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By Licence Administrator at 4:00 pm, Jun 02, 2011

## Memo

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<b>To:</b>	Chris Hanks	<b>Date:</b>	May 11, 2011
<b>Company:</b>	HBML	<b>From:</b>	Kelly Sexsmith
<b>Copy to:</b>		<b>Project #:</b>	1CH008.043.3600
<b>Subject:</b>	Formal SRK Responses to KIA Requests		

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SRK has prepared this memo in response to questions and comments by the Kitikmeot Inuit Association (KIA) on the "Hope Bay Doris North Waste Rock and Ore Management Plan" dated December 2010. The original comments by KIA and SRKs responses are provided below.

**KIA Question #1:** *Construction details (preferably "as-built" drawing) regarding the pollution control system, i.e., the waste rock and ore storage pads, and the sedimentation and pollution control ponds. These details would provide clarity regarding the adequacy and effectiveness of the system, such as, for example, the exchange between the sedimentation pond and the pollution control pond, the extent of coverage of the HDPE liner, etc., etc.*

**SRK response:** The IFC Drawings for the Sedimentation and Pollution Control Ponds as well as the waste rock storage pads are provided in Attachment 1. There are separate documents detailing both interim and operational water management plans for the site. Any comments on the design or operation of those facilities are should be provided in the context of the Water Management Plans.

**KIA Question #2:** *An estimate is requested regarding the maximum storage capacity of the waste rock pad based on maintaining stable slopes.*

**SRK response:** The expected volume of waste rock that is expected to be stored in the waste rock piles is 530,000 tonnes, including an allowance for the 30,000 tonnes of waste rock that will be displaced from use as backfill by cyanide residues. There is an additional contingency of 50,000 tonnes provided to address any small changes in the mine plans. The storage capacity of the pile, as shown in Figure 5c, is 601,000 tonnes. A five metre increase in height is possible in the eastern side of the pile, and could provide another 18,000 tonnes, or maximum storage capacity of 619,000 tonnes.

The storage capacities are based on an overall final slope angle of 2H:1V. A technical memo documenting the pile stability analysis for the 619,000 tonne pile configuration is provided in Attachment 2.

**KIA Question #3:** *Details and justification are needed regarding the characterization, classification and segregation of waste rock. More specifically:*

- The proposed two categories (mineralized and non-mineralized rock) to classify waste rock do not necessarily correspond to PAG and Non-PAG rock*
- Will visual inspection, based on presence of sulphides, be sufficient to adequately characterize the waste rock?*
- No specific testing was proposed to be carried out on waste rock material designated for use in construction. The material needs be assessed for metal leaching potential, and the environmental impacts of the soluble metals species must be considered.*

**SRK Response:** SRK presented an up-to-date summary of the geochemical characterization data available for the Doris North waste rock in Section 2.5.1 and Appendix 1 of the waste rock management plan. This

includes a statistical summary of critical ABA parameters according to the proposed management units, some key findings on carbonate mineralogy from the mineralogical testing, and some key findings on sulphate and metal release rates from the kinetic testing. HBML plans to issue a complete update on this testwork with the upcoming Amendment 3 package. In the interim, a meeting was held between HBML, KIA, BGC (KIA's technical consultant), and SRK on various aspects of the Hope Bay project. Key findings of the waste rock characterization programs were presented and discussed during the meeting, and KIA were provided with an opportunity to ask further questions. With respect to the specific comments provided by KIA prior to that meeting, we are providing the following responses:

a: In developing the Waste Rock Management Plan, SRK has recommended a more conservative segregation criteria that not only recognizes the ARD potential of the rock, but also the potential for increased risk of sulphate and metal leaching in non-PAG rocks that contain elevated levels of both sulphide and carbonate. Based on the segregation criteria proposed in the management plan, all of the "non-mineralized" rock is expected to be non-PAG with low (<0.5%)<sup>1</sup> sulphide concentrations, while the "mineralized" rock is expected to include: 1) any rock with >0.5% sulphides, regardless of ARD potential, 2) any rock that is not spatially abundant, and therefore not well characterized or understood (e.g. the gabbro), and 3) any rock that is located in the alteration zones that surround the ore, regardless of actual sulphide content or ARD potential due to the anticipated challenges of separating rock in this area. Therefore, the "mineralized" rock will include all of the PAG and non-PAG material with "elevated" sulphides. This degree of conservatism is appropriate given that there is more than excess capacity for all of the mineralized rock to be backfilled in the underground mine.

Additionally, the segregation plan takes considers the characteristics of each of the major rock units, and is only proposed for units that are well characterized and that consistently contain large amounts of NP, such as the basalt and the buffer zone material (also basalt). The high NP content ensures that even if the segregation was not complete, there would still be an extremely low risk of ARD. The characteristics of the diabase are very uniform as would be expected for an intrusive volcanic that post-dates the mineralization that is associated with the gold deposits, and therefore segregated as non-mineralized on the basis of rock type (but with visual inspections to confirm rock type and low sulphide content). The gabbro and any other minor rock units that have had more limited testing (for example other types of mafic dykes), are being handled as mineralized rock due to limited information and/or lower NP content.

b: Visual inspection has been proposed as the primary screening method for segregating the waste rock into mineralized and non-mineralized storage areas on the pile. The inspection will include both rock type and sulphide content. The geologists have been further instructed to classify rock as "mineralized" if there are any doubts as to their ability to see or estimate the abundance of sulphides. However, HBML would like to emphasize that samples are also being collected at a minimum frequency of one sample per 5,000 tonnes for acid-base accounting (ABA) testing, with additional samples providing a sampling density of one sample per 1,000 tonnes for total inorganic carbon (TIC) and total sulphur content in units that have a higher degree of variability such as basalt or alteration zone material. The final destination of these materials in the dump is also being tracked so that the laboratory results can be directly tied to specific locations within the non-mineralized area of the pile.

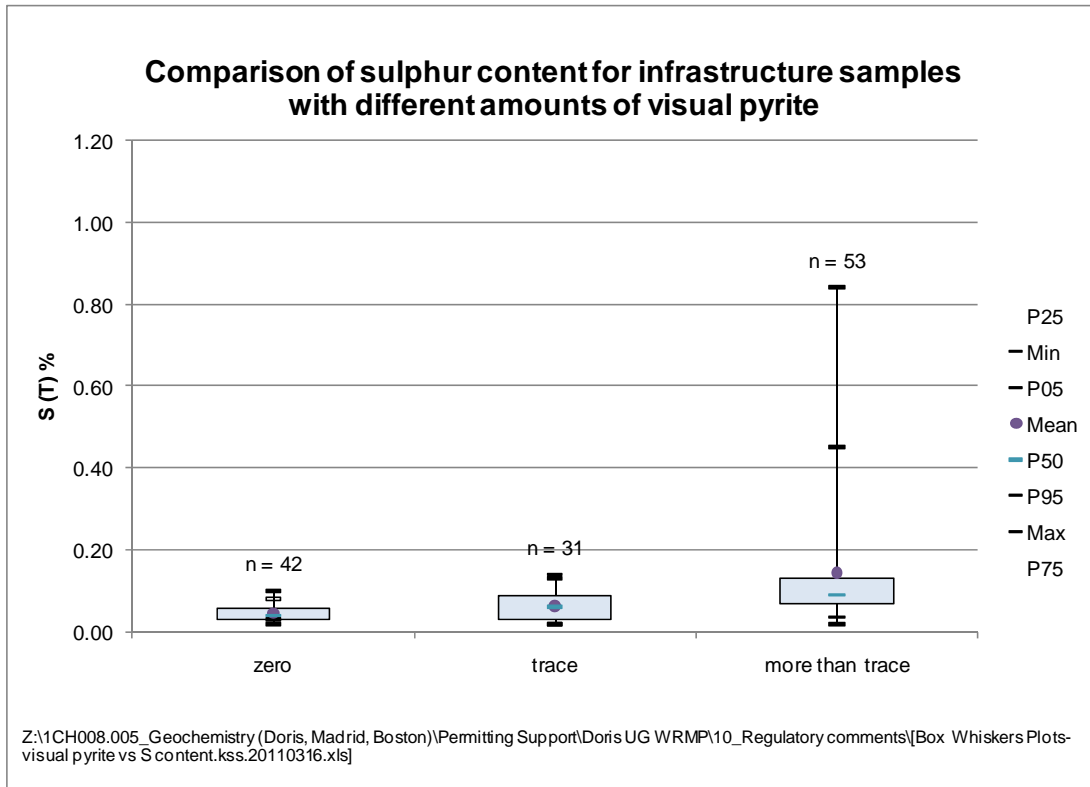
The laboratory results are closely reviewed on receipt and are being used to assess the accuracy of the visual determinations. Once sufficient data are available for samples representing a variety of rock types, HBML will assess the accuracy of the visual methods, and may propose further changes in the sampling plans. However, in the current plan, the laboratory data will be the primary basis for assessing whether the non-mineralized rock can be used in construction. If the segregation programs do not prove to be effective, and the non-mineralized rock does not meet the criteria considered acceptable for construction, then it will remain in the waste rock stockpile area until backfilling commences. In the event that small amounts of mineralized rock remain within the non-mineralized area of the pile at closure, the abundance of carbonate minerals present in all of the basalt rock units that would surround these materials is expected to provide an appropriate level of

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<sup>1</sup> Note that this corresponds to 0.25% sulphur content, or an acid potential of <8 kg CaCO<sub>3</sub> eq/tonne.

control for long term metal leaching and/or ARD issues. It is also likely that permafrost will aggrade into the rock, which would provide an additional level of control for these processes.

The assumption that visual methods would be an appropriate means of segregating the rock is based on correlations between observations of sulphides in the geological logs and corresponding test results in data from recent construction characterization programs (SRK 2010a-d) in which the site geologists were asked to pay particular attention to the visual estimates. As shown in Figure 1, samples with higher sulphur values based on laboratory testing corresponded to observations of more than trace amounts of pyrite.



**Figure 1: Comparison of sulphur content for infrastructure samples with different amounts of visual pyrite**

c: Specific testing of the material designated for use in construction has been proposed in the Waste Rock Management Plan. Section 3.3.3 describes the confirmatory testing programs and the criteria that will be used to establish whether the waste rock is suitable for use in construction. Section 5.3.2 described the additional testing that would be completed following construction. Section 2.5.1 describes the characterization testing that was completed on waste rock from the underground mining area prior to mining.

The confirmatory testing programs include acid base accounting tests (as described under response b and in the plan) and shake flask extraction tests to characterize the soluble sulphate and metal content of the rock, as well as any residual nutrients from blasting.

The pre-development characterization programs include kinetic testing and field barrel tests to characterize the soluble metal content. Rates of metal leaching in kinetic test samples with less than 0.5% sulphide are comparable to that of the quarry rock that is currently being used in construction (SRK 2007).

**KIA Question #4:** Details are needed regarding the projected volumes and proportions of the different waste rock management units (i.e., mineralized and non-mineralized waste rock). What is the contingency plan if there is excess “mineralized” waste rock (i.e., exceeding backfill capacity) when backfilling begins?

**SRK Response:** On the basis of rock type and the available laboratory testing data, the proportions of mineralized and non-mineralized waste rock have been estimated and used to estimate the relative quantities of mineralized and non-mineralized rock from the underground mine. The estimates, shown in Attachment 3, indicate that there would be approximately 343,000 tonnes of non-mineralized rock and 163,000 tonnes of mineralized rock<sup>2</sup>. Given that there is capacity to backfill 370,000 tonnes of waste rock in the underground mine, there should be more than sufficient capacity to backfill all of the mineralized rock, even if there is some conservatism in the segregation procedures (i.e. leading to more non-mineralized rock in the mineralized pile). For example, even if twice as much basalt and buffer zone material was classified as mineralized, there would still be an excess of 176,000 tonnes of backfill capacity remaining in the mine.

**KIA Question #5:** *Clarification and details are requested regarding whether HBML is designing the pollution control system to withstand 1:100 year 24-hour duration storms, or 1:25 year 24-hour duration storms.*

**SRK response:** Information on the design criteria for the water management facilities is provided in the water management plans. A separate response will be provided to address this comment.

**References:**

SRK Consulting, 2010a. Geochemical Characterization and Recommendations for Portal Face-Off Area Construction Rock, Doris North, Hope Bay Project, April 22, 2010.

SRK Consulting, 2010b. Geochemical Characterization and Recommendations for Doris North Fuel Tank Farm and Mill Pad, Doris North, Hope Bay Project, April 23, 2010.

SRK Consulting, 2010c. Geochemical Characterization and Recommendations for Roberts Bay Fuel Tank Farm, Doris North, Hope Bay Project, April 23, 2010.

SRK Consulting, 2010d. Geochemical Characterization and Recommendations for Quarry 5, Doris North, Hope Bay Project. Memo dated June 8, 2010.

SRK Consulting, 2007. Geochemical Characterization of Quarry Materials, Doris North Project, Hope Bay, Nunavut, Canada (Revised March 2007). Prepared for Miramar Hope Bay Ltd., March 2007.

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<sup>2</sup> Note that these volumes reflect the total expected underground waste production figures, as opposed to the peak waste rock storage quantities that take backfilling into consideration.







