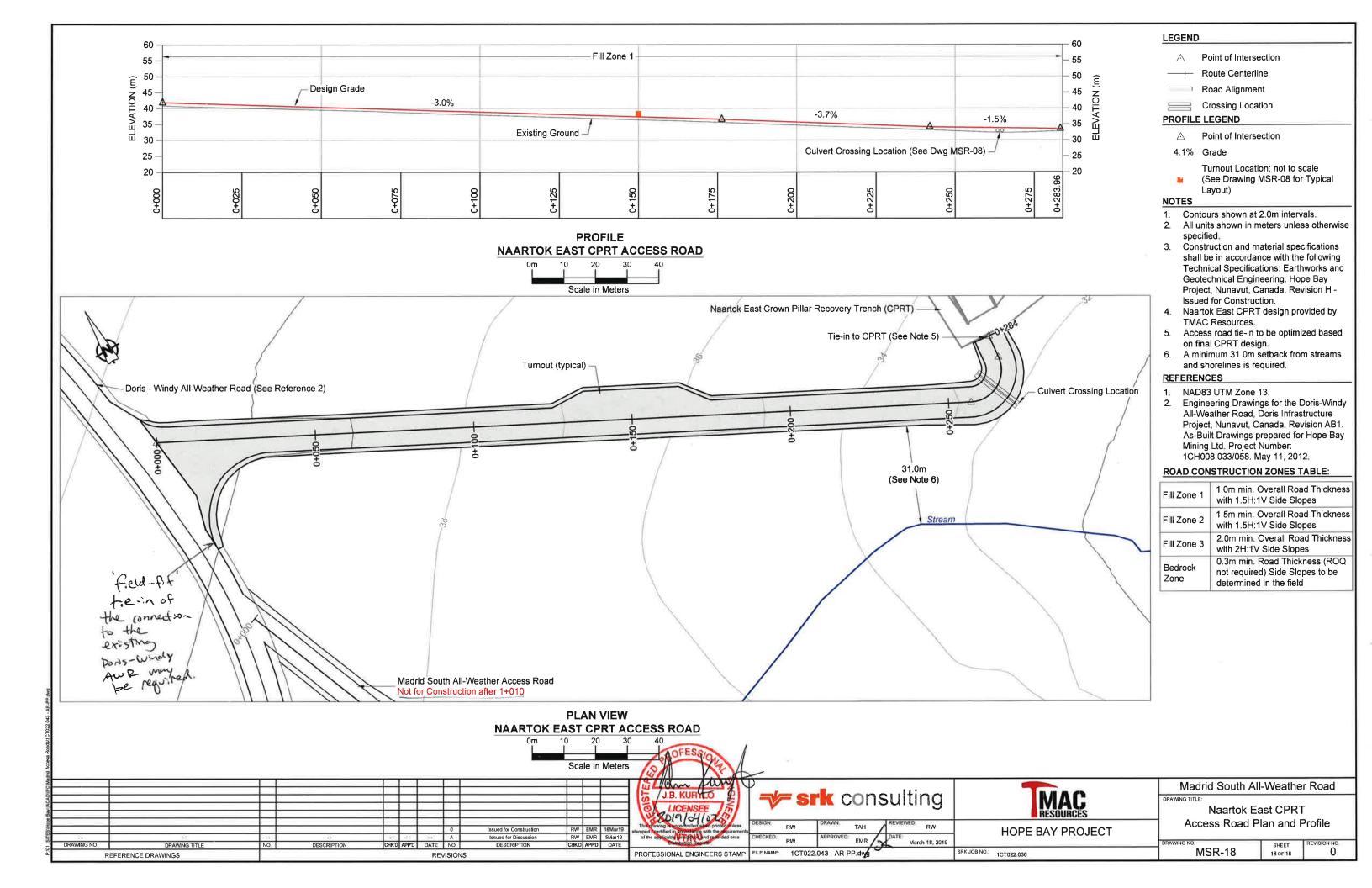














Engineering Drawings for the Madrid South All-Weather Road (0 - 1km), Hope Bay Project, Nunavut, Canada

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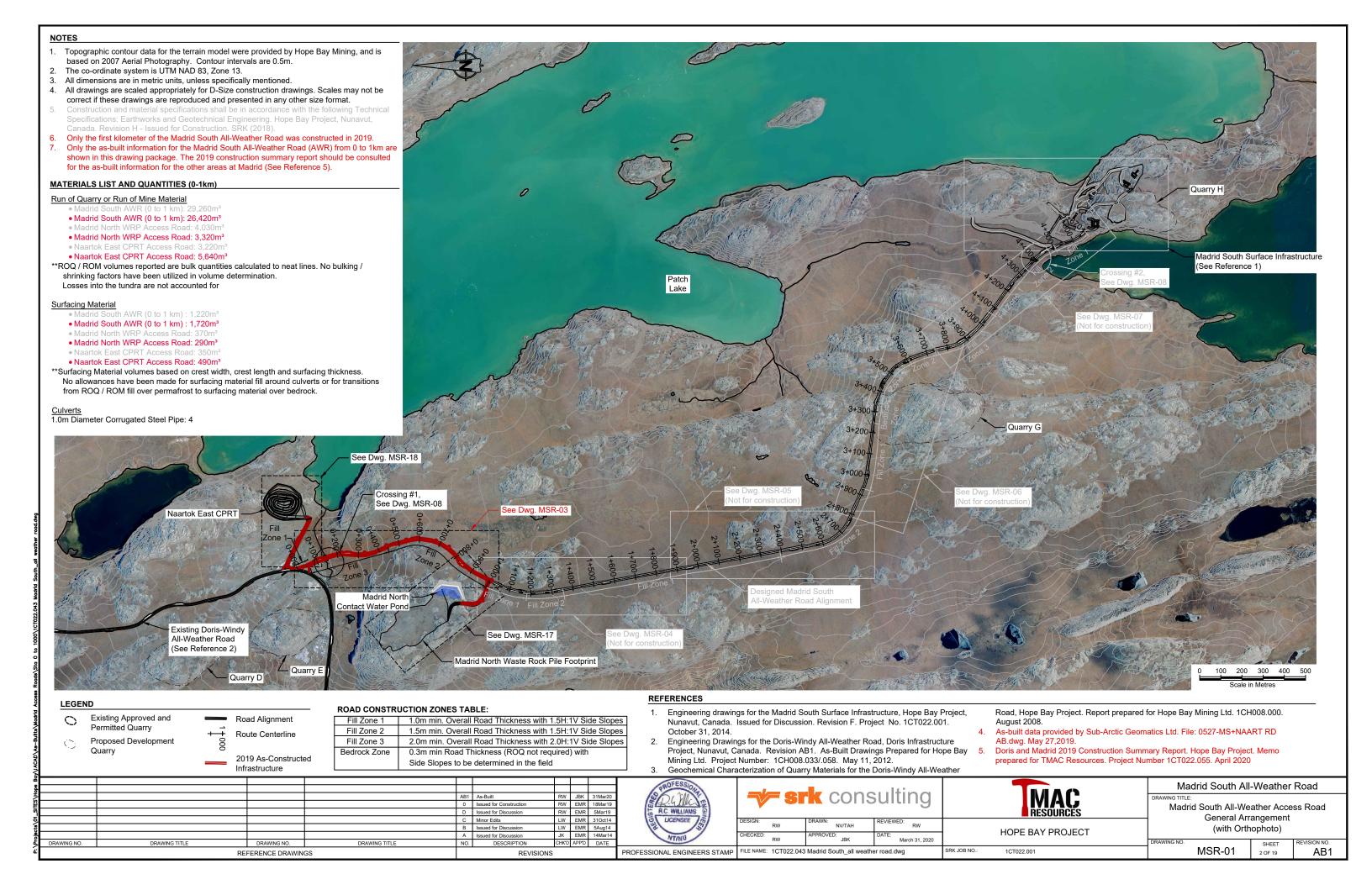
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MSR-00	Engineering Drawings for the Madrid South All-Weather Road, Hope Bay Project, Nunavut, Canada	AB1	Mar. 31, 2020	As-Built
MSR-01	General Arrangement (with Orthophoto)	AB1	Mar. 31, 2020	As-Built
MSR-02	General Arrangement	AB1	Mar. 31, 2020	As-Built
MSR-03	Road Alignment Plan and Profile (1 of 5)	AB1	Mar. 31, 2020	As-Built
MSR-04	Road Alignment Plan and Profile (2 of 5)	Α	Mar. 14, 2014	Issued For Discussion
MSR-05	Road Alignment Plan and Profile (3 of 5)	Α	Mar. 14, 2014	Issued For Discussion
MSR-06	Road Alignment Plan and Profile (4 of 5)	Α	Mar. 14, 2014	Issued For Discussion
MSR-07	Road Alignment Plan and Profile (5 of 5)	D	Mar. 14, 2014	Issued For Discussion
MSR-08	Typical Road and Crossing Details	AB1	Mar. 31, 2020	As-Built
MSR-09	Animal Crossings Plan and Sections	AB1	Mar. 31, 2020	As-Built
MSR-10	Crossing #1 - Bridge Option - Plan, Section and Detail (1 of 2)	Α	Mar. 14, 2014	Discontinued (Removed)
MSR-11	Crossing #1 - Bridge Option - Plan, Section and Detail (2 of 2)	Α	Mar. 14, 2014	Discontinued (Removed)
MSR-12	Crossing #1 - Arch Culvert Option	Α	Mar. 14, 2014	Discontinued (Removed)
MSR-13	Crossing #2 - Arch Culvert	Α	Mar. 14, 2014	Discontinued (Removed)
MSR-14	Typical Arch Culvert Plan and Profile (1 of 2)	Α	Mar. 14, 2014	Discontinued (Removed)
MSR-15	Typical Arch Culvert Details (2 of 2)	Α	Mar. 14, 2014	Discontinued (Removed)
MSR-16	Material List and Quantity Estimates	В	Aug. 5, 2014	Discontinued (Removed)
MSR-17	Madrid North Waste Rock Access Road Plan and Profile	AB1	Mar. 31, 2020	As-Built
MSR-18	Naartok East CPRT Access Road Plan and Profile	AB1	Mar. 31, 2020	As-Built
MSR-19	Typical Road Fill Comparison Sections	AB1	Mar. 31, 2020	As-Built

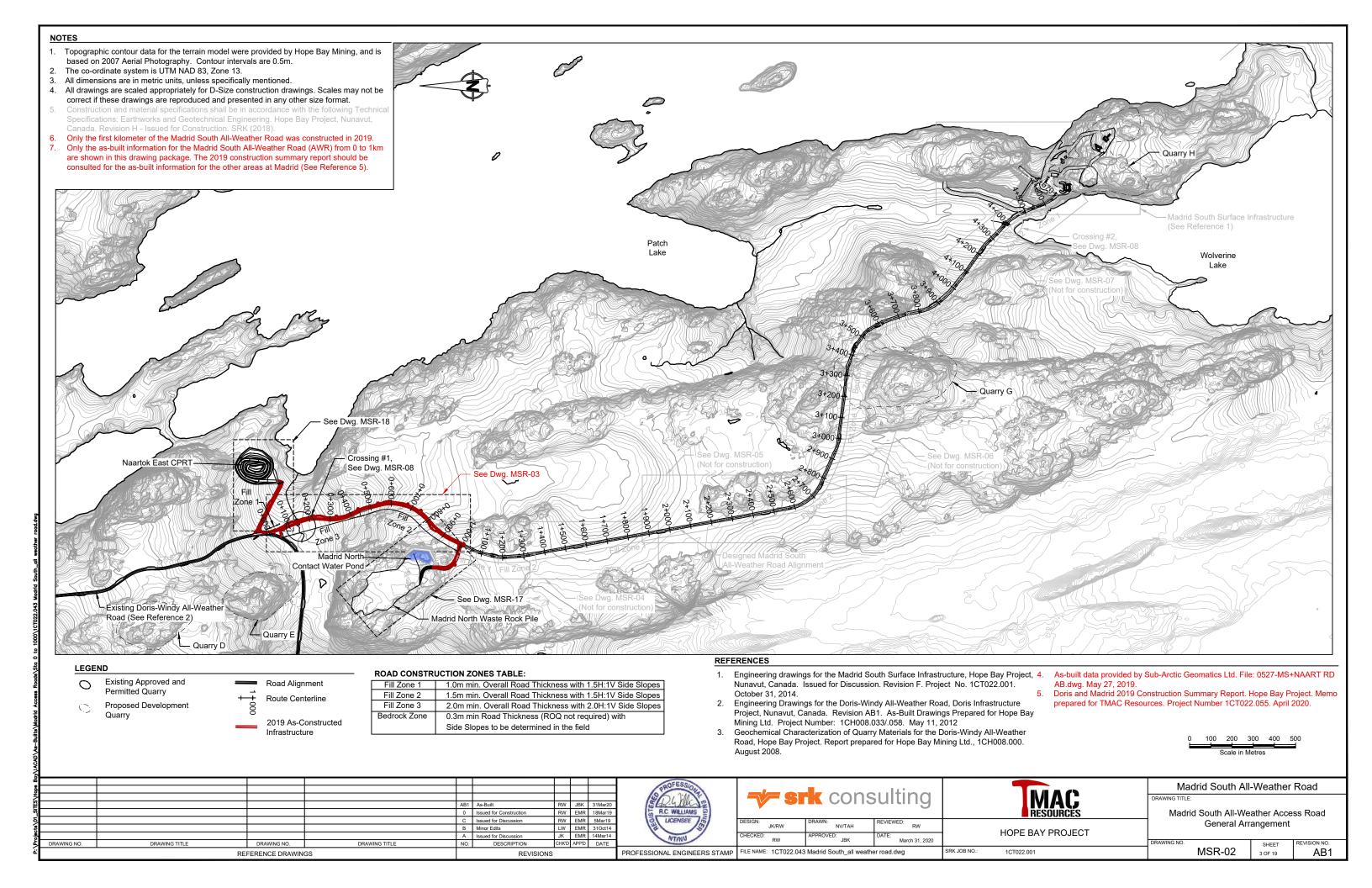


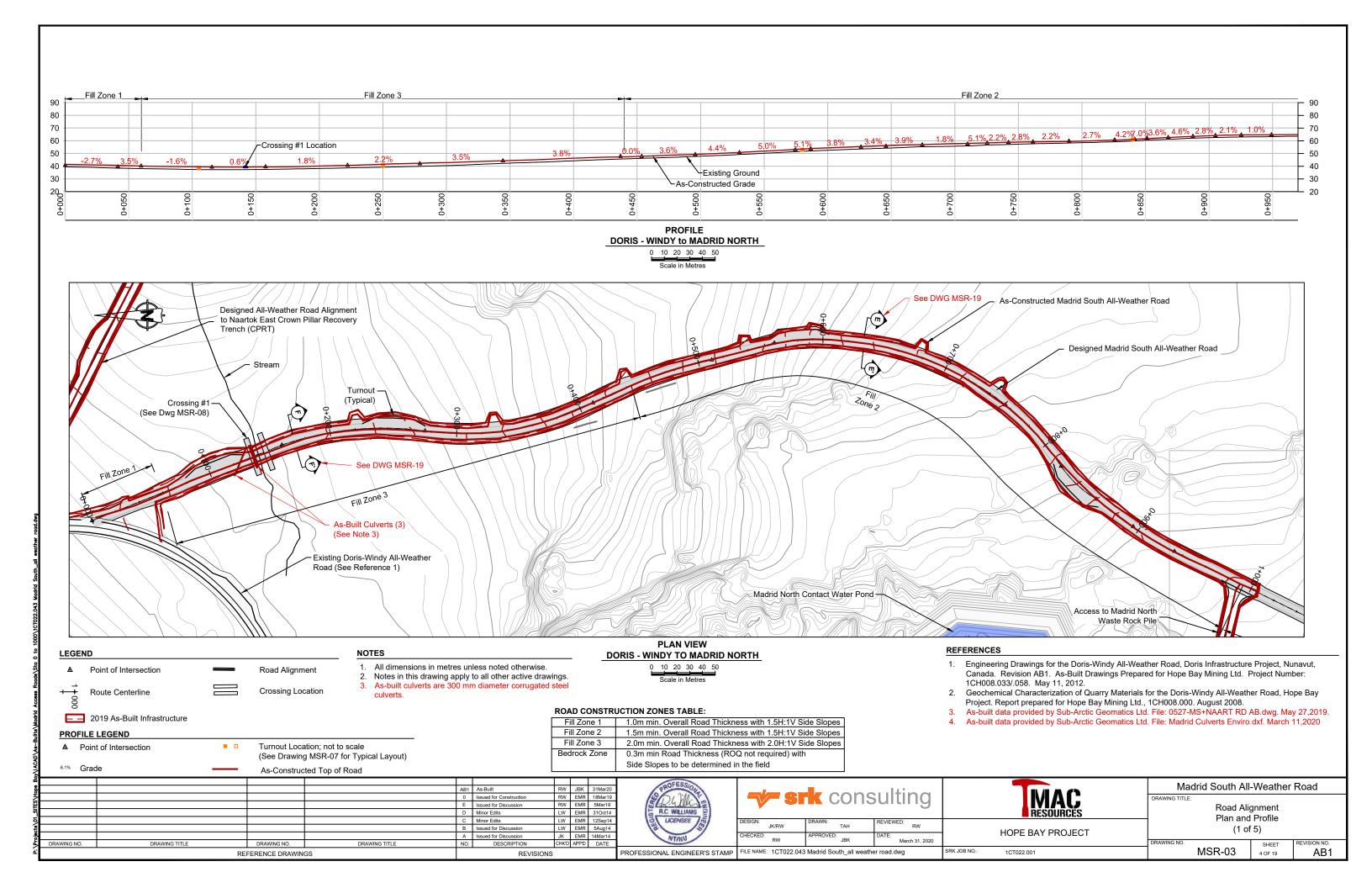


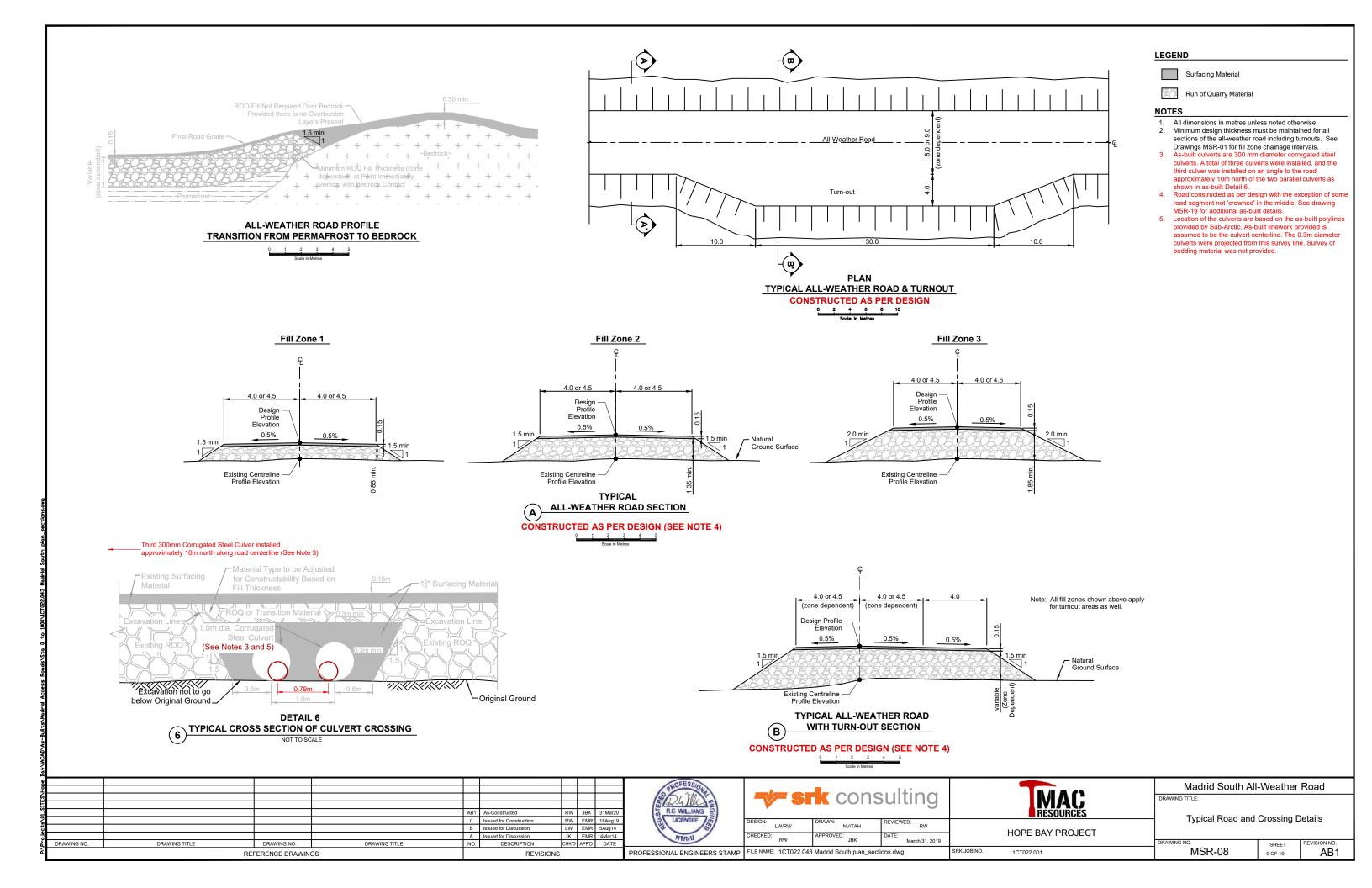


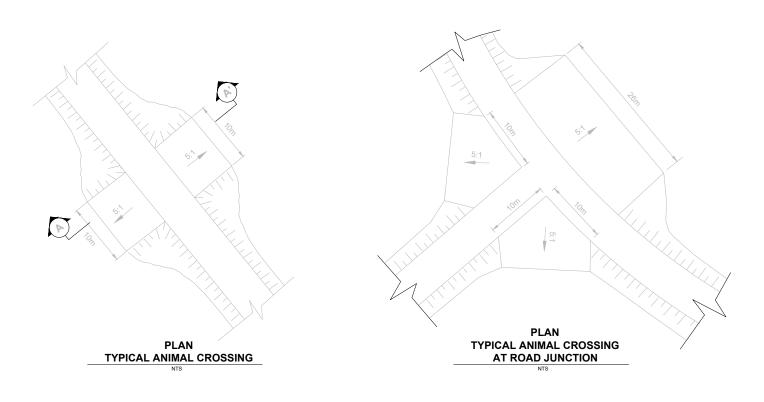
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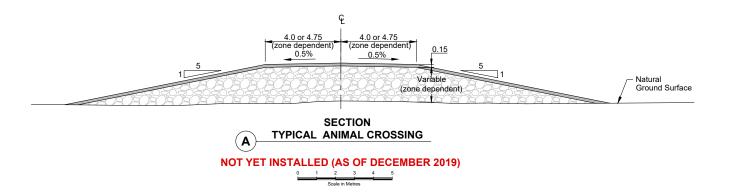


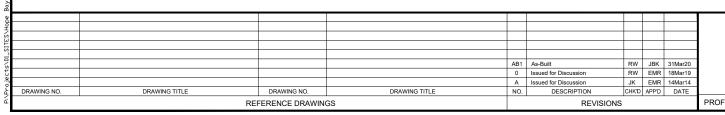














* srk consulting						
DESIGN:	DRAWN:	REVIEWED:				
LW/RW	NV	RW				
CHECKED:	APPROVED:	DATE:				
RW	JBK					

MAC

1CT022.001

SRK JOB NO.:

HOPE BAY PROJECT

Madrid South All-Weather Road

DRAWING TITLE:

LEGEND

NOTES

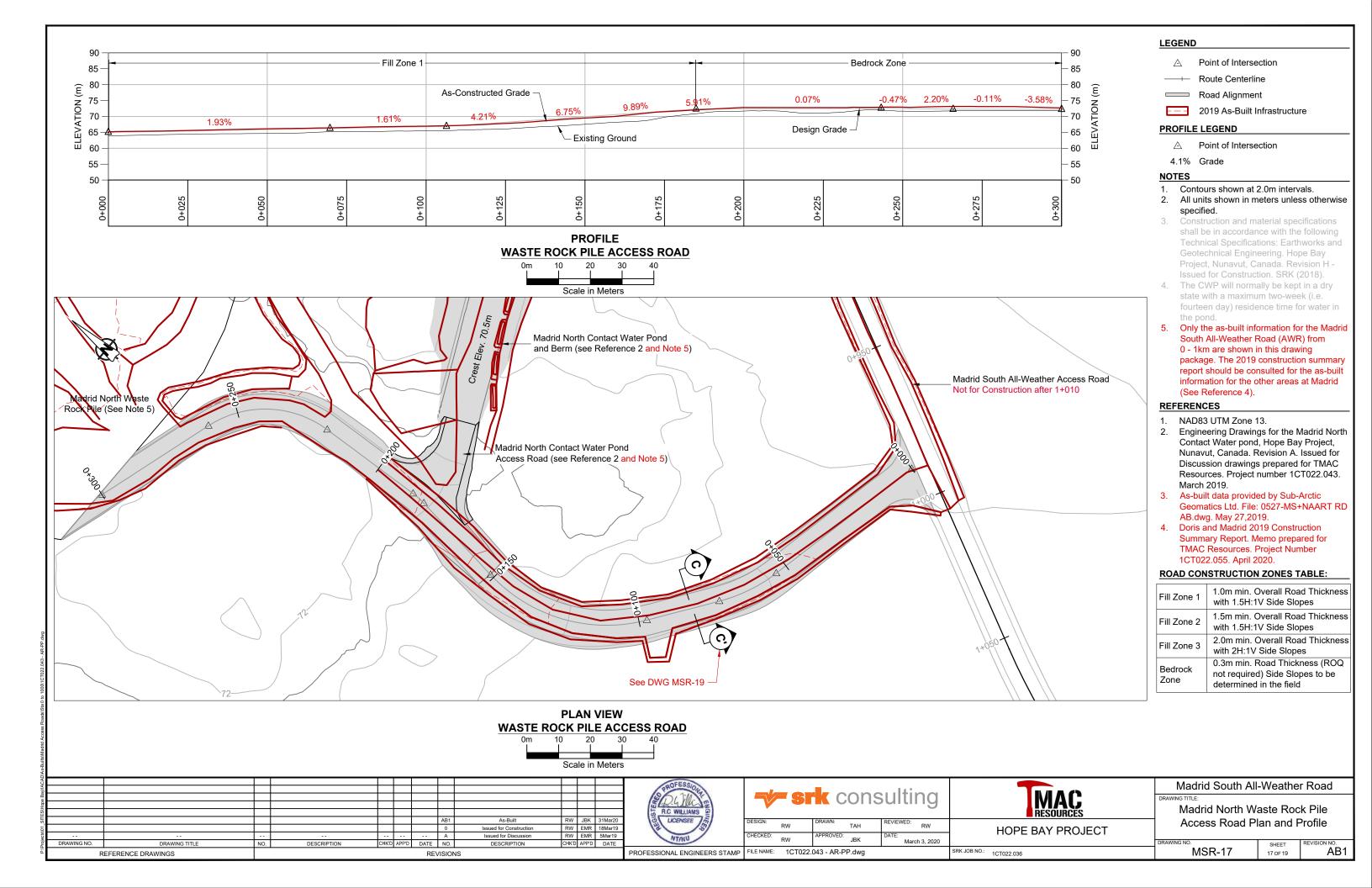
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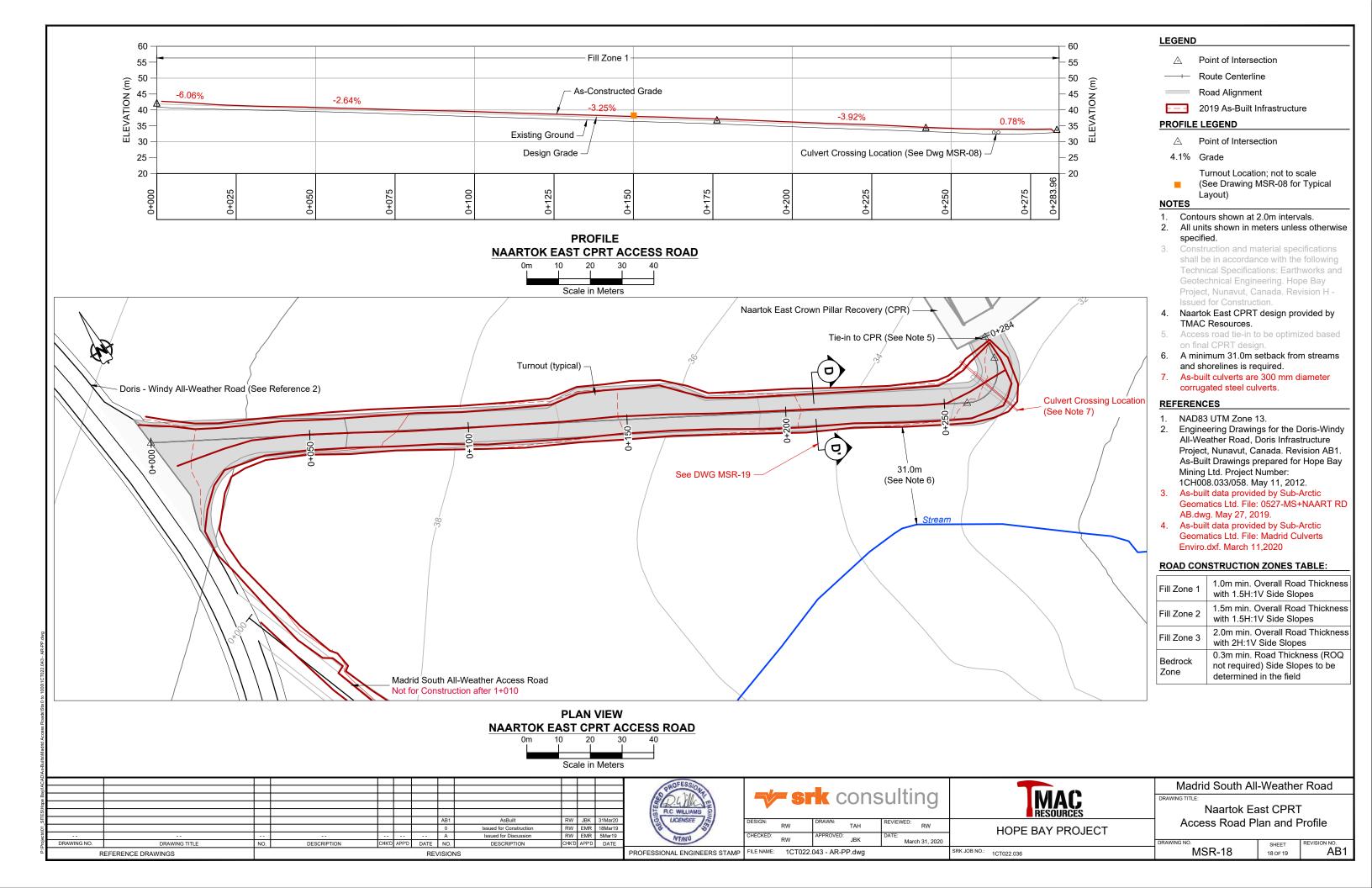
All dimensions in metres unless noted otherwise.
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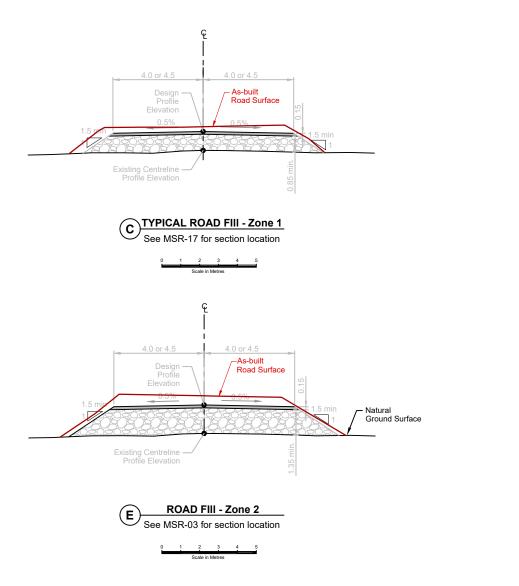
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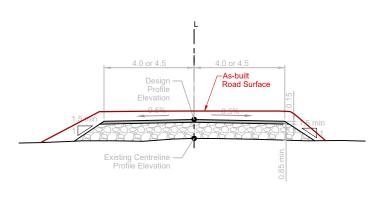
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SHEET AB1

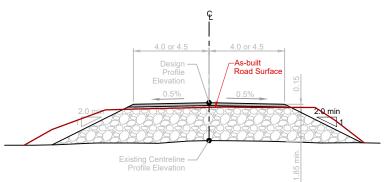








TYPICAL ROAD FIII - Zone 1
See MSR-18 for section location



FTYPICAL ROAD FIII - Zone 3

See MSR-03 for section location

LEGEND

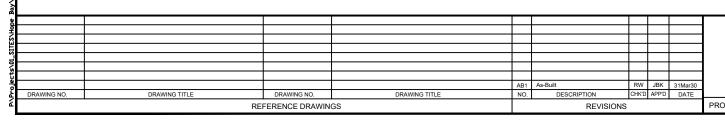
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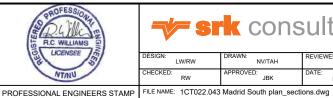
Run of Quarry Material

_____ 2019 As-Built Infrastructure

NOTES

- All dimensions in metres unless noted otherwise.
- Minimum design thickness must be maintained for all sections of the all-weather road including turnouts. See Drawings MSR-01 for fill zone chainage intervals.