# Engineering Drawings for the Doris North Camp Area, Doris North Project, Nunavut, Canada

## ACTIVE DRAWING STATUS

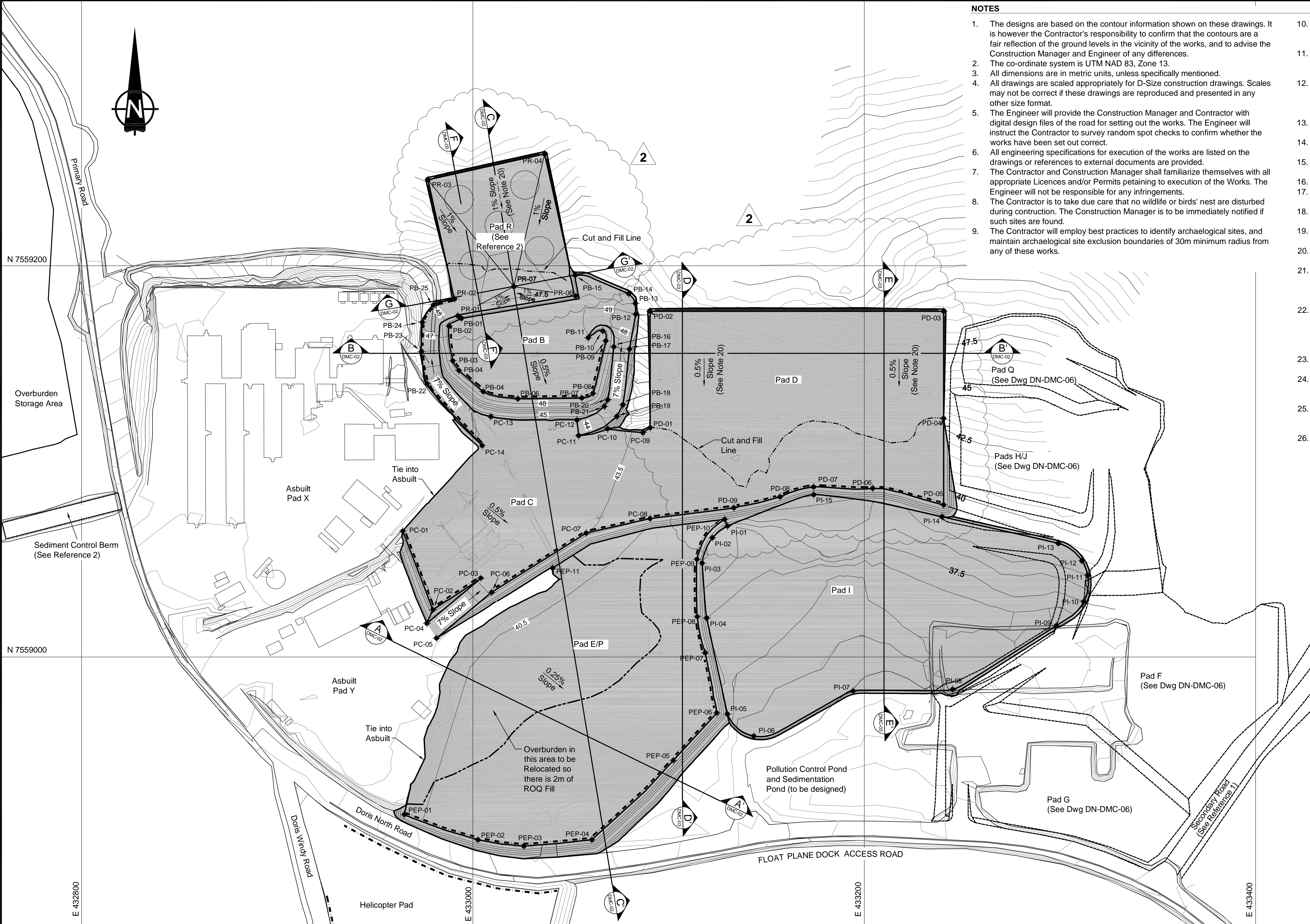
SRK DWG NUMBER	NEWMONT DWG NUMBER	DRAWING TITLE	REVISION	DATE	STATUS	OLD/REPLACED REVISIONS		
DN-DMC-00	HB+D-CIV-CIV-OND-0039	Engineering Drawings for Doris North Camp Area	4	September 29, 2010	Issued for Construction	Rev. 3, June 30, 2010	Rev. 0, June 24, 2010	Rev. A, June 4, 2010
DN-DMC-01	HB+D-CIV-CIV-OND-0030	Doris North Camp Grading Plan (West Side)	3	June 30, 2010	Issued for Construction	Rev. 2, June 24, 2010	Rev. 1, June 4, 2010	Rev. 0, May 26, 2010
DN-DMC-02	HB+D-CIV-CIV-OND-0040	Doris North Camp Sections and Details (West Side)	1	June 30, 2010	Issued for Construction	Rev. 0, June 24, 2010	Rev. A, June 4, 2010	
DN-DMC-03	HB+D-CIV-CIV-OND-0041	Doris North Camp Sections and Details (West Side)	1	June 30, 2010	Issued for Construction	Rev. 0, June 24, 2010	Rev. A, June 4, 2010	
DN-DMC-04	HB+D-CIV-CIV-OND-0042	Doris North Camp Sections and Details (West Side)	1	June 30, 2010	Issued for Construction	Rev. 0, June 24, 2010	Rev. A, June 13, 2010	
DN-DMC-05	HB+D-CIV-CIV-OND-0052	Material Specifications (West Side)	1	June 30, 2010	Issued for Construction	Rev. 0, June 24, 2010	Rev. A, June 4, 2010	
DN-DMC-06	HB+D-CIV-CIV-OND-0048	Doris North Camp Grading Plan (East Side)	0	June 24, 2010	Issued for Construction			
DN-DMC-07	HB+D-CIV-CIV-OND-0049	Doris North Camp Sections and Details (East Side)	0	June 24, 2010	Issued for Construction			
DN-DMC-08	HB+D-CIV-CIV-OND-0050	Doris North Camp Sections and Details (East Side)	0	June 24, 2010	Issued for Construction			
DN-DMC-09	HB+D-CIV-CIV-OND-0051	Material Specifications (East Side)	0	June 24, 2010	Issued for Construction			
DN-DMC-010	HB+D-CIV-CIV-OND-0070	Sedimentation and Pollution Control Ponds Grading Plan	0	September 29, 2010	Issued for Construction	Rev. A, July 28, 2010		
DN-DMC-011	HB+D-CIV-CIV-OND-0071	Sedimentation and Pollution Control Ponds Sections	0	September 29, 2010	Issued for Construction	Rev. A, July 28, 2010		
DN-DMC-012	HB+D-CIV-CIV-OND-0072	Float Plane Dock Access Road and Dyke Profiles	0	September 29, 2010	Issued for Construction	Rev. A, July 28, 2010		
DN-DMC-013	HB+D-CIV-CIV-OND-0073	Material Specifications (Sedimentation & Pollution Control Ponds)	0	September 29, 2010	Issued for Construction	Rev. A, July 28, 2010		
DN-DMC-014	HB+D-CIV-CIV-OND-0080	Sedimentation and Pollution Control Ponds Typical Details	0	September 29, 2010	Issued for Construction	Rev. A, July 28, 2010		
DN-DMC-015	HB+D-CIV-CIV-OND-0082	Pad R - Raised Access Ramp	1	August 08, 2010	Issued for Construction			
DN-DMC-016	HB+D-CIV-CIV-OND-0088	Pollution Control Pond Surfaces 1 and 2	0	September 29, 2010	Issued for Construction			
DN-DMC-017	HB+D-CIV-CIV-OND-0089	Pollution Control Pond Surfaces 3 and 4	0	September 29, 2010	Issued for Construction			
DN-DMC-018	HB+D-CIV-CIV-OND-0091	Pad C Expansion	0	September 8, 2010	Issued for Construction			
DN-DMC-019	HB+D-CIV-CIV-OND-0109	Doris North Access Road General Arrangement	0	September 24, 2010	Issued for Construction			
DN-DMC-020	HB+D-CIV-CIV-OND-0110	Doris North Access Road Profiles	0	September 24, 2010	Issued for Construction			
DN-DMC-021	HB+D-CIV-CIV-OND-0111	Doris North Access Road Sections and Details Sheet 1 of 2	0	September 24, 2010	Issued for Construction			
DN-DMC-022	HB+D-CIV-CIV-OND-0112	Doris North Access Road Sections and Details Sheet 2 of 2	0	September 24, 2010	Issued for Construction			
DN-DMC-023	HB+D-CIV-CIV-OND-0113	Material Specifications (Doris North Access Road)	0	September 24, 2010	Issued for Construction			

**HOPE BAY MINING LTD.**



PROJECT NO: 1CH008.027  
ISSUED FOR CONSTRUCTION  
Revision 4  
September 29, 2010  
DN-DMC-00 / HB+D-CIV-CIV-OND-0039





NOTES

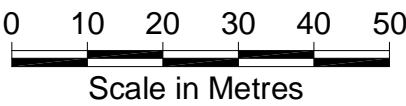
- The designs are based on the contour information shown on these drawings. It is however the Contractor's responsibility to confirm that the contours are a fair reflection of the ground levels in the vicinity of the works, and to advise the Construction Manager and Engineer of any differences.
- The co-ordinate system is UTM NAD 83, Zone 13.
- All dimensions are in metric units, unless specifically mentioned.
- All drawings are scaled appropriately for D-Size construction drawings. Scales may not be correct if these drawings are reproduced and presented in any other size format.
- The Engineer will provide the Construction Manager and Contractor with digital design files of the road for setting out the works. The Engineer will instruct the Contractor to survey random spot checks to confirm whether the works have been set out correct.
- All engineering specifications for execution of the works are listed on the drawings or references to external documents are provided.
- The Contractor and Construction Manager shall familiarize themselves with all appropriate Licences and/or Permits pertaining to execution of the Works. The Engineer will not be responsible for any infringements.
- The Contractor is to take due care that no wildlife or birds' nest are disturbed during construction. The Construction Manager is to be immediately notified if such sites are found.
- The Contractor will employ best practices to identify archaeological sites, and maintain archaeological site exclusion boundaries of 30m minimum radius from any of these works.
- These works must be executed in accordance with the standard HBML health and safety, and environmental standards and protocols. It is the Contractors responsibility to familiarize himself with these documents.
- Construction of the camp pad may not commence without on-site presence of an Engineers' representative. The Contractor shall notify the Engineer at least 5 days in advance of intended construction start-up.
- The placement of rockfill material will be by CAT 773 and CAT 730 haul trucks. The Contractor must supply the Construction Manager and Engineer with a written procedure for how these works will be constructed using these trucks prior to the start of any construction.
- The Contractor shall notify the Engineer at least 3 days in advance if an inspection is required for acceptance of works at any stage.
- The Contractor is responsible to develop rock quarries within the general designated boundaries shown on this drawing.
- Prior to quarry development, the Contractor must provide the Construction Manager and Engineer with a detailed quarry development plan for approval.
- The Contractor is responsible for creating access to the rock quarries.
- The Contractor will employ best practices to control at source run-off, fugitive dust, blast vibrations, and fly rock.
- The Contractor shall employ best practices to ensure sediment control and to prevent erosion.
- The terrain model is based on current original ground survey by Nuna Logistics and as-built survey by SNC Lavalin.
- All excavated bedrock surfaces are to be free draining as shown. This slope is independent of the final finished surface elevation and grade.
- Additional OG survey is required along the eastern section of Pads Q, H/J, F and G as shown on this drawing. Until this survey has been completed and provided to the Engineer, the Engineer cannot confirm that the grade and elevation is appropriate in those areas.
- The lines on this drawing provides the final grade and elevation of all pads (R, B, C, D, E/P, I, H/J, Q, F and G). These grades include an allowance for placing a 150mm thick layer of surfacing grade material on all surfaces. The Contractor must make the appropriate adjustments to the grades set out for the Works.
- The Engineer provided the Contractor and Owner with and ACAD file of the Works for setting out the works as indicated on this drawing.
- Construction shall be in accordance with the following Technical Specifications: Earthworks and Geotechnical Engineering, Hope Bay project, Nunavut, Canada, revision C -Issue for Construction.
- A minimum fill of 2m rock must be placed over the overburden pile. The drawing denotes a portion of the pile that will have to be relocated in order to accommodate this requirement.
- Notes in this drawing apply to all other active drawings.

LEGEND

- New Camp Pads
- Stake-out Point
- Future Camp Pads/Road
- Existing Buildings
- Safety Berms (See Typical Berm Barrier Options Detail on Dwg DN-DMC-03)

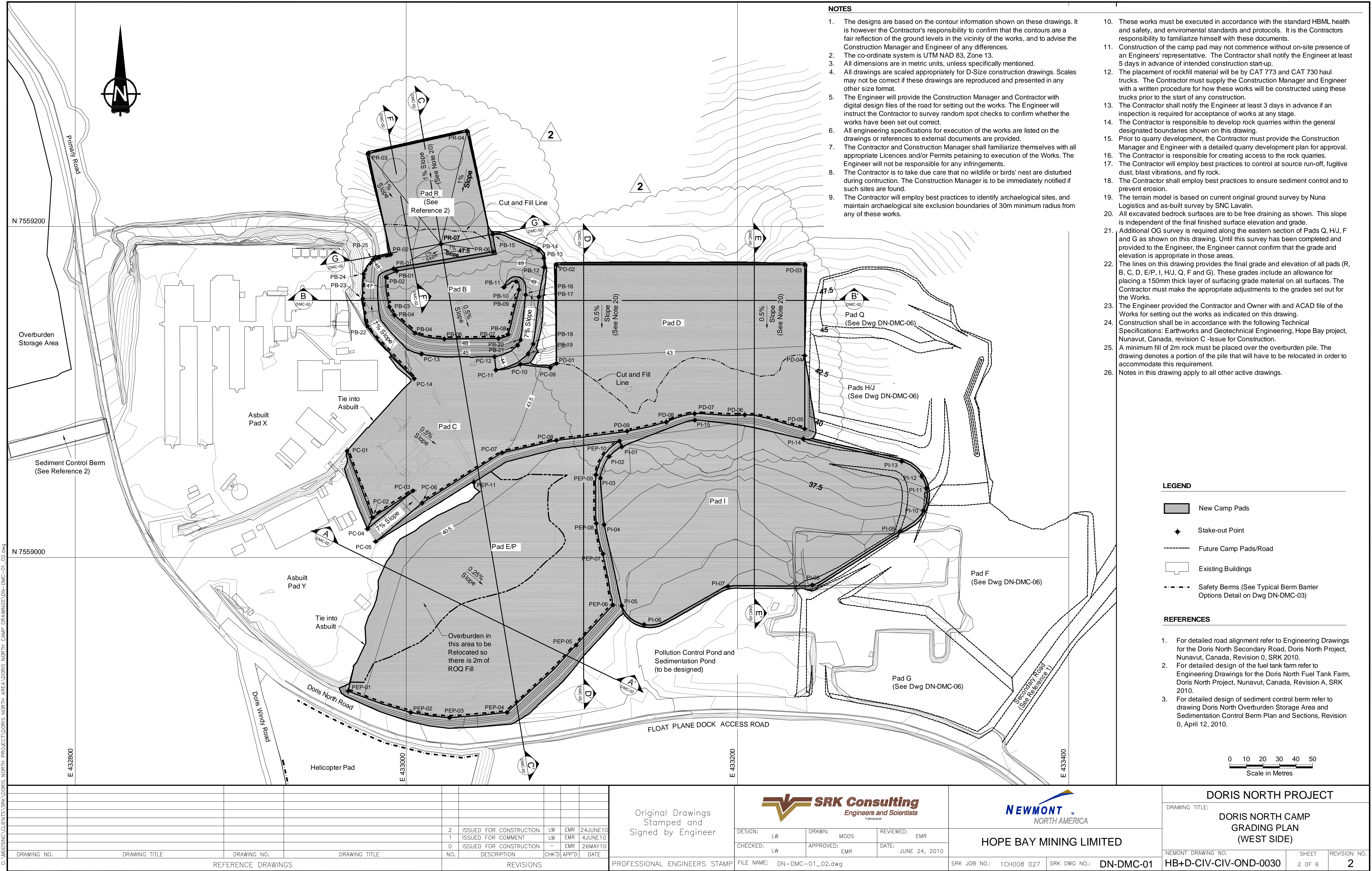
REFERENCES

- For detailed road alignment refer to Engineering Drawings for the Doris North Secondary Road, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.
- For detailed design of the fuel tank farm refer to Engineering Drawings for the Doris North Fuel Tank Farm, Doris North Project, Nunavut, Canada, Revision A, SRK 2010.
- For detailed design of sediment control berm refer to drawing Doris North Overburden Storage Area and Sedimentation Control Berm Plan and Sections, Revision 0, April 12, 2010.



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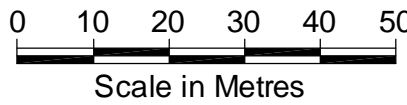
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- The Engineer provided the Contractor and Owner with and ACAD file of the Works for setting out the works as indicated on this drawing.
- Construction shall be in accordance with the following Technical Specifications: Earthworks and Geotechnical Engineering, Hope Bay project, Nunavut, Canada, revision C -Issue for Construction.
- A minimum fill of 2m rock must be placed over the overburden pile. The drawing denotes a portion of the pile that will have to be relocated in order to accommodate this requirement.
- Notes in this drawing apply to all other active drawings.

LEGEND

- New Camp Pads
- Stake-out Point
- Future Camp Pads/Road
- Existing Buildings
- Safety Berms (See Typical Berm Barrier Options Detail on Dwg DN-DMC-03)

REFERENCES

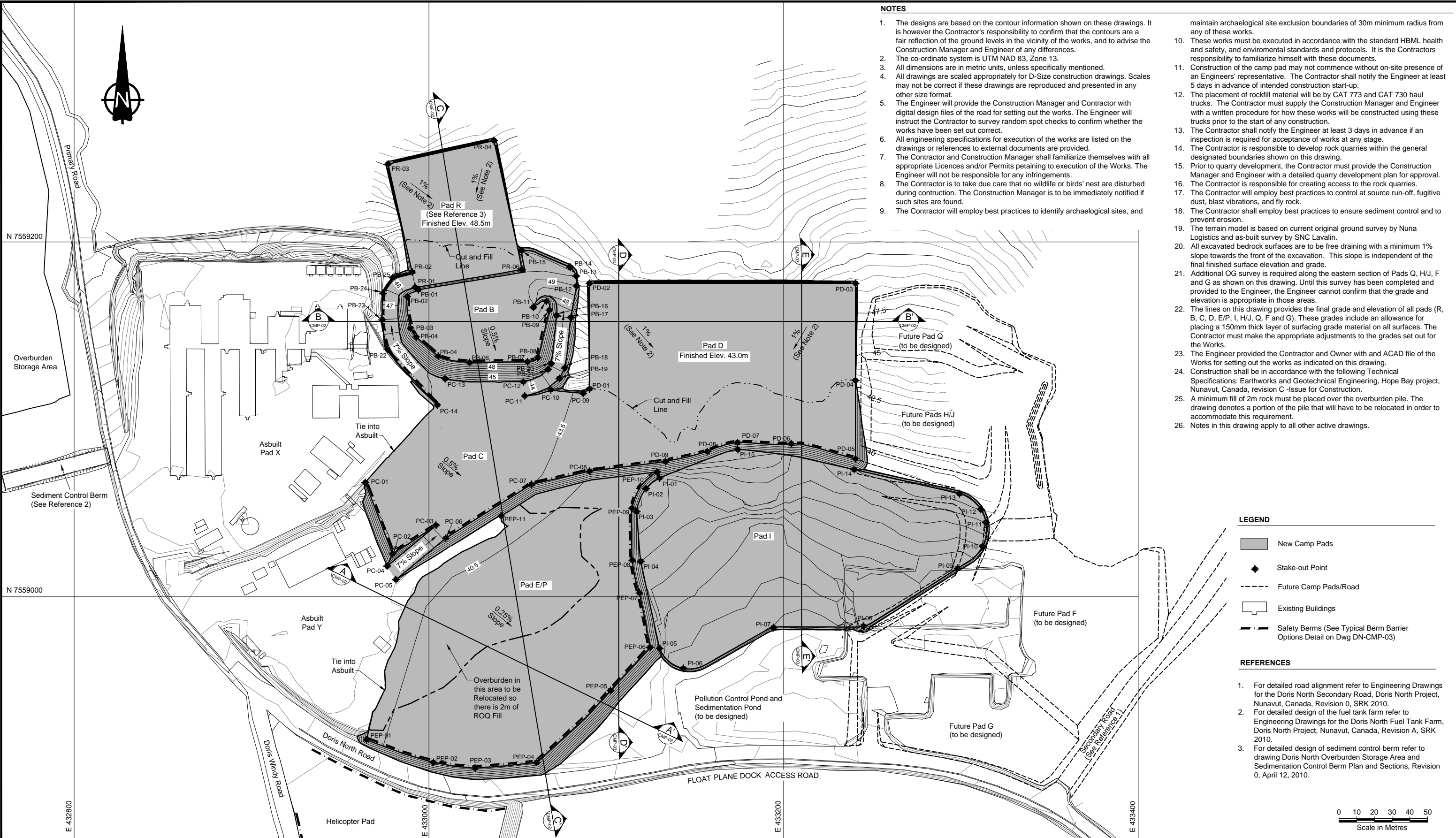
- For detailed road alignment refer to Engineering Drawings for the Doris North Secondary Road, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.
- For detailed design of the fuel tank farm refer to Engineering Drawings for the Doris North Fuel Tank Farm, Doris North Project, Nunavut, Canada, Revision A, SRK 2010.
- For detailed design of sediment control berm refer to drawing Doris North Overburden Storage Area and Sedimentation Control Berm Plan and Sections, Revision 0, April 12, 2010.



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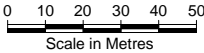
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- The Engineer provided the Contractor and Owner with and ACAD file of the Works for setting out the works as indicated on this drawing.
- Construction shall be in accordance with the following Technical Specifications: Earthworks and Geotechnical Engineering, Hope Bay project, Nunavut, Canada, revision C -Issue for Construction.
- A minimum fill of 2m rock must be placed over the overburden pile. The drawing denotes a portion of the pile that will have to be relocated in order to accommodate this requirement.
- Notes in this drawing apply to all other active drawings.

LEGEND

- New Camp Pads
- Stake-out Point
- Future Camp Pads/Road
- Existing Buildings
- Safety Berms (See Typical Berm Barrier Options Detail on Dwg DN-CMP-03)

REFERENCES

- For detailed road alignment refer to Engineering Drawings for the Doris North Secondary Road, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.
- For detailed design of the fuel tank farm refer to Engineering Drawings for the Doris North Fuel Tank Farm, Doris North Project, Nunavut, Canada, Revision A, SRK 2010.
- For detailed design of sediment control berm refer to drawing Doris North Overburden Storage Area and Sedimentation Control Berm Plan and Sections, Revision 0, April 12, 2010.

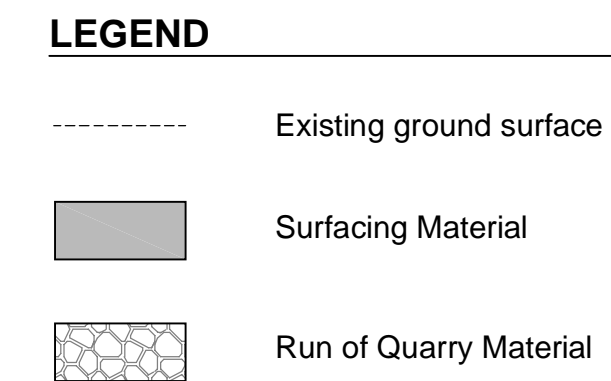


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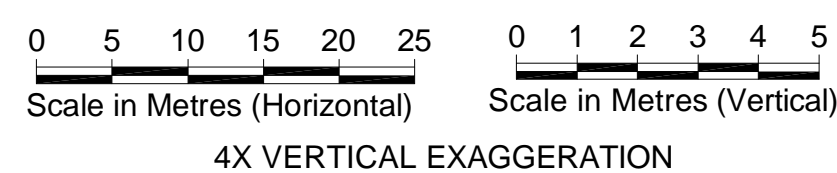


[illegible]





- ## NOTES
1. All dimensions in metres unless noted otherwise.
  2. Where the thickness of the pads is greater than 3.0m allow for the placement of barriers.
  3. The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
  4. Notes in this drawing apply to all other active drawings.

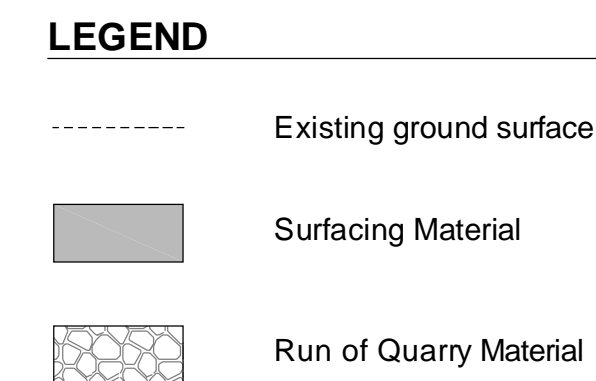


**C** SECTION

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- ## NOTES
1. All dimensions in metres unless noted otherwise.
  2. Where the thickness of the pads is greater than 3.0m allow for the placement of barriers.
  3. The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
  4. Notes in this drawing apply to all other active drawings.

## REFERENCES

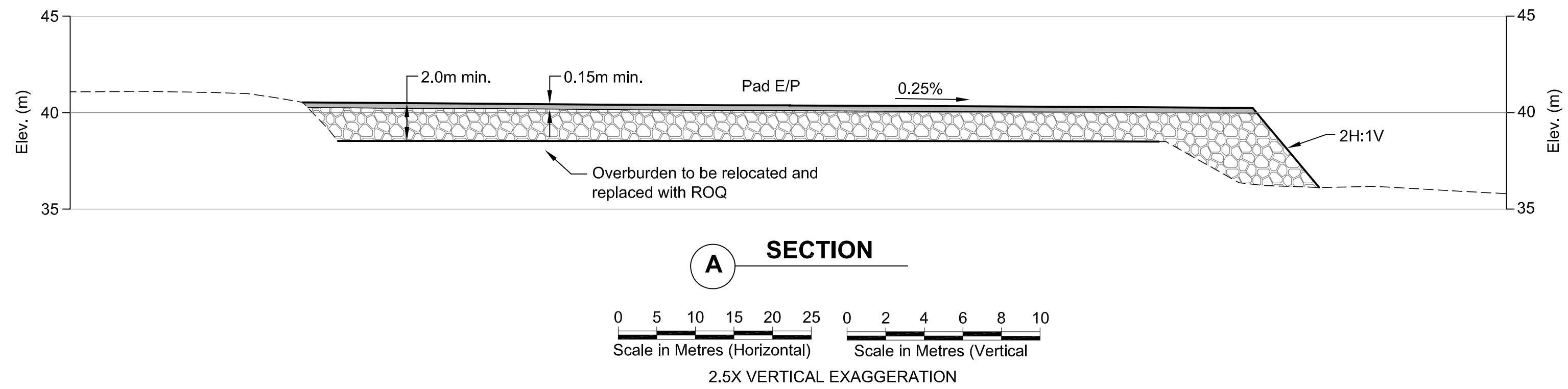
1. For detailed design of the fuel tank farm refer to Engineering Drawings for the Doris North Fuel Tank Farm, Doris North Project, Nunavut, Canada, Revision A, SRK 2010.



**C** **SECTION**

[illegible]

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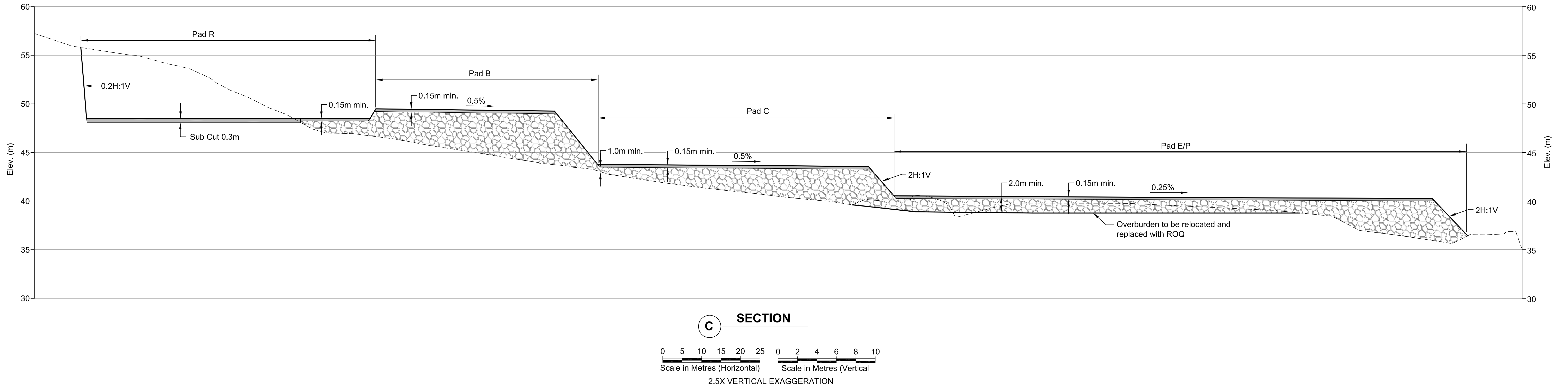
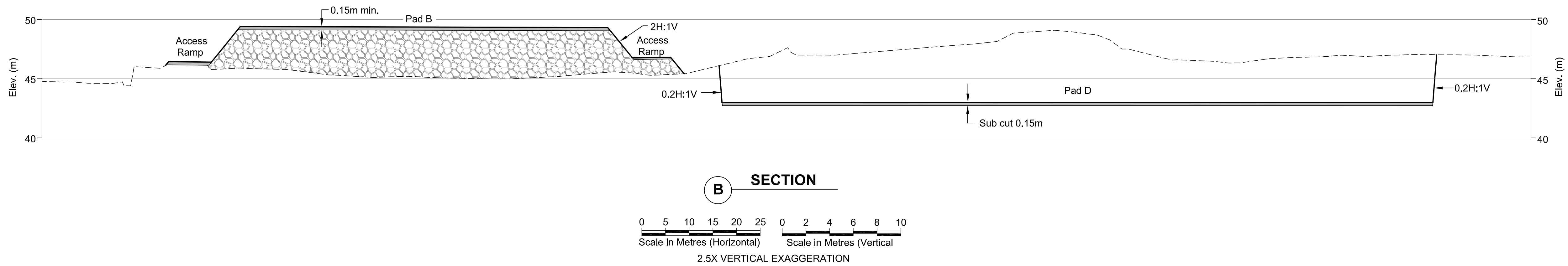


**LEGEND**

- Existing ground surface
- Surfacing Material
- Run of Quarry Material


**NOTES**

- All dimensions in metres unless noted otherwise.
- Where the thickness of the pads is greater than 3.0m allow for the placement of barriers.
- The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
- Notes in this drawing apply to all other active drawings.




CMP-01	DORIS NORTH CAMP GRADING PLAN			A	ISSUED FOR COMMENT	LW	EMR	4JUNE10	
DRAWING NO.	DRAWING TITLE	DRAWING NO.	DRAWING TITLE	NO.	DESCRIPTION	CHK'D	APP'D	DATE	
REFERENCE DRAWINGS				REVISIONS					

PROFESSIONAL ENGINEERS' STAMP



**SRK Consulting**  
Engineers and Scientists  
Vancouver

DESIGN:	LW	DRAWN:	MDDS	REVIEWED:	EMR
CHECKED:	LW	APPROVED:	EMR	DATE:	JUNE 4, 2010
FILE NAME: DN-CMP-01_02.dwg					



**NEWMONT**  
NORTH AMERICA

**HOPE BAY MINING LIMITED**

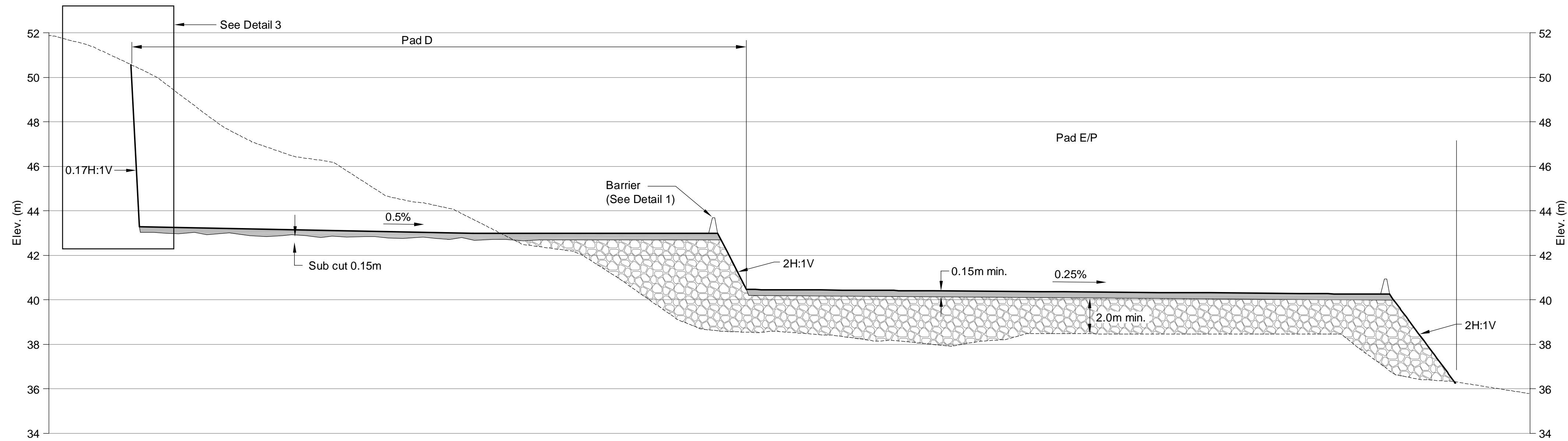
SRK JOB NO.:	1CH008 027
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DORIS NORTH PROJECT		
DRAWING TITLE:		
DORIS NORTH CAMP SECTIONS AND DETAILS		
DRAWING NO.	SHEET	REVISION NO.
DN-CMP-02	3 OF 5	A