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DESIGN: LW/RW	DRAWN: NV/TAH	REVIEWED: RW				
CHECKED: RW	APPROVED: JBK	DATE: March 31, 2020				



1CT022.001

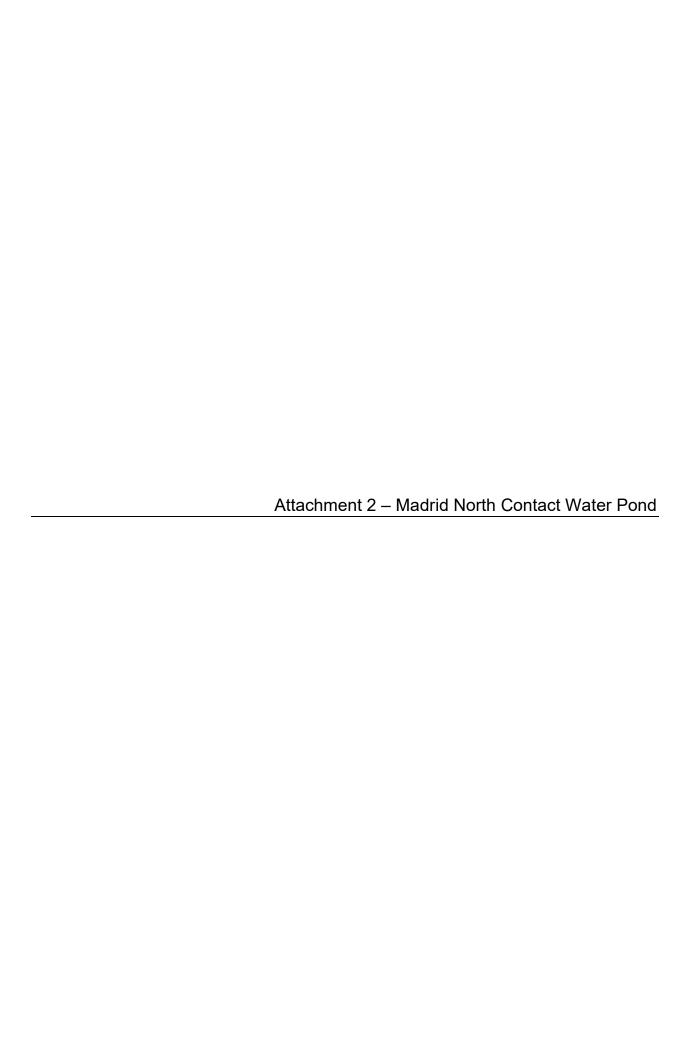
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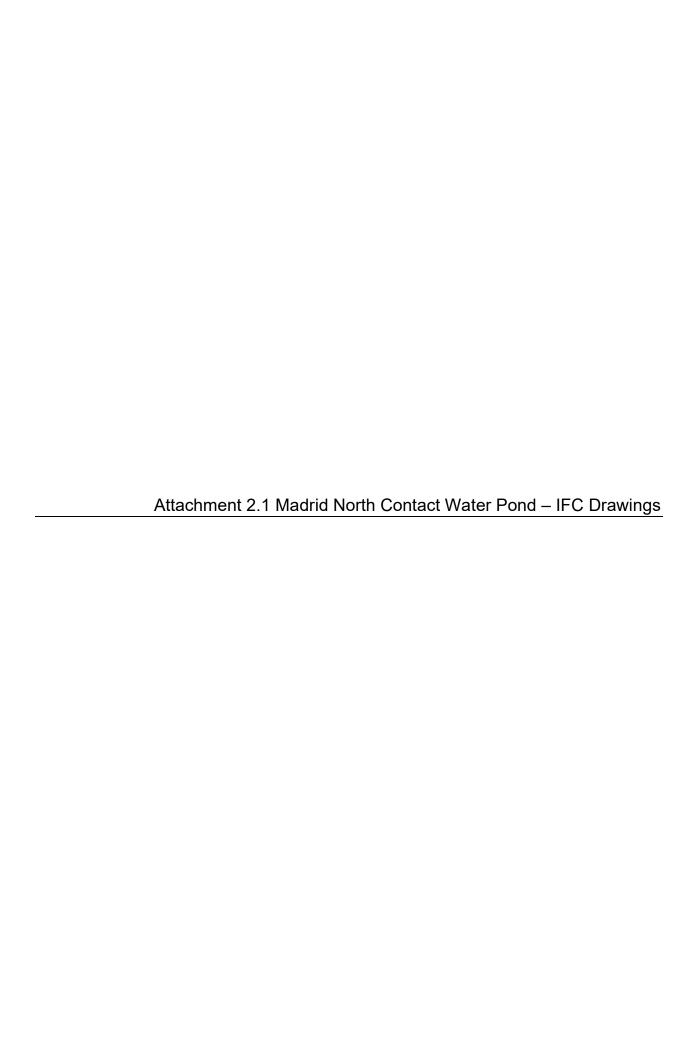
Madrid South All-Weather Road DRAWING TITLE:

Typical Road Fill Comparison

MSR-19

SHEET AB1





Engineering Drawings for the Madrid North Contact Water Pond Hope Bay Project, Nunavut, Canada

Active Drawing Status

Drawing Number	Drawing Title	Issue	Date	Revision
MN-CWP-01	General Arrangement (With Orthophoto)	Issued for Construction	March 19, 2019	0
MN-CWP-02	General Arrangement	Issued for Construction	March 19, 2019	0
MN-CWP-03	Contact Water Pond Anticipated Foundation Conditions Plan and Profile	Issued for Construction	March 19, 2019	0
MN-CWP-04	Contact Water Pond Plan and Profile	Issued for Construction	March 19, 2019	0
MN-CWP-05	Contact Water Pond Typical Sections	Issued for Construction	March 19, 2019	0
MN-CWP-06	Contact Water Pond Typical Details	Issued for Construction	March 19, 2019	0
MN-CWP-07	Liner Tie-in Typical Details	Issued for Construction	March 19, 2019	0

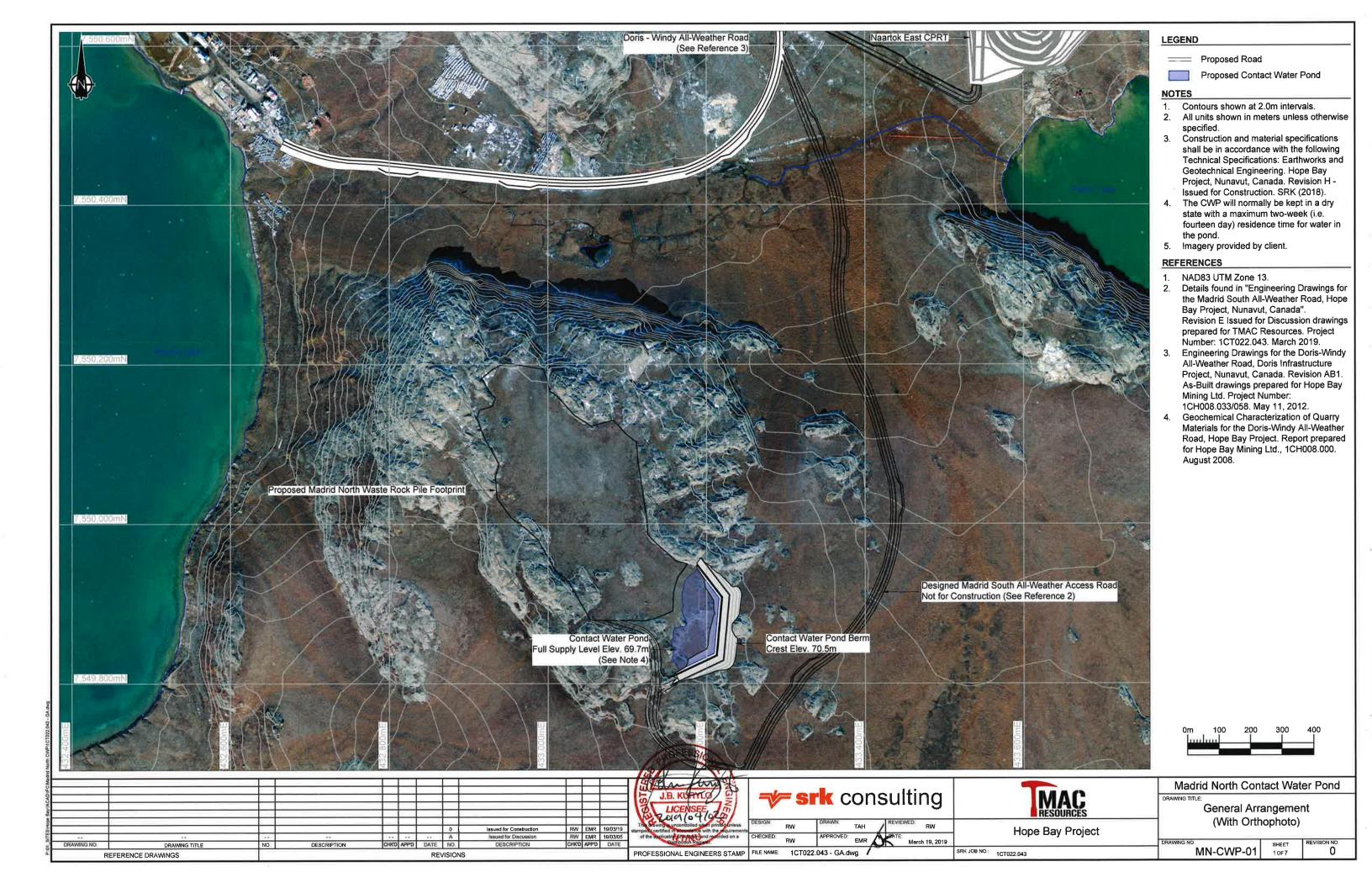


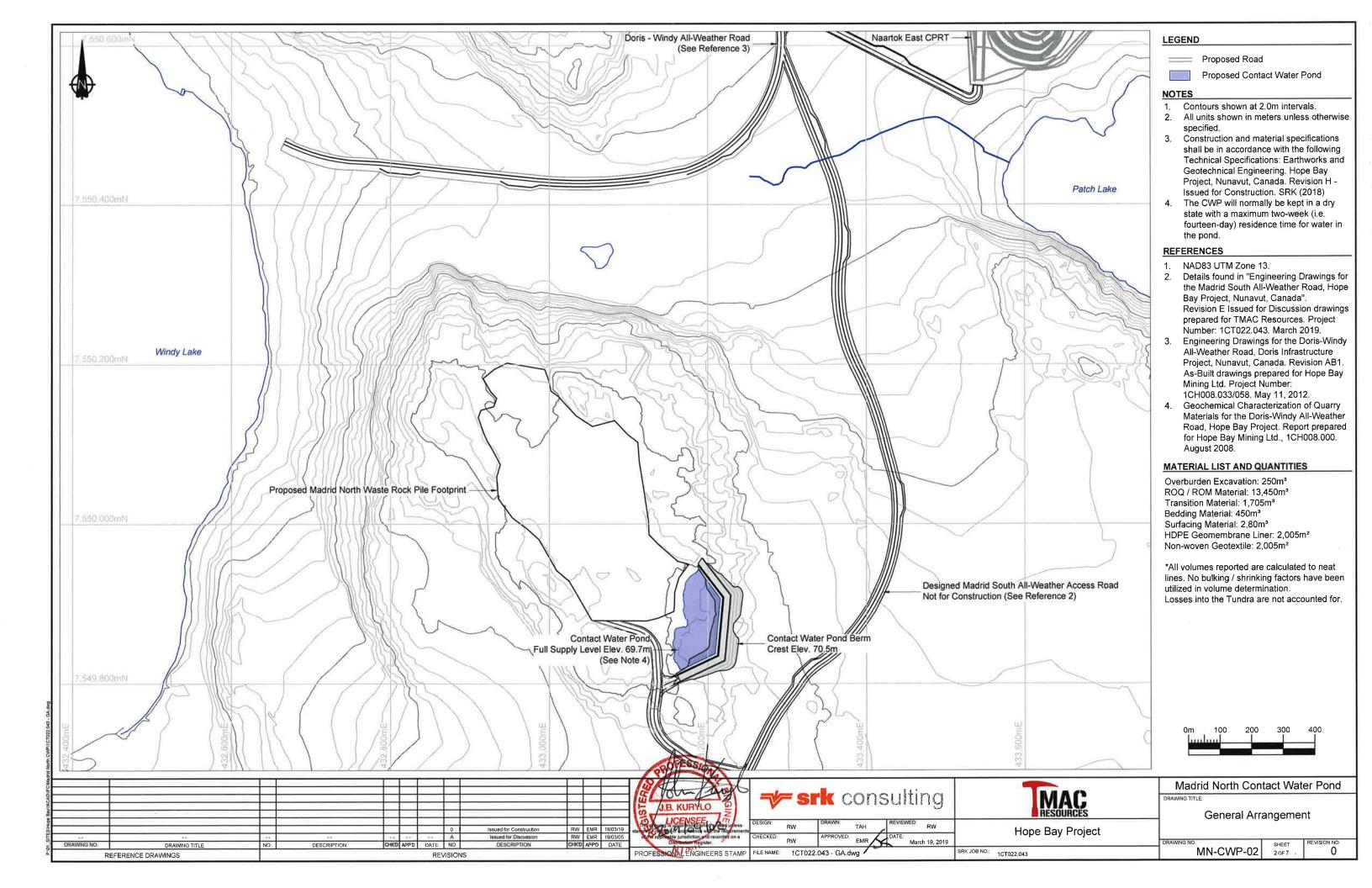
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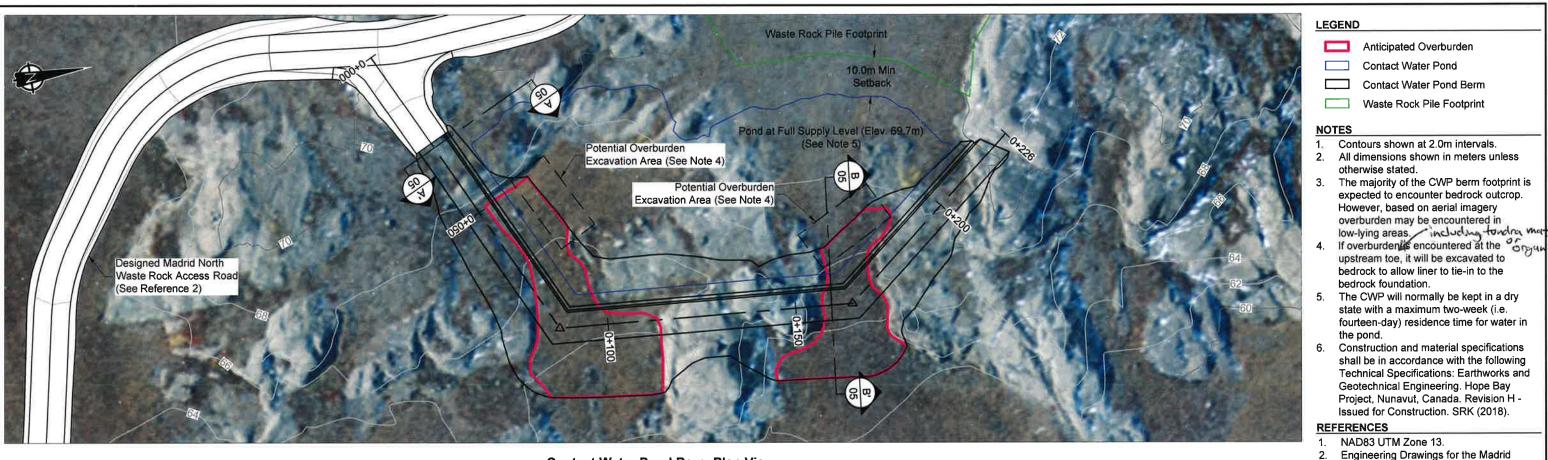
2019/04/02

STK CONSUL

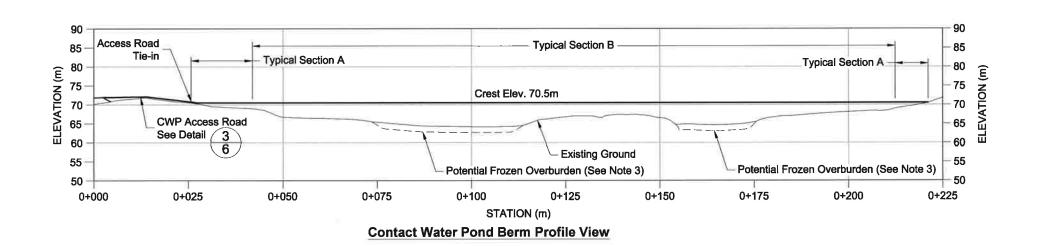
Project Number: 1CT022.043

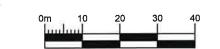






Contact Water Pond Berm Plan View





Contact Water Pond Berm Waste Rock Pile Footprint

South All-Weather Road, Hope ay Project, Nunavut, Canada. Issued for Discussion. Revision A. Project No. 1CT022.043.

Geochemical Characterization of Quarry Materials for the Doris-Windy All-Weather Road, Hope Bay Project. Report prepared for Hope Bay Mining Ltd., 1CH008.000.

March 2019.

August 2008.

DESCRIPTION PROFESSIONAL ENGINEERS STAMP | FILE NAME: 1CT022.043 - OB.dwg REVISIONS

DRAWING TITLE

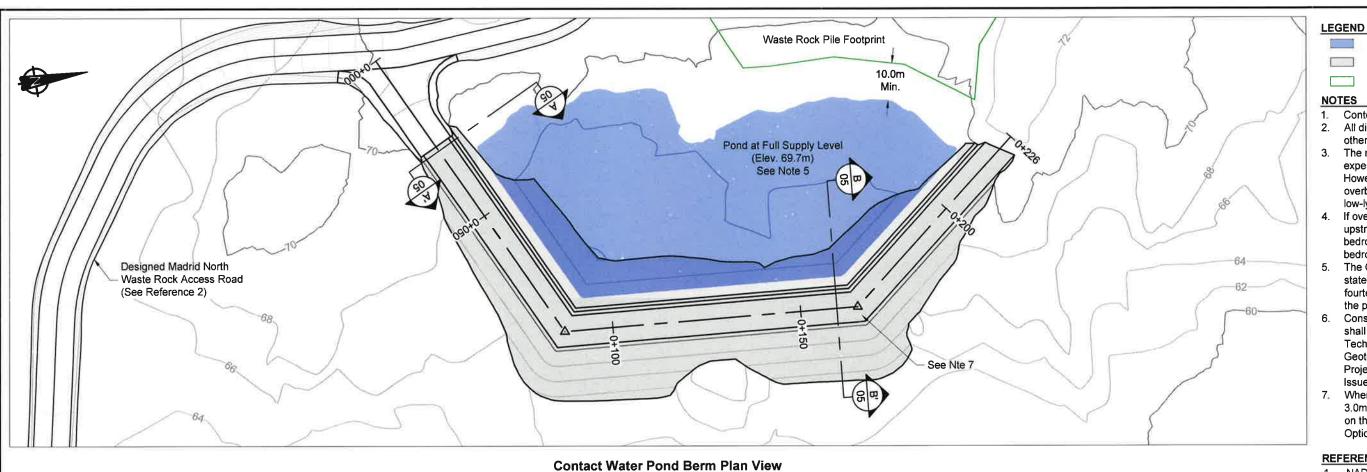
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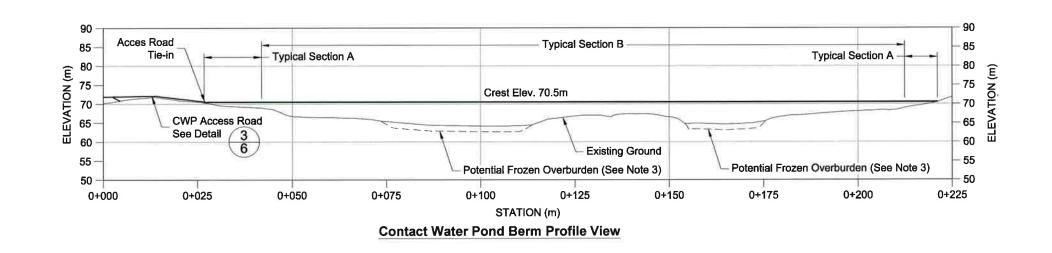
srk consulting

Hope Bay Project SRK JOB NO : 1CT022.043

Madrid North Contact Water Pond DRAWING TITLE: Contact Water Pond **Anticipated Foundation Conditions** Plan and Profile

MN-CWP-03





Contact Water Pond

Contact Water Pond Berm

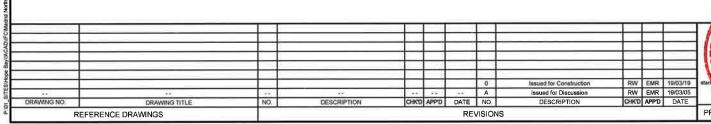
Waste Rock Pile Footprint

- Contours shown at 2.0m intervals.
- All dimensions shown in meters unless otherwise stated.
- The majority of the CWP berm footprint is expected to encounter bedrock outcrop. However, based on aerial imagery overburden may be encountered in low-lying areas.
- If overburden is encountered at the upstream toe, it will be excavated to bedrock to allow liner to tie-in to the bedrock foundation.
- 5. The CWP will normally be kept in a dry state with a maximum two-week (i.e. fourteen-day) residence time for water in the pond.
- Construction and material specifications shall be in accordance with the following Technical Specifications: Earthworks and Geotechnical Engineering. Hope Bay Project, Nunavut, Canada, Revision H -Issued for Construction, SRK (2018).
- Where the height of the berm exceeds 3.0m, safety berms or barries are required on the crest. See Typical Berm Barrier Options on drawing MN-CWP-06.

REFERENCES

- NAD83 UTM Zone 13.
- Engineering Drawings for the Madrid South All-Weather Road, Hope ay Project, Nunavut, Canada. Issued for Discussion. Revision A. Project No. 1CT022.043 March 2019.
- Geochemical Characterization of Quarry Materials for the Doris-Windy All-Weather Road, Hope Bay Project. Report prepared for Hope Bay Mining Ltd., 1CH008.000. August 2008.







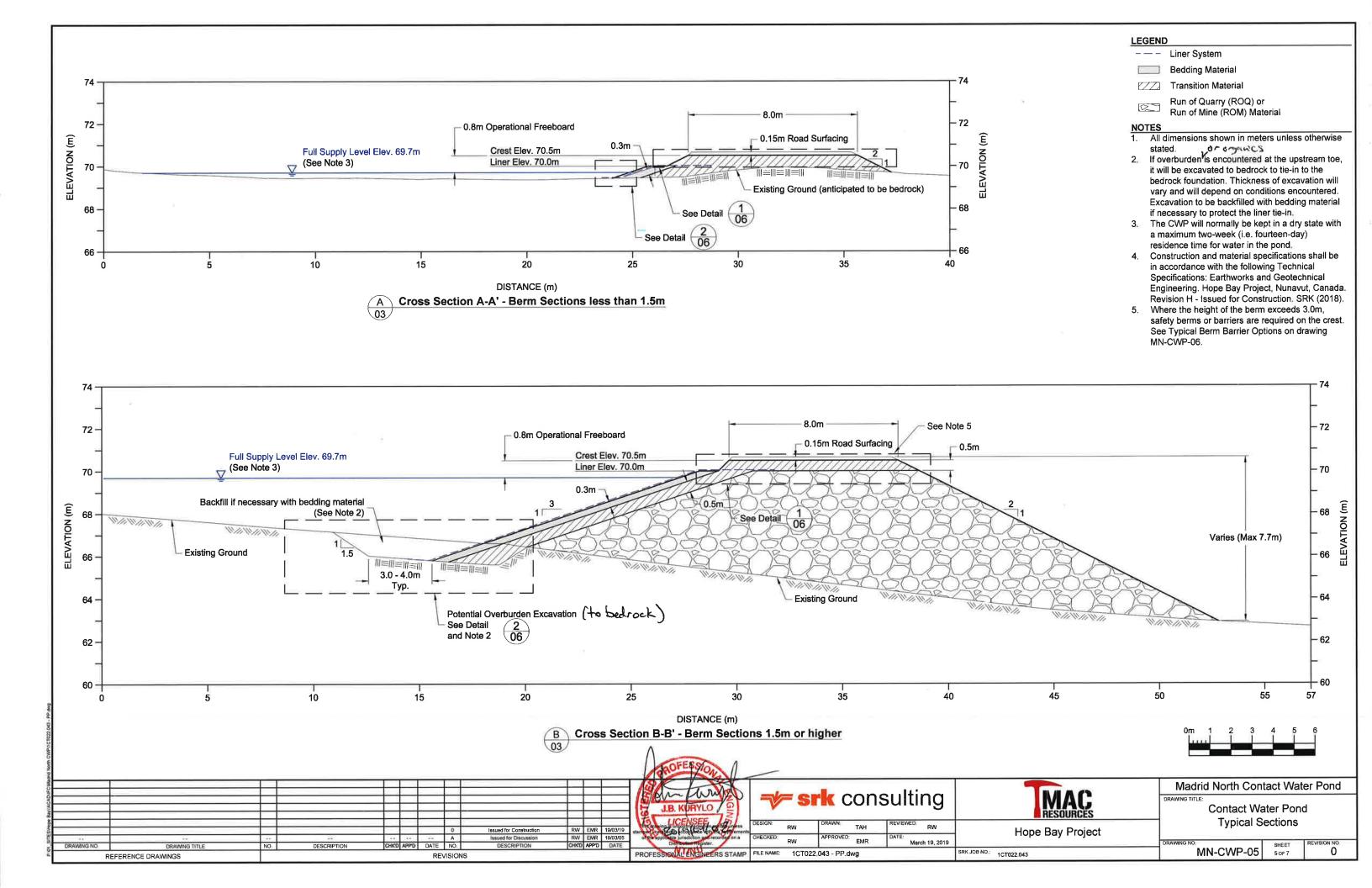


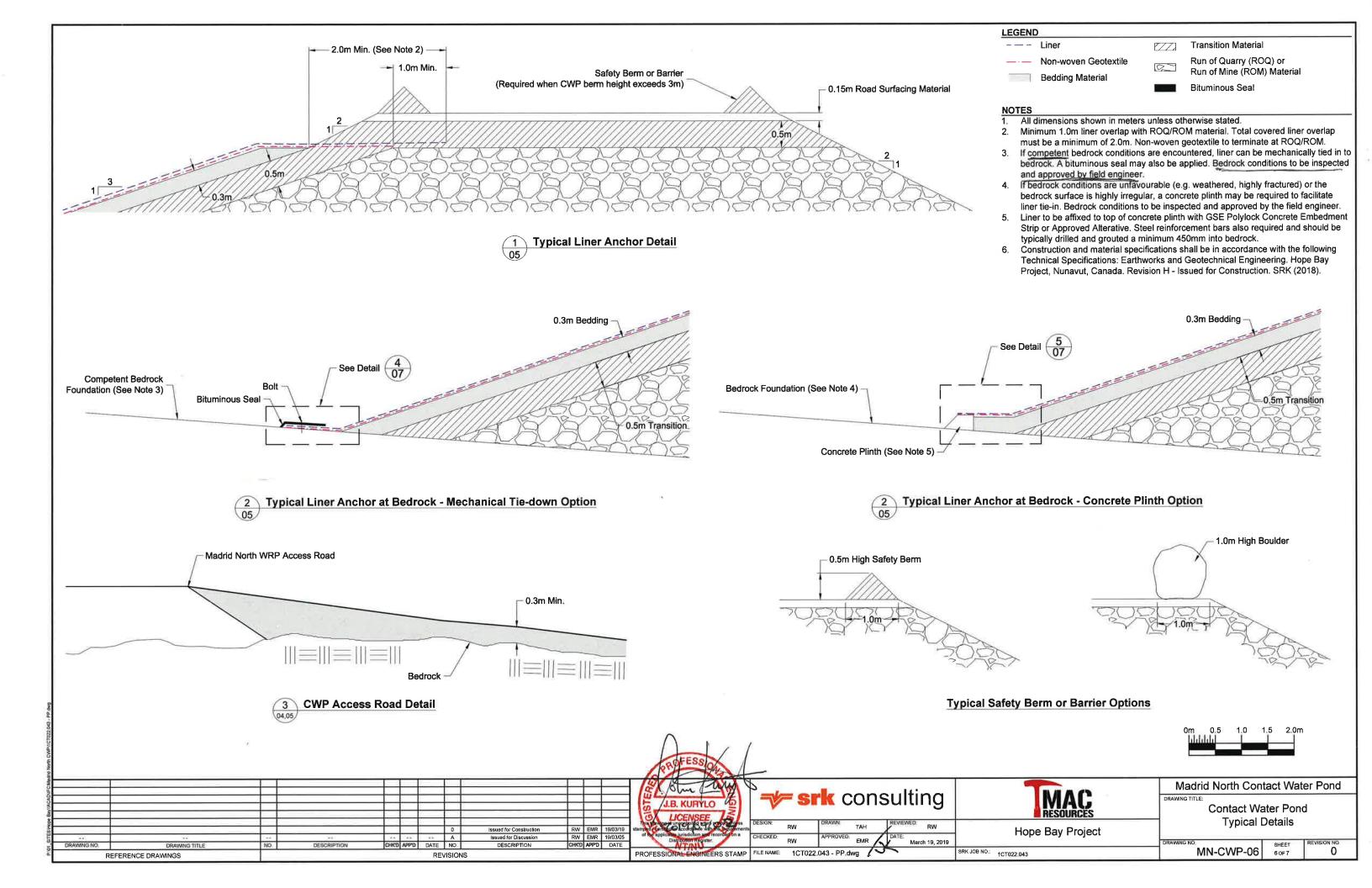


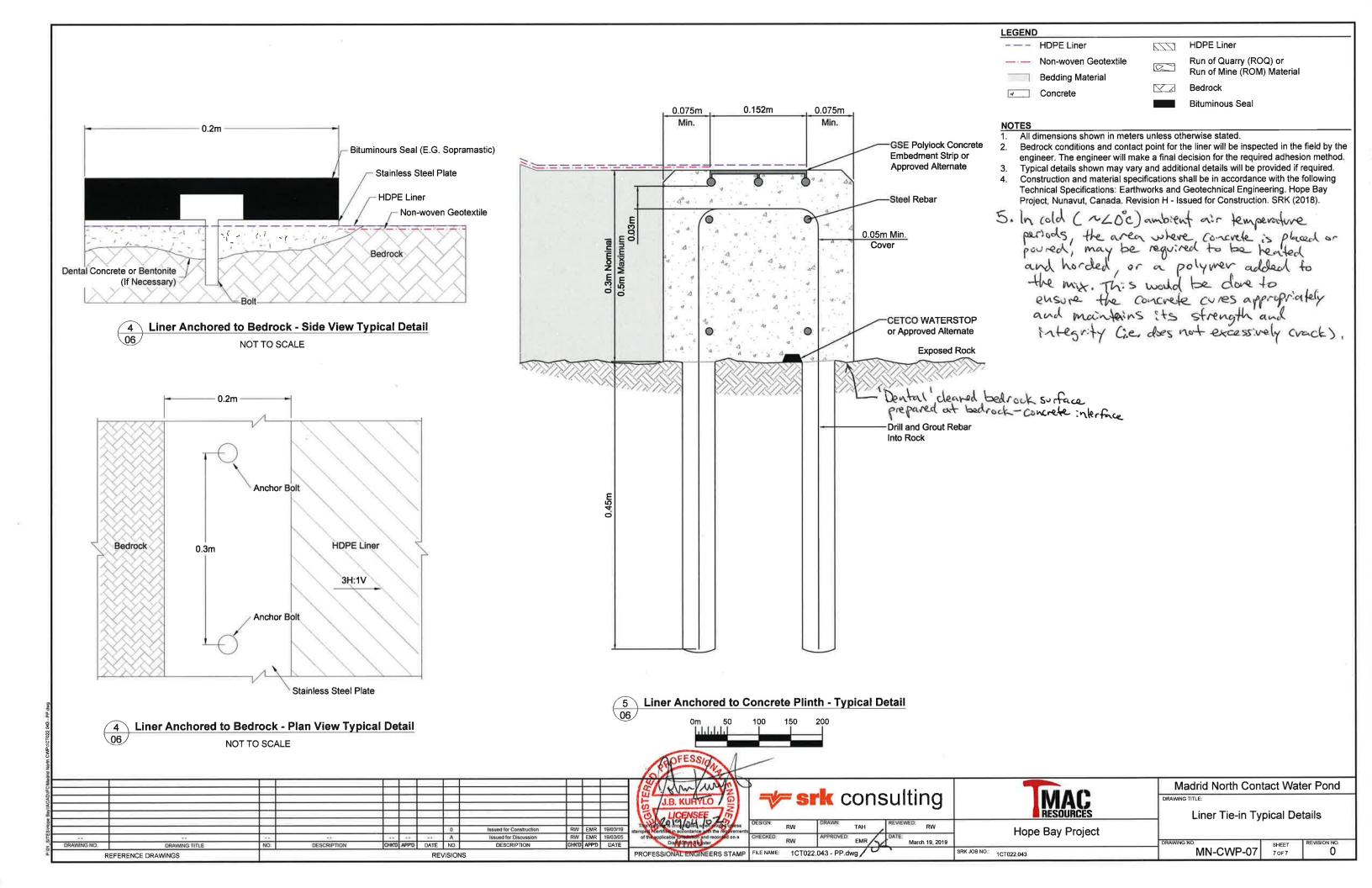
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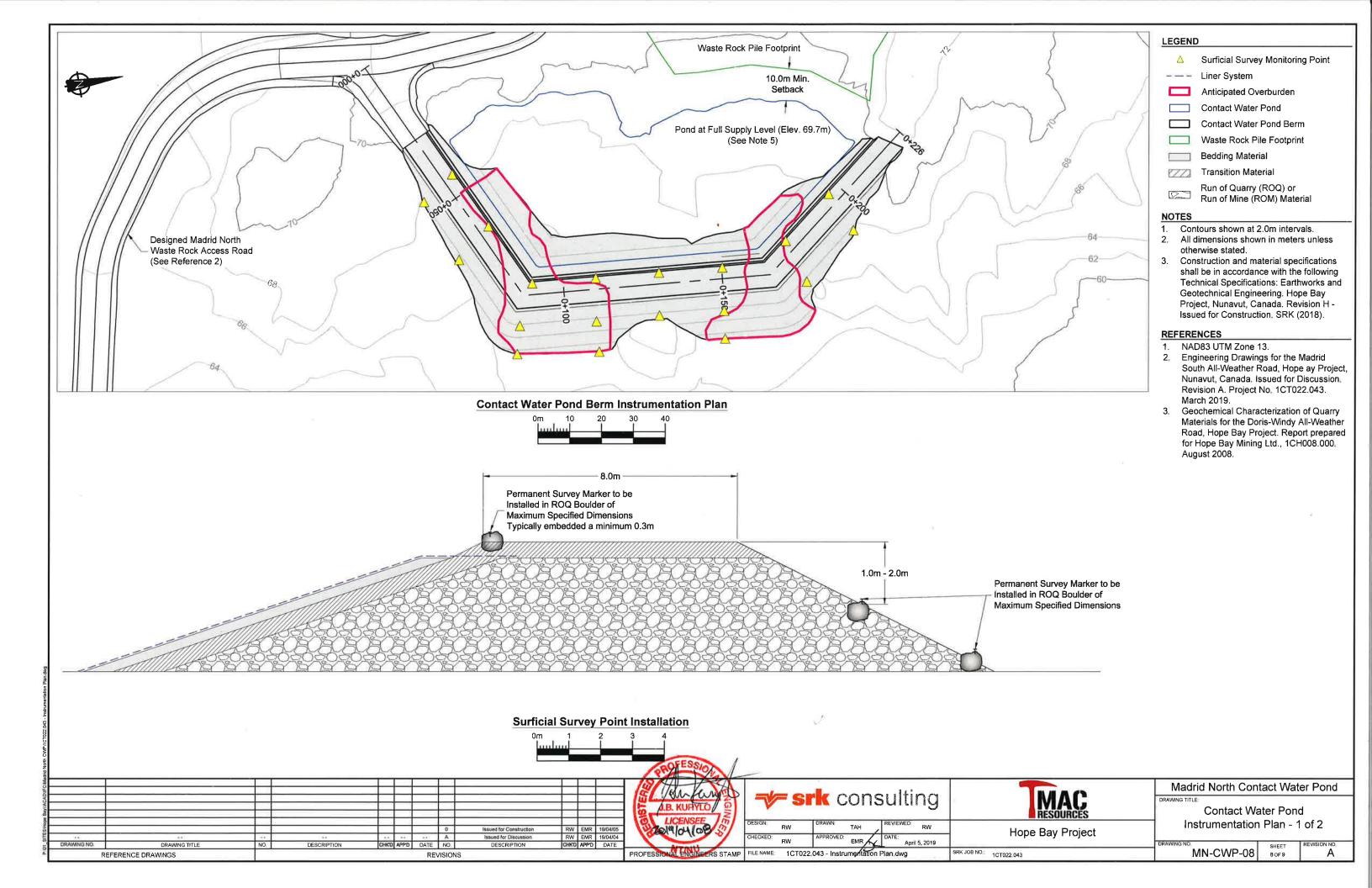
Contact Water Pond Plan and Profile