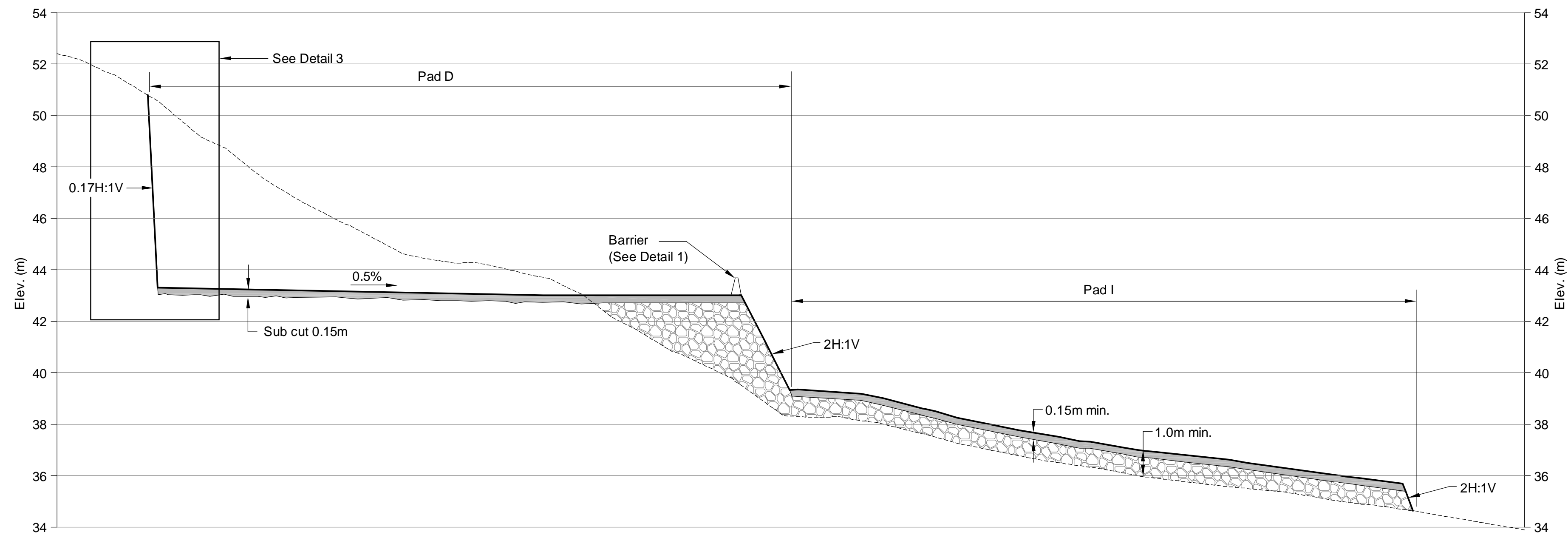




G:\MODS\CLIENTS\SRK\DORIS NORTH PROJECT\DORIS NORTH AREA\DORIS NORTH CAMP DRAWINGS\DN-DMC-01\_02.dwg



**D** SECTION



**E** SECTION

**LEGEND**

- Existing ground surface
- Run of Quarry Material
- Surfacing Material

**NOTES**

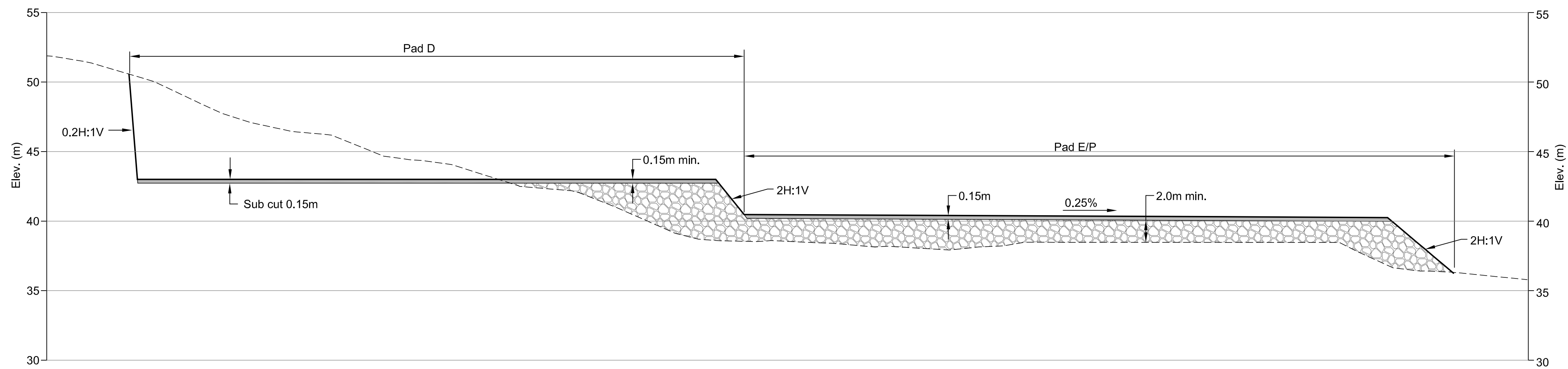
- All dimensions in metres unless noted otherwise.
- Minimum design thickness must be maintained for all sections of the Pads
- Where the thickness of the pads is greater than 2.0m allow for the placement of barriers.
- The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
- Notes in this drawing apply to all other active drawings.



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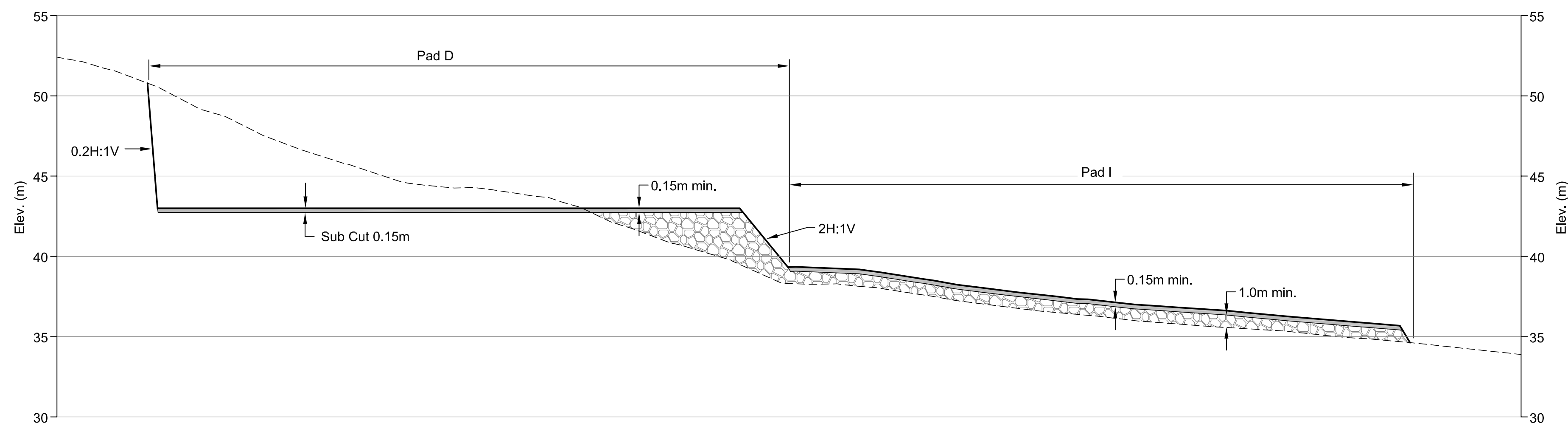


#### LEGEND

- Existing ground surface
- Run of Quarry Material
- Surfacing Material

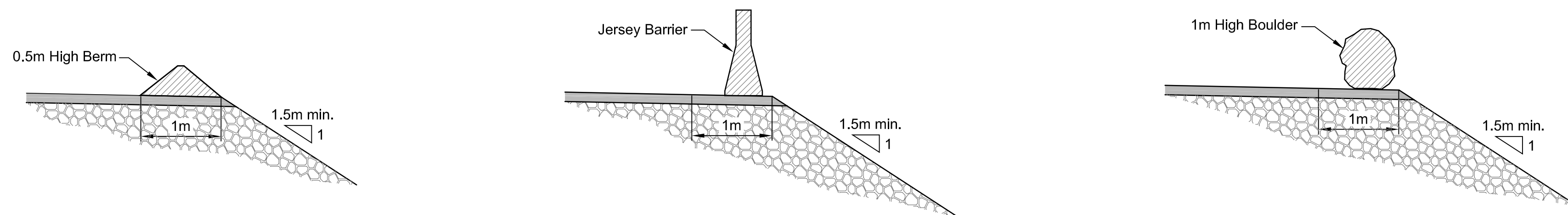
#### NOTES

- All dimensions in metres unless noted otherwise.
- Minimum design thickness must be maintained for all sections of the Pads
- Where the thickness of the pads is greater than 2.0m allow for the placement of barriers.
- The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
- Notes in this drawing apply to all other active drawings.



#### SECTION E

0 5 10 15 20 25 0 2 4 6 8 10  
Scale in Metres (Horizontal) Scale in Metres (Vertical)  
2.5X VERTICAL EXAGGERATION