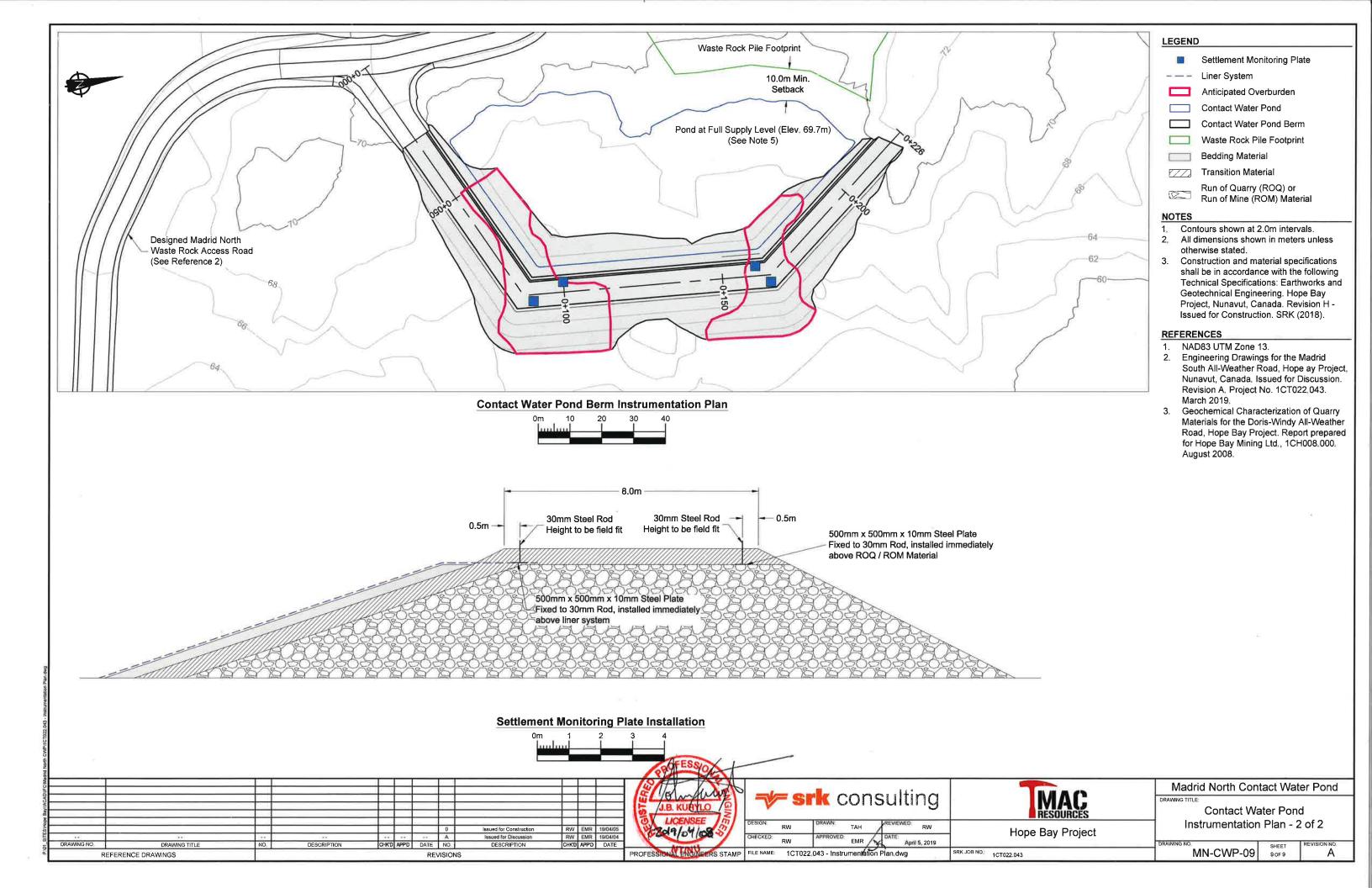
MN-CWP-04	SHEET 4 OF 7	REVISION NO.
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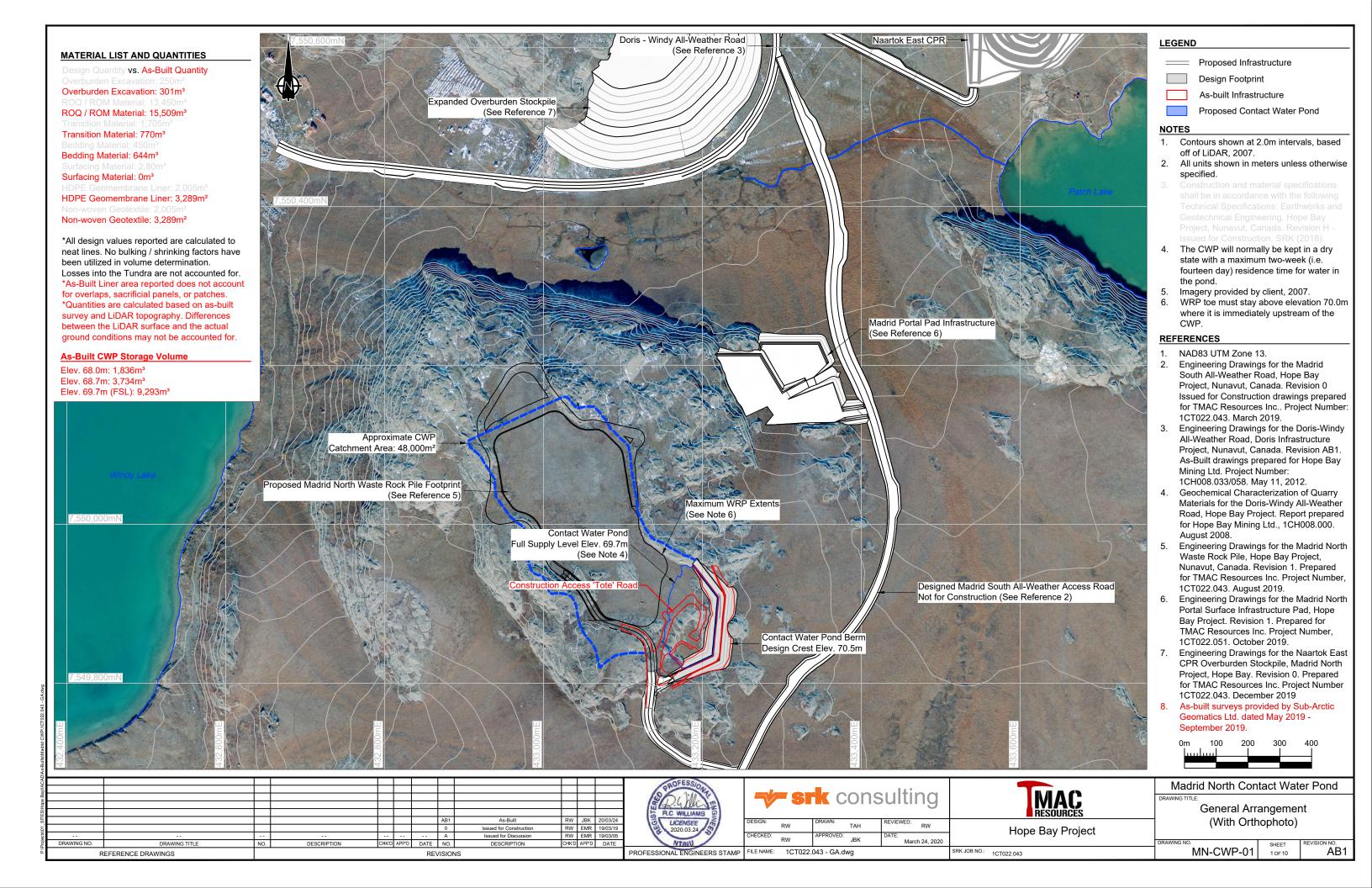
As-Built Drawings for the Madrid North Contact Water Pond Hope Bay Project, Nunavut, Canada

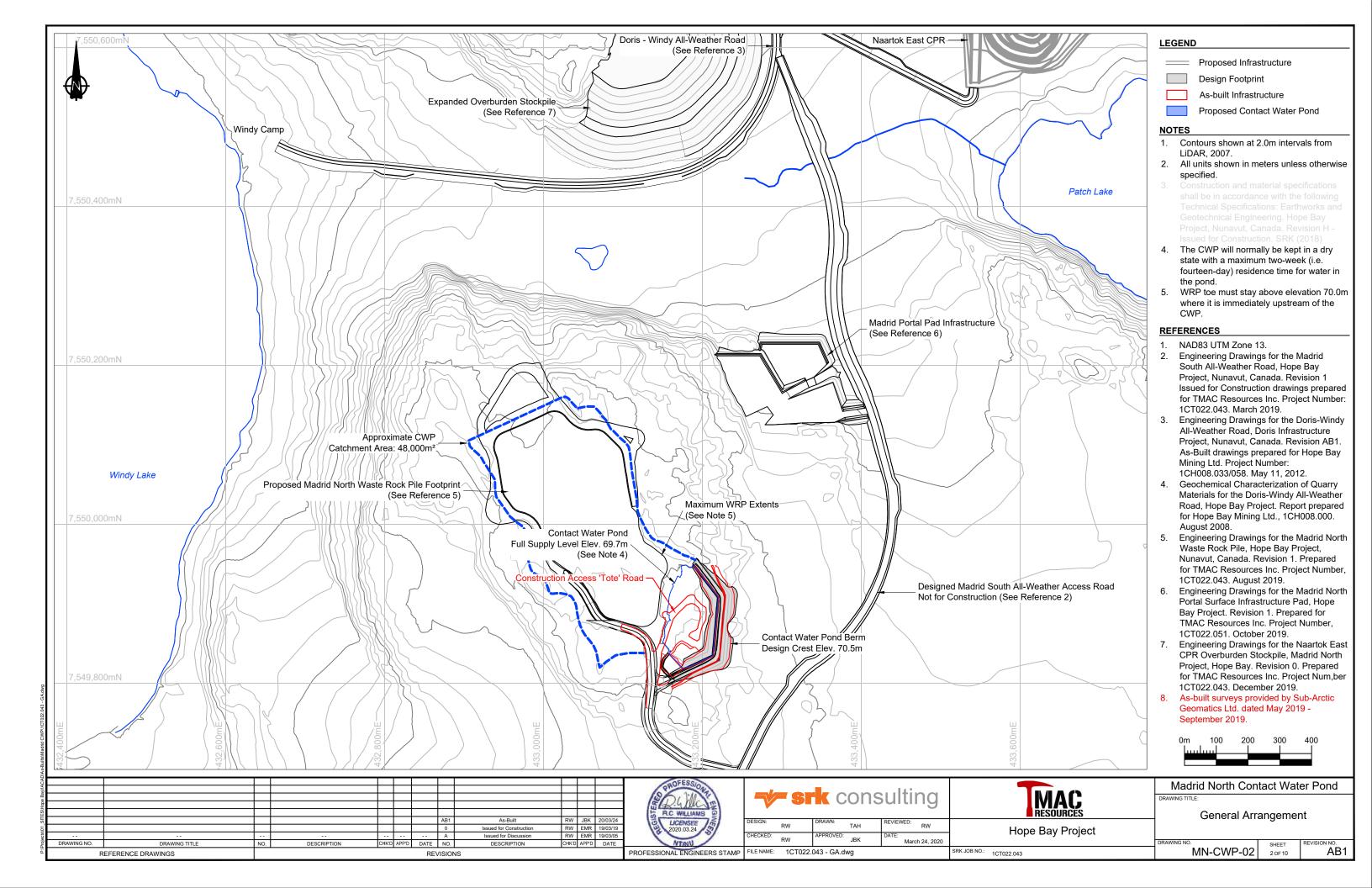
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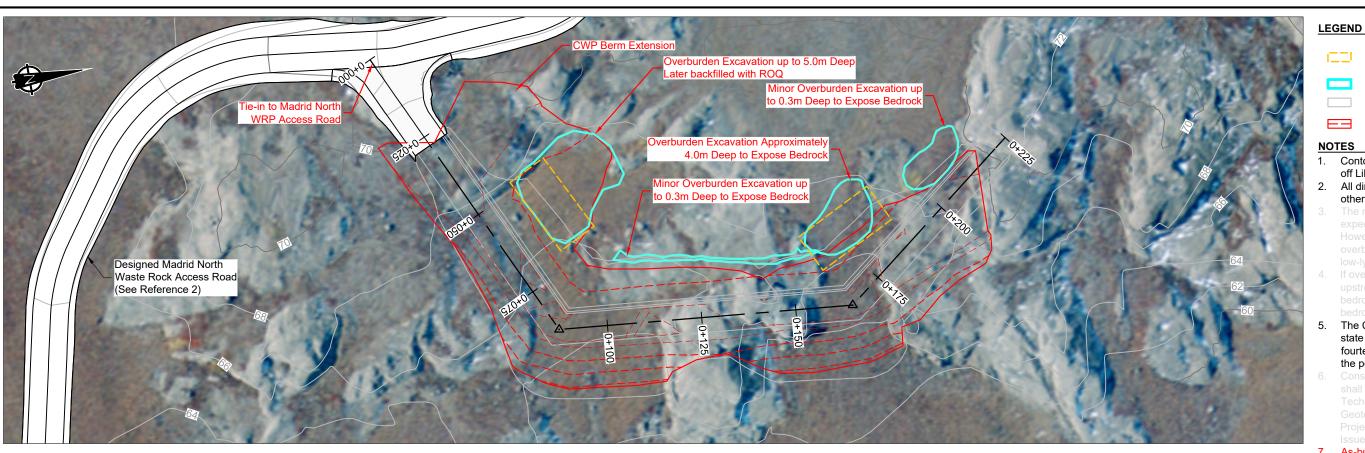
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MN-CWP-02	General Arrangement	As-Built	2020/03/24	AB1
MN-CWP-03	Contact Water Pond Foundation Conditions Plan and Profile	As-Built	2020/03/24	AB1
MN-CWP-04	Contact Water Pond As-Built Berm and Concrete Plinths	As-Built	2020/03/24	AB1
MN-CWP-05	Contact Water Pond Typical Sections	As-Built	2020/03/24	AB1
MN-CWP-06	Contact Water Pond Typical Sections	As-Built	2020/03/24	AB1
MN-CWP-07	Contact Water Pond Typical Details	As-Built	2020/03/24	AB1
MN-CWP-08	Liner Tie-in Typical Details	As-Built	2020/03/24	AB1
MN-CWP-09	Contact Water Pond Instrumentation Plan	As-Built	2020/03/24	AB1
MN-CWP-10	As-Built Liner Panel Layout	As-Built	2020/03/24	AB1



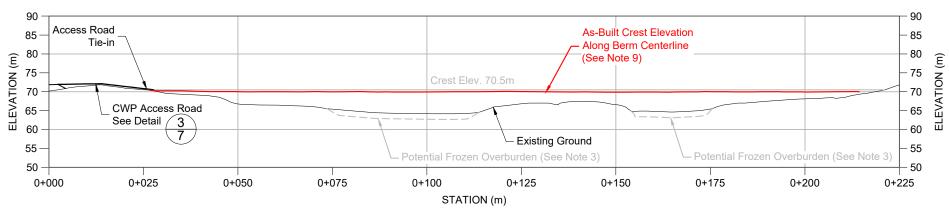








Contact Water Pond Berm Plan View



Contact Water Pond Berm Profile View

Anticipated Overburden Excavation Area (Design)



As-built Overburden Excavation (See Note 7)



Design Contact Water Pond Berm

- As-Built Contact Water Pond Berm
- Contours shown at 1.0m intervals, based off LiDAR, 2007.
- All dimensions shown in meters unless otherwise stated.

- The CWP will normally be kept in a dry state with a maximum two-week (i.e. fourteen-day) residence time for water in the pond.
- As-built overburden excavation extents are approximate, and are based on as-built surveys and site observation.
- Construction access 'tote' road (MN-CWP-01 & 02) is not shown on this drawing.
- As-built crest elevation along berm centerline is approximately 70.0m (i.e. minimum elevation of the liner). As-built crest elevation along the upstream and downstream crest edge is approximately 70.5m. See as-built cross sections on MN-CWP-05 & 06.

REFERENCES

- NAD83 UTM Zone 13.
- Engineering Drawings for the Madrid South All-Weather Road, Hope ay Project, Nunavut, Canada. Issued for Discussion. Revision A. Project No. 1CT022.043. March 2019.
- Geochemical Characterization of Quarry Materials for the Doris-Windy All-Weather Road, Hope Bay Project. Report prepared for Hope Bay Mining Ltd., 1CH008.000. August 2008.
- As-built surveys provided by Sub-Arctic Geomatics Ltd. dated May 2019 -September 2019.



As-Built RW EMR 19/03/19 Issued for Discussion REFERENCE DRAWINGS REVISIONS

R.C. WILLIAM PROFESSIONAL ENGINEERS STAMP

	₹ SI	k cons	sulting		
	DESIGN: RW	DRAWN: TAH	REVIEWED: RW		Нор
	CHECKED: RW	APPROVED: JBK	DATE: March 24, 2020		ПОР
,	FILE NAME: 1CT022.	043 - OB.dwg		SRK JOB NO.:	1CT022.043

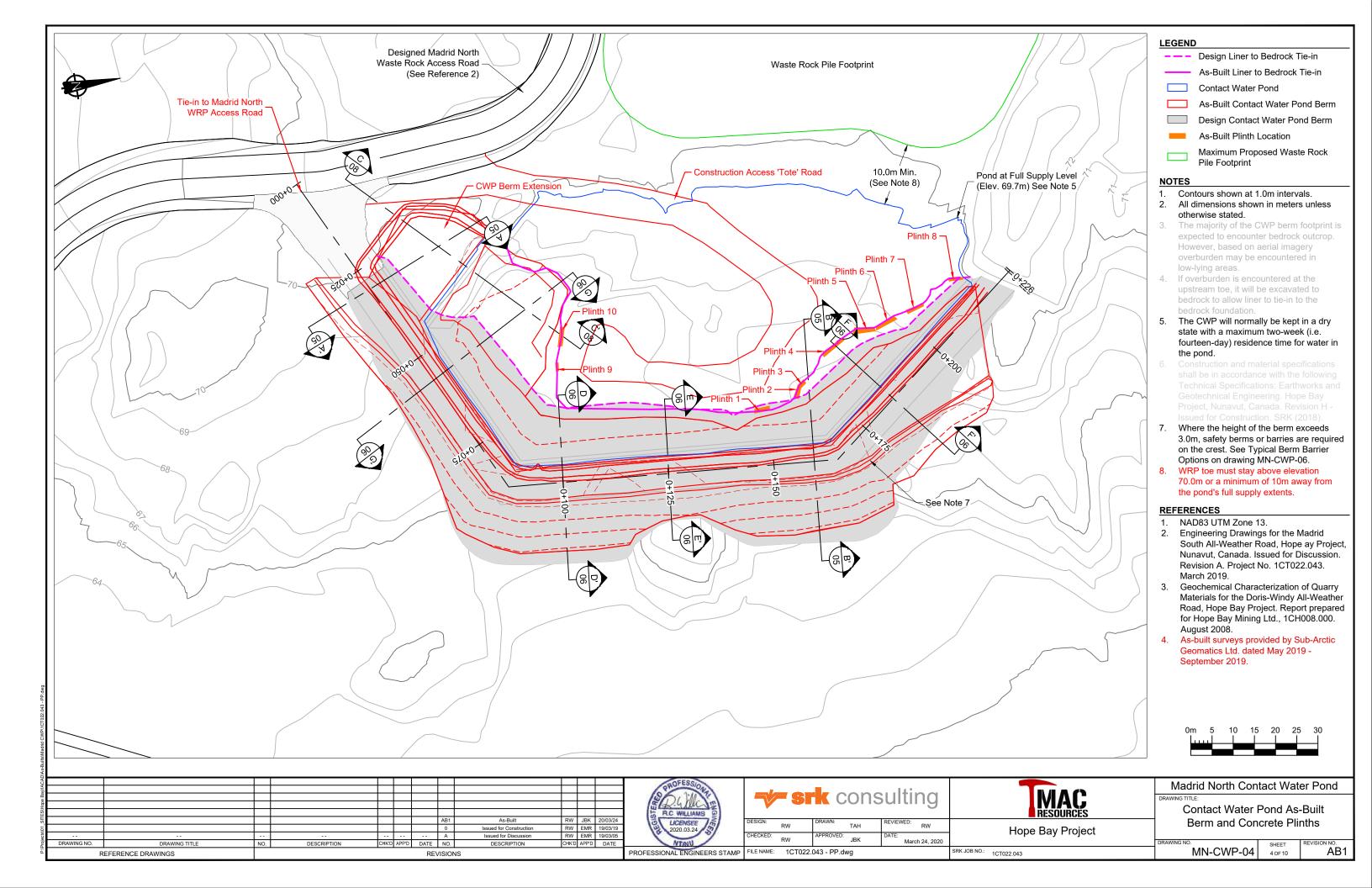
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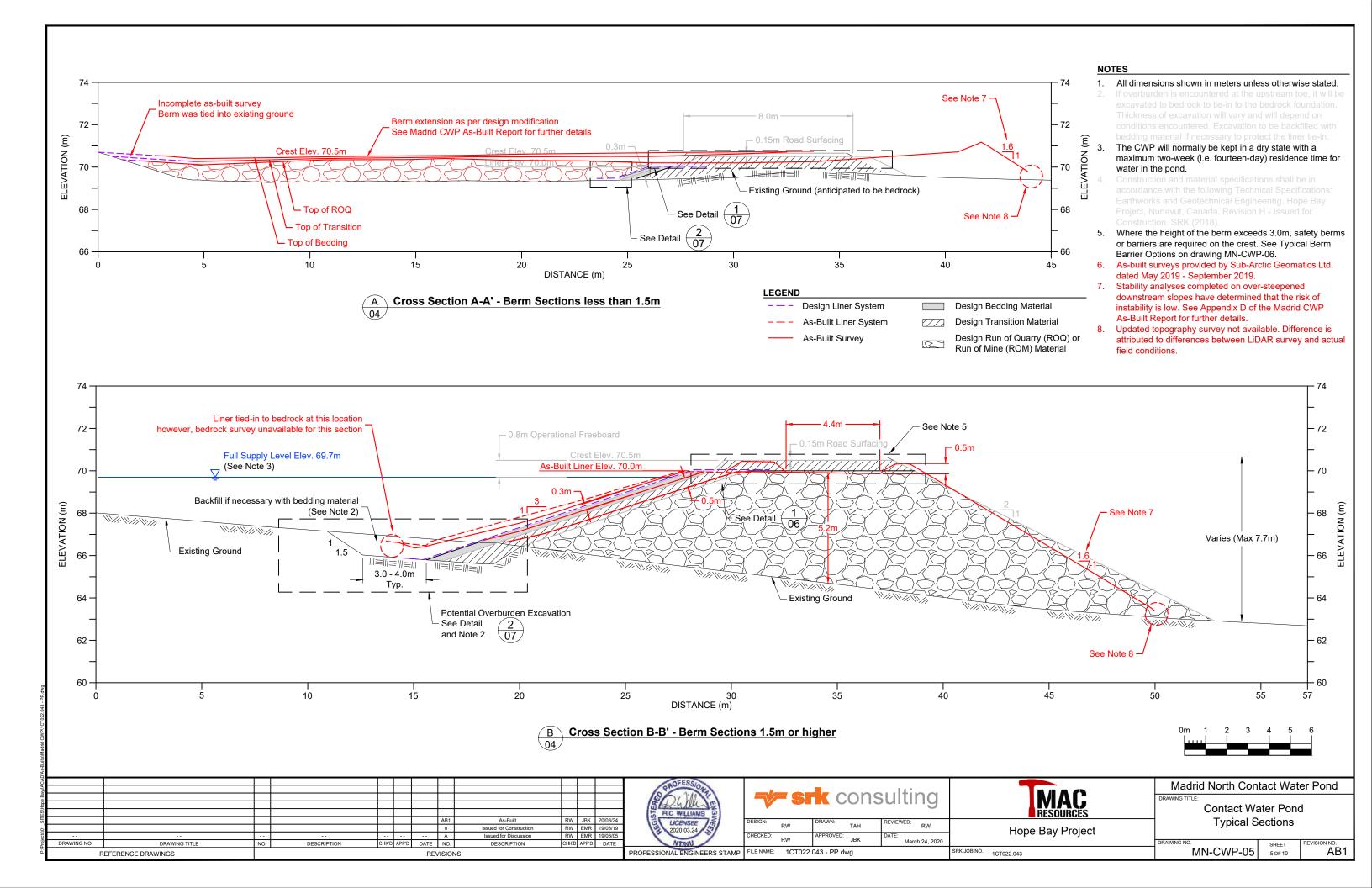
Madrid North Contact Water Pond

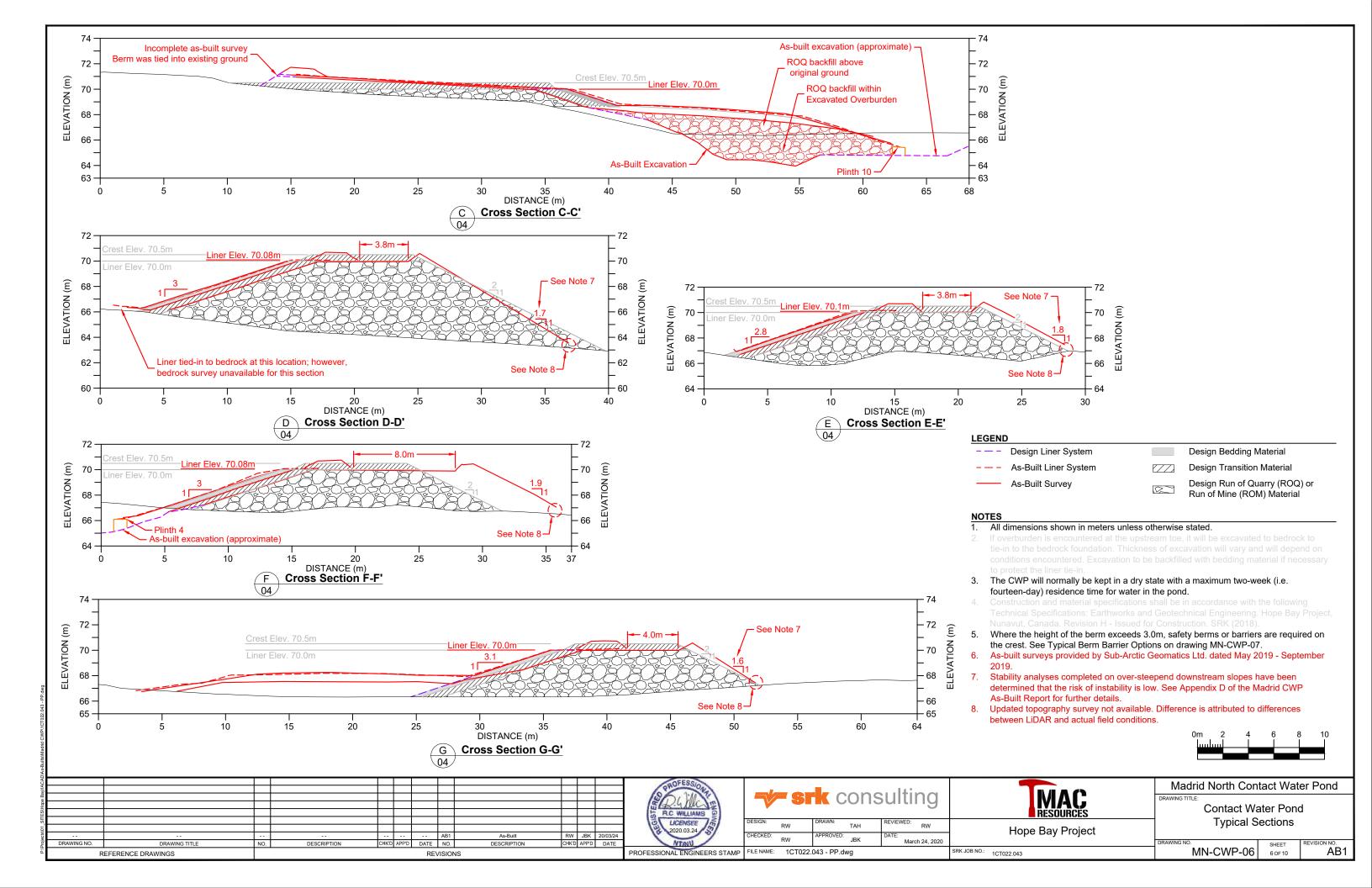
Contact Water Pond Foundation Conditions Plan and Profile

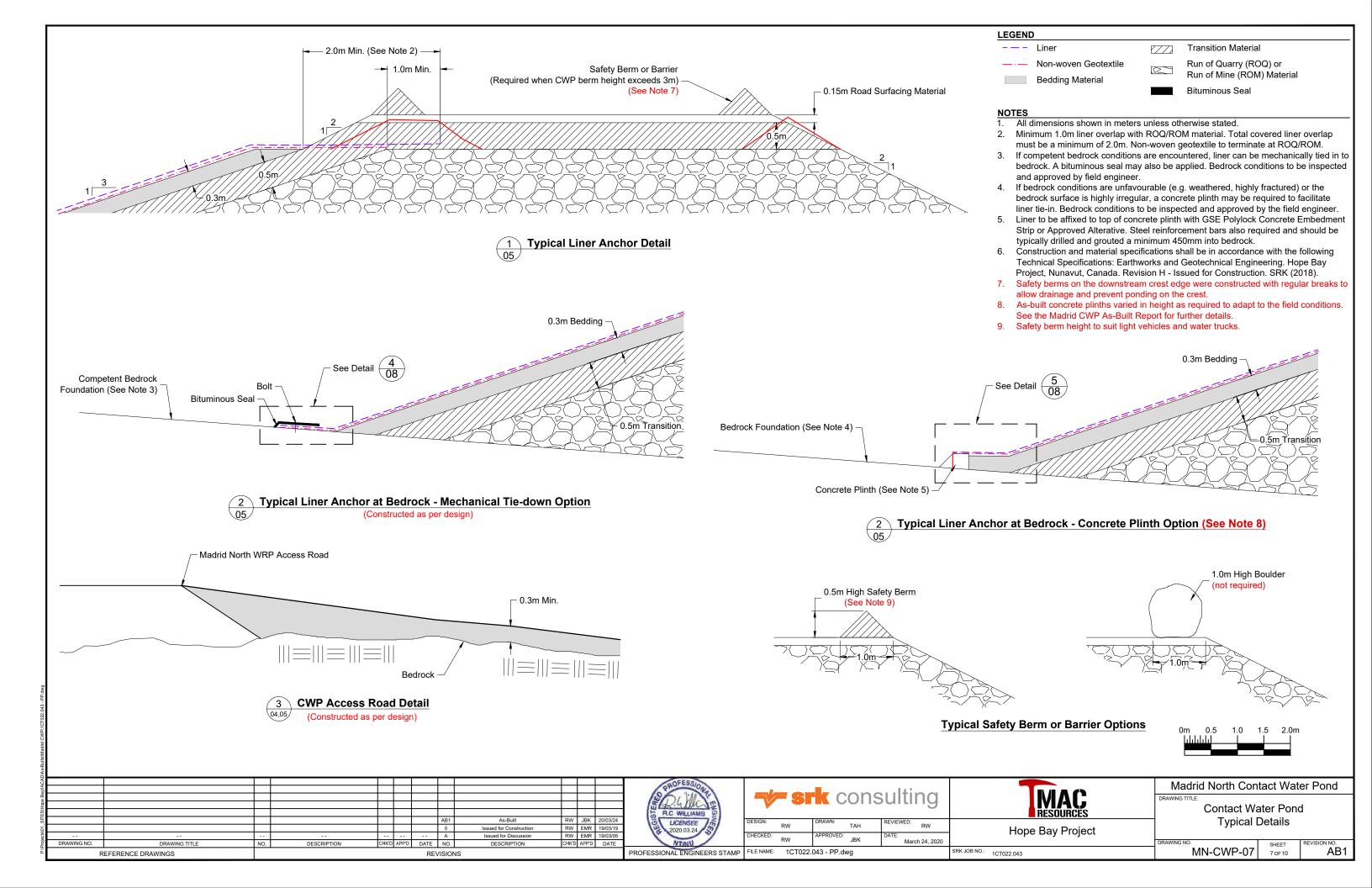
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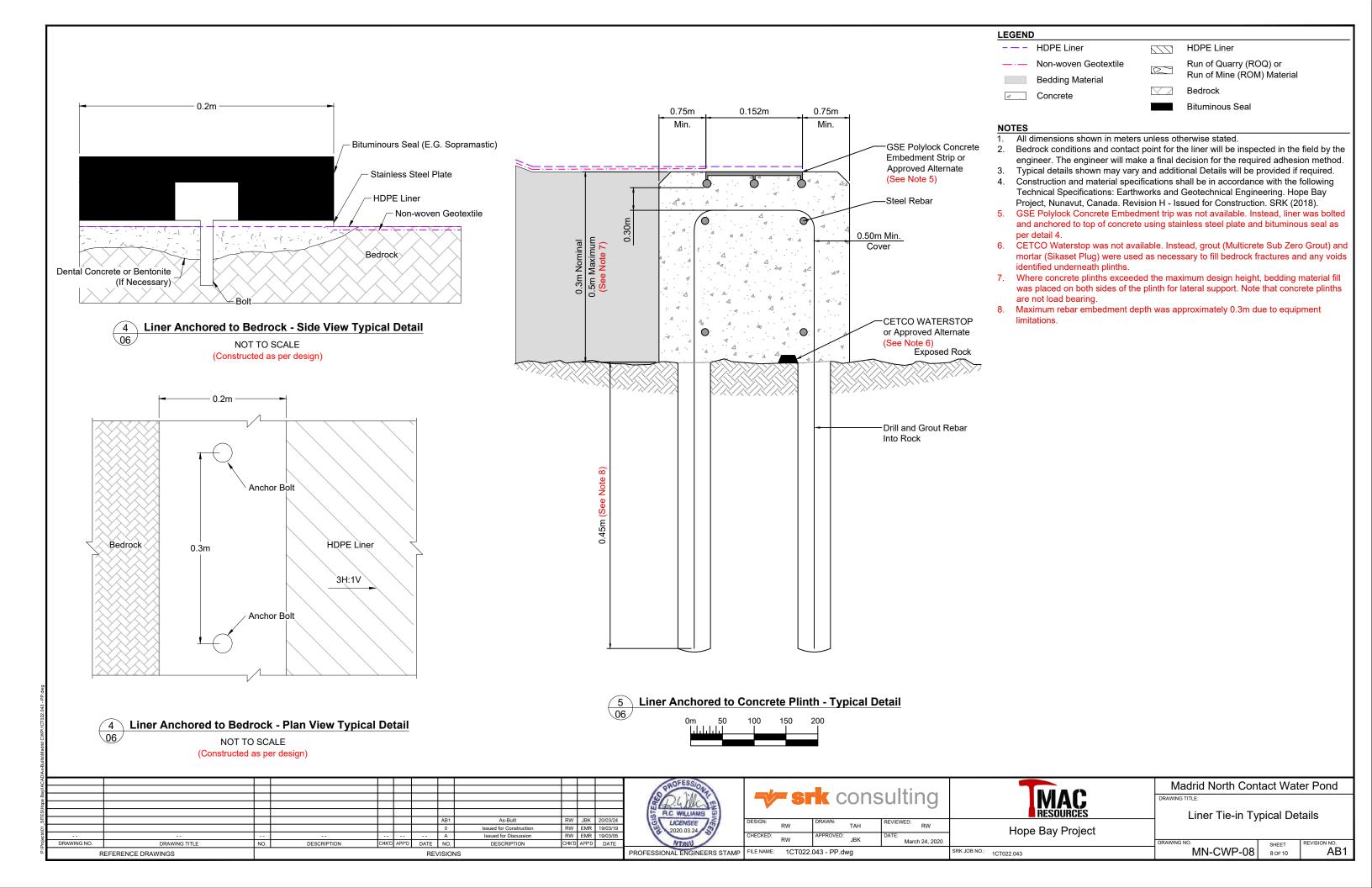
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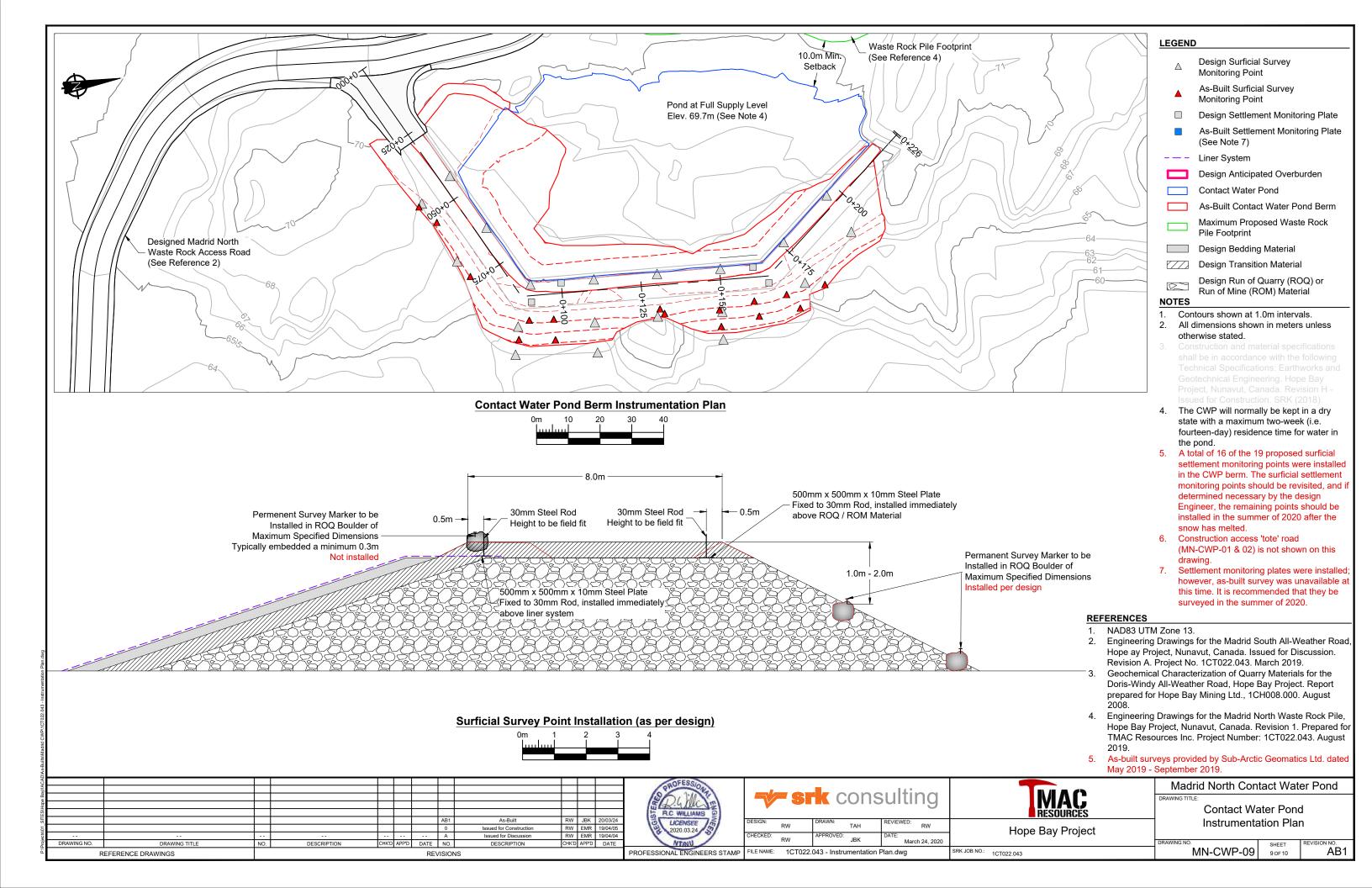


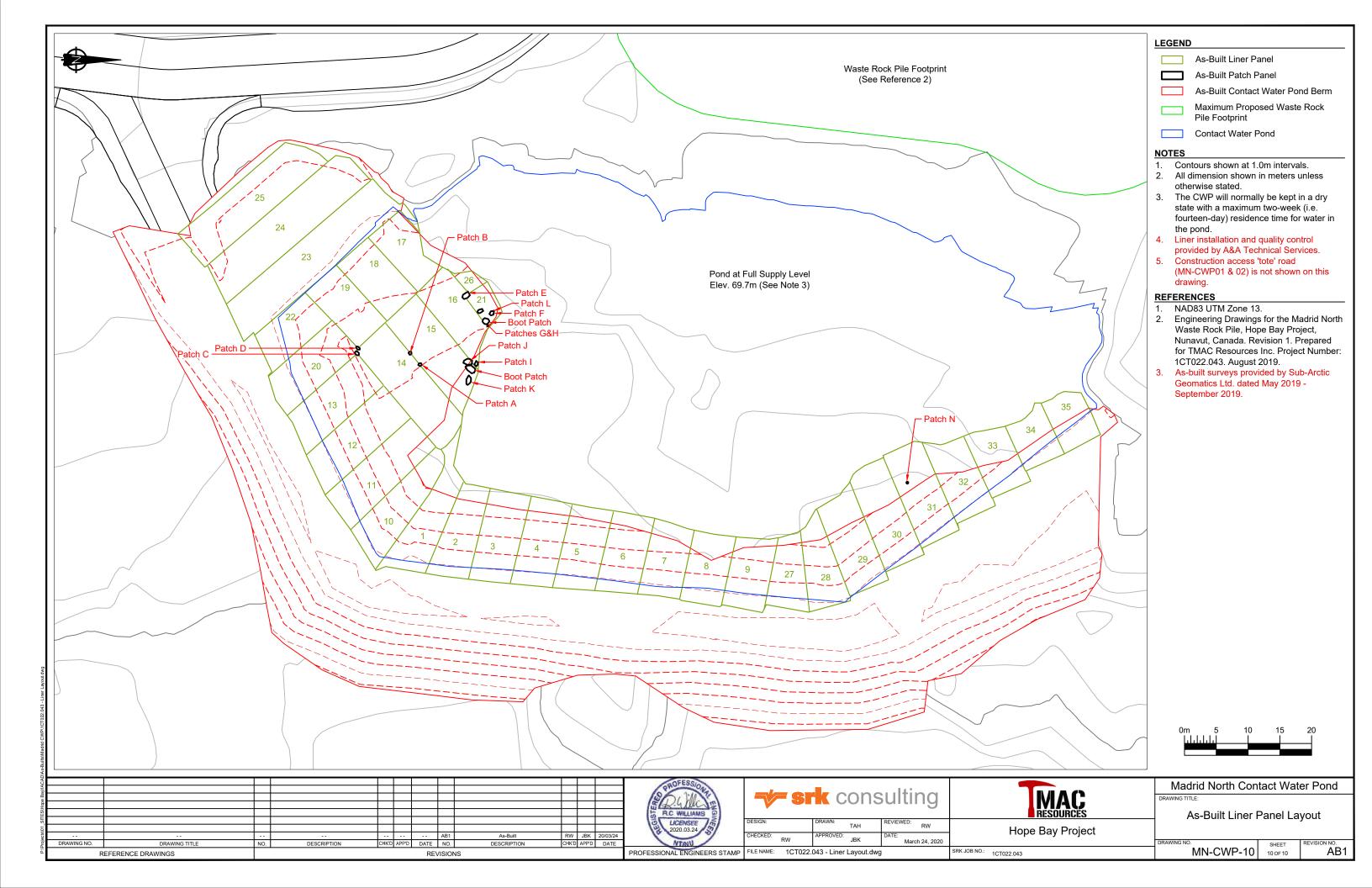


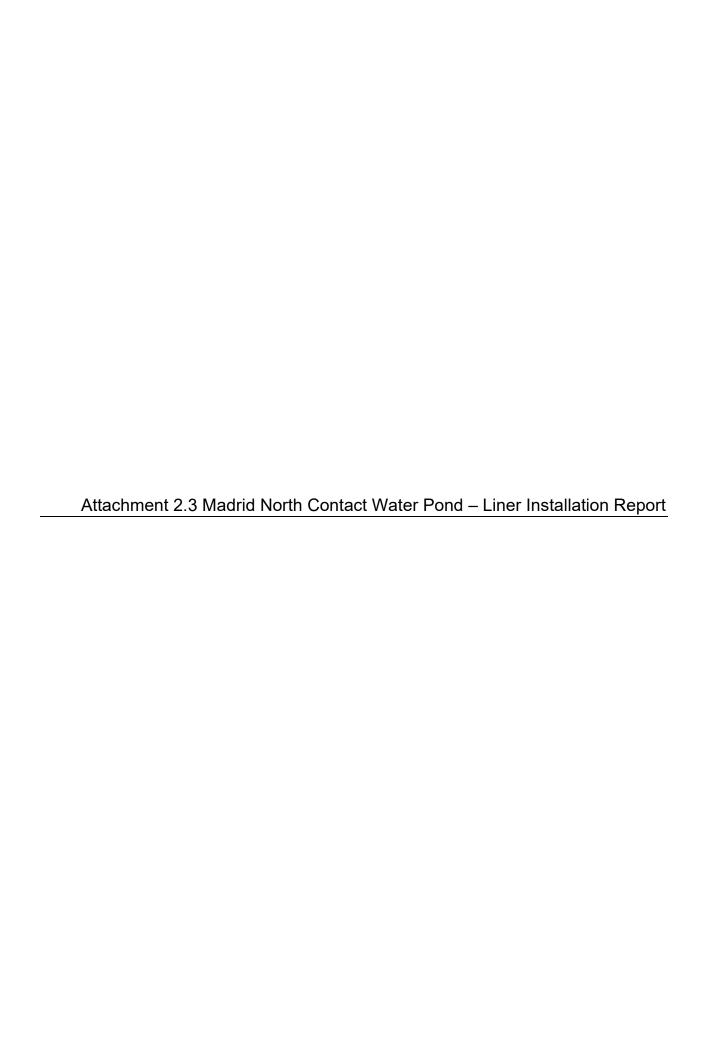












A&A Technical Services Yellowknife NT

TMAC Resources
Hope Bay
Madrid CWP HDPE Liner installation
June 25 - July 24, 2019

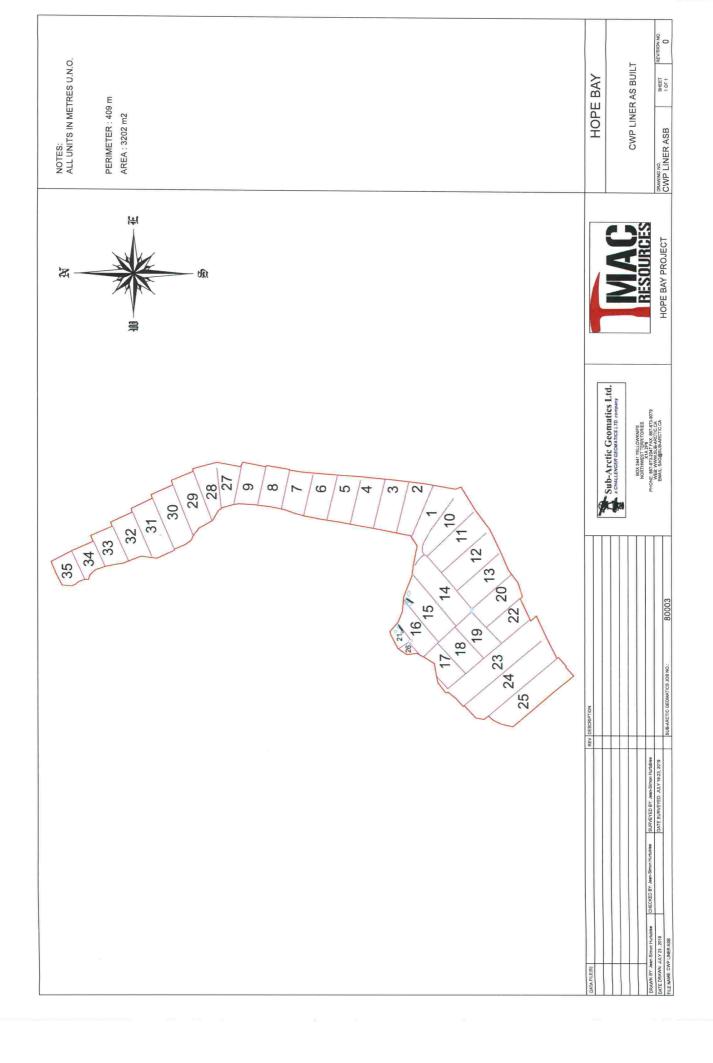


A&A Technical Services Yellowknife NT

TMAC Resources Hope Bay Madrid CWP HDPE Liner installation June 25 - July 24, 2019

Page	Table of contents
1	As built survey drawing
2	Liner panel dimension log
3+4	Daily welder qualifications and destruct sample Q/C data.
5+6	Non-destructive air pressure test Q/C data.
6+7	IAGI CWT certificates
8	Subgrade acceptance and warranty





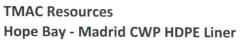
A&A Technical Services Yellowknife NT

TMAC Resources Hope Bay - Madrid CWP HDPE Liner Panel log



				11sc
Panel #	Solmax roll number	Length m	width	m2
P-1	138390	22.5	6.7	150.8
P-2	138390	14.5	6.7	99.3
P-3	138390	14.3	6.7	97.2
P-4	138390	14.5	6.7	95.8
P-5	138390	13.5	6.7	97.2
P-6	138390	11.8	6.7	90.5
P-7	138390	11.4	6.7	79.1
P-8	138390	11.0	6.7	76.4
P-9	138390	12.0	6.7	73.7
P-10	138390	11.0	6.7	80.4
P-11	138390	16.0	6.7	73.7
P-12	138390	15.0	6.7	107.2
P-13	138390	15.0	6.7	100.5
P-14	138390	19.5	6.7	100.5
P-15	138390	18.4	6.7	130.7
P-16	138390	17.6	6.7	123.3
P-17	138390	6.0	6.7	117.9
P-18	138390	12.0	6.7	40.2
P-19	138390	12.0	6.7	80.4
P-20	138401	12.0	6.7	80.4
P-21	138401	7.0	6.7	80.4
P-22	138401	11.3	2.3	46.9
P-23	138401	28.5	6.7	26.0
P-24	138401	27.1	6.7	191.0
P-25	138401	28.1	6.7	181.6
P-26	138401	3.7	1.1	188.3
P-27	138401	4.0	5.5	4.1
P-28	138401	11.2	5.5	22.0
P-29	138401	14.5	6.8	61.6
P-30	135267	14.7	6.6	98.6
P-31	135267	16.1	6.7	97.0
P-32	135267	15.1	6.7	107.9
P-33	135267	11.5	6.7	101.2
P-34	135267	9.1	6.7	77.1
P-35	135267	7.9	6.7	61.0
P-36	135267	8.9	6.7	52.9
			Total m2	3292.26

A&A Technical Services Yellowknife NT June/July 2019



Daily welder qualifiction tests



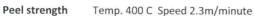
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28-Jun-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	136	141	91
2	140	140	91
3	142	136	91
4	145	142	91
28-Jun-19	Shear Strength		Minimum ppi (lbs/inch)
1	161		120
2	155		120

	Peel strength	Temp. 400 C Spe	eed 2.3m/minute
05-Jul-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	144	135	91
2	146	150	91
3	148	151	91
4	146	149	91
05-Jul-19	Shear Strength		Minimum ppi (lbs/inch)
1	161		120
2	160		120

Peel strength Temp. 400 C Speed 2.3m/minute			eed 2.3m/minute
08-Jul-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	140	142	91
2	146	138	91
3	139	144	91
4	142	145	91
08-Jul-19	Shear Strength		Minimum ppi (lbs/inch)
1	158		120
2	155		120







remp. 400 c speed 2.5m/milite			ca 2.5m/mmate
16-Jul-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	144	142	91
2	140	137	91
3	148	142	91
4	139	139	91
16-Jul-19	Shear Strength		Minimum ppi (lbs/inch)
1	158		120
2	161		120

Peel strength	Temp. 400 C	Speed 2.3m/minute
---------------	-------------	-------------------

22-Jul-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	144	145	91
2	135	139	91
3	140	143	91
4	145	140	91
22-Jul-19	Shear Strength		Minimum ppi (lbs/inch)
1	153		120
2	159		120

Extrusion welder qualification Preheat 250 C Extrudite 250 C

08-Jul-19	Peel strength	Minimum ppi (lbs/inch)
1	138	78
2	144	78
	Shear Strength	
1	158	120
2	152	120

Extrusion welder qualification Preheat 250 C Extrudite 250 C

theater travelouser		
22-Jul-19	Peel strength	Minimum ppi (lbs/inch)
1	141	78
2	139	78
	Shear Strength	
1	162	120
2	158	120

A&A Technical Services Yellowknife NT June/July 2019

TMAC Resources Hope Bay - Madrid CWP HDPE Liner



Non destructive air pressure testing of wedge weld seams. (5minutes)

Date	Technician	Seam location	Start psi	Finish psi	Pass/Fail	Comments
28-Jun-19	АН	P1-P2	35	35	Pass	
28-Jun-19	АН	P2-P3	30	30	Pass	
28-Jun-19	АН	P-3-P4	32	32	Pass	
28-Jun-19	АН	P-4-P5	32	32	Pass	
28-Jun-19	АН	P-5-P-6	35	35	Pass	
28-Jun-19	АН	P6-P7	30	30	Pass	
03-Jul-19	АН	P7-P8	30	30	Pass	Witnessed by SRK
03-Jul-19	АН	P8-P9	35	35	Pass	
05-Jul-19	АН	P1-P10	30	30	Pass	
05-Jul-19	АН	P10-P11	35	35	Pass	
05-Jul-19	АН	P11-P12	35	35	Pass	
05-Jul-19	АН	P12-P13	35	35	Pass	
08-Jul-19	АН	P14-P15a	35	35	Pass	
08-Jul-19	AH	P14-P15b	32	32	Pass	
08-Jul-19	АН	P15-P16	35	35	Pass	
16-Jul-19	GH	P17-P18	30	30	Pass	
16-Jul-19	GH	P18-P19	30	30	Pass	
16-Jul-19	GH	P13-P20	30	30	Pass	4
16-Jul-19	GH	P20-P22	30	30	Pass	
16-Jul-19	GH	Toe P11	30	30	Pass	
16-Jul-19	GH	Toe P12	30	30	Pass	
16-Jul-19	GH	Toe P13	30	30	Pass	
16-Jul-19	GH	Toe P20	30	30	Pass	
16-Jul-19	GH	Toe P22	30	30	Pass	
16-Jul-19	GH	P22-P23	30	30	Pass	
17-Jul-19	GH	P19-P23	30	30	Pass	
17-Jul-19	GH	P18-P23	30	30	Pass	
17-Jul-19	GH	P17-P23	30	30	Pass	
17-Jul-19	GH	P23-P24	30	30	Pass	
17-Jul-19	GH	P21-P26	32	32	Pass	
17-Jul-19	GH	P26-P16	30	30	Pass	
22-Jul-19	GH	P9-P27	30	30	Pass	
22-Jul-19	GH	P27-P28	30	30	Pass	
22-Jul-19	GH	P28-P29	30	30	Pass	

P1 = Liner panel number in relation to drawing

A&A Technical Services Yellowknife NT June/July 2019

TMAC Resources Hope Bay - Madrid CWP HDPE Liner



Non destructive air pressure testing of wedge weld seams. (5minutes)

Technician	Seam location	Start psi	Finish psi	Pass/Fail	Comments
GH	P29-P30	30	30		
GH	P30-P31	30	30		
GH	P31-P32	30	30		
GH	P32-P33	30	30		
GH	P34-P35	30	30		
Vaccum box	testing of extr				
Technician	Location			Pass/Fail	
patches on seam				Pass	
	P14-P15				
Patches at toe of				Pass	
	P20 and P13				
	t-l P15 P16	225		Dana	
Patches on P15-P16-P26				Pass	
	-				
	GH GH GH GH GH TH GH GH GH GH Vaccum box Technician	GH P29-P30 GH P30-P31 GH P31-P32 GH P32-P33 GH P34-P35 Vaccum box testing of extra Technician Location patches on seam P14-P15 Patches at toe of P20 and P13	GH P29-P30 30 GH P30-P31 30 GH P31-P32 30 GH P32-P33 30 GH P34-P35 30 Vaccum box testing of extrusion patches Technician Location patches on seam P14-P15 Patches at toe of	GH P29-P30 30 30 GH P30-P31 30 30 GH P31-P32 30 30 GH P32-P33 30 30 GH P34-P35 30 30 Vaccum box testing of extrusion patches Technician Location patches on seam P14-P15 Patches at toe of P20 and P13	GH P29-P30 30 30 GH P30-P31 30 30 GH P31-P32 30 30 GH P32-P33 30 30 GH P34-P35 30 30 Vaccum box testing of extrusion patches Technician Location Pass/Fail patches on seam Pass P14-P15 Patches at toe of Pass P20 and P13 P20 and P13 Pass

P1 = Liner panel number in relation to drawing

CERTIFIED WELDING TECHNICIAN



The International Association of Geosynthetic Installers Certifies:

ALAN HARMAN

hands-on skills, knowledge and experience in the welding and installation of polyethylene (PE) geomembranes, and As a Certified Welding Technician, in polyethylene wedge and extrusion welding, having demonstrated superior having basic mechanical aptitude for working with welders and equipment on the job site.

Registration number: CWT162010

Valid 07 June 2016 — 07 June 2021

President, IAGI

Geosynthetic Installers

Managing Director, IAG.

CERTIFIED WELDING TECHNICIAN



The International Association of Geosynthetic Installers Certifies:

GUY HORESAY

hands-on skills, knowledge and experience in the welding and installation of polyethylene (PE) geomembranes, and As a Certified Welding Technician, in polyethylene wedge and extrusion welding, having demonstrated superior having basic mechanical aptitude for working with welders and equipment on the job site.

Registration number: CWT170010

Valid 07 June 2016 — 07 June 2021

President, IAGI

Managing Director, IAGI

A&A Technical Services Subgrade acceptance and warranty

Client –TMAC Resources
Hope Bay
Madrid CWP HDPE Liner installation
June 25 - July 24, 2019

Upon arrival to site the fuel tank pad area to be lined was thoroughly inspected by A&A Technical Services installation supervisor and deemed to be a suitable surface on which to place the HDPE lining system. The SOLMAX 60mil textured HDPE liner was under laid with a layer of 540g/m2 non-woven geotextile supplied by the client.

Warranties issued by A&A Technical Services shall cover only the cost of replacement and/or repair of defective installations, determined or agreed to be the responsibility of A&A Technical Services, provide that the warranty work will be performed to the same standards and scope of work set out in the contract documents. A&A's installation warranty shall commence upon acceptance of the individual geosynthetic components by the owner or its representative as such components are completed. The installation warranty period shall not exceed beyond 1 years. Our installation warranty is rendered null and void if the installed geosynthetics are subject to abuse by machinery, equipment or personnel not under the control of A&A, harmful chemicals or unusual weather conditions or catastrophic earthworks failures.

A&A Technical Services shall not be held liable for defects, damage and/or deficient materials and installations, either in whole or in part should the defects, damage or deficient materials and installations arise as the result from the use of poor quality and inappropriate or unsuitable earthworks material or site preparation. This limitation of liability extends to improper and/or construction techniques, and methods and equipment used to create the earthworks covering all or any portion of the completed geosynthetic installation.

Dated: July 25, 2019

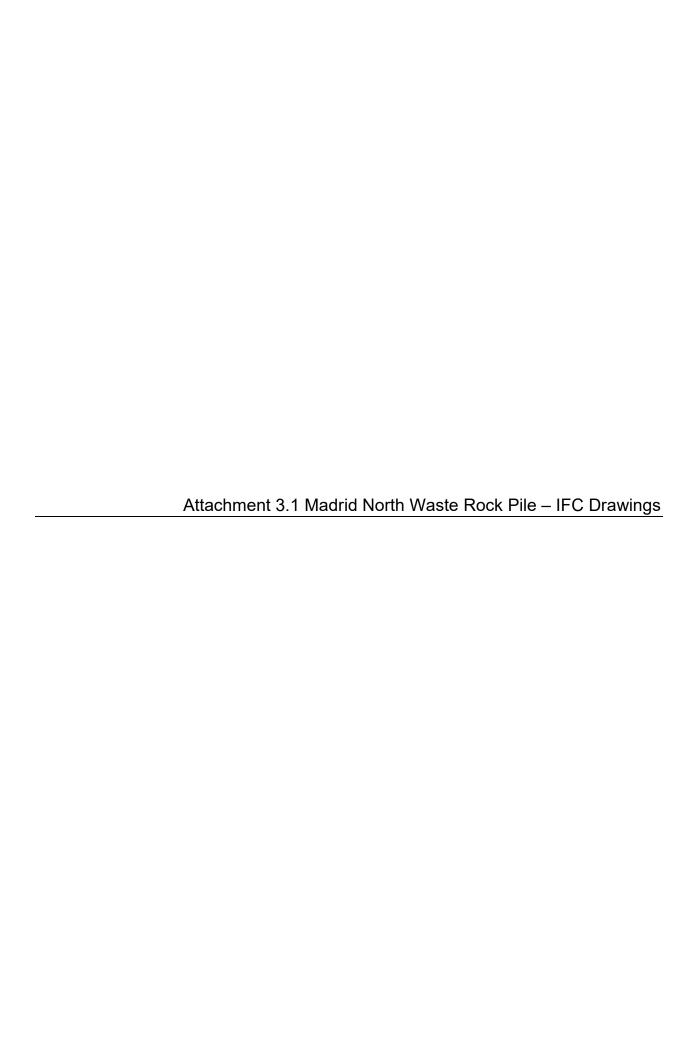
Signed: (

Al Harman President

A&A Technical Services

Yellowknife NT





Engineering Drawings for the Madrid North Waste Rock Pile Hope Bay Project, Nunavut, Canada

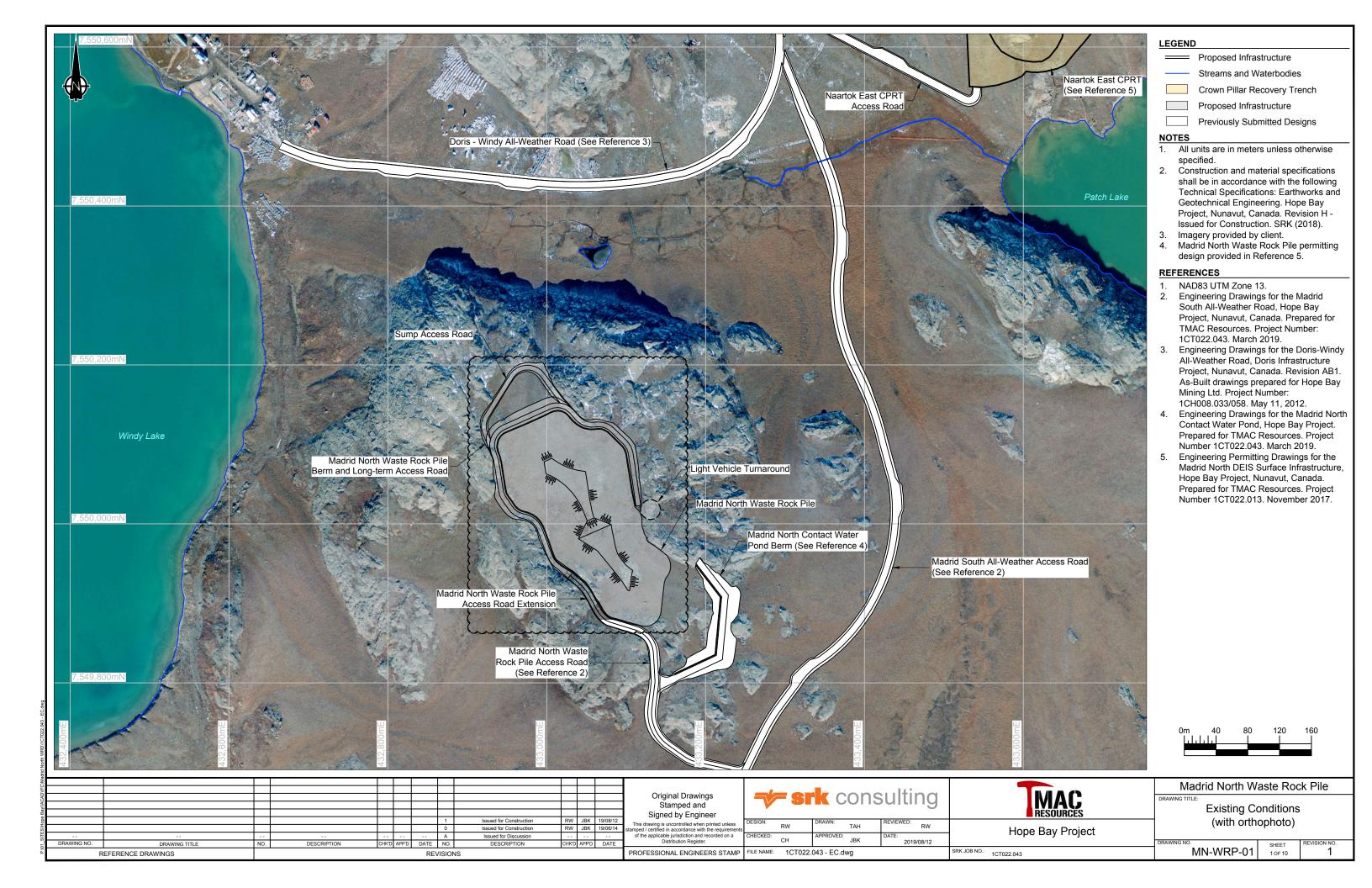
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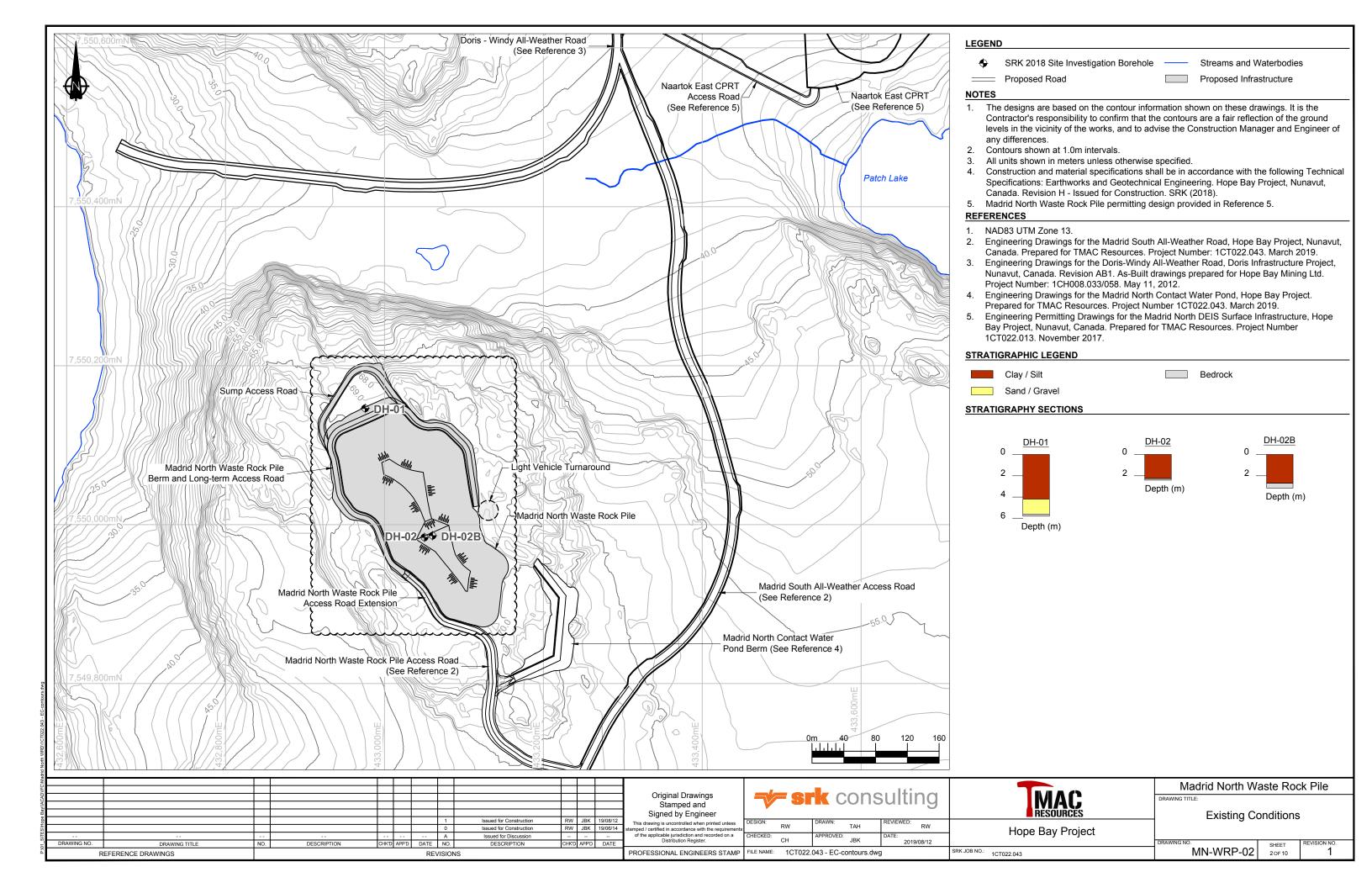
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MN-WRP-02	Existing Conditions	Issued for Construction	2019/08/13	1
MN-WRP-03	Existing Drainage Flowpaths	Issued for Construction	2019/08/13	1
MN-WRP-04	Waste Rock Pile Pad - Part 1	Issued for Construction	2019/08/13	1
MN-WRP-05	Waste Rock Pile Pad - Part 2	Issued for Construction	2019/08/13	1
MN-WRP-06	Waste Rock Pile Pad Sections	Issued for Construction	2019/08/13	1
MN-WRP-07	Waste Rock Pile	Issued for Construction	2019/08/13	1
MN-WRP-08	Waste Rock Pile Sections	Issued for Construction	2019/08/13	1
MN-WRP-09	Typical Details	Issued for Construction	2019/08/13	1
MN-WRP-10	Typical Sump Details	Issued for Construction	2019/08/13	0