DETAIL  
TYPICAL BERM BARRIER OPTIONS  
NTS

										DORIS NORTH PROJECT		
										DRAWING TITLE:		
										DORIS NORTH CAMP SECTIONS AND DETAILS		
										DRAWING NO.		
										DN-CMP-03		
										SHEET		
										4 OF 5		
										REVISION NO.		
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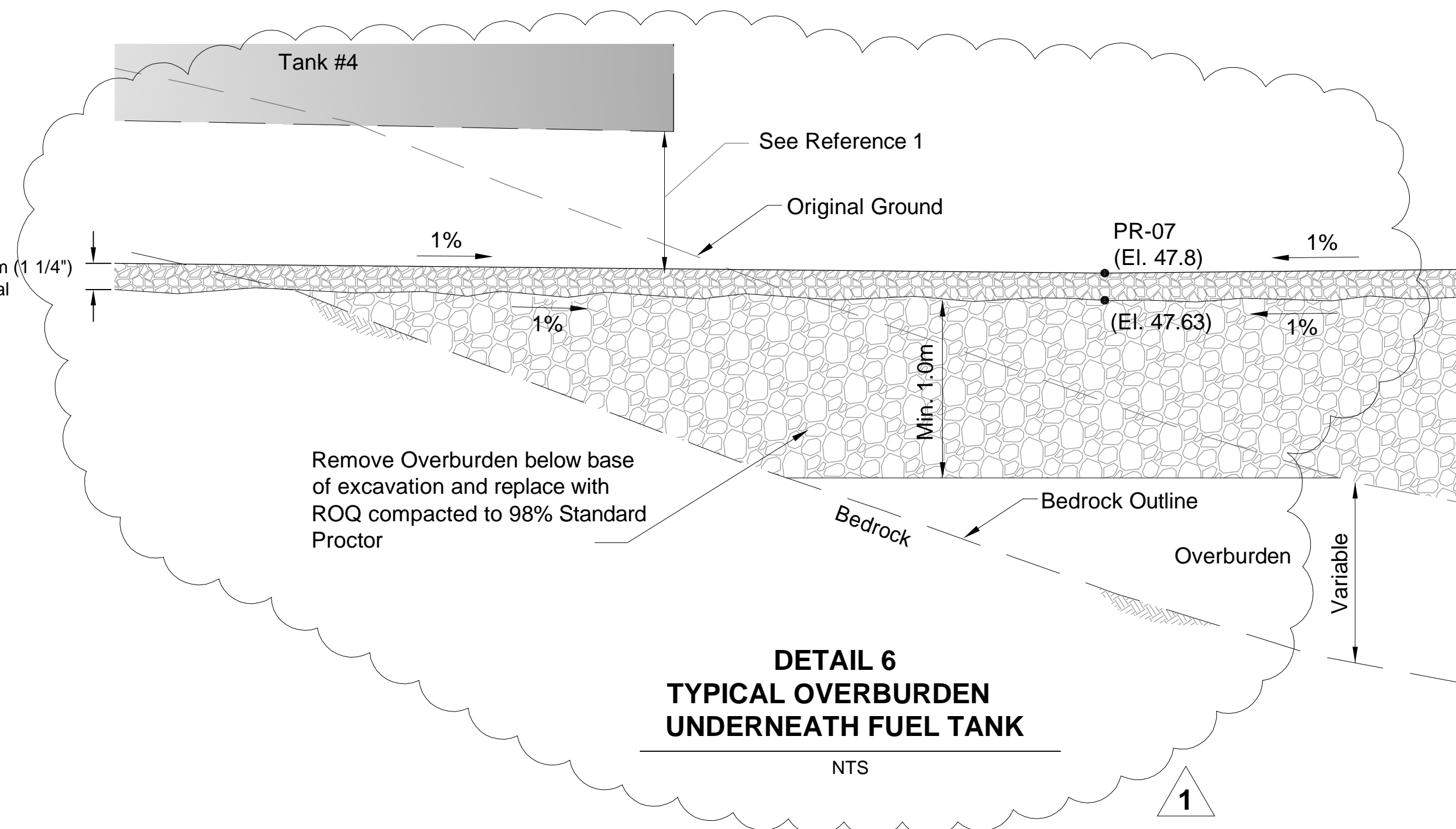
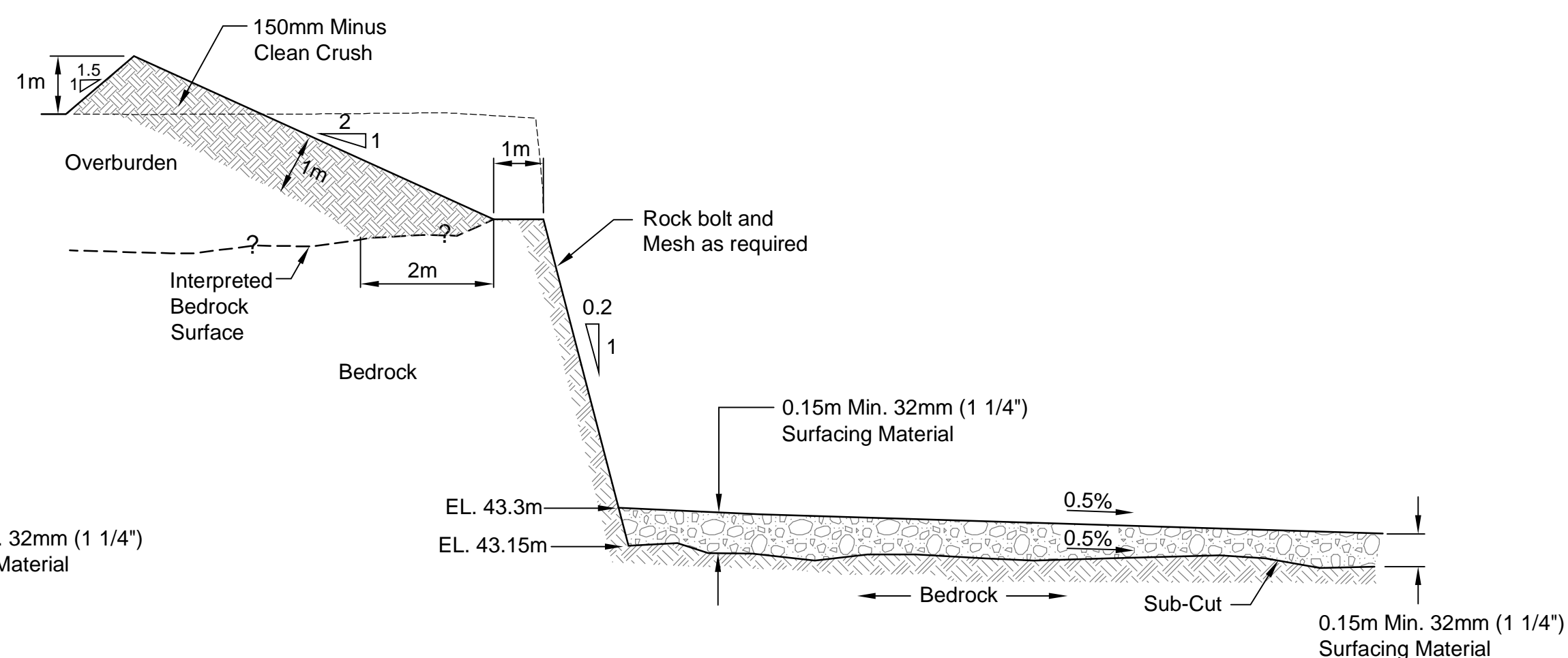
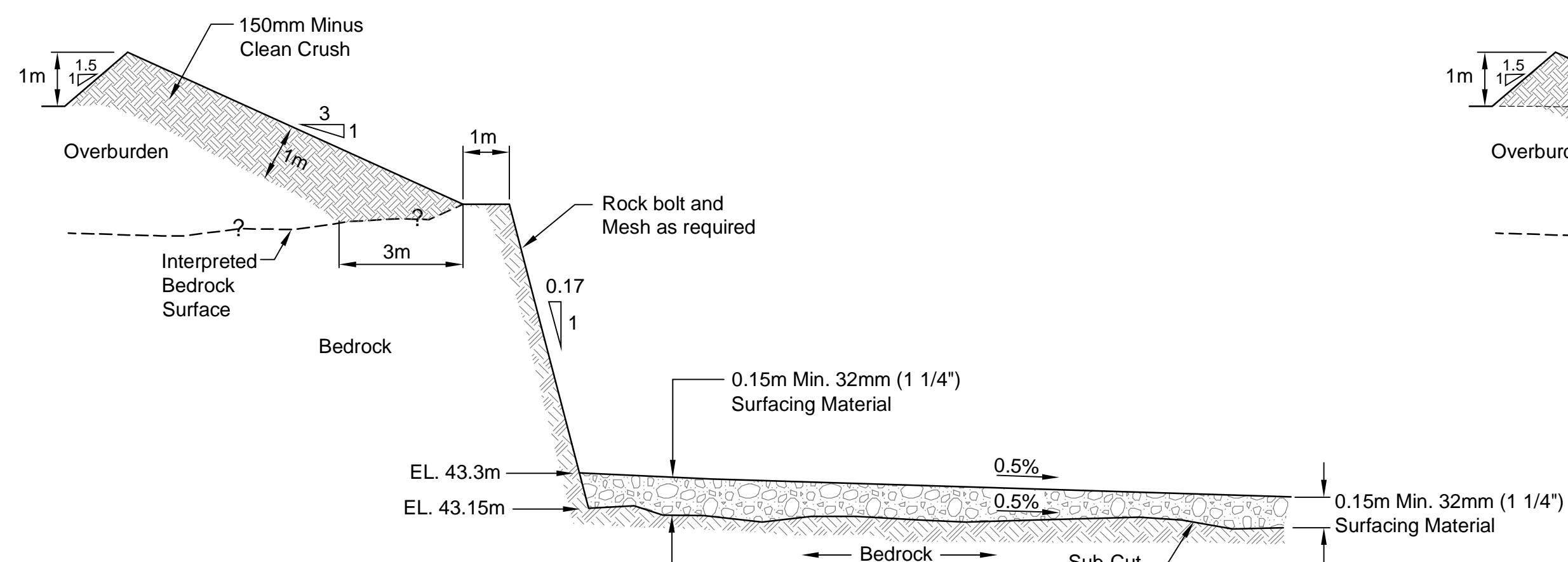
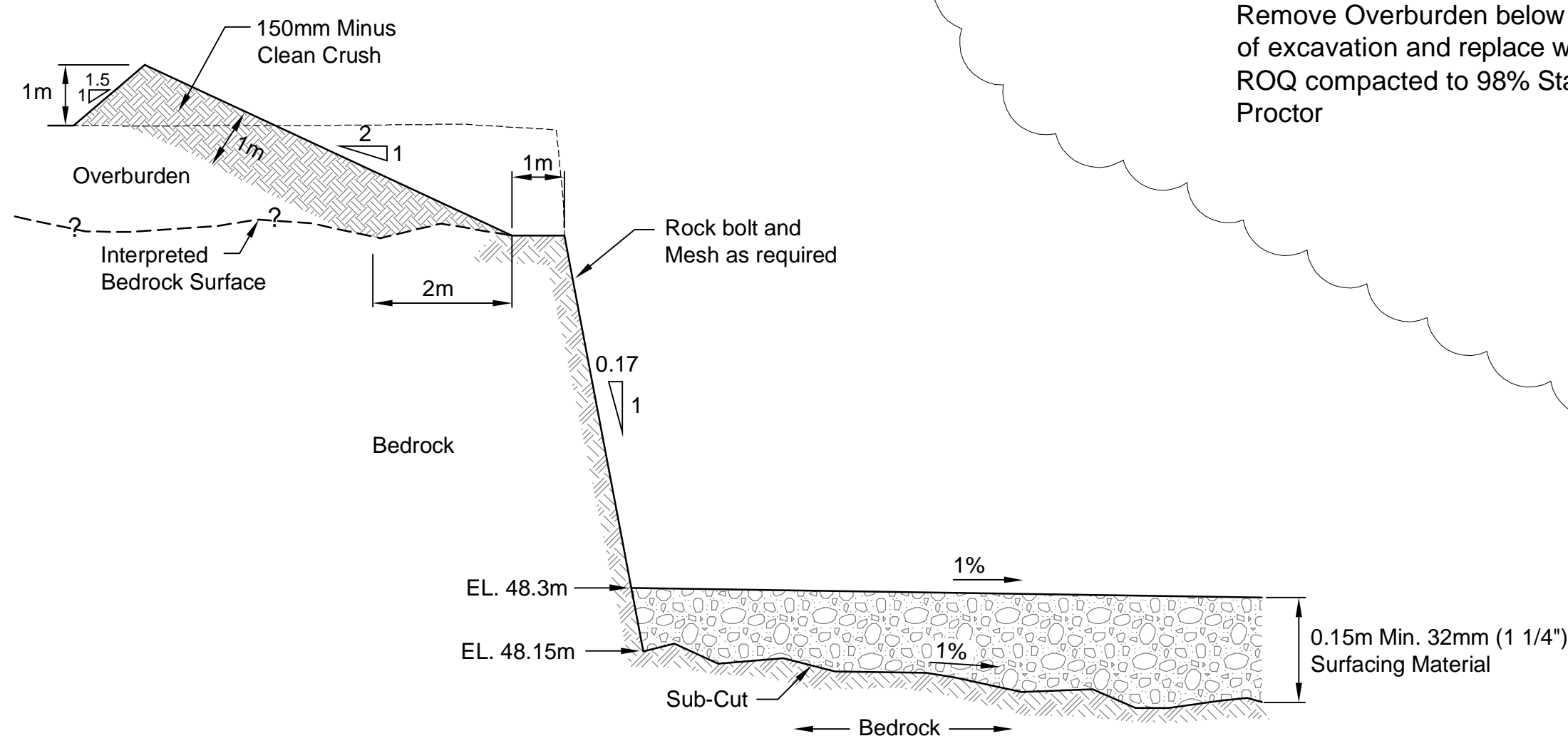
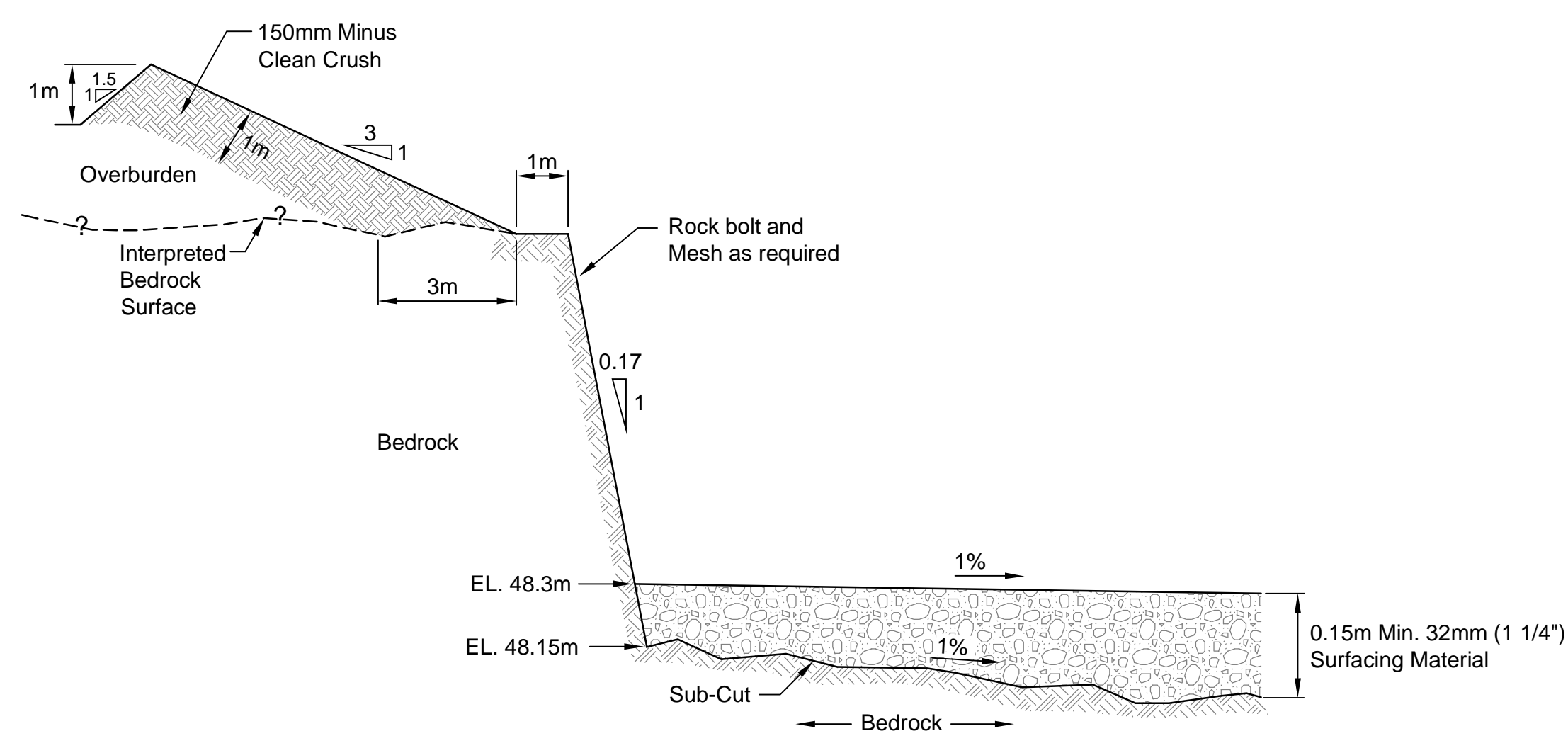
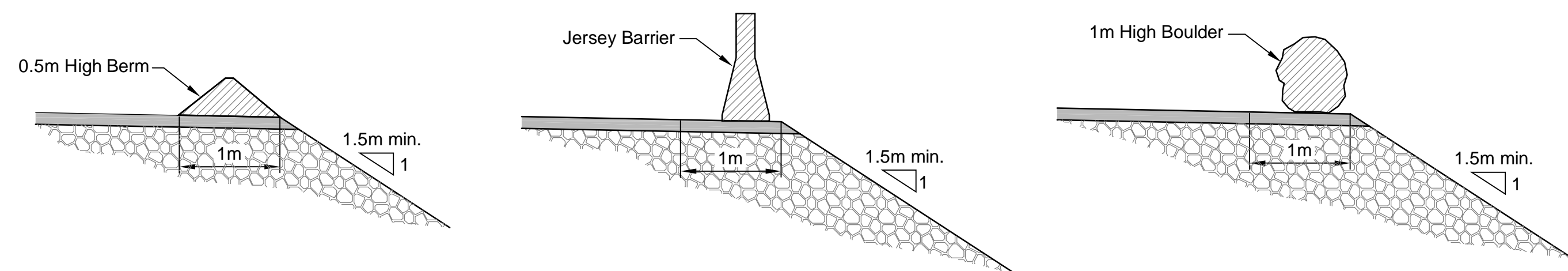
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										DRAWN: MDDS		
										REVIEWED: EMR		
										CHECKED: LW		
										APPROVED: EMR		
										DATE: JUNE 4, 2010		

										HOPE BAY MINING LIMITED		
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										DORIS NORTH CAMP SECTIONS AND DETAILS		
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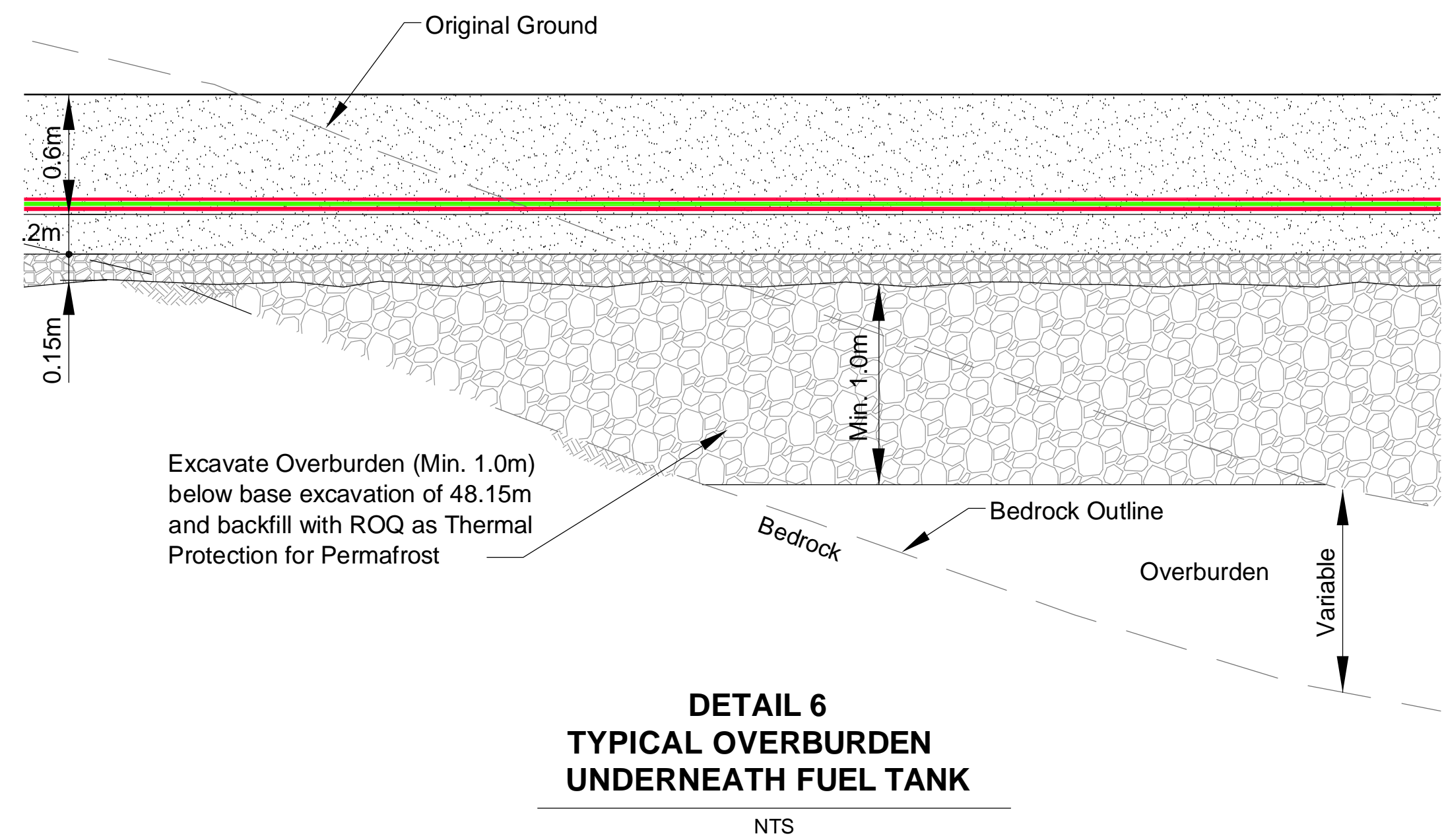
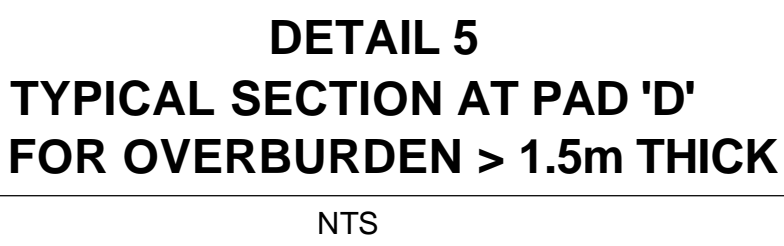
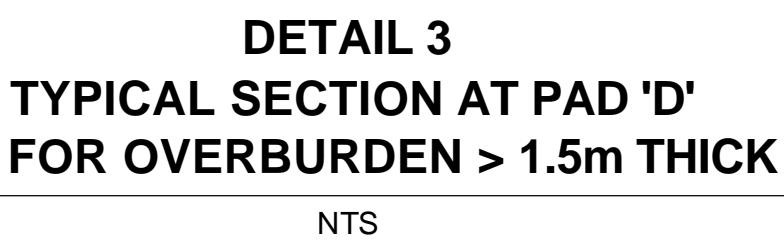
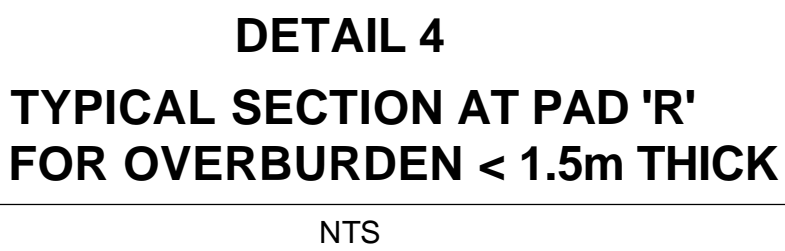
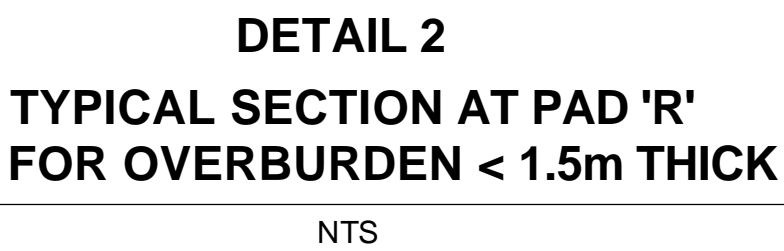
										DRAWING NO.		
										DN-CMP-03		
										SHEET		
										4 OF 5		
										REVISION NO.		
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- ## REFERENCE
- 
1. For detailed design of the fuel tank farm refer to Engineering Drawings for the Doris North Fuel Tank Farm, Doris North Project. Nunavut, Canada, Revision A, SRK 2010.