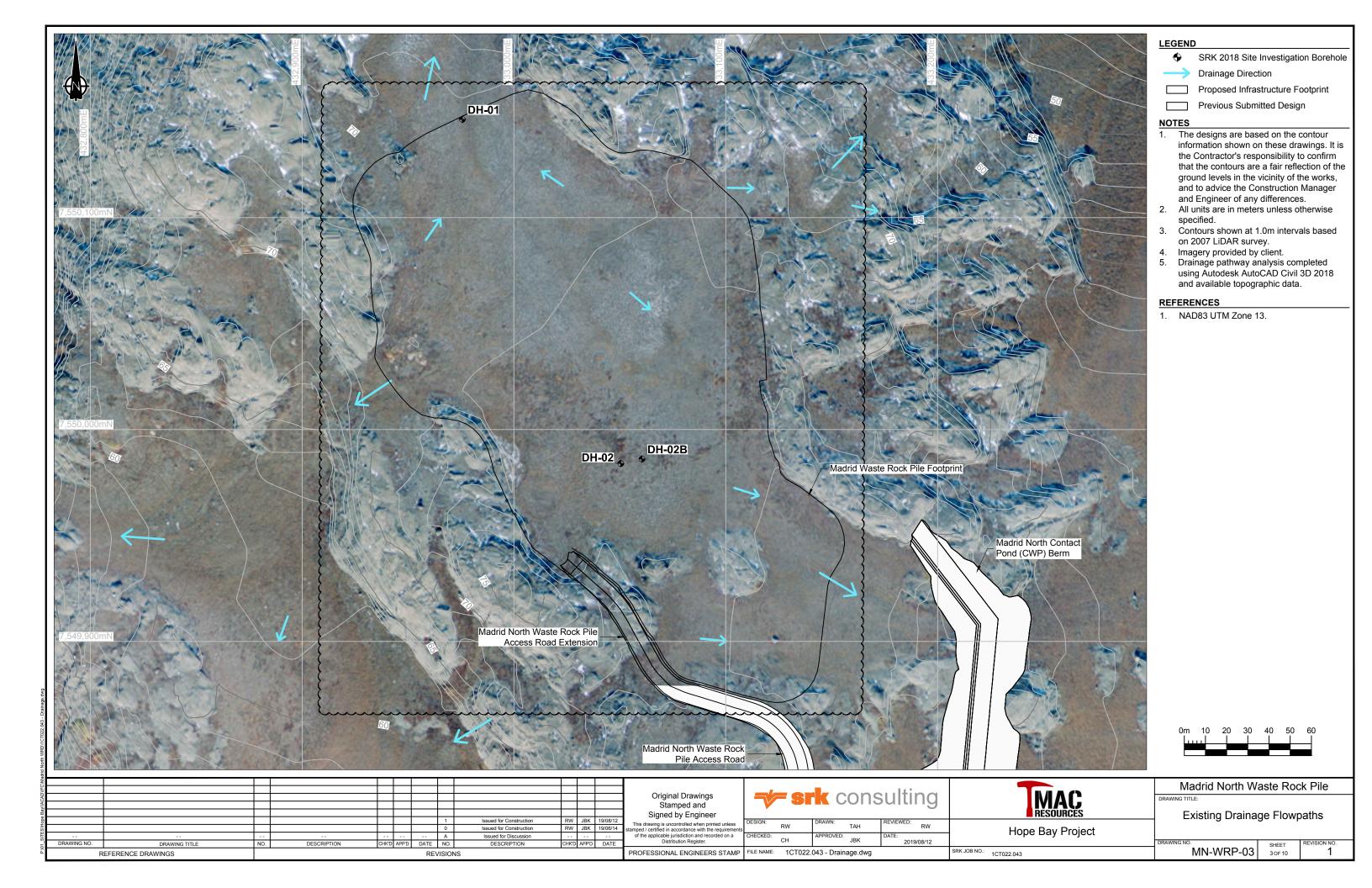


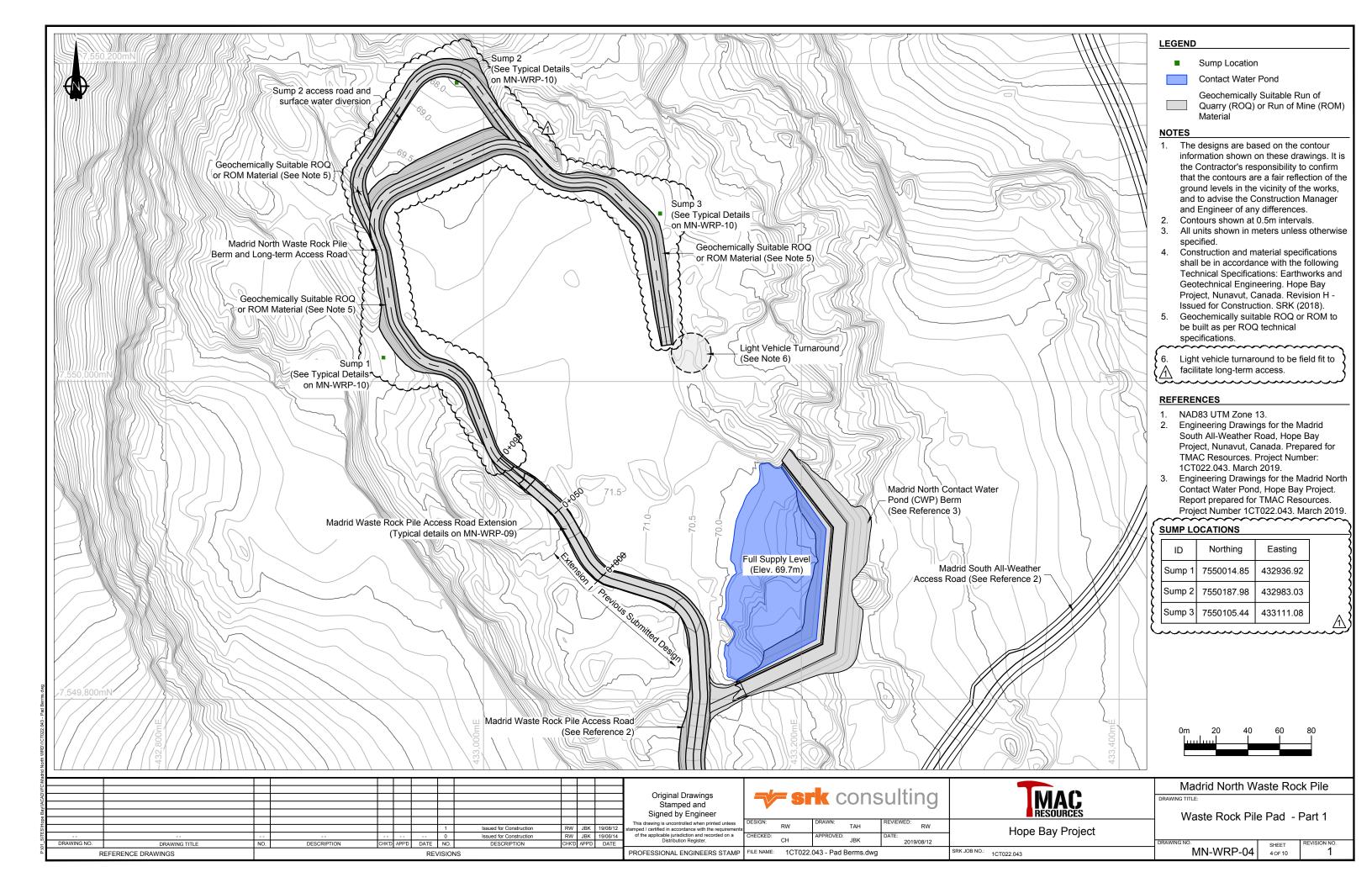


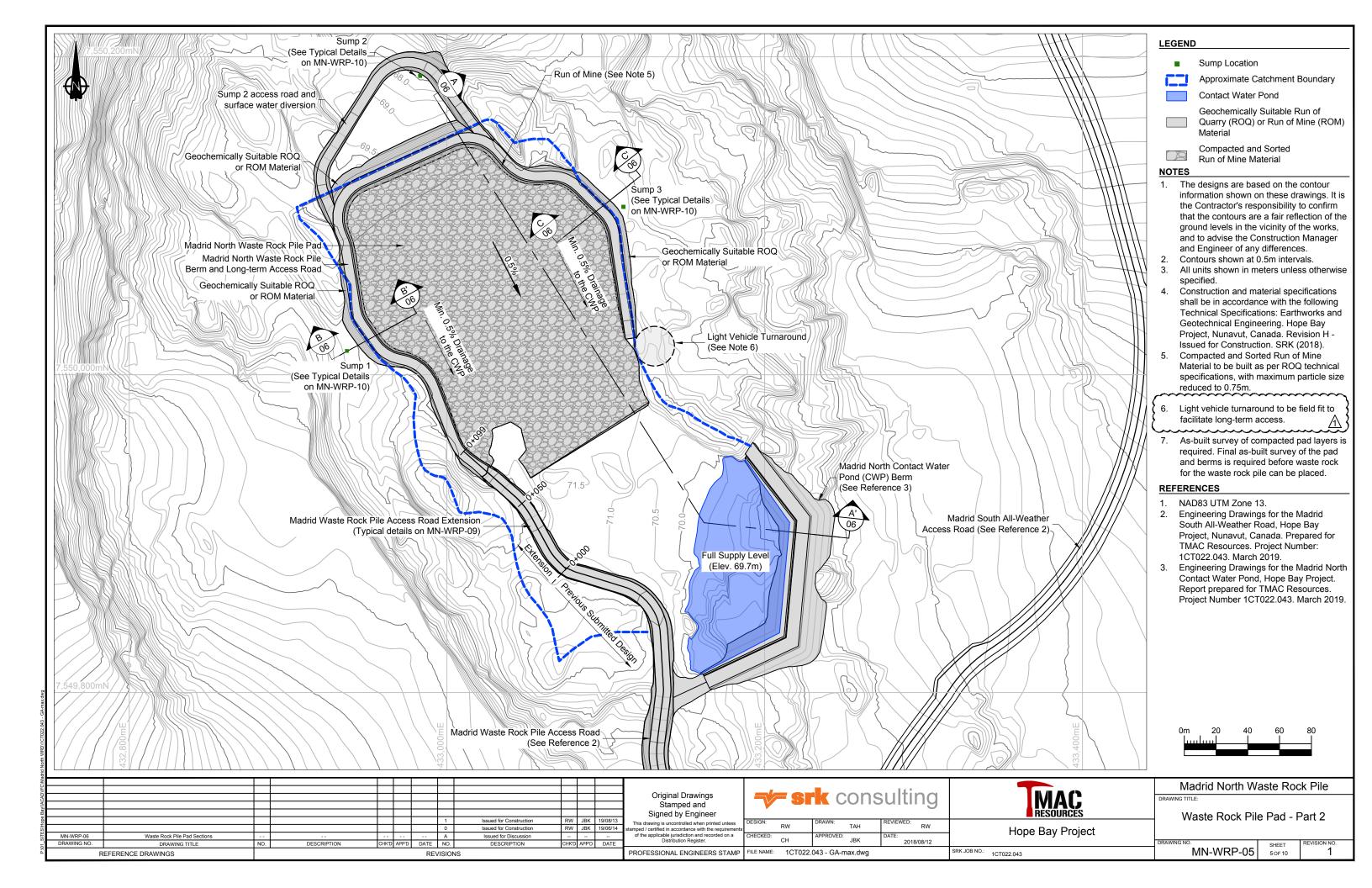
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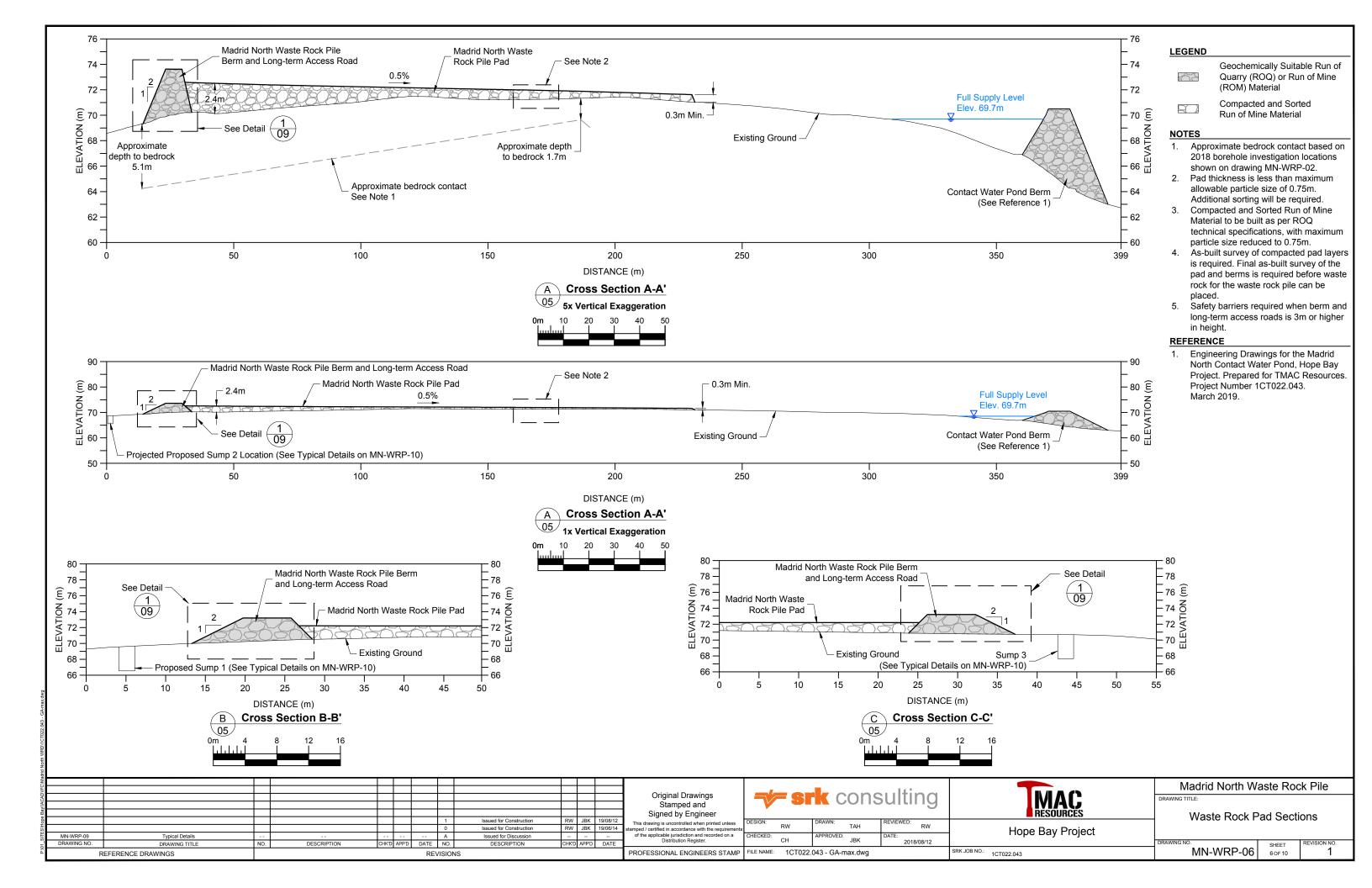


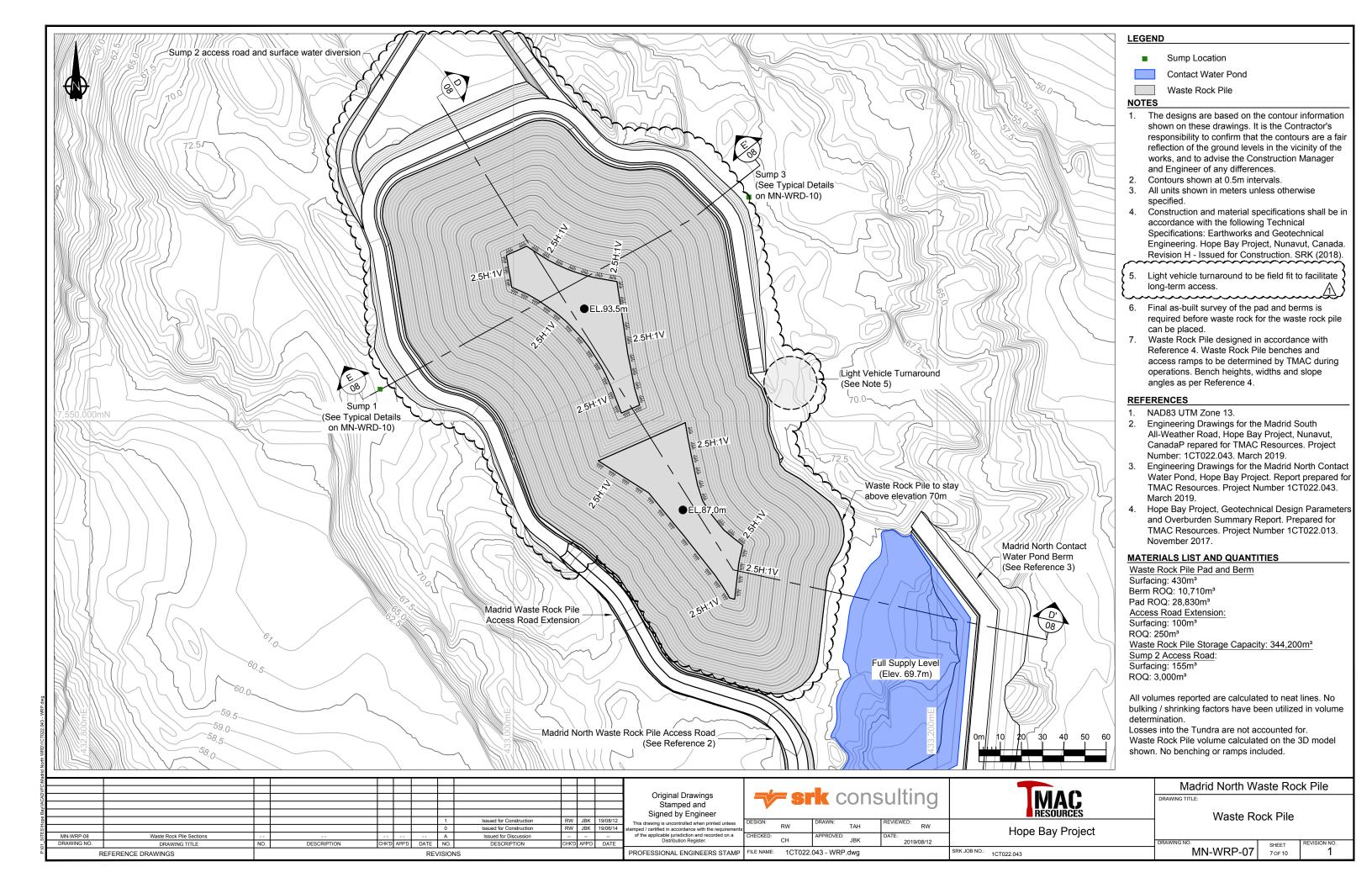


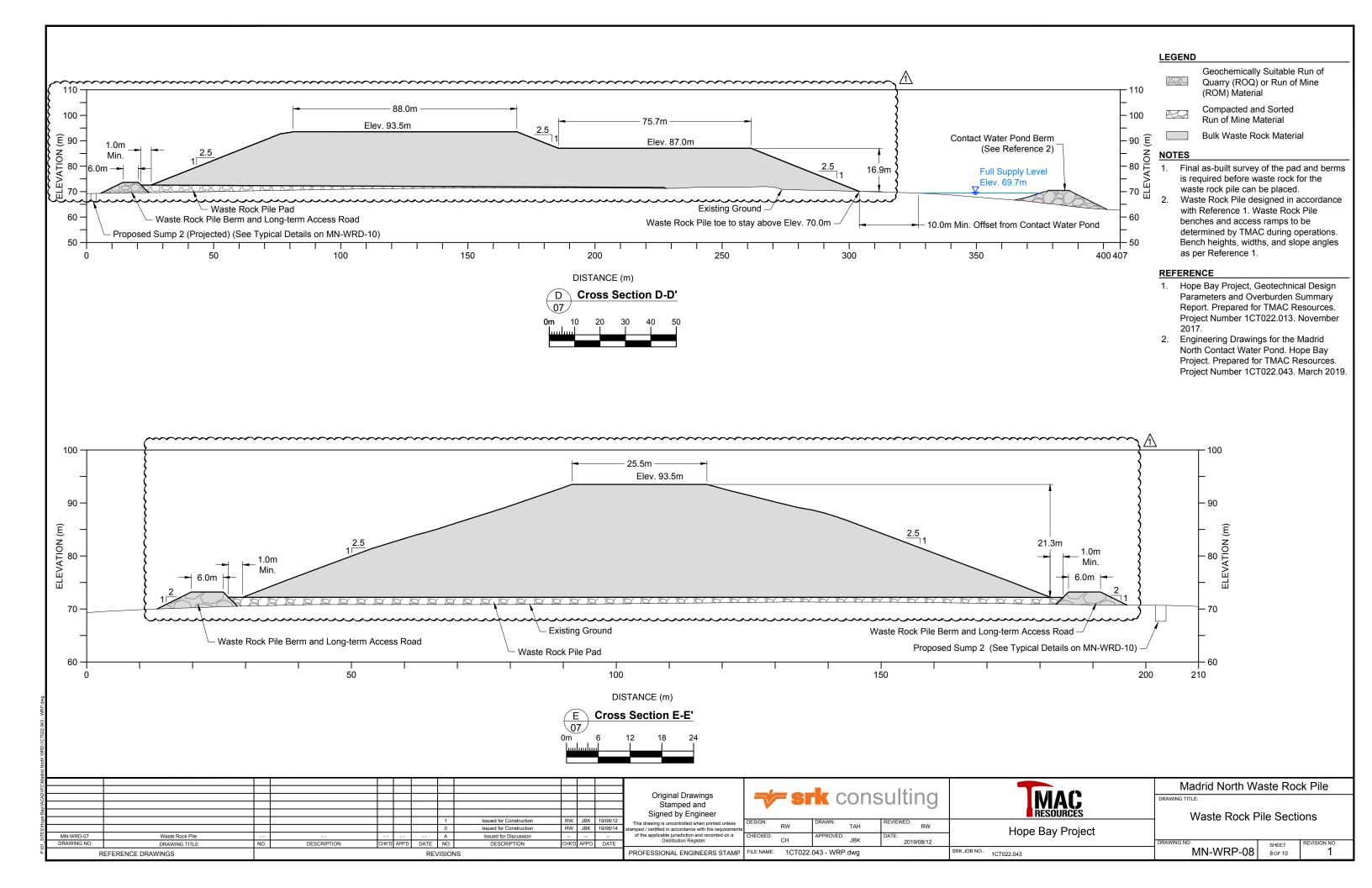


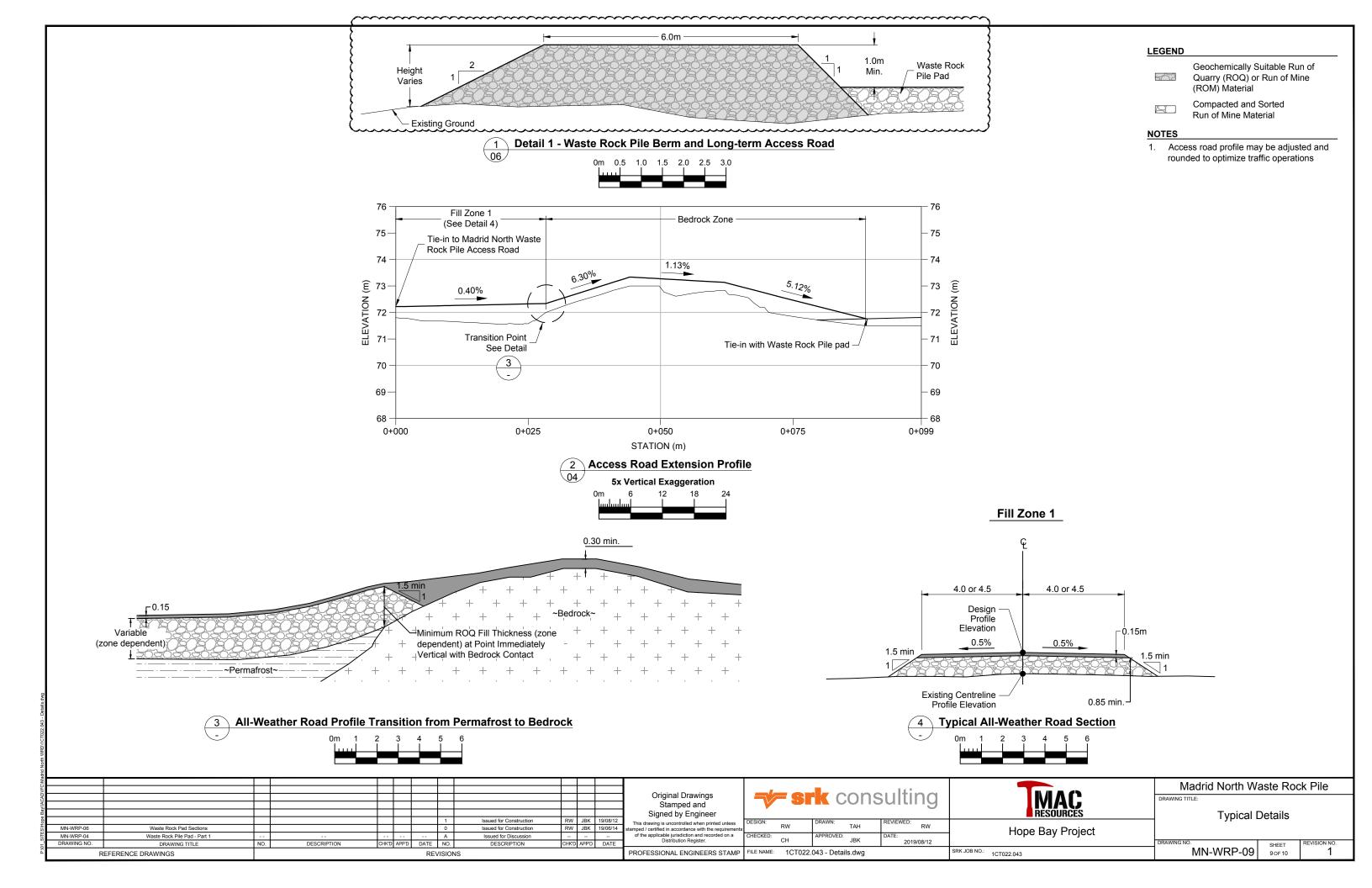


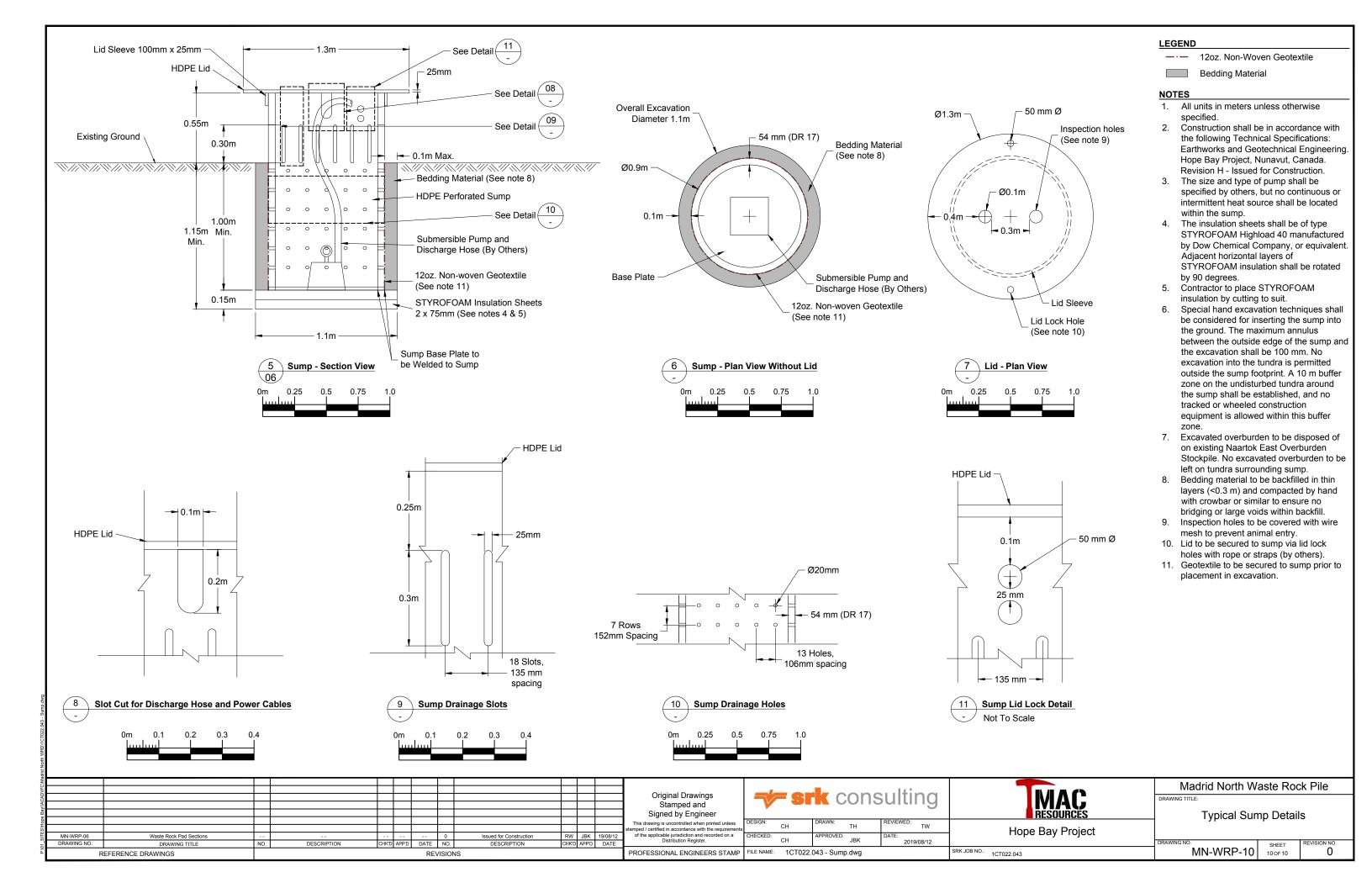














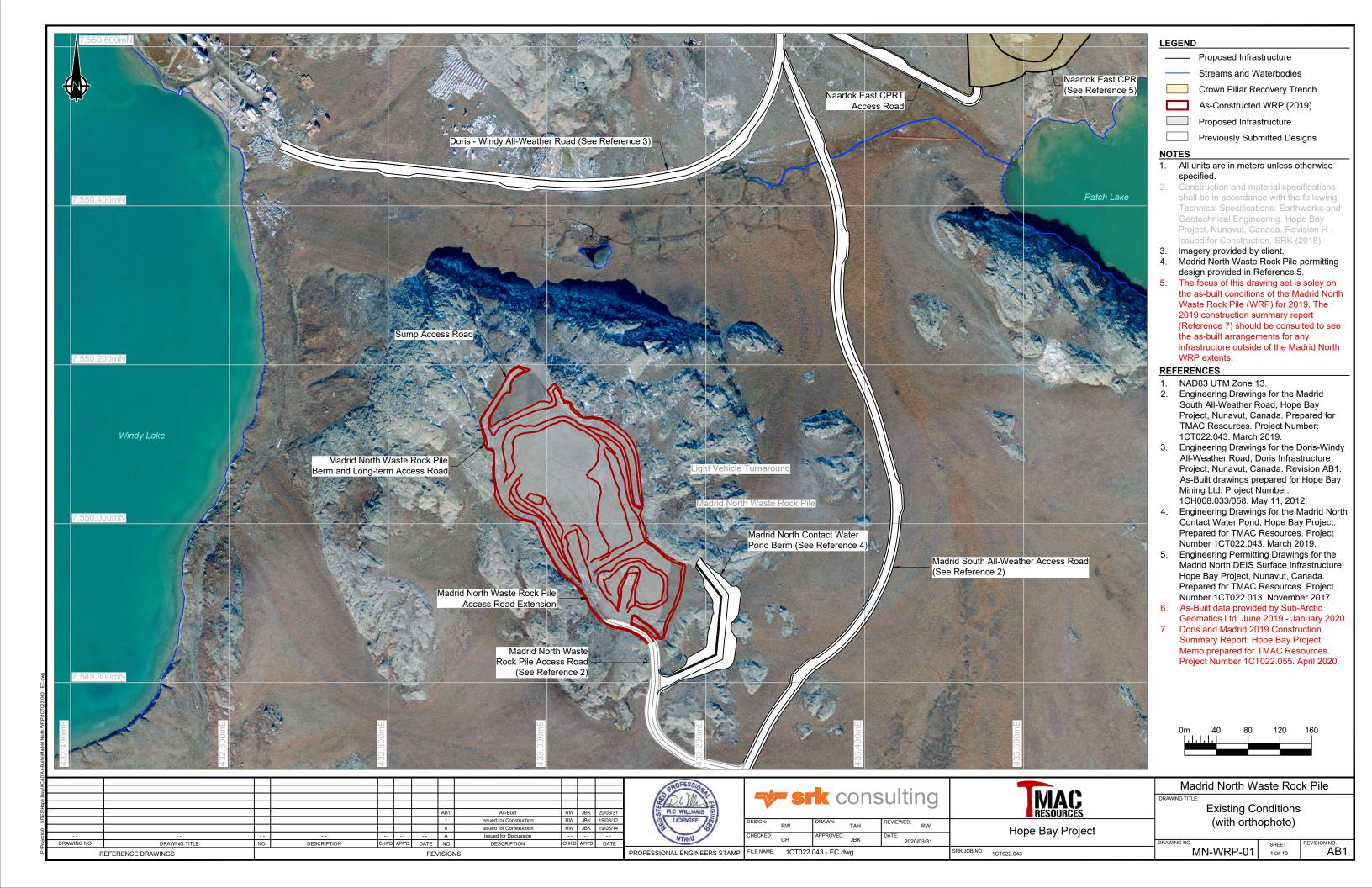
Engineering Drawings for the Madrid North Waste Rock Pile Hope Bay Project, Nunavut, Canada