[illegible]



[illegible]



NOTES

1. Soil classification for these works are based on the Unified Soil Classification System (USCS).
2. On bare tundra surfaces the maximum snow thickness allowed prior to fill placement shall be 102mm (4"). On all other surfaces complete snow removal is required. The Engineer must approve all surfaces prior to placement of any construction material.
3. Snow and ice on construction material must be removed prior to loading for construction use.
4. Due care must be taken when placing fill materials such that no damage occurs to the subgrade and/or culverts. Any damage must be immediately reported to the Engineer.
5. Maximum lift thickness is 1.85m. Staged construction will be required where fill thickness exceeds 1.85m.
6. In areas where staged construction is required, all snow shall be removed and the surface scarified prior to placing the next lift. The Engineer will approve such staged construction.
7. Run of Quarry, and Surfacing material has to be compacted after placement.
8. Compaction will be a field specification, based on trial compaction tests to be carried out by the Contractor to the satisfaction of the Engineer.
9. It is the Contractor's responsibility to create the construction materials as specified through appropriate crushing. Any deviations must be approved by the Engineer.
10. Construction fill material shall be from approved rock quarries, shall be non-acid generating, free of organic material or similar impurities, as well as snow and ice.
11. Construction fill material must be free of overburden soils. Such unsuitable material shall be disposed of in a designated on site disposal area as outlined in the Contractors' quarry development plan.
12. Construction fill material will not have to be washed to remove blast residues or fines, unless specifically instructed by the Engineer.
13. Run of Quarry (ROQ) shall be well-graded, containing sufficient quantities of gravel, sand, and silt sized material. For fill thickness <0.85m the maximum boulder size shall not exceed 500mm. For fill thickness >0.85m the maximum boulder size shall not exceed 900mm.
14. Surfacing material shall be a well-graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 51mm (2") but no smaller than 32mm (1¼").
15. ROQ material shall be visually inspected by the Engineer on a routine basis and the Contractor will be advised if the material does not meet the specification in Note 17.
16. The Contractor shall collect samples of the surfacing material directly from the crusher stockpile and submit for laboratory testing including but not limited to grain size distribution, and moisture content at least 1 sample every 8,000m<sup>3</sup>. The Engineer may conduct additional sampling and testing as deemed necessary.
17. Sample collection and testing of ROQ, and surface material for geochemical suitability is required and will be carried out by the Site Environmental Manager in accordance with procedures developed by SRK.

Materials List and Quantities

Item	Quantity / Area / Volume		Description
1. Run of Quarry Material	Pad B	16,964 m³	Volumes derived by Eagle Point 7.2. - Side slopes 1.5H:1V for fill less than 2m - Side slopes 2H:1V for fill greater than 2m - Fills are min. 1.0m - Volumes derived by merging Topography/As-built to Pad Design Surfaces.
	Pad R Cut	15,660 m³	
	Pad R Fill	1,134 ³	
	Pad C	16,197 m³	
	Pad D Fill	8,309 m³	
	Pad D Cut	37,000 m³	
	Pad E/P	31,372 m³	
	Pad I	14,526 m³	
	Total Fill:	87,368 m³	
	Total Cut:	37,000 m³	
2. Surface Grade Material	Pad B	1,267 m³	
	Pad R	1,148 m³	
	Pad C	1,153 m³	
	Pad D	2,167 m³	
	Pad E/P	2,628 m³	
	Pad I	2,474 m³	
	Total Fill:	10,837 m³	
3. Volume of Overburden to be Relocated	Pad E/P	3,000 m³	Approximate

PAD E/P STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PEP-01	7,558,919.48	432,964.90	40.52
PEP-02	7,558,906.59	433,002.59	40.43
PEP-03	7,558,903.36	433,026.07	40.37
PEP-04	7,558,906.62	433,060.82	40.29
PEP-05	7,558,947.15	433,102.43	40.25
PEP-06	7,558,971.42	433,124.60	40.27
PEP-07	7,559,002.17	433,118.72	40.33
PEP-08	7,559,020.66	433,114.68	40.37
PEP-09	7,559,049.99	433,114.54	40.43
PEP-10	7,559,070.39	433,128.67	40.42
PEP-11	7,559,045.52	433,040.74	40.54

PAD I STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PI-01	7,559,066.95	433,130.11	38.63
PI-02	7,559,060.99	433,122.40	38.89
PI-03	7,559,047.97	433,117.24	38.96
PI-04	7,559,019.96	433,119.52	38.00
PI-05	7,558,970.86	433,130.17	37.53
PI-06	7,558,959.60	433,143.56	36.22
PI-07	7,558,982.64	433,194.28	35.97
PI-08	7,558,983.45	433,245.15	37.82
PI-09	7,559,016.12	433,298.27	37.57
PI-10	7,559,028.39	433,312.20	38.50
PI-11	7,559,041.74	433,314.16	39.50
PI-12	7,559,049.18	433,311.31	40.00
PI-13	7,559,058.11	433,299.18	40.34
PI-14	7,559,071.72	433,239.77	39.98
PI-15	7,559,083.02	433,174.19	41.04

PAD D STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PD-01	7,559,117.24	433,090.58	43.00
PD-02	7,559,176.74	433,090.58	43.00
PD-03	7,559,176.74	433,240.58	43.00
PD-04	7,559,121.97	433,240.58	43.00
PD-05	7,559,077.76	433,240.58	43.00
PD-06	7,559,086.30	433,204.33	43.00
PD-07	7,559,086.93	433,174.09	43.00
PD-08	7,559,081.90	433,156.97	43.00
PD-09	7,559,076.33	433,133.35	43.00

PAD C STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PC-01	7,559,064.43	432,964.17	43.87
PC-02	7,559,024.15	432,979.50	43.86
PC-03	7,559,040.04	433,003.85	43.64
PC-04	7,559,017.28	432,976.48	41.12
PC-05	7,559,009.66	432,981.39	41.05
PC-06	7,559,033.11	433,009.57	43.60
PC-07	7,559,063.38	433,057.89	43.52
PC-08	7,559,070.80	433,090.58	43.00
PC-09	7,559,114.73	433,086.93	43.47
PC-10	7,559,116.72	433,068.83	43.56
PC-11	7,559,113.24	433,054.04	43.66
PC-12	7,559,121.20	433,053.17	43.71
PC-13	7,559,122.92	433,009.12	43.86
PC-14	7,559,108.11	433,004.73	43.85

PAD B STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PB-01	7,559,173.16	432,994.14	49.49
PB-02	7,559,169.27	432,987.78	49.48
PB-03	7,559,151.46	432,989.70	49.39
PB-04	7,559,146.47	432,992.83	49.36
PB-05	7,559,135.72	433,005.25	49.30
PB-06	7,559,132.08	433,022.95	49.26
PB-07	7,559,132.48	433,055.63	49.23
PB-08	7,559,134.83	433,061.37	49.23
PB-09	7,559,161.67	433,067.92	49.36
PB-10	7,559,166.67	433,066.40	49.38
PB-11	7,559,163.24	433,058.91	49.37
PB-12	7,559,175.26	433,083.23	49.00
PB-13	7,559,180.79	433,083.05	49.50
PB-14	7,559,185.77	433,079.65	50.24
PB-15	7,559,195.22	433,052.24	50.00
PB-16	7,559,158.60	433,072.01	47.00
PB-17	7,559,157.48	433,080.02	47.00
PB-18	7,559,128.98	433,076.64	45.09
PB-19	7,559,123.04	433,073.57	45.09
PB-20	7,559,131.51	433,068.94	44.82
PB-21	7,559,128.15	433,067.20	44.82
PB-22	7,559,139.71	432,976.60	45.50
PB-23	7,559,156.19	432,973.70	46.50
PB-24	7,559,171.16	432,974.14	47.38
PB-25	7,559,179.89	432,979.71	48.00

PAD R STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PR-01	7,559,174.38	432,992.43	48.50
PR-02	7,559,182.97	432,990.53	48.50
PR-03	7,559,243.98	432,977.03	48.50
PR-04	7,559,257.17	433,036.67	48.50
PR-05	7,559,193.47	433,050.72	48.50
PR-06	7,559,184.44	433,052.90	48.50

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NOTES

1.

Soil classification for these works are based on the Unified Soil Classification System (USCS).
2.

On bare tundra surfaces the maximum snow thickness allowed prior to fill placement shall be 102mm (4"). On all other surfaces complete snow removal is required. The Engineer must approve all surfaces prior to placement of any construction material.
3.

Snow and ice on construction material must be removed prior to loading for construction use.
4.

Due care must be taken when placing fill materials such that no damage occurs to the subgrade and/or culverts. Any damage must be immediately reported to the Engineer.
5.

Maximum lift thickness is 1.85m. Staged construction will be required where fill thickness exceeds 1.85m.
6.

In areas where staged construction is required, all snow shall be removed and the surface scarified prior to placing the next lift. The Engineer will approve such staged construction.
7.

Run of Quarry, and Surfacing material has to be compacted after placement.
8.

Compaction will be a field specification, based on trial compaction tests to be carried out by the Contractor to the satisfaction of the Engineer.
9.

It is the Contractor's responsibility to create the construction materials as specified through appropriate crushing. Any deviations must be approved by the Engineer.
10.

Construction fill material shall be from approved rock quarries, shall be non-acid generating, free of organic material or similar impurities, as well as snow and ice.
11.

Construction fill material must be free of overburden soils. Such unsuitable material shall be disposed of in a designated on site disposal area as outlined in the Contractors' quarry development plan.
12.

Construction fill material will not have to be washed to remove blast residues or fines, unless specifically instructed by the Engineer.
13.

Run of Quarry (ROQ) shall be well-graded, containing sufficient quantities of gravel, sand, and silt sized material. For fill thickness <0.85m the maximum boulder size shall not exceed 500mm. For fill thickness >0.85m the maximum boulder size shall not exceed 900mm.
14.

Surfacing material shall be a well-graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 51mm (2") but no smaller than 32mm (1¼").
15.

¾" Finishing material shall be well graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 32mm (1¼") but no smaller than 19mm (¾")
16.

ROQ material shall be visually inspected by the Engineer on a routine basis and the Contractor will be advised if the material does not meet the specification in Note 17.
17.

The Contractor shall collect samples of the surfacing material directly from the crusher stockpile and submit for laboratory testing including but not limited to grain size distribution, and moisture content at least 1 sample every 8,000m³. The Engineer may conduct additional sampling and testing as deemed necessary.
18.

Sample collection and testing of ROQ, and surface material for geochemical suitability is required and will be carried out by the Site Environmental Manager in accordance with procedures developed by SRK.