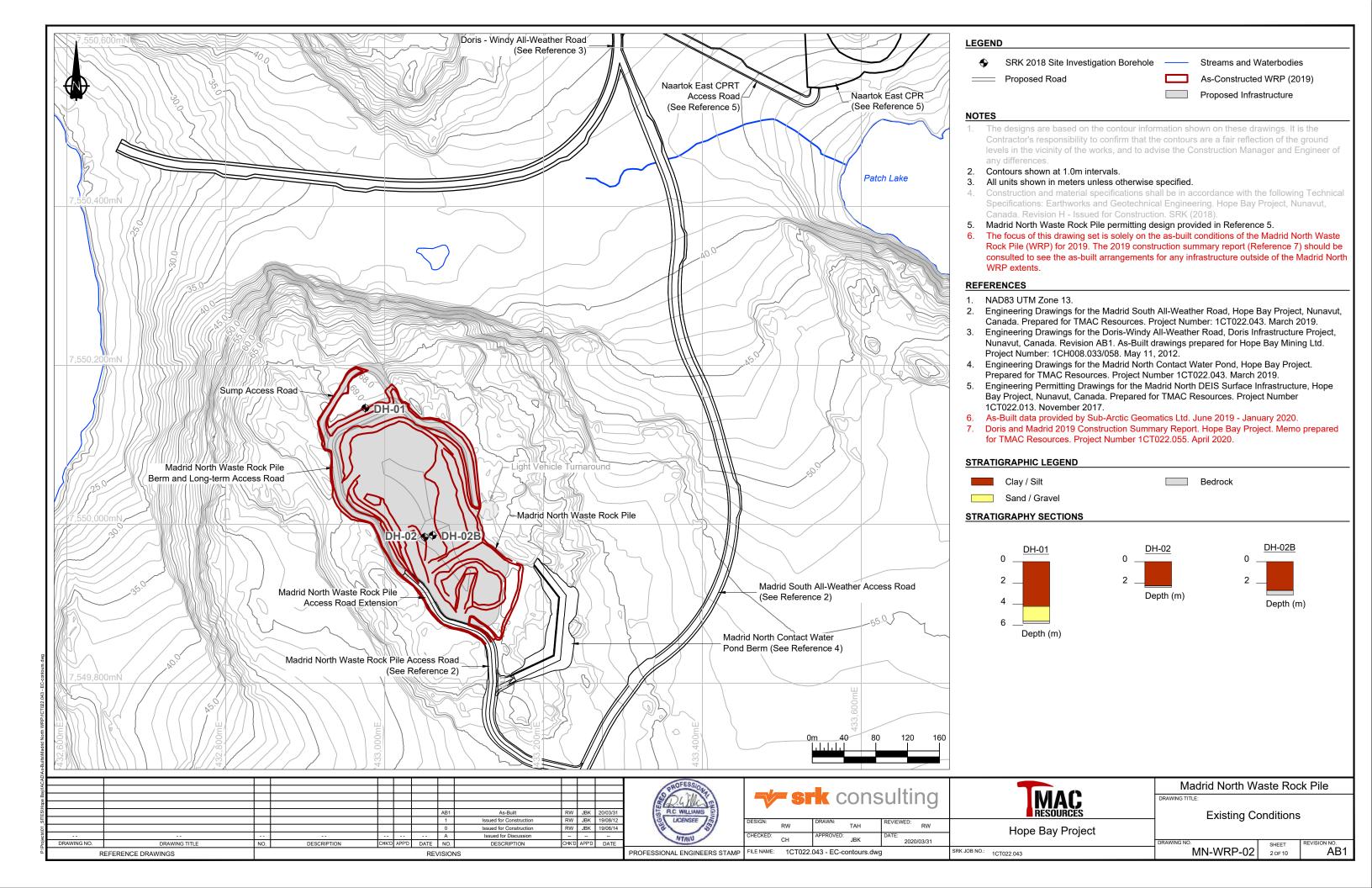
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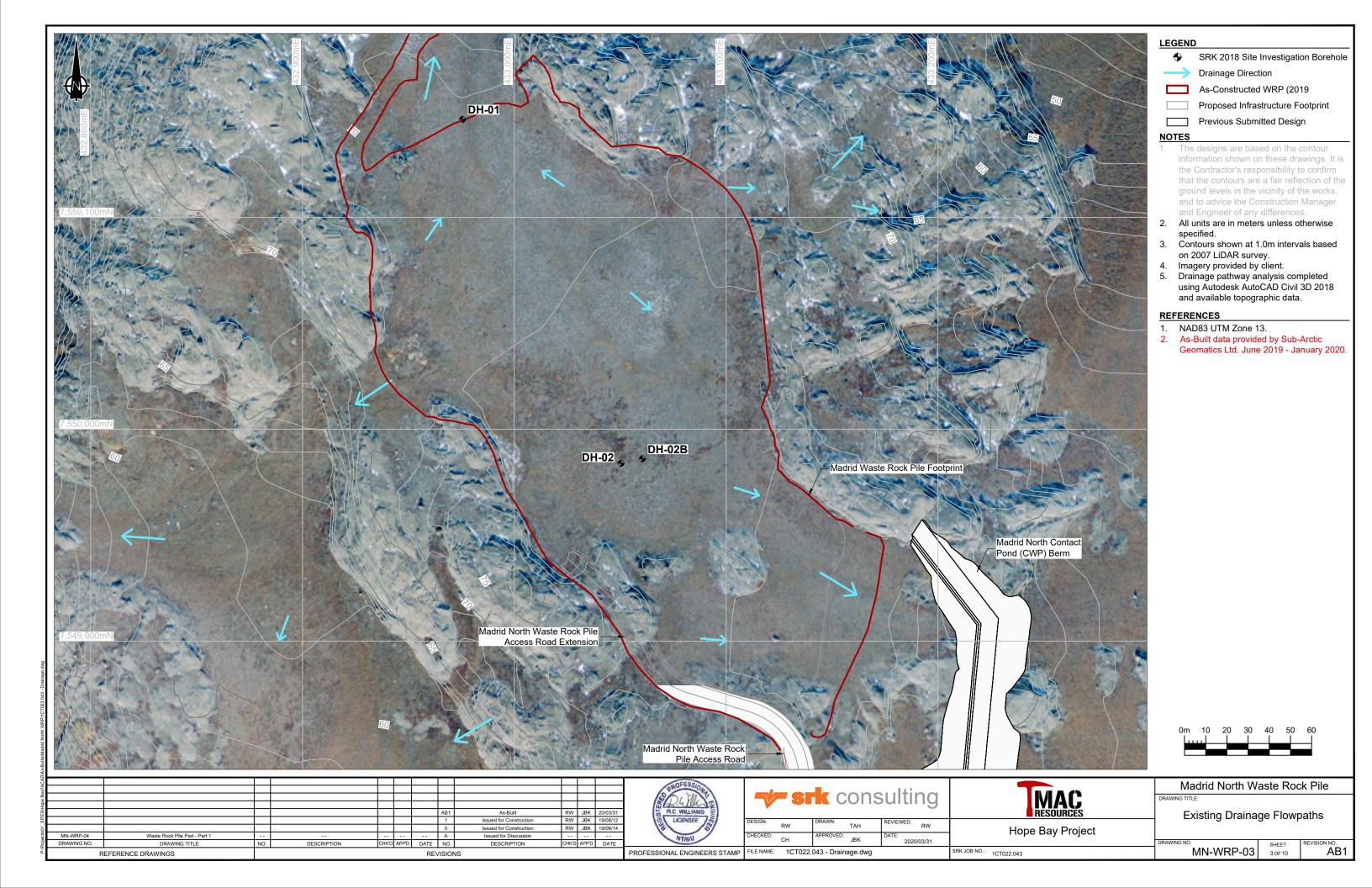
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MN-WRP-01	Existing Conditions (with orthophoto)	As-Built	2020/03/31	AB1
MN-WRP-02	Existing Conditions	As-Built	2020/03/31	AB1
MN-WRP-03	Existing Drainage Flowpaths	As-Built	2020/03/31	AB1
MN-WRP-04	Waste Rock Pile Pad - Part 1	As-Built	2020/03/31	AB1
MN-WRP-05	Waste Rock Pile Pad - Part 2	As-Built	2020/03/31	AB1
MN-WRP-06	Waste Rock Pile Pad Sections	As-Built	2020/03/31	AB1
MN-WRP-07	Waste Rock Pile	As-Built	2020/03/31	AB1
MN-WRP-08	Waste Rock Pile Sections	As-Built	2020/03/31	AB1
MN-WRP-09	Typical Details	As-Built	2020/03/31	AB1
MN-WRP-10	Typical Sump Details	As-Built	2020/03/31	AB1