Materials List and Quantities

Item	Quantity / Area / Volume		Description
1. Run of Quarry Material	Pad B	16,964 m³	Volumes derived by Eagle Point 7.2. - Side slopes 1.5H:1V for fill less than 2m - Side slopes 2H:1V for fill greater than 2m - Fills are min. 1.0m - Volumes derived by merging Topography/ As-built to Pad Design Surfaces.
	Pad R Fill	1,134 m³	
	Pad C	16,197 m³	
	Pad D Fill	8,309 m³	
	Pad E/P	31,372 m³	
	Pad I	14,526 m³	
Total Fill:		88,502 m³	
2. Surface Grade Material	Pad B	1,267 m³	
	Pad R	1,148 m³	
	Pad C	1,153 m³	
	Pad D	2,167 m³	
	Pad E/P	2,628 m³	
	Pad I	2,474 m³	
Total Fill:		10,837 m³	
3. Volume of Overburden to be Relocated	Pad E/P	3,000 m³	Approximate
4. Volume of Rock Cut	Pad R Cut	15,660 m³	
	Pad D Cut	37,000 m³	
	Total Cut:	52,660 m³	

PAD E/P STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PEP-01	7,558,919.48	432,964.90	40.52
PEP-02	7,558,906.59	433,002.59	40.43
PEP-03	7,558,903.36	433,026.07	40.37
PEP-04	7,558,906.62	433,060.82	40.29
PEP-05	7,558,947.15	433,102.43	40.25
PEP-06	7,558,971.42	433,124.60	40.27
PEP-07	7,559,002.17	433,118.72	40.33
PEP-08	7,559,020.66	433,114.68	40.37
PEP-09	7,559,049.99	433,114.54	40.43
PEP-10	7,559,070.39	433,128.67	40.42
PEP-11	7,559,045.52	433,040.74	40.54

PAD I STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PI-01	7,559,066.95	433,130.11	38.63
PI-02	7,559,060.99	433,122.40	38.89
PI-03	7,559,047.97	433,117.24	38.96
PI-04	7,559,019.96	433,119.52	38.00
PI-05	7,558,970.86	433,130.17	37.53
PI-06	7,558,959.60	433,143.56	36.22
PI-07	7,558,982.64	433,194.28	35.97
PI-08	7,558,983.45	433,245.15	37.82
PI-09	7,559,016.12	433,298.27	37.57
PI-10	7,559,028.39	433,312.20	38.50
PI-11	7,559,041.74	433,314.16	39.50
PI-12	7,559,049.18	433,311.31	40.00
PI-13	7,559,058.11	433,299.18	40.34
PI-14	7,559,071.72	433,239.77	39.98
PI-15	7,559,083.02	433,174.19	41.04

PAD C STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PC-01	7,559,064.43	432,964.17	43.87
PC-02	7,559,024.15	432,979.50	43.86
PC-03	7,559,040.04	433,003.85	43.64
PC-04	7,559,017.28	432,976.48	41.12
PC-05	7,559,009.66	432,981.39	41.05
PC-06	7,559,033.11	433,009.57	43.60
PC-07	7,559,063.38	433,057.89	43.52
PC-08	7,559,070.80	433,090.58	43.00
PC-09	7,559,114.73	433,086.93	43.47
PC-10	7,559,116.72	433,068.83	43.56
PC-11	7,559,113.24	433,054.04	43.66
PC-12	7,559,121.20	433,053.17	43.71
PC-13	7,559,122.92	433,009.12	43.86
PC-14	7,559,108.11	433,004.73	43.85

PAD D STAKE-OUT TABLE				
ID	Northing	Easting	Base Elev.	Surface Elev.
PD-01	7,559,117.24	433,090.58	43.08	43.23
PD-02	7,559,176.74	433,090.58	43.38	43.53
PD-03	7,559,176.74	433,240.58	43.34	43.49
PD-04	7,559,121.97	433,240.58	43.07	43.22
PD-05	7,559,077.76	433,240.58	42.85	43.00
PD-06	7,559,086.30	433,204.33	42.85	43.00
PD-07	7,559,086.93	433,174.09	42.85	43.00
PD-08	7559081.901	433156.97	42.85	43.00
PD-09	7559076.325	433133.353	42.85	43.00

PAD B STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PB-01	7,559,173.16	432,994.14	49.49
PB-02	7,559,169.27	432,987.78	49.48
PB-03	7,559,151.46	432,989.70	49.39
PB-04	7,559,146.47	432,992.83	49.36
PB-05	7,559,135.72	433,005.25	49.30
PB-06	7,559,132.08	433,022.95	49.26
PB-07	7,559,132.48	433,055.63	49.23
PB-08	7,559,134.83	433,061.37	49.23
PB-09	7,559,161.67	433,067.92	49.36
PB-10	7,559,166.67	433,066.40	49.38
PB-11	7,559,163.24	433,058.91	49.37
PB-12	7,559,175.26	433,083.23	49.00
PB-13	7,559,180.79	433,083.05	49.50
PB-14	7,559,185.77	433,079.65	50.24
PB-15	7,559,195.22	433,052.24	50.00
PB-16	7,559,158.60	433,072.01	47.00
PB-17	7,559,157.48	433,080.02	47.00
PB-18	7,559,128.98	433,076.64	45.09
PB-19	7,559,123.04	433,073.57	45.09
PB-20	7,559,131.51	433,068.94	44.82
PB-21	7,559,128.15	433,067.20	44.82
PB-22	7,559,139.71	432,976.60	45.50
PB-23	7,559,156.19	432,973.70	46.50
PB-24	7,559,171.16	432,974.14	47.38
PB-25	7,559,179.89	432,979.71	48.00

PAD R STAKE-OUT TABLE				
ID	Northing	Easting	Base Elev.	Surface Elev.
PR-01	7,559,174.38	432,992.43	47.54	48.13
PR-02	7,559,182.97	432,990.53	47.63	48.16
PR-03	7,559,243.98	432,977.03	48.25	48.50
PR-04	7,559,257.17	433,036.67	48.25	48.50
PR-05	7,559,200.53	433,049.16	47.70	48.20
PR-06	7,559,184.44	433,052.90	47.54	48.14
PR-07	7,559,189.37	433,020.91	47.63	47.80

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1. Soil classification for these works is based on the Unified Soil Classification System (USCS).
2. On bare tundra surfaces the maximum snow thickness allowed prior to fill placement shall be 102mm (4"). On all other surfaces complete snow removal is required. The Engineer must approve all surfaces prior to placement of any construction material.
3. Snow and ice on construction material must be removed prior to loading for construction use.
4. Due care must be taken when placing fill materials such that no damage occurs to the subgrade and/or culverts. Any damage must be immediately reported to the Engineer.
5. Maximum lift thickness is 1.85m. Staged construction will be required where fill thickness exceeds 1.85m.
6. In areas where staged construction is required, all snow shall be removed and the surface scarified prior to placing the next lift. The Engineer will approve such staged construction.
7. Run of Quarry, and Surfacing material has to be compacted after placement.
8. Compaction will be a field specification, based on trial compaction tests to be carried out by the Contractor to the satisfaction of the Engineer.
9. It is the Contractor's responsibility to create the construction materials as specified through appropriate crushing. Any deviations must be approved by the Engineer.
10. Construction fill material shall be from approved rock quarries, shall be non-acid generating, free of organic material or similar impurities, as well as snow and ice.
11. Construction fill material must be free of overburden soils. Such unsuitable material shall be disposed of in a designated on site disposal area as outlined in the Contractors' quarry development plan.
12. Construction fill material will not have to be washed to remove blast residues or fines, unless specifically instructed by the Engineer.
13. Run of Quarry (ROQ) shall be well-graded, containing sufficient quantities of gravel, sand, and silt sized material. For fill thickness <0.85m the maximum boulder size shall not exceed 500mm. For fill thickness >0.85m the maximum boulder size shall not exceed 900mm.
14. Surfacing material shall be a well-graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 51mm (2") but no smaller than 32mm (1 $\frac{1}{4}$ ").
15.  $\frac{3}{4}$ " Finishing material shall be well graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 32mm (1 $\frac{1}{4}$ ") but no smaller than 19mm ( $\frac{3}{4}$ " )
16. ROQ material shall be visually inspected by the Engineer on a routine basis and the Contractor will be advised if the material does not meet the specification in Note 17.
17. The Contractor shall collect samples of the surfacing material directly from the crusher stockpile and submit for laboratory testing including but not limited to grain size distribution, and moisture content at least 1 sample every 8,000m<sup>3</sup>. The Engineer may conduct additional sampling and testing as deemed necessary.
18. Sample collection and testing of ROQ, and surface material for geochemical suitability is required and will be carried out by the Site Environmental Manager in accordance with procedures developed by SRK.

Item	Quantity / Area / Volume		Description
1. Run of Quarry Material	Pad B Pad R Fill Pad C Pad D Fill Pad E/P Pad I <hr/> Total Fill:	16,964 m³ 1,134 m³ 16,197 m³ 8,309 m³ 31,372 m³ 14,526 m³  88,502 m³	Volumes derived by Eagle Point 7.2. - Side slopes 1.5H:1V for fill less than 2m - Side slopes 2H:1V for fill greater than 2m - Fills are min. 1.0m - Volumes derived by merging Topography/ As-built to Pad Design Surfaces.
2. Surface Grade Material	Pad B Pad R Pad C Pad D Pad E/P Pad I <hr/> Total Fill:	1,267 m³ 1,148 m³ 1,153 m³ 2,167 m³ 2,628 m³ 2,474 m³  10,837 m³	
3. Volume of Overburden to be Relocated	Pad E/P	3,000 m³	Approximate
4. Volume of Rock Cut	Pad R Cut Pad D Cut <hr/> Total Cut:	15,660 m³ 37,000 m³  52,660 m³	

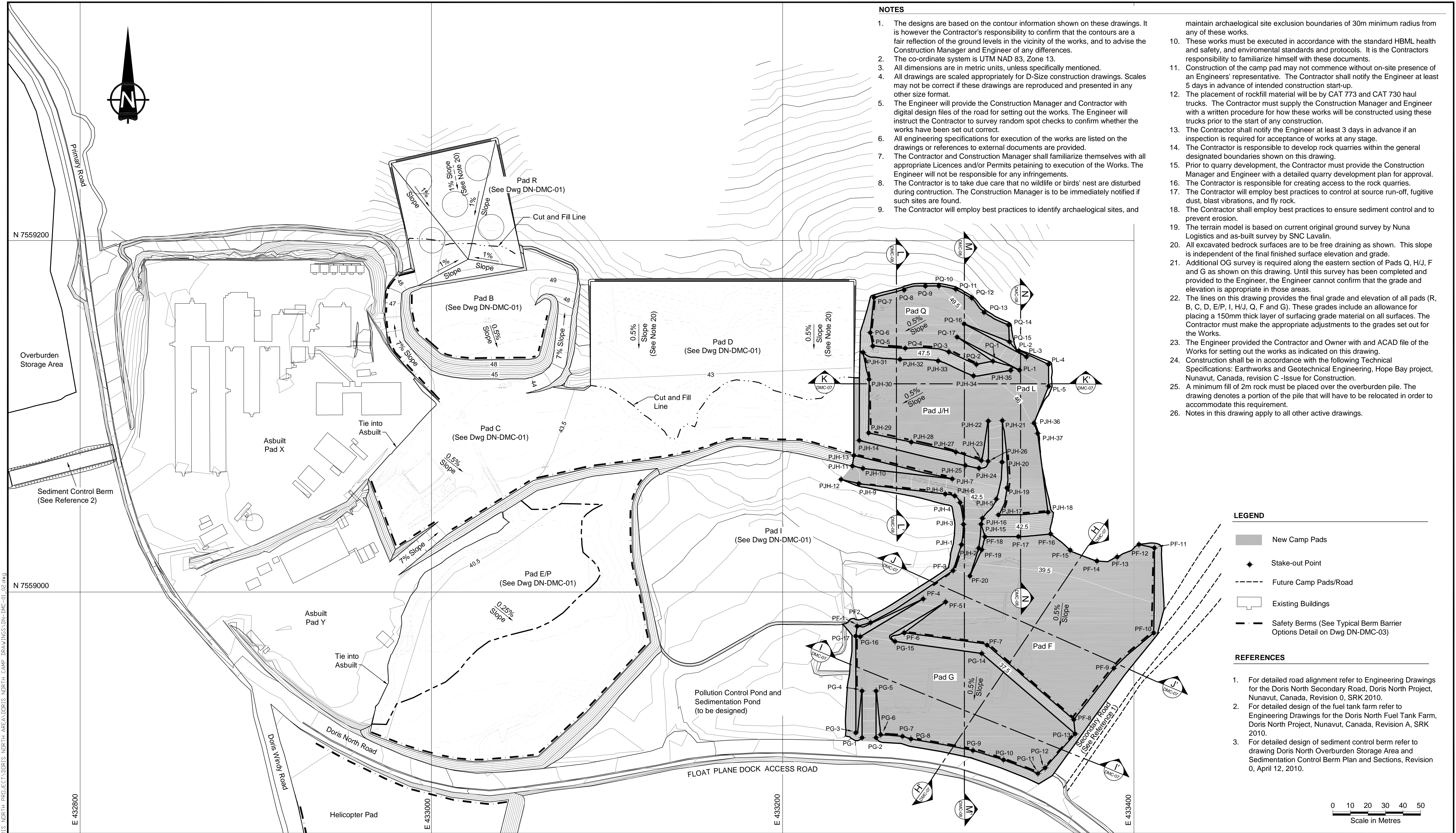
PAD I STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PI-01	7,559,066.95	433,130.11	38.63
PI-02	7,559,060.99	433,122.40	38.89
PI-03	7,559,047.97	433,117.24	38.96
PI-04	7,559,019.96	433,119.52	38.00
PI-05	7,558,970.86	433,130.17	37.53
PI-06	7,558,959.60	433,143.56	36.22
PI-07	7,558,982.64	433,194.28	35.97
PI-08	7,558,983.45	433,245.15	37.82
PI-09	7,559,016.12	433,298.27	37.57
PI-10	7,559,028.39	433,312.20	38.50
PI-11	7,559,041.74	433,314.16	39.50
PI-12	7,559,049.18	433,311.31	40.00
PI-13	7,559,058.11	433,299.18	40.34
PI-14	7,559,071.72	433,239.77	39.98
PI-15	7,559,083.02	433,174.19	41.04

PAD D STAKE-OUT TABLE				
ID	Northing	Easting	Base Elev.	Surface Elev.
PD-01	7,559,117.24	433,090.58	43.25	43.40
PD-02	7,559,176.74	433,090.58	43.85	44.00
PD-03	7,559,176.74	433,240.58	43.85	44.00
PD-04	7,559,121.97	433,240.58	43.25	43.40
PD-05	7,559,077.76	433,240.58	42.85	43.00
PD-06	7,559,086.30	433,204.33	42.85	43.00
PD-07	7,559,086.93	433,174.09	42.85	43.00
PD-08	7559081.901	433156.97	42.85	43.00
PD-09	7559076.325	433133.353	42.85	43.00

PAD R STAKE-OUT TABLE				
ID	Northing	Easting	Base Elev.	Surface Elev.
PR-01	7,559,174.38	432,992.43	47.54	48.13
PR-02	7,559,182.97	432,990.53	47.63	48.16
PR-03	7,559,243.98	432,977.03	48.25	48.50
PR-04	7,559,257.17	433,036.67	48.25	48.50
PR-05	7,559,200.53	433,049.16	47.70	48.20
PR-06	7,559,184.44	433,052.90	47.54	48.14
PR-07	7,559,189.37	433,020.91	47.63	47.80

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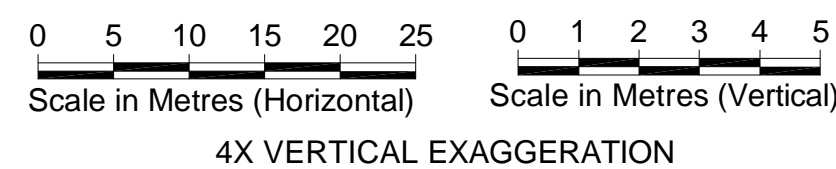




For detailed road alignment refer to Engineering Drawings for the Doris North Secondary Road, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.

- |   |                         |
|---|-------------------------|
|  | Existing ground surface |
|  | Surfacing Material      |
|  | Run of Quarry Material  |

1. All dimensions in metres unless noted otherwise.
2. Where the thickness of the pads is greater than 3.0m allow for the placement of barriers.
3. The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
4. Notes in this drawing apply to all other active drawings.

[illegible]

Original Drawings  
Stamped and  
Signed by Engineer



DESIGN: LW	DRAWN: MDDS	REVIEWED: EMR
CHECKED: LW	APPROVED: EMR	DATE: JUNE 24, 2010

FILE NAME: DN-DMC-06\_08.dwg



HOPE BAY MINING LIMITED

SRK JOB NO.: 1CH008 027

SRK DWG NO.: DN-DMC-07

DORIS NORTH PROJECT

DRAWING TITLE:

DORIS NORTH CAMP  
SECTIONS AND DETAILS  
(EAST SIDE)

NEMONT DRAWING NO.