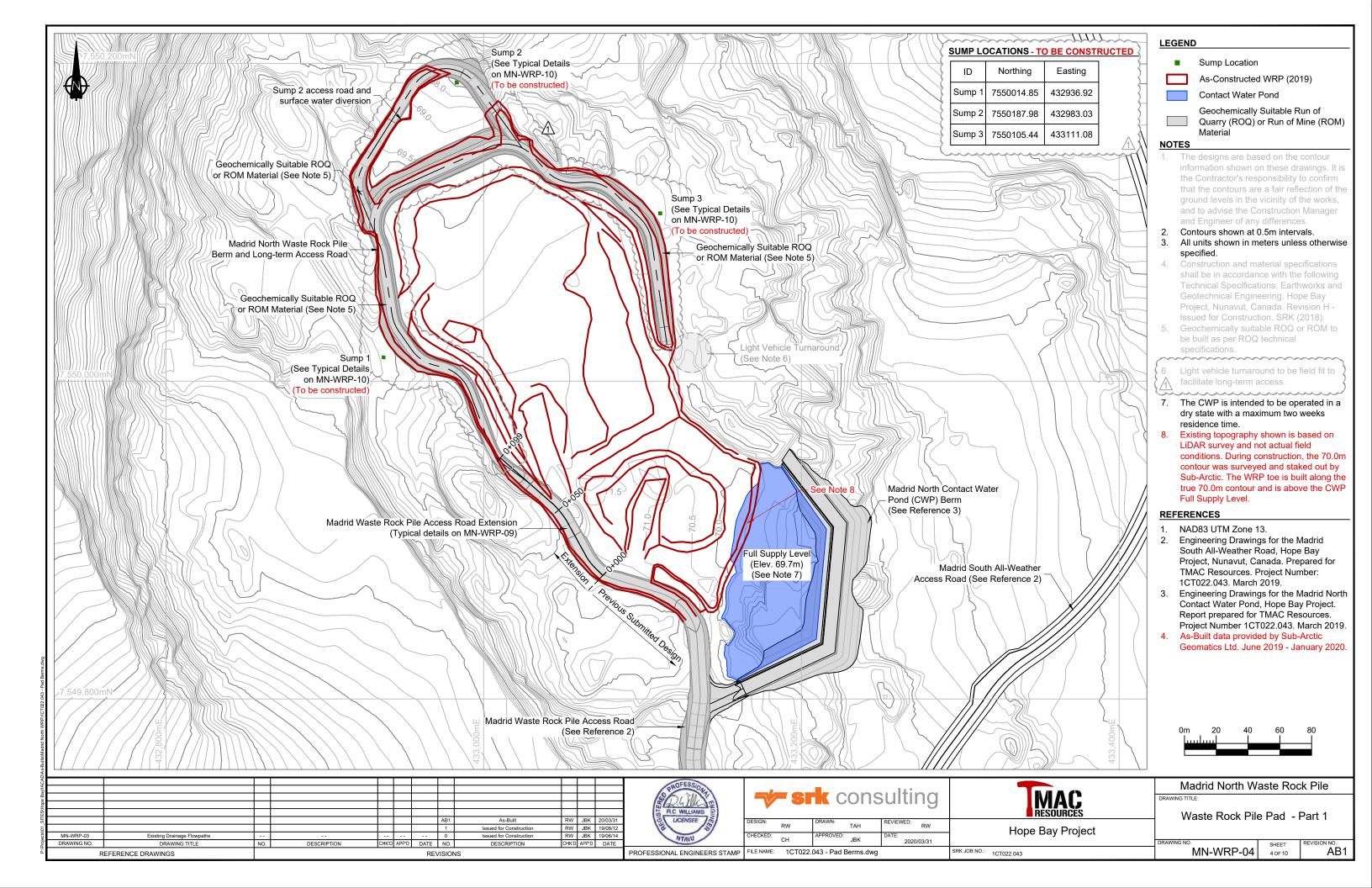


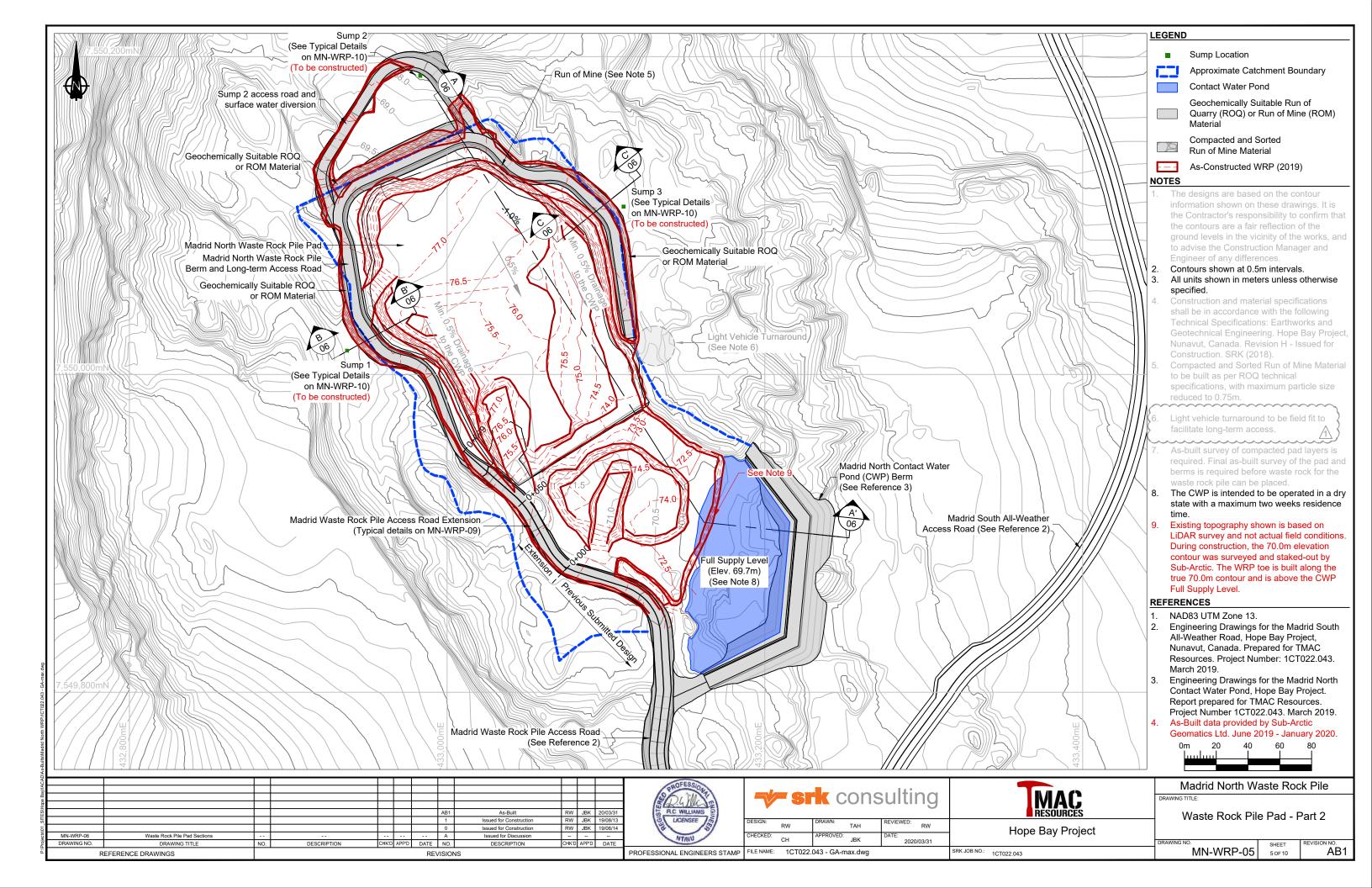


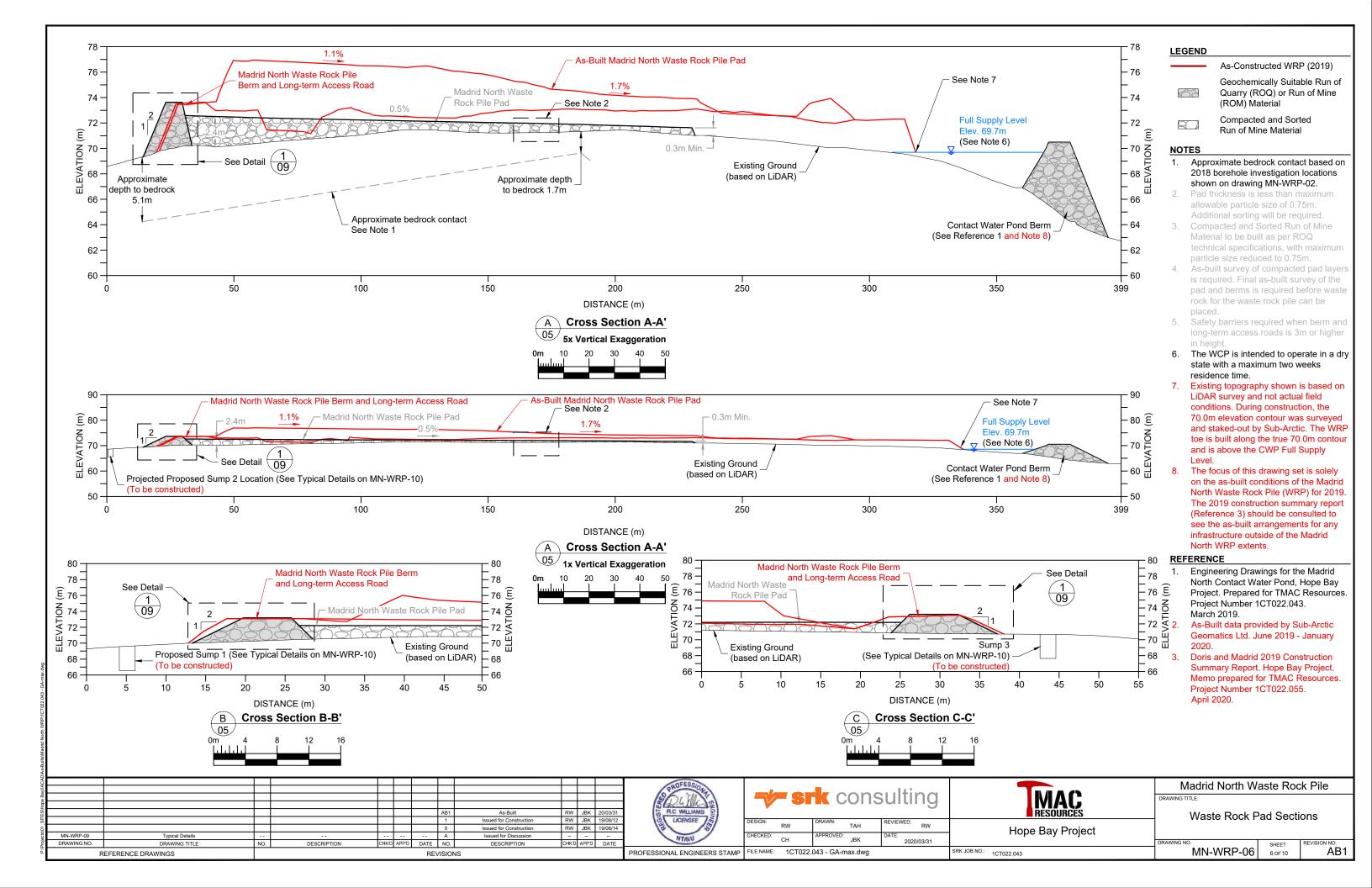


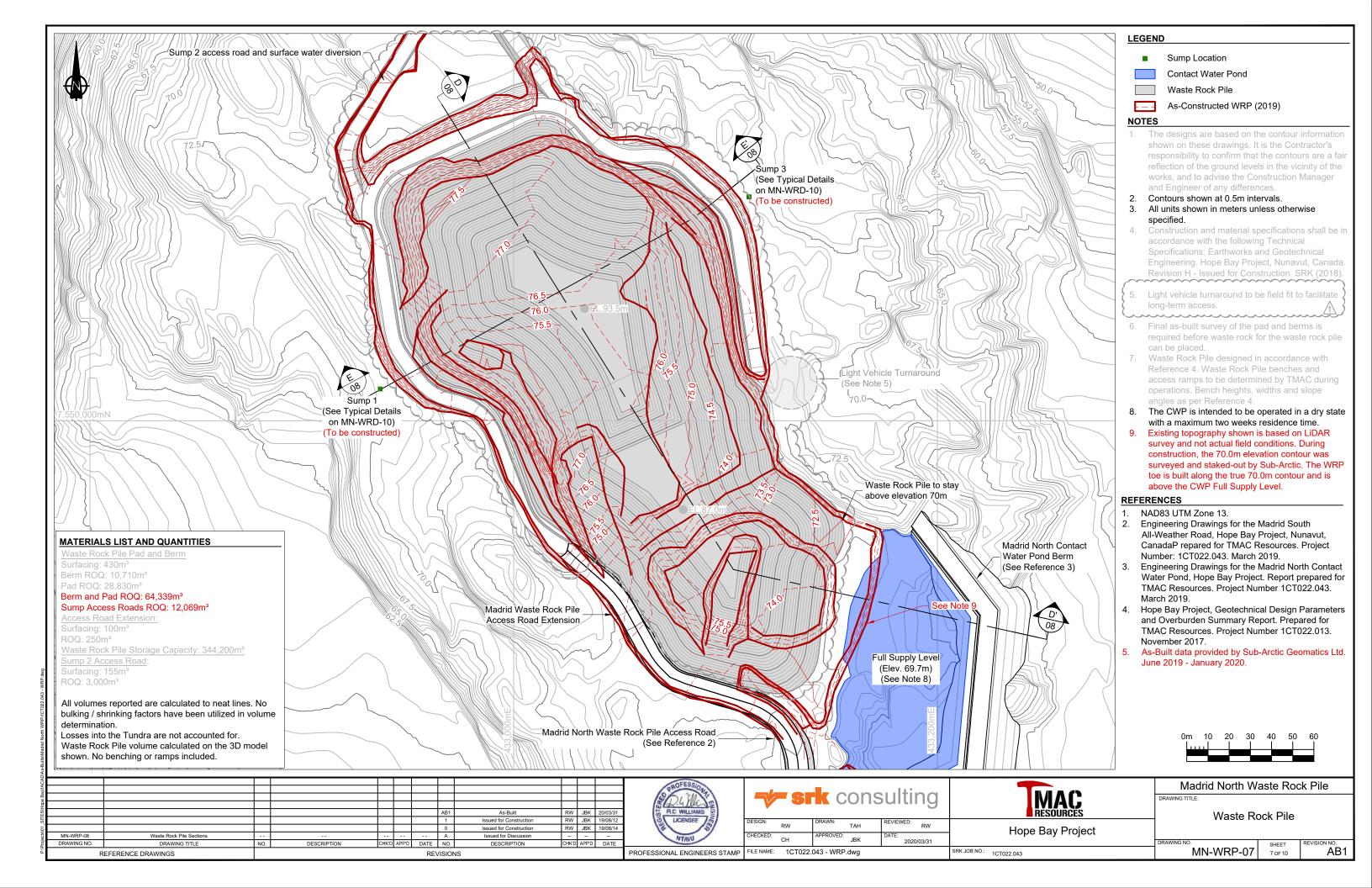


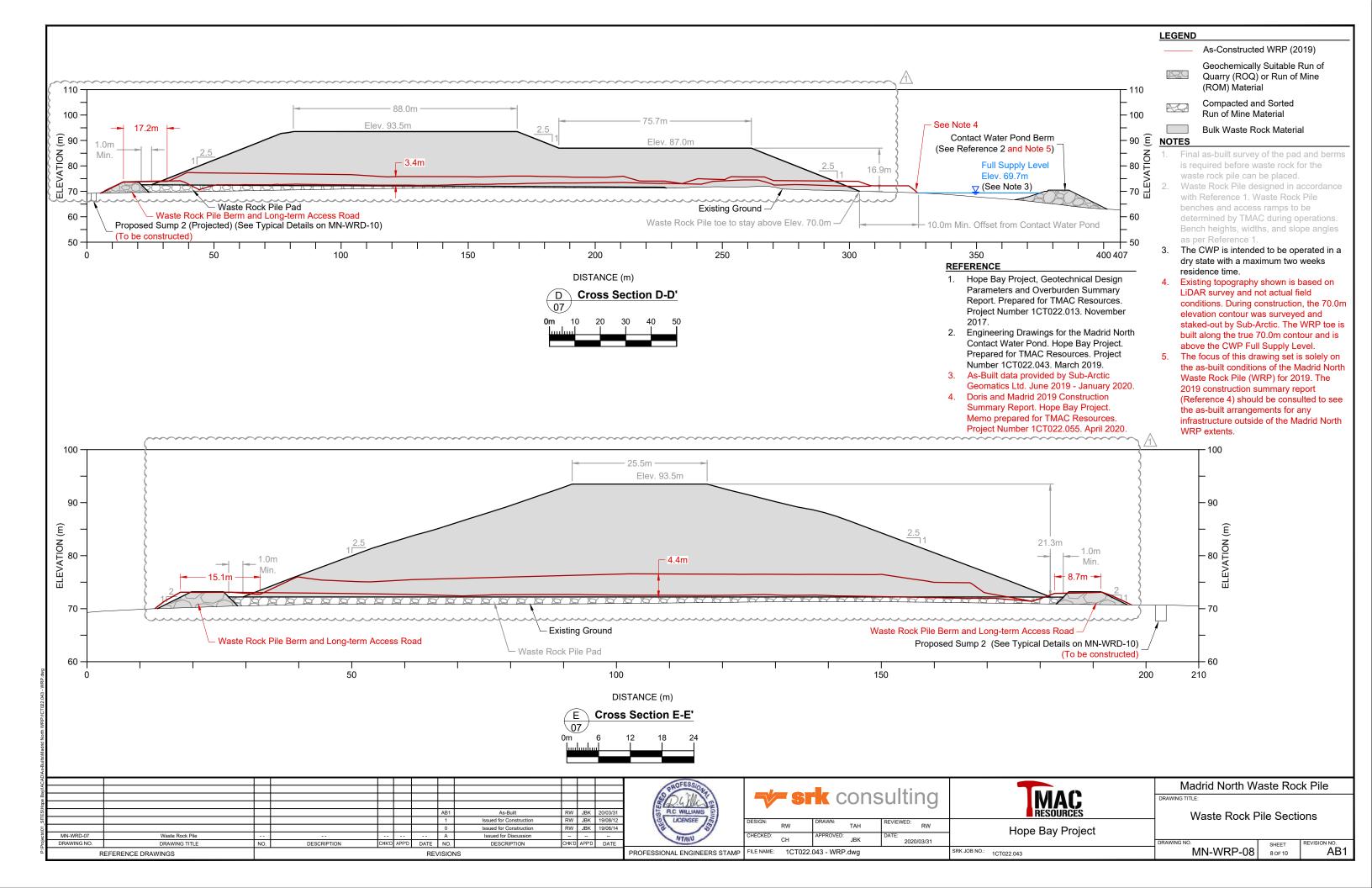


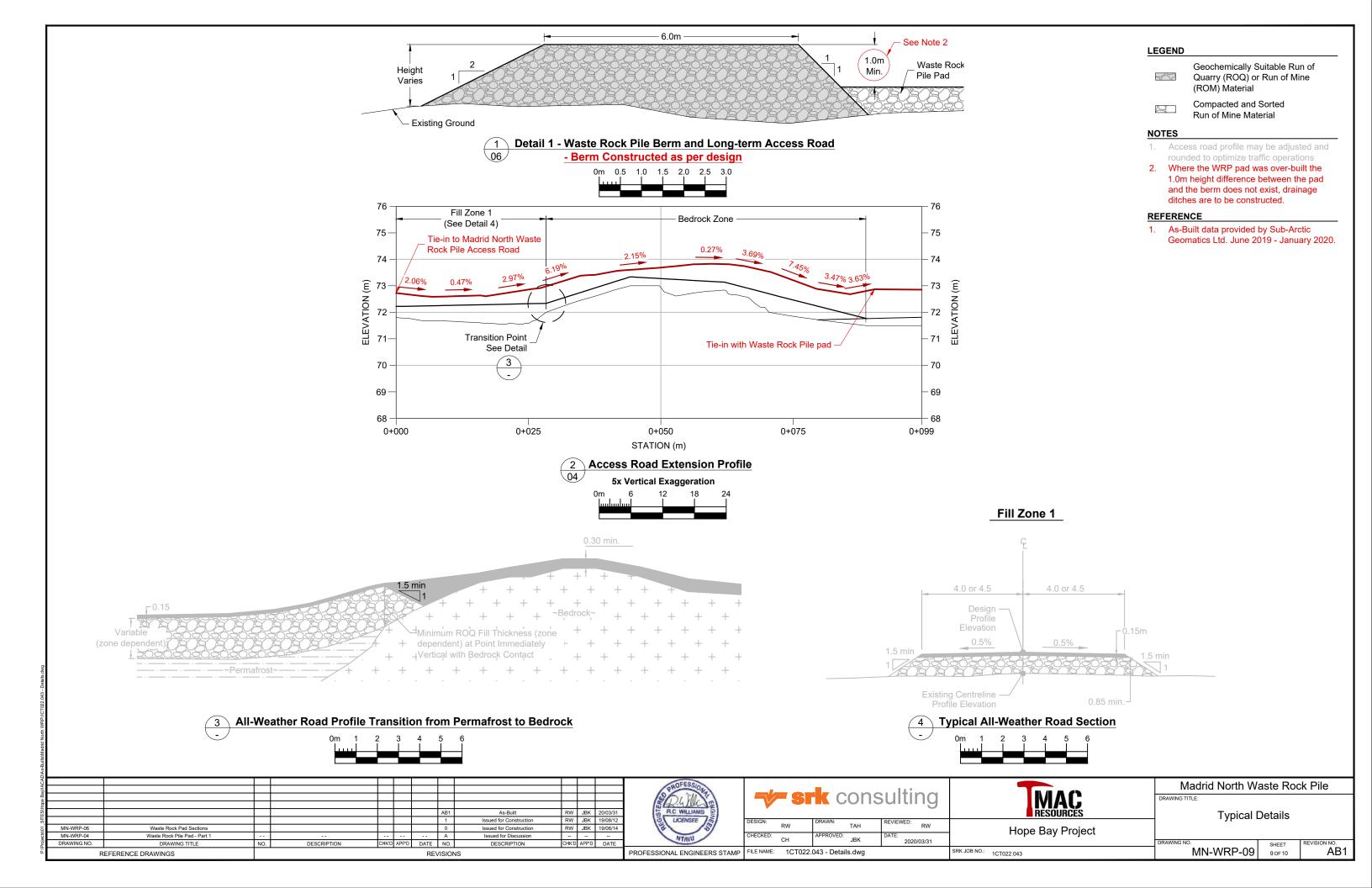


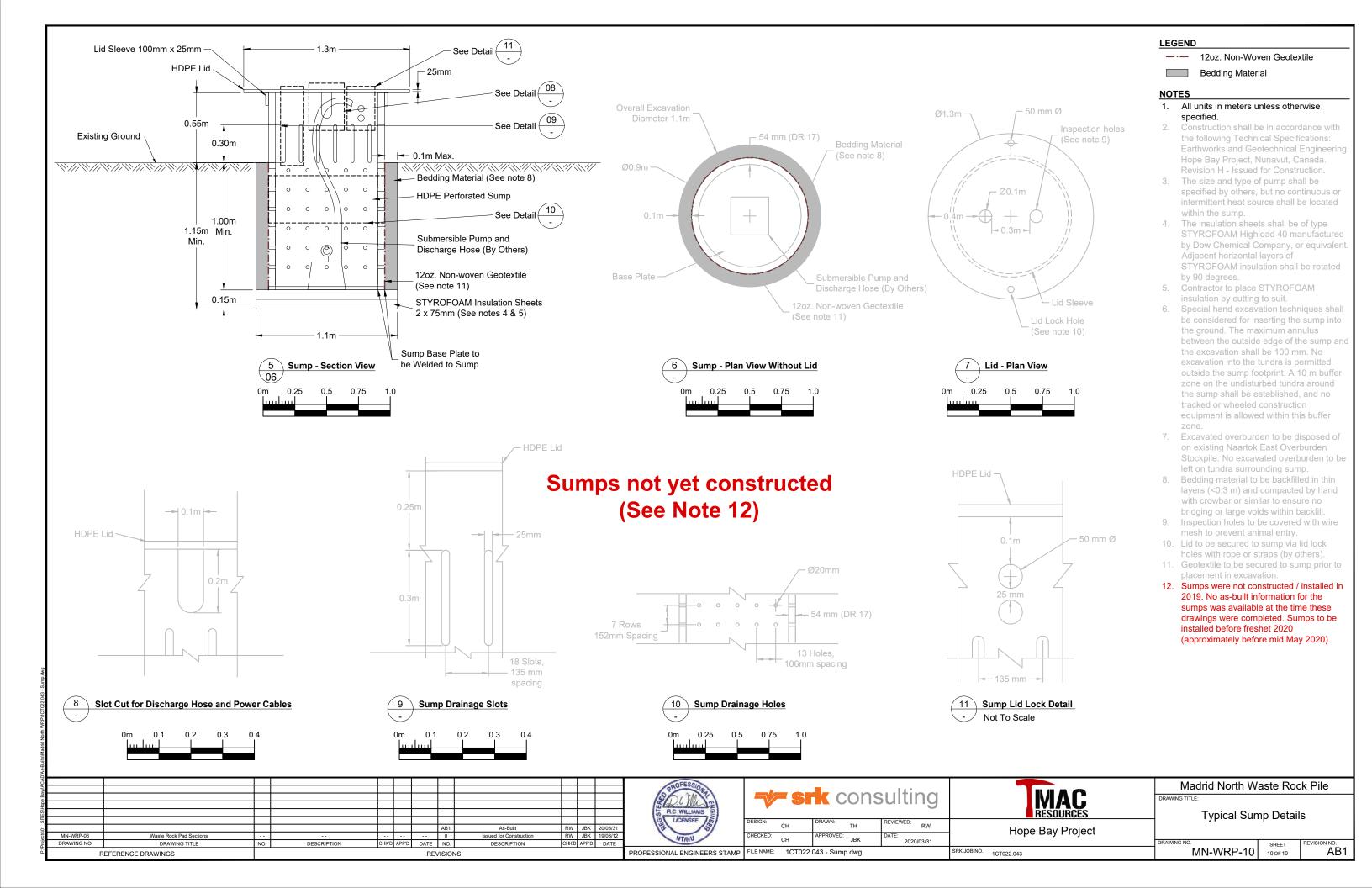


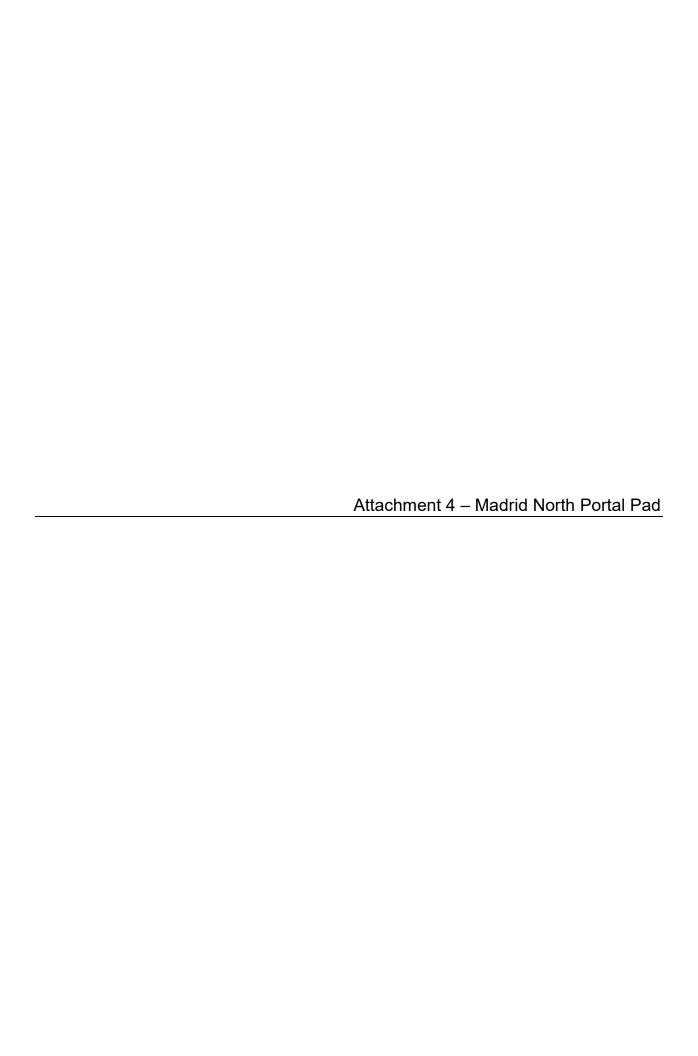














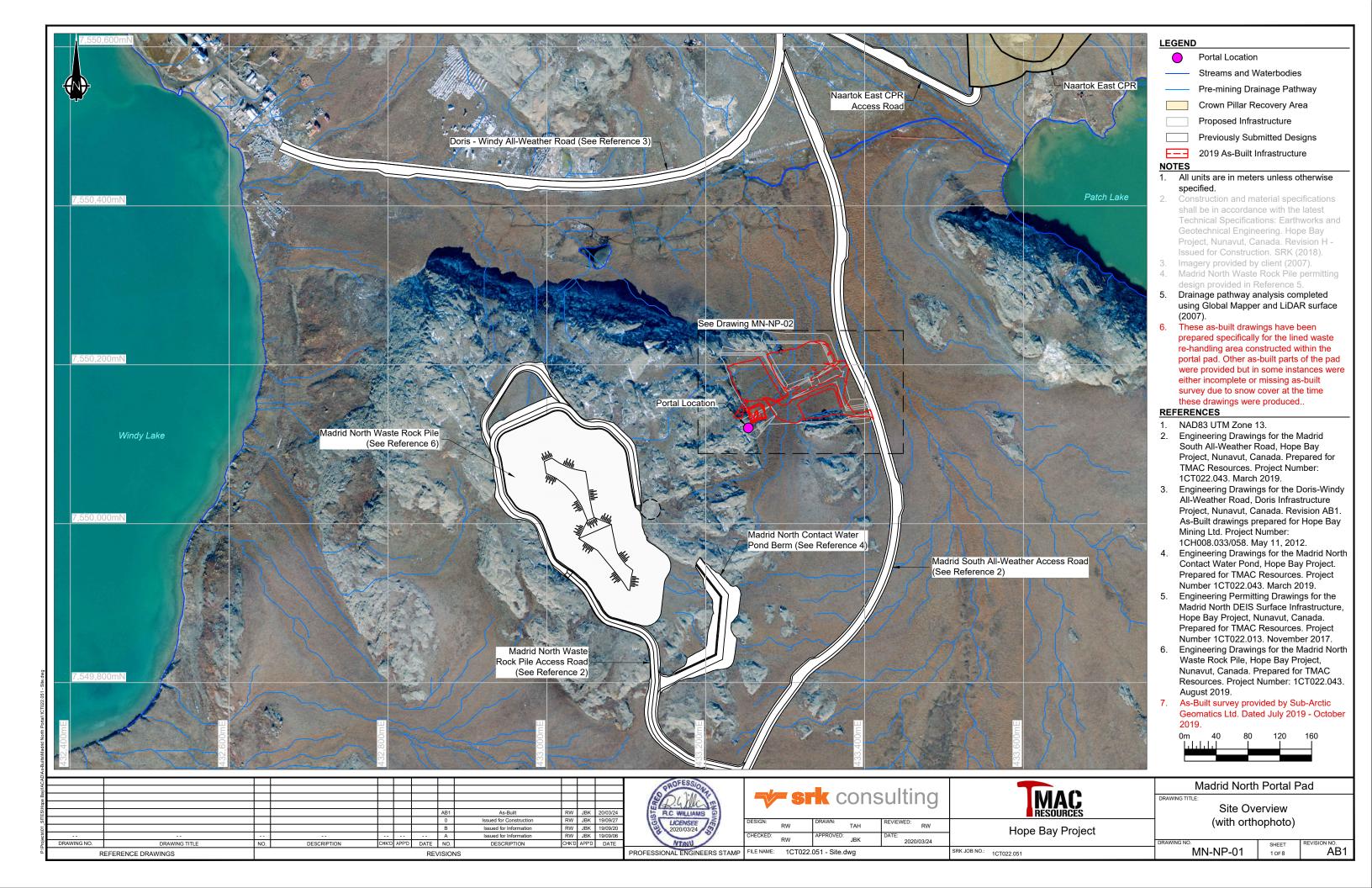
Engineering Drawings for the Lined Waste Re-handling Area at the Madrid North Portal Pad Hope Bay Project, Nunavut, Canada

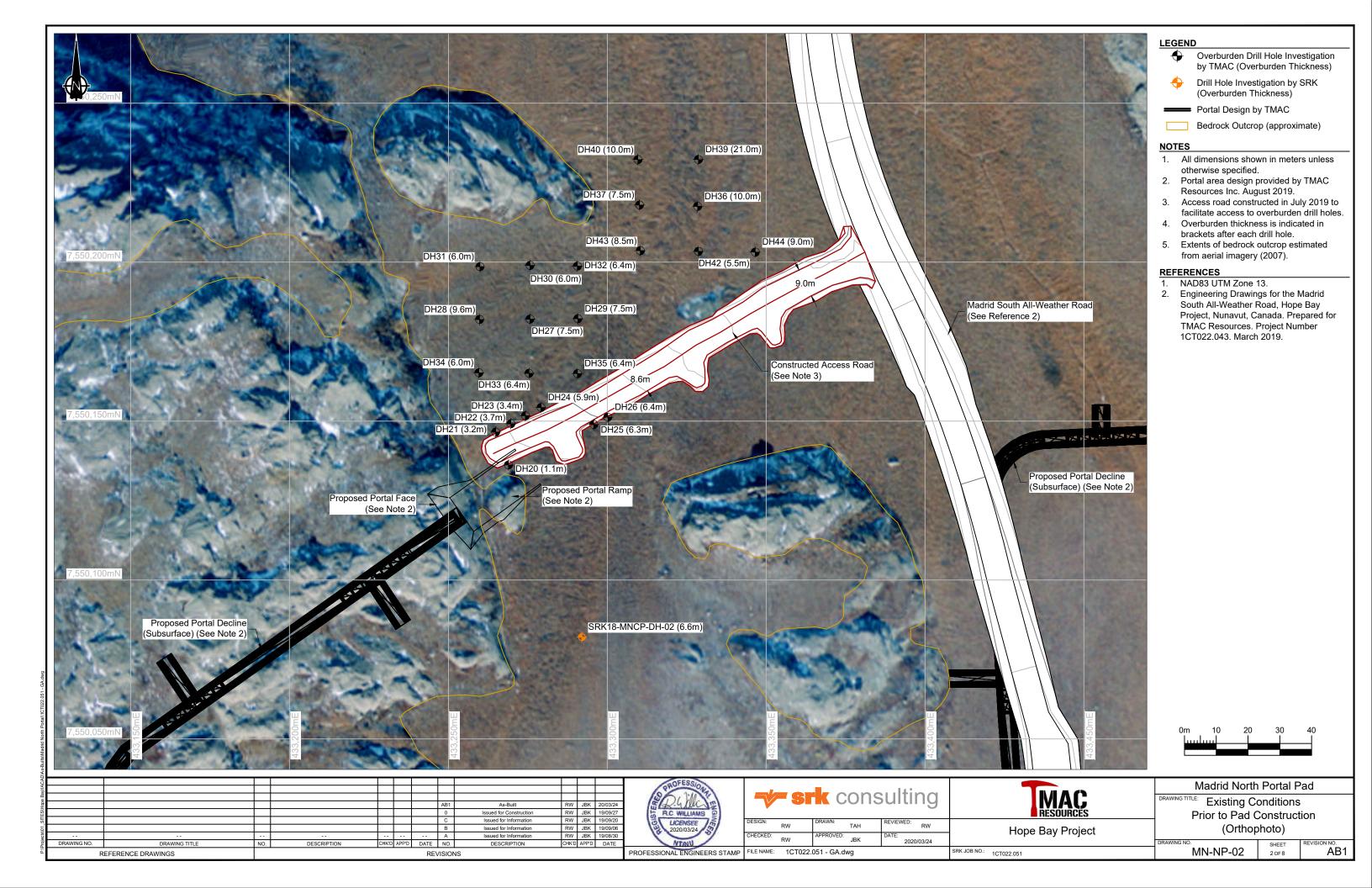
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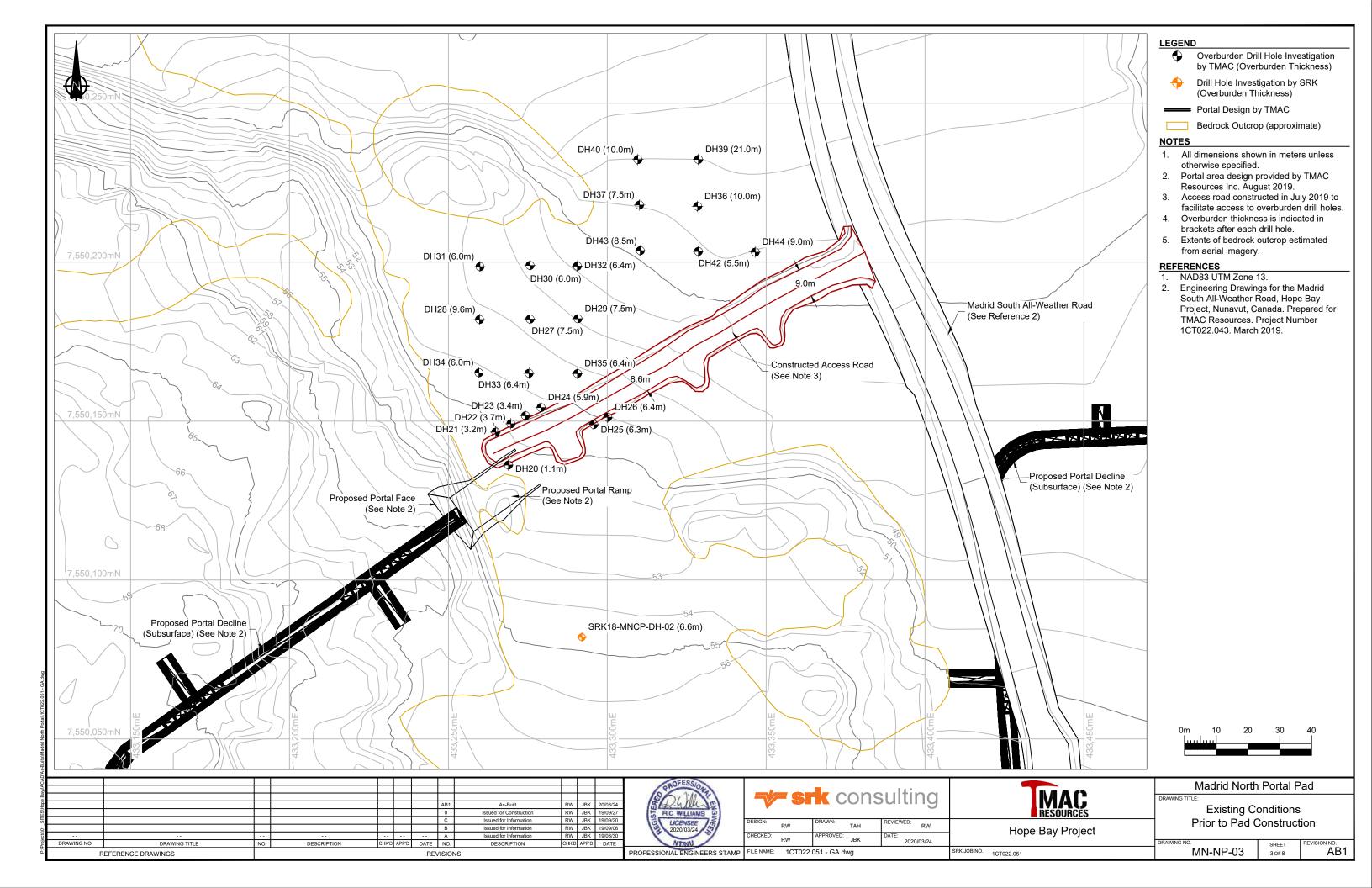
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MN-NP-03	Existing Conditions Prior to Pad Construction	As-Built	2020/03/24	AB1
MN-NP-04	Portal Pad Infrastructure	As-Built	2020/03/24	AB1
MN-NP-05	Portal Pad Infrastructure Sections Sheet 1/2	As-Built	2020/03/24	AB1
MN-NP-06	Portal Pad Infrastructure Sections Sheet 2/2	As-Built	2020/03/24	AB1
MN-NP-07	Typical Details	As-Built	2020/03/24	AB1
MN-NP-08	As-Built Liner Layout	As-Built	2020/03/24	AB1

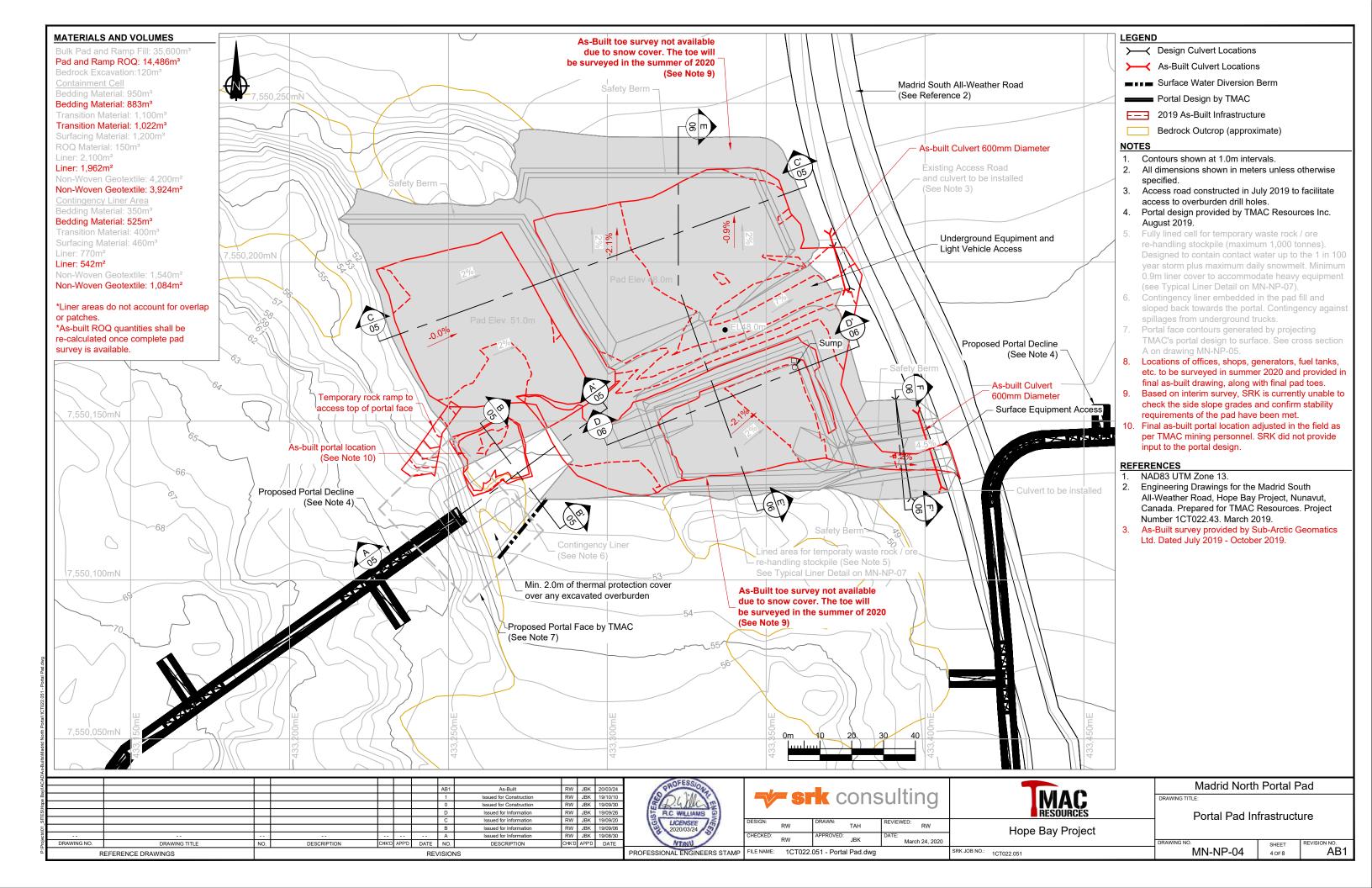


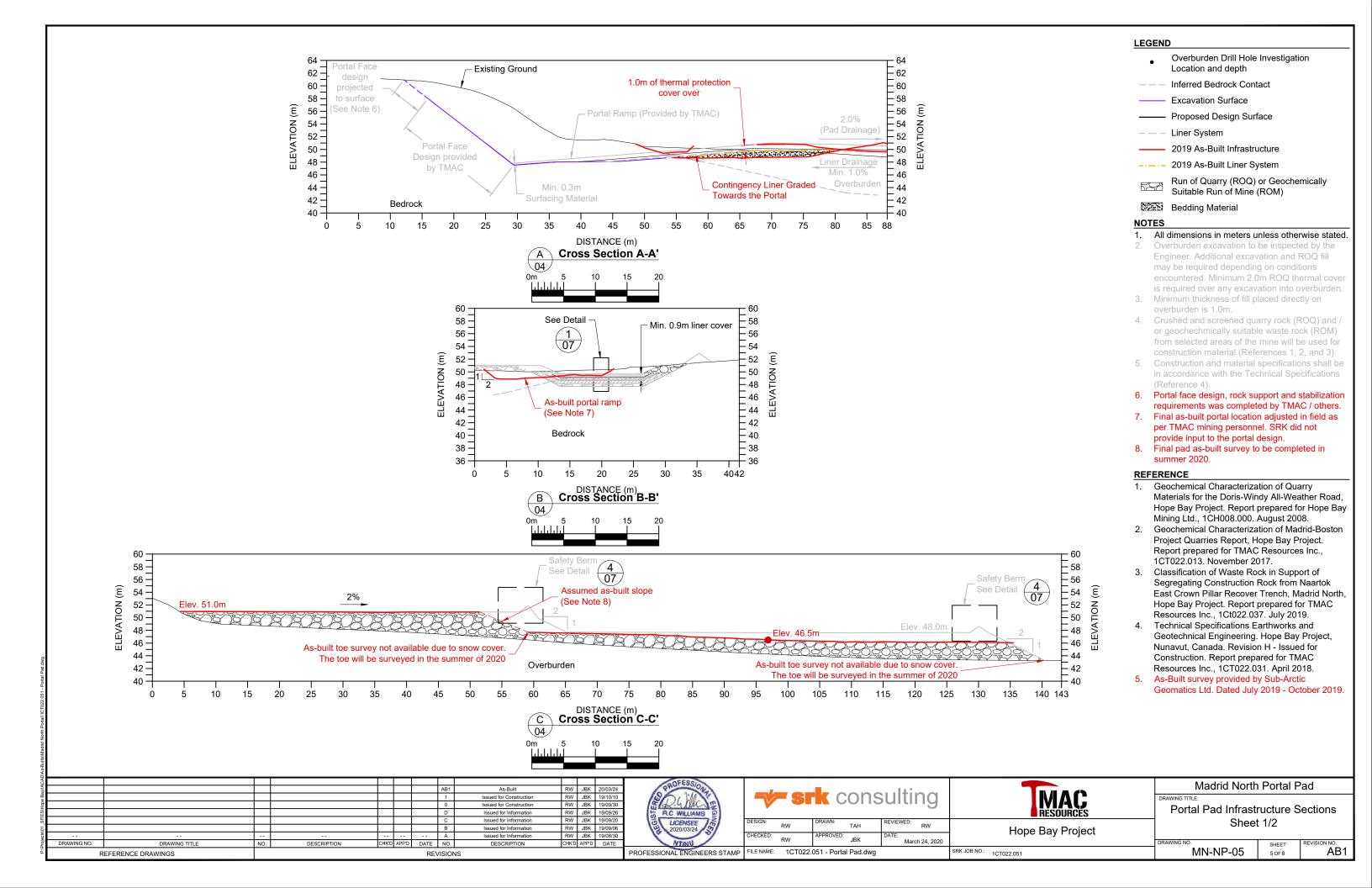


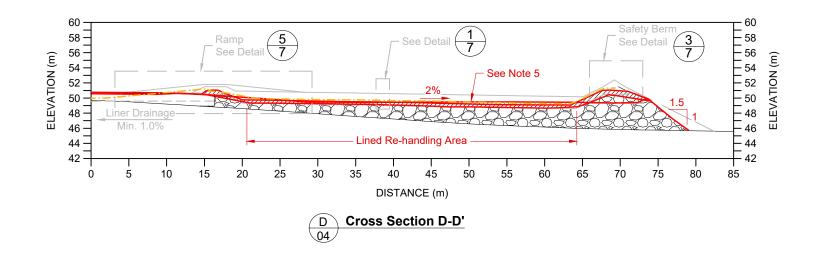


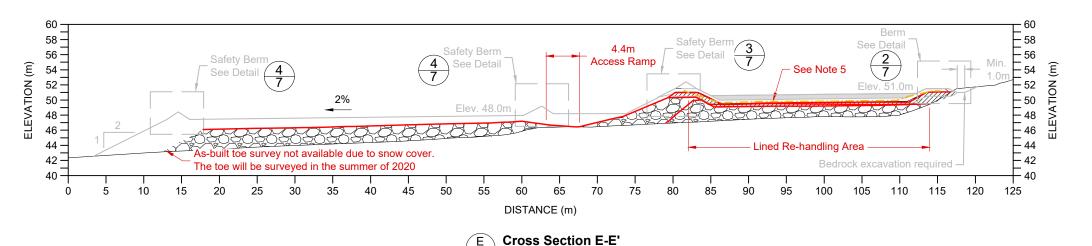




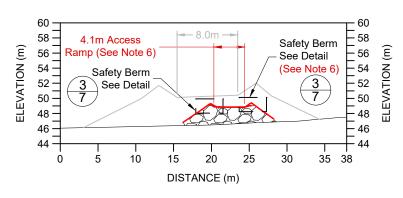








04



Cross Section F-F'

1CT022.051 - Portal Pad.dwg

MAC Hope Bay Project

— Proposed Design Surface

- - Liner System

LEGEND

Excavation Surface

- 2019 As-Built Infrastructure ---- 2019 As-Built Liner System

Run of Quarry (ROQ) or Geochemically

Suitable Run of Mine (ROM)

Surfacing Material

Transition Material

NOTES

- All dimensions in meters unless otherwise stated.
- Minimum thickness of fill placed directly on overburden is 1.0m.
- Crushed and screened quarry rock (ROQ) and/or geochemically suitable waste rock (ROM) from selected areas of the mine wil be used for construction material (References 1, 2, and 3).
- Construction and material specifications shall be in accordance with the Technical Specifications (Reference 4)
- As-built data above liner was unavailable at the time of as-built drawing updates.
- As-built survey of access road from waste re-handling area to Madrid South All-Weather Road to be re-surveyed in summer 2020 to confirm minimum design road width and berm heights are met.

REFERENCE

- 1. Geochemical Characterization of Quarry Materials for the Doris-Windy All-Weather Road, Hope Bay Project. Report prepared for Hope Bay Mining Ltd., 1CH008.000. August 2008.
- 2. Geochemical Characterization of Madrid-Boston Project Quarries Report, Hope Bay Project. Report prepared for TMAC Resources Inc., 1CT022.013. November 2017.
- Classification of Waste Rock in Support of Segregating Construction Rock from Naartok East Crown Pillar Recover Trench, Madrid North, Hope Bay Project. Report prepared for TMAC Resources Inc. 1Ct022.037. July 2019.
- Technical Specifications Earthworks and Geotechnical Engineering. Hope Bay Project, Nunavut, Canada. Revision H -Issued for Construction. Report prepared for TMAC Resources Inc., 1CT022.031. April 2018.
- As-Built survey provided by Sub-Arctic Geomatics Ltd. Dated July 2019 - October 2019.

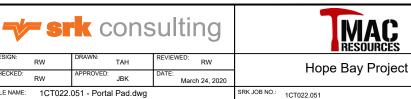


As-Built RW JBK 20/03/24 Issued for Construction RW JBK 19/09/30 RW JBK 19/09/20 RW JBK 19/09/06 Issued for Information RW JBK 19/08/30

REVISIONS

REFERENCE DRAWINGS



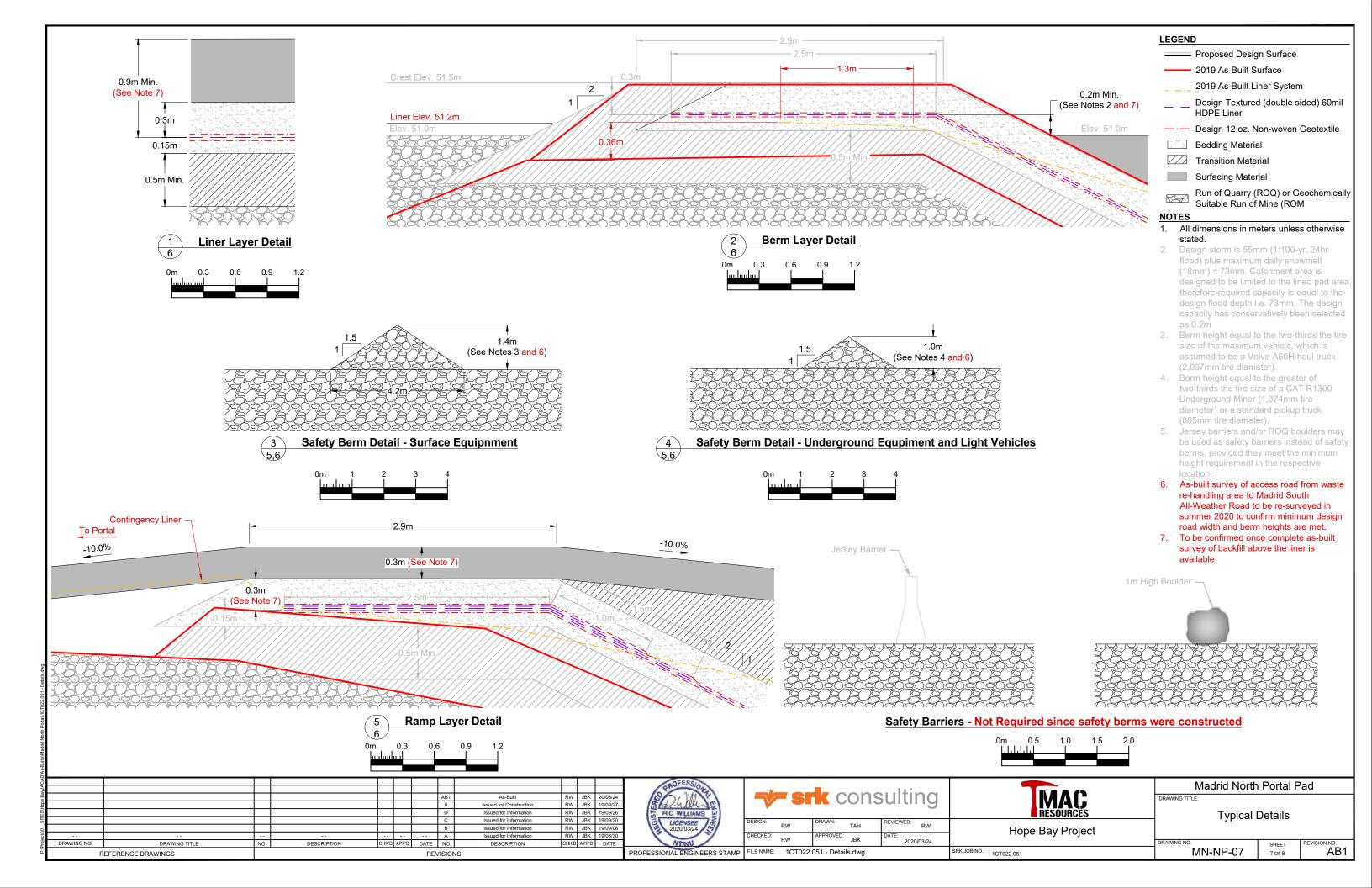


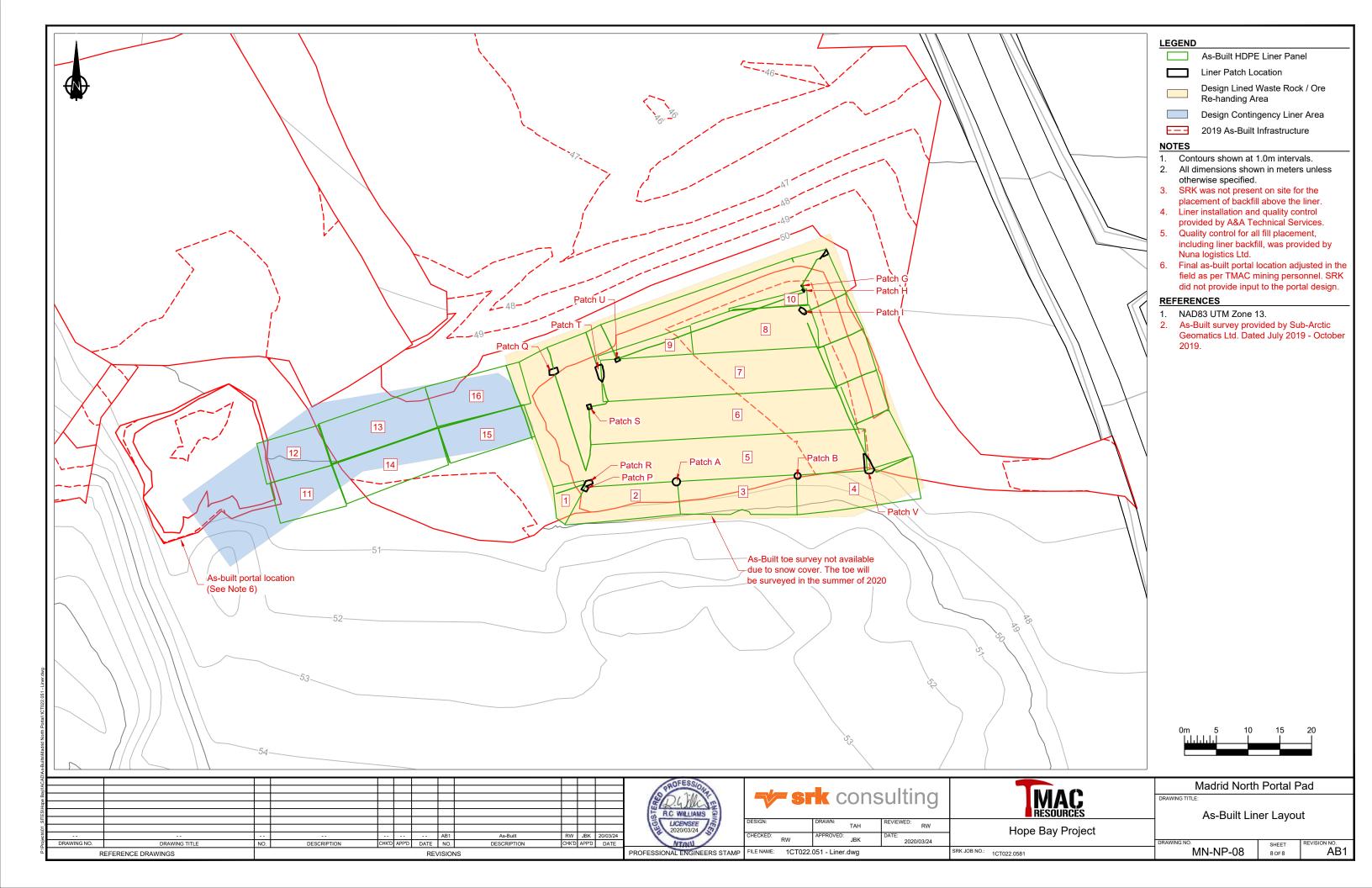
Madrid North Portal Pad

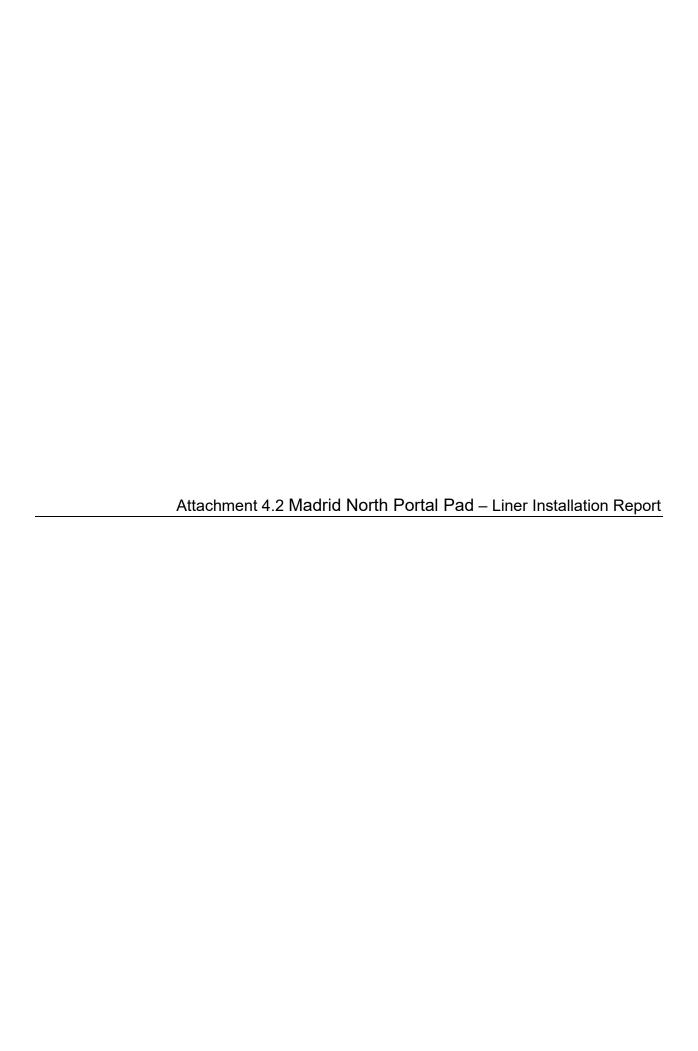
Portal Pad Infrastructure Sections Sheet 2/2

MN-NP-06

AB1



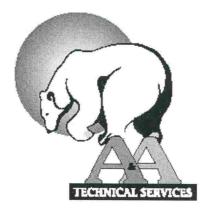




A&A Technical Services Yellowknife NT

Madrid Portal 60mil HDPE liner Installation October 23 – November 7, 2019

Client: TMAC Resources Inc.



A&A Technical Services Yellowknife NT

Madrid Portal 60mil HDPE liner Installation October 23 – November 7, 2019

Client: TMAC Resources Inc.

August 25 – Oct 5, 2019

Page	Table of contents
1	Panel and seam layout drawing
2-5	Daily wedge/extrusion welder qualification data and destruct sample test results.
6	Non-destructive air pressure test data
7-8	International Association of Geosynthetic Installers (IAGI) Certified Welding Technician (CWT) certificates.
9	A&A Technical Services subgrade acceptance and warranty.



A&A Technical Services Yellowknife NT

TMAC Resources Inc. Hope Bay NT Madrid Portal HDPE Liner

 $60 mil\ textured\ HDPE\ sandwiched\ between\ 540 g/m2\ non-woven\ geotextile.$ Panel layout drawing.



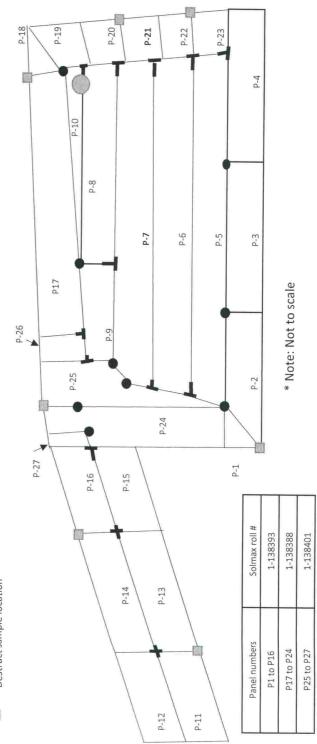
Extrusion Patch

Z

Extrusion T weld

Sump

Destruct sample location



A&A Technical Services Yellowknife NT October 22- November 7, 2019

TMAC Resources

Hope Bay - Madrid Portal Solmax 60mil single textured HDPE Daily wedge and extrusion welder qualification and destruct sample tests.

Test welds were completed in ambient conditions at site. Peel and shear tests were conducted inside Madrid office trailer at room temperature for prequalification of welders and destruct test samples.

Portable hoarding was used for all wedge and extrusion welding.

Wedge Welder - Demtech Prowedge #2

Tech: AH

Peel strength Temp. 420 C Speed 1.8m minute

27-Oct-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	142	145	91
2	138	136	91
3	145	142	91
4	139	141	91
27-Oct-19	Shear Strength		Minimum ppi (lbs/inch)
1	160		120
2	162		120

Wedge Welder - Demtech Prowedge #2

Tech: AH

Peel strength Temp. 420 C Speed 1.8m minute

30-Oct-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	144	135	91
2	146	150	91
3	148	151	91
4	146	149	91
30-Oct-19	Shear Strength		Minimum ppi (lbs/inch)
1	161		120
2	160		120

Wedge Welder - Demtech Prowedge #2

Tech: GH

Peel strength Temp. 420 C Speed 1.8m minute

02-Nov-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	140	145	91
2	142	150	91
3	139	142	91
4	135	142	91
02-Nov-19	Shear Strength		Minimum ppi (lbs/inch)
1	161		120
2	164		120

Wedge Welder - Demtech Prowedge #2

Tech: GH

Peel strength Temp. 420 C Speed 1.8m minute

8 - John 120 C Speed From Himate			
03-Nov-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	137	141	91
2	139	145	91
3	144	138	91
4	140	148	91
03-Nov-19	Shear Strength		Minimum ppi (lbs/inch)
1	159		120
2	162		120



Wedge Welder - Demtech Prowedge #2

Tech: GH

Peel strength Temp. 420 C Speed 1.8m minute

04-Nov-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	139	148	91
2	144	140	91
3	142	139	91
4	136	139	91
04-Nov-19	Shear Strength		Minimum ppi (lbs/inch)
1	155		120
2	155		120

Extrusion welder daily prequalifications

Extrusion Welder- ProXL #1

Tech: AH

Hot air temp: - 260 C Extrudite temp: 250 C

27-Oct-19	Peel strength	Minimum ppi (lbs/inch)
1	138	78
2	144	78
3	132	78
4	138	78
27-Oct-19	Shear Strength	
1	158	120
2	152	120

Extrusion Welder- ProXL #1

Tech: GH

Hot air temp: - 260 C Extrudite temp: 250 C

I HE S WALL SELL	.p. 200 c	Extradite temp. 2	30 0
30-Oct-19	Peel strength		Minimum ppi (lbs/inch)
1	141	144	78
2	132	135	78
3	140	131	78
4	136	136	78
30-Oct-19	Shear Strength		
1	158		120
2	160		120

Extrusion Welder- ProXL #1

Tech: GH

Hot air temp: - 260 C

Extrudite temp: 250 C

03-Nov-19	Peel strength	Minimum ppi (lbs/inch)
1	145	78
2	139	78
3	140	78
4	132	78
03-Nov-19	Shear Strength	
1	152	120
2	158	120



Destruct samples from wedge weld seams

Location: Top of seam panels 1 and 2.

Tech: AH

Peel strength

27.0 -+ 10	hand the second of	0	The state of the s
27-Oct-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	137	139	91
2	139	134	91
3	142	141	91
4	140	144	91
27-Oct-19			Minimum ppi (lbs/inch)
1	157		120
2	163		120

Location: S end of seam P11 and P13

Tech: AH

Peel strength Temp. 420 C Speed 1.8m minute

30-Oct-19 Inside weld		Outside weld	Minimum ppi (lbs/inch)	
1	144	139	91	
2	138	135	91	
3	140	140	91	
4	144	140	91	
30-Oct-19	Shear Strength		Minimum ppi (lbs/inch)	
1	167		120	
2	155		120	

Location: N end of seam P14 and P16

Tech: AH

Peel strength

	i cer strength			
30-Oct-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)	
1	144	144	91	
2	144	138	91	
3	142	141	91	
4	136	145	91	
30-Oct-19	Shear Strength		Minimum ppi (lbs/inch)	
1	158		120	
2	162	120		

Location: Top of seam P17 and P-18

Tech: GH

Peel strength

	i cei sti ciigtii		
02-Nov-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	141	132	91
2	140	145	91
3	137	135	91
4	138	139	91
02-Nov-19	Shear Strength		Minimum ppi (lbs/inch)
1	161		120
2	157		120



Location: Top of seam P20 and P21

Tech: GH

Peel strength

02-Nov-19			Minimum ppi (lbs/inch)
1	139	144	91
2	142	141	91
3	136	139	91
4	137	135	91
02-Nov-19	Shear Strength		Minimum ppi (lbs/inch)
1	161		120
2	163		120

Location: Top of seam P22 and P23

Tech: GH

Peel strength

	. cer ou en gun		
02-Nov-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)
1	137	144	91
2	141	142	91
3	137	142	91
4	144	142	91
02-Nov-19	Shear Strength		Minimum ppi (lbs/inch)
1	154		120
2	159		120

Location : Top of seam P24 and P25

Tech: GH

Peel strength

03-Nov-19	Inside weld	Outside weld	Minimum ppi (lbs/inch)	
1	144	142	91	
2	142	145	91	
3	134	144	91	
4	149	142	91	
03-Nov-19	Shear Strength		Minimum ppi (lbs/inch)	
1	160		120	
2	165	120		

A&A Technical Services Yellowknife NT October 22- November 7, 2019



TMAC Resources

Hope Bay - Madrid Portal Solmax 60mil single textured HDPE

Non - Destructive air pressure testing of dual wedge weld seams. $% \label{eq:control_eq} % \label{eq:control_eq}$

Minimum 30 psi over 5 minutes with less than 4 psi loss in pressure = Pass

		5 minutes w		T		= Pass
Date	Technician	Seam location	Start psi	Finish psi	Pass/Fail	Comments
Oct. 27/19	AH	P-1 - P-2	30	30	Pass	
Oct. 27/19	AH	P-2 - P-3	35	35	Pass	
Oct. 27/19	AH	P-3- P-4	35	35	Pass	
Oct. 27/19	AH	P-4 - P-5	35	35	Pass	
Oct. 27/19	АН	P-3 - P-5	35	35	Pass	
Oct. 27/19	AH	P-2 - P-5	35	35	Pass	
Oct. 27/19	АН	P-8 -P-9	34	34	Pass	
Oct. 27/19	АН	P-5-P-6	34	34	Pass	
Oct. 27/19	AH	P-6-P-7	35	35	Pass	
Oct. 27/19	АН	P-7 - P-8	35	35	Pass	
Oct. 27/19	АН	P-8 P-10	35	35	Pass	
	Due to broke	n valve on air ta	ank, weather a	and time cons	traints additio	nal destruct samples
	were taken f	rom wedge we	ld seams inste	ad of air testin	ıg.	
		·				
						page 6

CERTIFIED WELDING TECHNICIAN



The International Association of Geosynthetic Installers Certifies:

ALAN HARMAN

hands-on skills, knowledge and experience in the welding and installation of polyethylene (PE) geomembranes, and As a Certified Welding Technician, in polyethylene wedge and extrusion welding, having demonstrated superior having basic mechanical aptitude for working with welders and equipment on the job site.

Registration number: CWT162010

Valid 07 June 2016 — 07 June 2021

International Association of Geosynthetic Installers

President, IAGI

1 1 1

Managing Director, IAG

CERTIFIED WELDING TECHNICIAN



The International Association of Geosynthetic Installers Certifies

GUY HORESAY

hands-on skills, knowledge and experience in the welding and installation of polyethylene (PE) geomembranes, and As a Certified Welding Technician, in polyethylene wedge and extrusion welding, having demonstrated superior having basic mechanical aptitude for working with welders and equipment on the job site. Registration number: CWT170010

Valid 07 June 2016 — 07 June 2021

International Association of Geographics Installers

President, IAGI

Man of the state o

Managing Director, IAGI

A&A Technical Services Yellowknife NT

Madrid Portal 60mil HDPE liner Installation October 23 - November 7, 2019

Client: TMAC Resouces Inc.

Upon arrival to site the containment area to be lined was thoroughly inspected by A&A Technical Services installation supervisor and deemed to be a suitable surface on which to place the HDPE lining system. The Solmax 60 mil single textured HDPE was sandwiched between two layers of 540g/m2 non-woven geotextile. The lining system was placed over a compacted layer of -25mm crushed rock and backfilled with the same.

Warranties issued by A&A Technical Services shall cover only the cost of replacement and/or repair of defective installations, determined or agreed to be the responsibility of A&A Technical Services, provide that the warranty work will be performed to the same standards and scope of work set out in the contract documents. A&A's installation warranty shall commence upon acceptance of the individual geosynthetic components by the owner or its representative as such components are completed. The installation warranty period shall not exceed beyond 1 years. Our installation warranty is rendered null and void if the installed geosynthetics are subject to abuse by machinery, equipment or personnel not under the control of A&A. harmful chemicals or unusual weather conditions or catastrophic earthworks failures.

A&A Technical Services shall not be held liable for defects, damage and/or deficient materials and installations, either in whole or in part should the defects, damage or deficient materials and installations arise as the result from the use of poor quality and inappropriate or unsuitable earthworks material or site preparation. This limitation of liability extends to improper and/or construction techniques, and methods and equipment used to create the earthworks covering all or any portion of the completed geosynthetic installation.

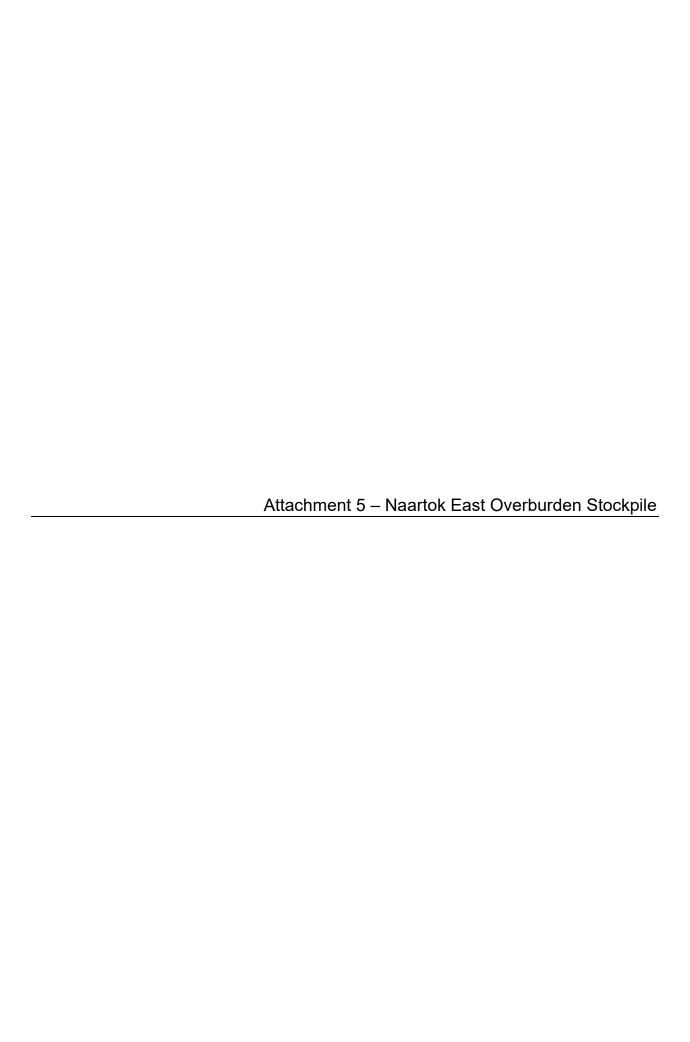
Signed: Olon Harran

Dated: November 7, 2019

Al Harman President

A&A Technical Services

Yellowknife NT







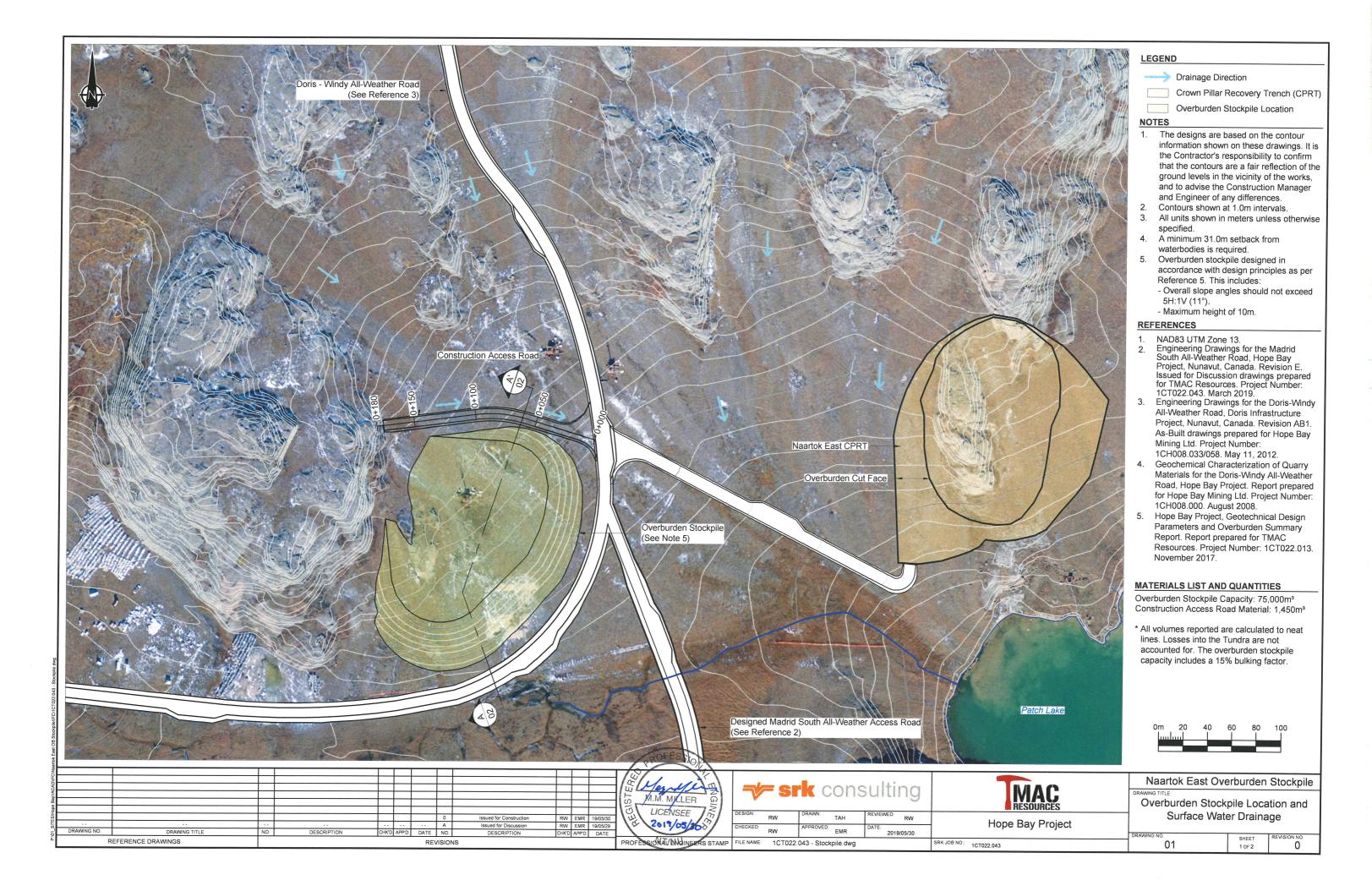
TMAC Resources Inc. Madrid North Project Naartok East CPRT Overburden Stockpile

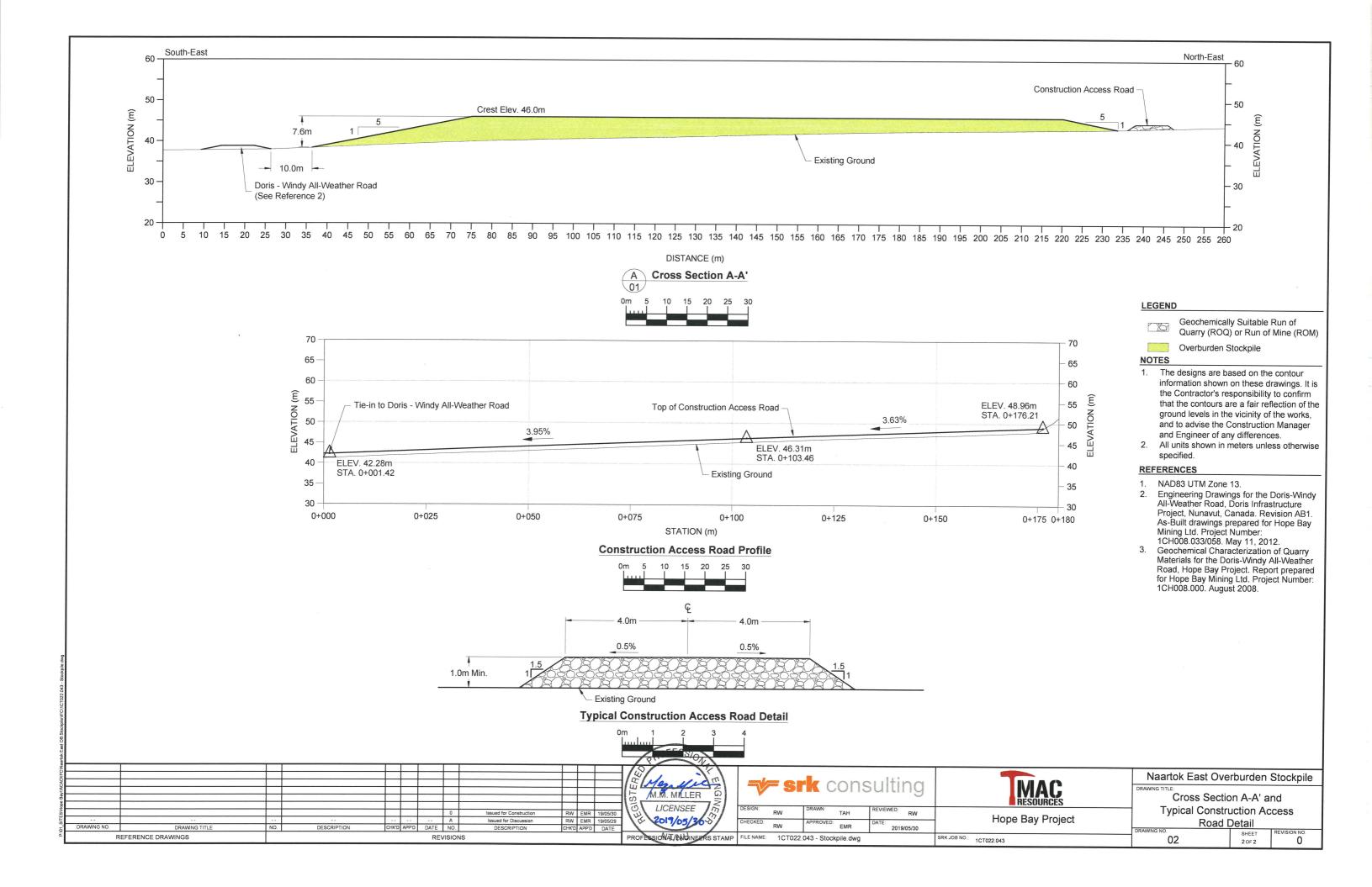
Drawing Number	Drawing Title	Issue	Date	Revision
01	Overburden Stockpile Location and Surface Water Drainage	Issued for Construction	2019/05/30	0
02	Cross Section A-A' and Typical Construction Access Road Detail	Issued for Construction	2019/05/30	0

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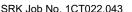
TMAC Resources Inc.

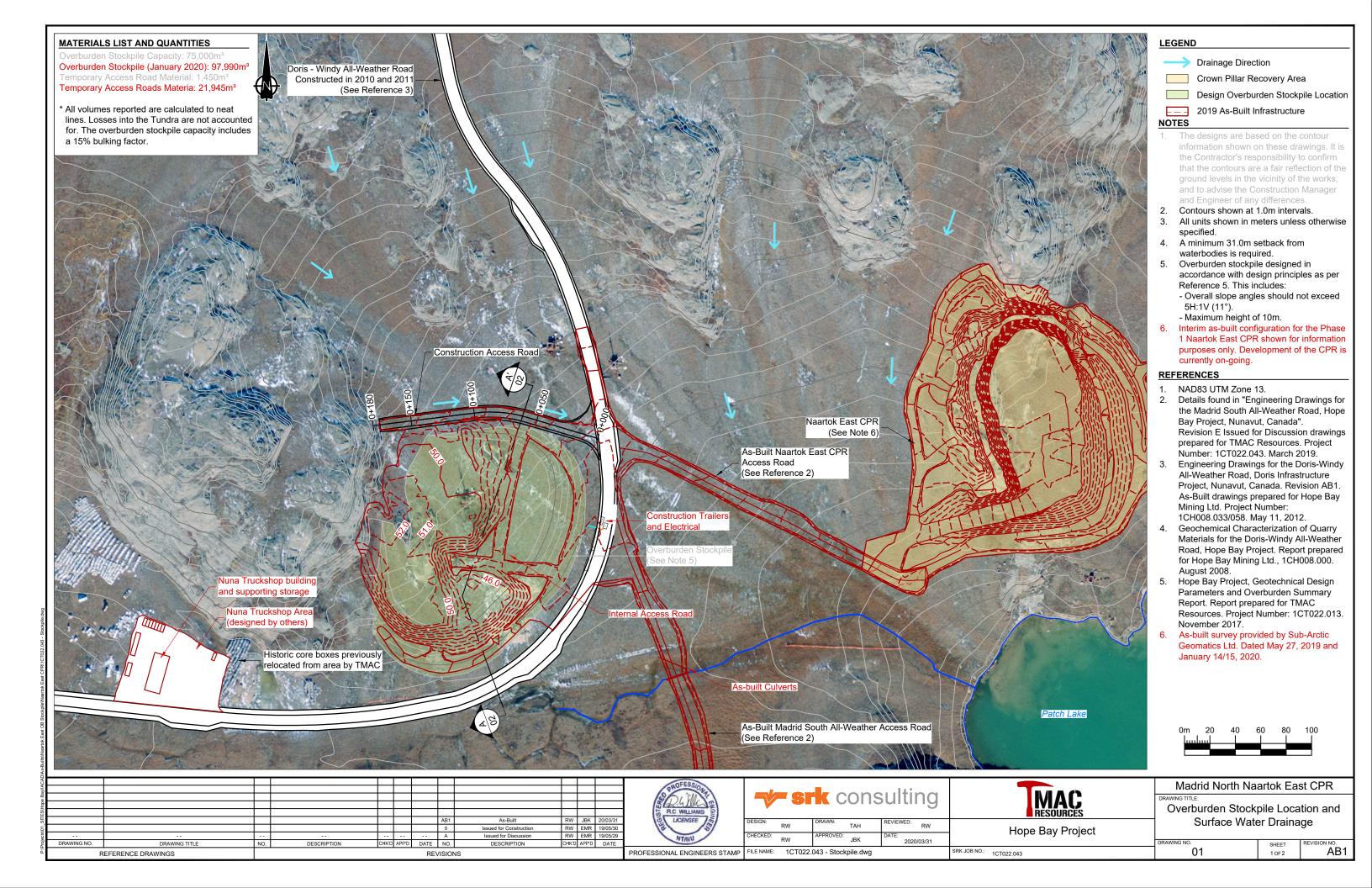
Engineering Drawings for the Naartok East Crown Pillar Recovery (CPR) Overburden Stockpile at Madrid North, Hope Bay Project, Nunavut, Canada

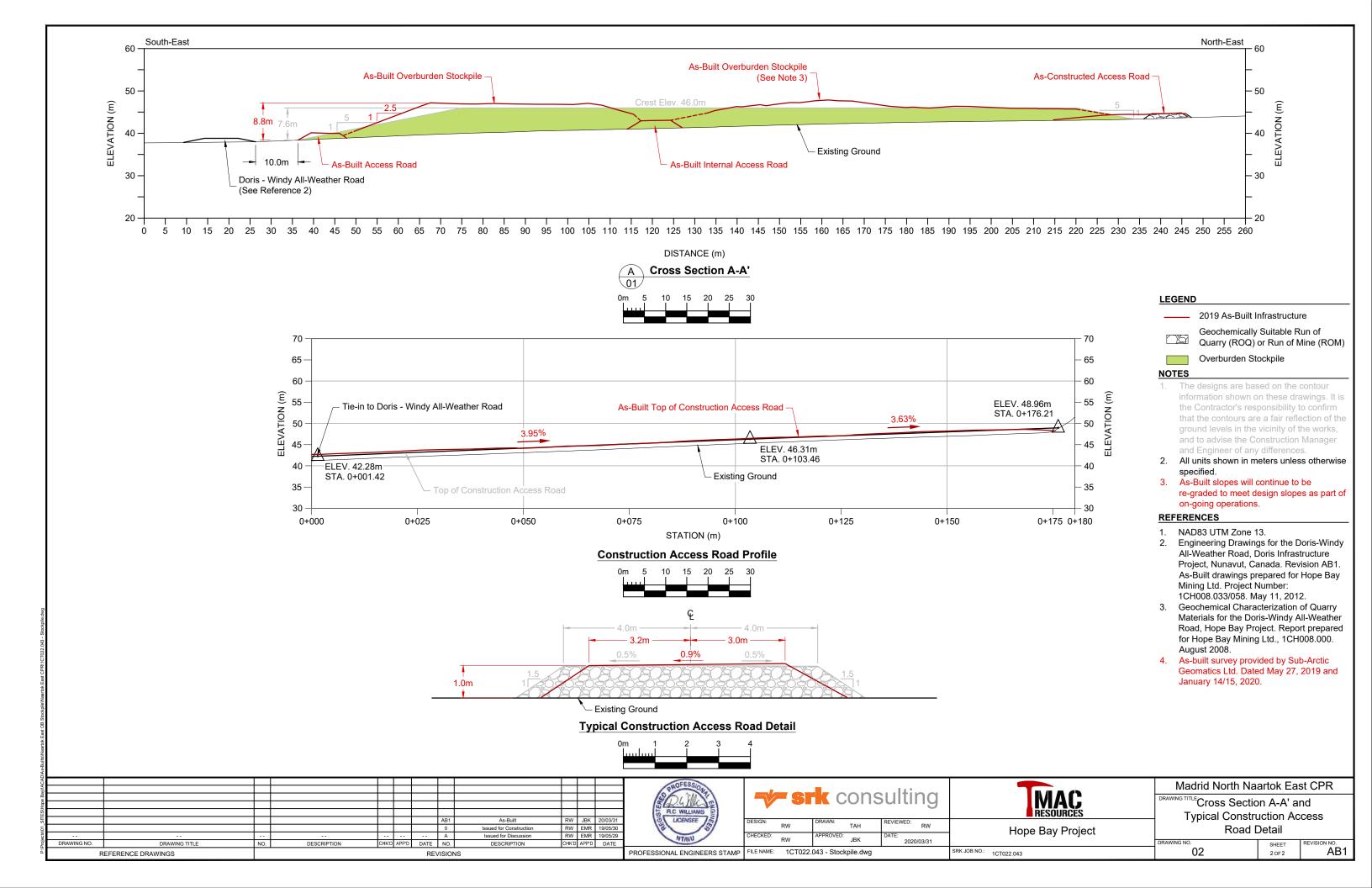
Drawing Number	Drawing Title	Issue	Date	Revision
01	Overburden Stockpile Location and Surface Water Drainage	As-Built	2020/03/31	AB1
02	Cross Section A-A' and Typical Construction Access Road Detail	As-Built	2020/03/31	AB1

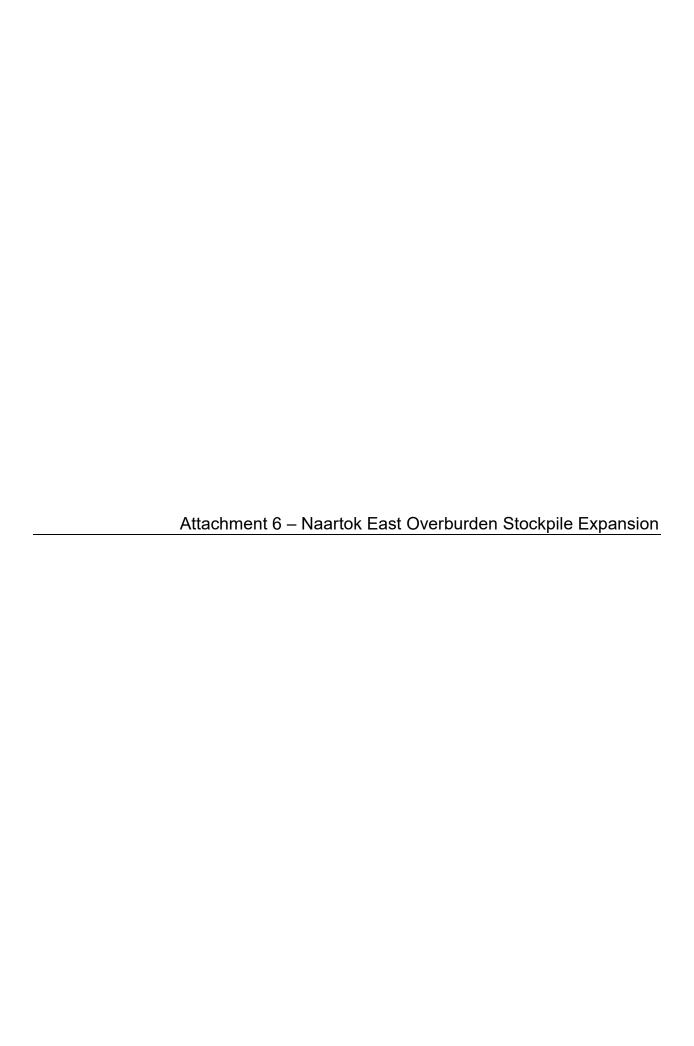
















TMAC Resources Inc. Madrid North Project Naartok East CPR Overburden Stockpile

Drawing Number	Drawing Title	Issue	Date	Revision
MN-OB-01	Overburden Stockpile Expansion Drainage Pathways	Issued for Construction	12/19/2019	0
MN-OB-02	Overburden Stockpile Expansion Location and Primary Filter Berm	Issued for Construction	12/19/2019	0
MN-OB-03	Cross Section A-A' and Typical Construction Access Road Centerline Profile	Issued for Construction	12/19/2019	0
MN-OB-04	Typical Details	Issued for Construction	12/19/2019	0

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2019/12/19

D. Willi