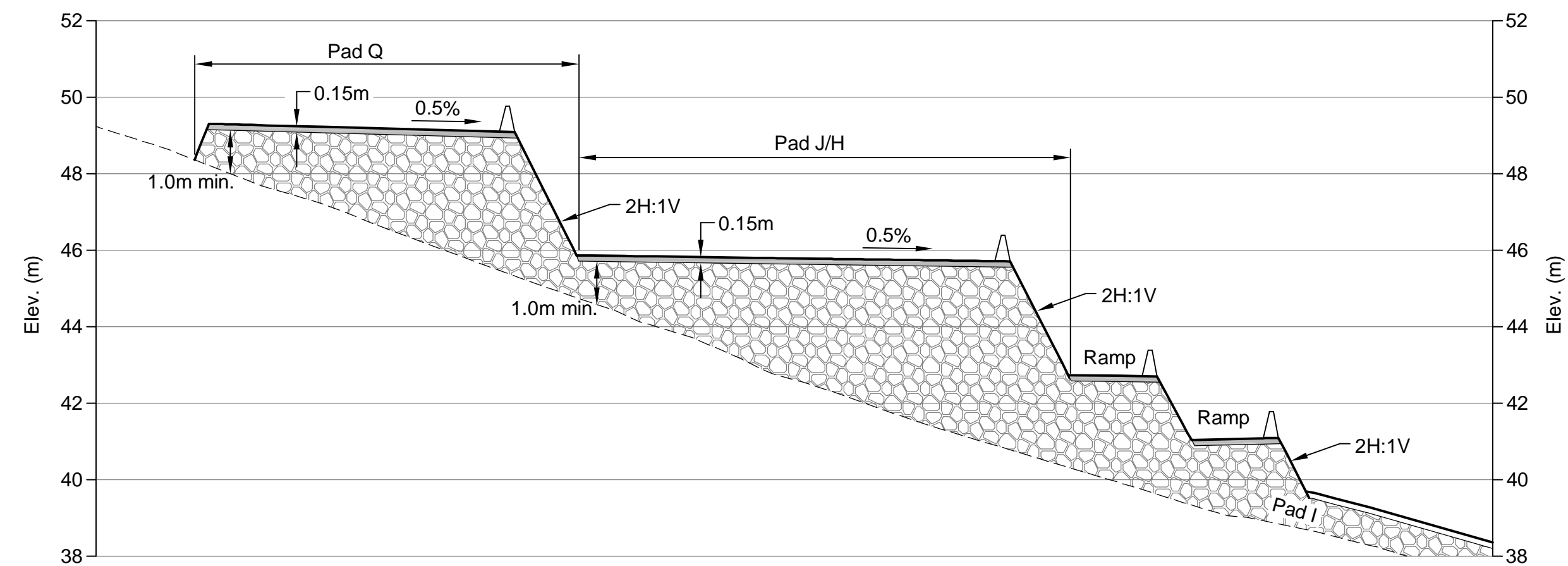
HB+D-CIV-CIV-OND-0049

SHEET

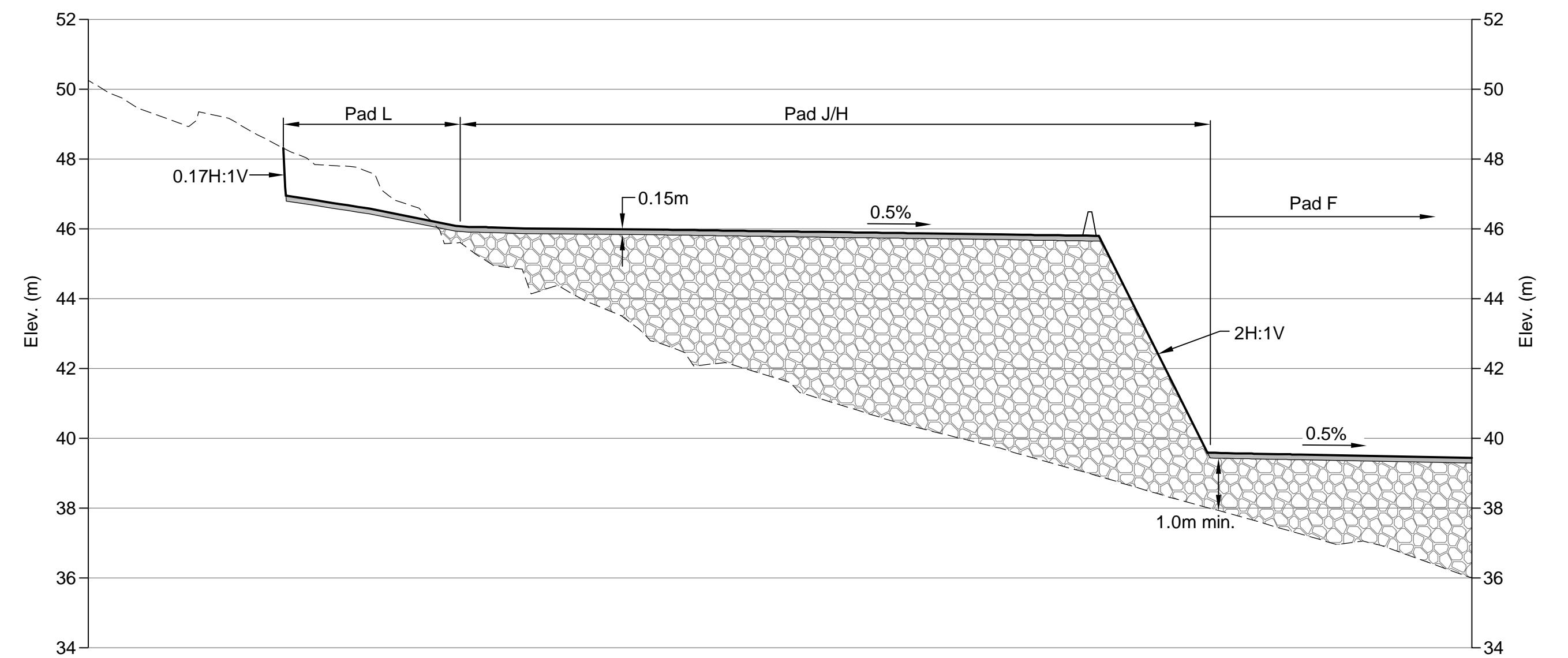
8 OF 10

DIVISION NO.

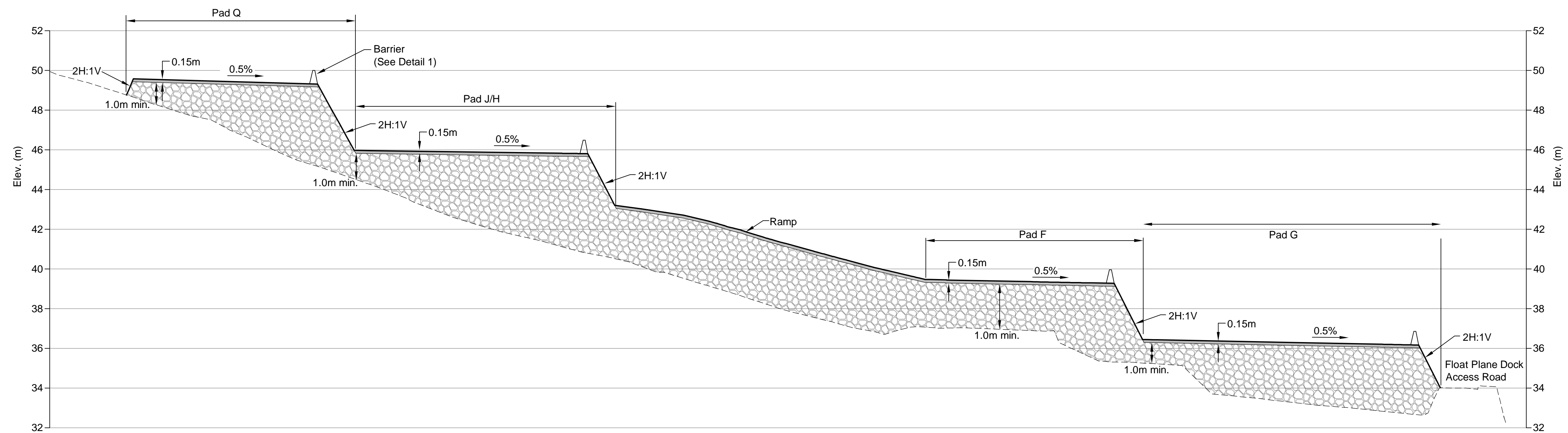
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**L** SECTION



**N** **SECTION**






**M** \_\_\_\_\_ **SECTION**

## REFERENCE

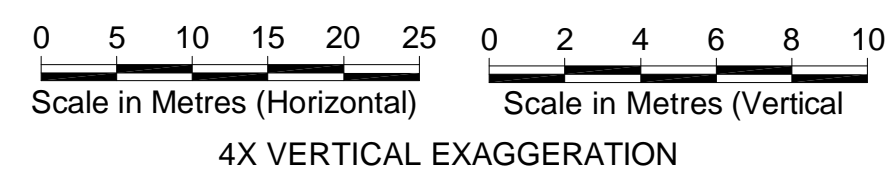
For detailed road alignment refer to Engineering Drawings for the Doris North Secondary Road, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.

## LEGEND

 Existing ground surface  
 Surfacing Material  
 Run of Quarry Material

## NOTES

1. All dimensions in metres unless noted otherwise.
2. Where the thickness of the pads is greater than 3.0m allow for the placement of barriers.
3. The barriers are to consist of boulders larger than 1m in diameter, jersey-barriers (1.82 long X 1.37m high X 0.61m wide) or a rock fill berm 0.5m high. Maximum spacing between barriers is 3.3m.
4. Notes in this drawing apply to all other active drawings.

[illegible]

Original Drawings  
Stamped and  
Signed by Engineer



DESIGN: LW	DRAWN: MDDS	REVIEWED: EMR
CHECKED: LW	APPROVED: EMR	DATE: JUNE 24, 2010

FILE NAME:	DN-DMC-06_08.dwg
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HOPE BAY MINING LIMITED

SRK JOB NO.:	1CH008 027
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SRK DWG NO.: DN-DMC-08

DORIS NORTH PROJECT

DRAWING TITLE:

DORIS NORTH CAMP  
SECTIONS AND DETAILS  
(EAST SIDE)

NEMONT DRAWING NO.

HB+D-CIV-CIV-OND-0050

SHEET	REVISION NO.
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9 OF 10

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NOTES

1. Soil classification for these works are based on the Unified Soil Classification System (USCS).
2. On bare tundra surfaces the maximum snow thickness allowed prior to fill placement shall be 102mm (4"). On all other surfaces complete snow removal is required. The Engineer must approve all surfaces prior to placement of any construction material.
3. Snow and ice on construction material must be removed prior to loading for construction use.
4. Due care must be taken when placing fill materials such that no damage occurs to the subgrade and/or culverts. Any damage must be immediately reported to the Engineer.
5. Maximum lift thickness is 1.85m. Staged construction will be required where fill thickness exceeds 1.85m.
6. In areas where staged construction is required, all snow shall be removed and the surface scarified prior to placing the next lift. The Engineer will approve such staged construction.
7. Run of Quarry, and Surfacing material has to be compacted after placement.
8. Compaction will be a field specification, based on trial compaction tests to be carried out by the Contractor to the satisfaction of the Engineer.
9. It is the Contractor's responsibility to create the construction materials as specified through appropriate crushing. Any deviations must be approved by the Engineer.
10. Construction fill material shall be from approved rock quarries, shall be non-acid generating, free of organic material or similar impurities, as well as snow and ice.
11. Construction fill material must be free of overburden soils. Such unsuitable material shall be disposed of in a designated on site disposal area as outlined in the Contractors' quarry development plan.
12. Construction fill material will not have to be washed to remove blast residues or fines, unless specifically instructed by the Engineer.
13. Run of Quarry (ROQ) shall be well-graded, containing sufficient quantities of gravel, sand, and silt sized material. For fill thickness <0.85m the maximum boulder size shall not exceed 500mm. For fill thickness >0.85m the maximum boulder size shall not exceed 900mm.
14. Surfacing material shall be a well-graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 51mm (2") but no smaller than 32mm (1 $\frac{1}{4}$ ").
15.  $\frac{3}{4}$ " Finishing material shall be well graded manufactured crush product produced from ROQ material. The screen size shall be no greater than 32mm (1 $\frac{1}{4}$ ") but no smaller than 19mm ( $\frac{3}{4}$ ")
16. ROQ material shall be visually inspected by the Engineer on a routine basis and the Contractor will be advised if the material does not meet the specification in Note 17.
17. The Contractor shall collect samples of the surfacing material directly from the crusher stockpile and submit for laboratory testing including but not limited to grain size distribution, and moisture content at least 1 sample every 8,000m<sup>3</sup>. The Engineer may conduct additional sampling and testing as deemed necessary.
18. Sample collection and testing of ROQ, and surface material for geochemical suitability is required and will be carried out by the Site Environmental Manager in accordance with procedures developed by SRK.

## Materials List and Quantities

Item	Quantity / Area / Volume	Description
1. Run of Quarry Material	<div> <div>Pad G/F</div> <div>Pad J/H</div> <div>Pad Q</div> </div> <div> <div>48,740 m³</div> <div>26,440 m³</div> <div>6,820 m³</div> </div> <hr/> <div> <div>Total Fill:</div> <div>Total Cut: (Pad L)</div> </div> <div> <div>82,000 m³</div> <div>1,293 m³</div> </div>	Volumes derived by Eagle Point 7.2. - Side slopes 1.5H:1V for fill less than 2m - Side slopes 2H:1V for fill greater than 2m - Fills are min. 1.0m - Volumes derived by merging Topography/ As-built to Pad Design Surfaces.
2. Surface Grade Material	<div> <div>Pad G/F</div> <div>Pad J/H</div> <div>Pad L</div> <div>Pad Q</div> </div> <div> <div>2,200 m³</div> <div>1,100 m³</div> <div>75 m³</div> <div>380 m³</div> </div> <hr/> <div> <div>Total Fill:</div> </div> <div> <div>3,755 m³</div> </div>	
3. Volume of Rock Cut	<div> <div>Pad L</div> </div> <div> <div>1,293 m³</div> </div> <hr/> <div> <div>Total Fill:</div> </div> <div> <div>1,293 m³</div> </div>	

PAD G STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PG-1	7,558,917.14	433,245.19	34.33
PG-2	7,558,917.14	433,253.19	34.33
PG-3	7,558,919.92	433,241.61	36.18
PG-4	7,558,943.72	433,245.19	36.19
PG-5	7,558,943.72	433,253.19	36.19
PG-6	7,558,918.85	433,255.82	36.18
PG-7	7,558,917.89	433,268.24	36.18
PG-8	7,558,916.38	433,273.04	36.18
PG-9	7,558,909.84	433,308.33	36.17
PG-10	7,558,904.51	433,325.76	36.15
PG-11	7,558,896.74	433,345.32	36.14
PG-12	7,558,899.95	433,349.42	36.16
PG-13	7,558,919.31	433,366.52	36.28
PG-14	7,558,965.06	433,313.33	36.45
PG-15	7,558,971.88	433,263.96	36.45
PG-16	7,558,974.62	433,244.15	36.46
PG-17	7,558,974.97	433,241.61	36.46

PAD L STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PL-1	7,559,126.33	433,334.82	46.57
PL-2	7,559,137.40	433,334.44	47.82
PL-3	7,559,133.85	433,338.80	46.62
PL-4	7,559,129.64	433,351.42	46.80
PL-5	7,559,117.13	433,351.48	46.80

### PAD F STAKE-OUT TABLE



PAD F STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PF-1	7,558,980.57	433,242.39	39.29
PF-2	7,558,982.64	433,250.10	39.30
PF-3	7,559,012.23	433,297.05	39.47
PF-4	7,558,996.12	433,280.03	39.39
PF-5	7,558,994.36	433,292.76	39.38
PF-6	7,558,976.86	433,269.22	39.28
PF-7	7,558,969.78	433,316.43	39.27
PF-8	7,558,927.43	433,365.74	39.11
PF-9	7,558,956.65	433,388.55	39.29
PF-10	7,558,976.76	433,411.24	39.42
PF-11	7,559,025.12	433,411.70	39.65
PF-12	7,559,027.09	433,402.83	39.65
PF-13	7,559,020.01	433,390.48	39.59
PF-14	7,559,017.67	433,378.95	39.56
PF-15	7,559,023.83	433,363.75	39.56
PF-16	7,559,033.08	433,352.56	39.61
PF-17	7,559,031.57	433,334.20	39.59
PF-18	7,559,031.46	433,315.17	39.58
PF-19	7,559,024.15	433,313.39	39.54
PF-20	7,559,009.14	433,306.58	39.46

### PAD J/H STAKE-OUT TABLE

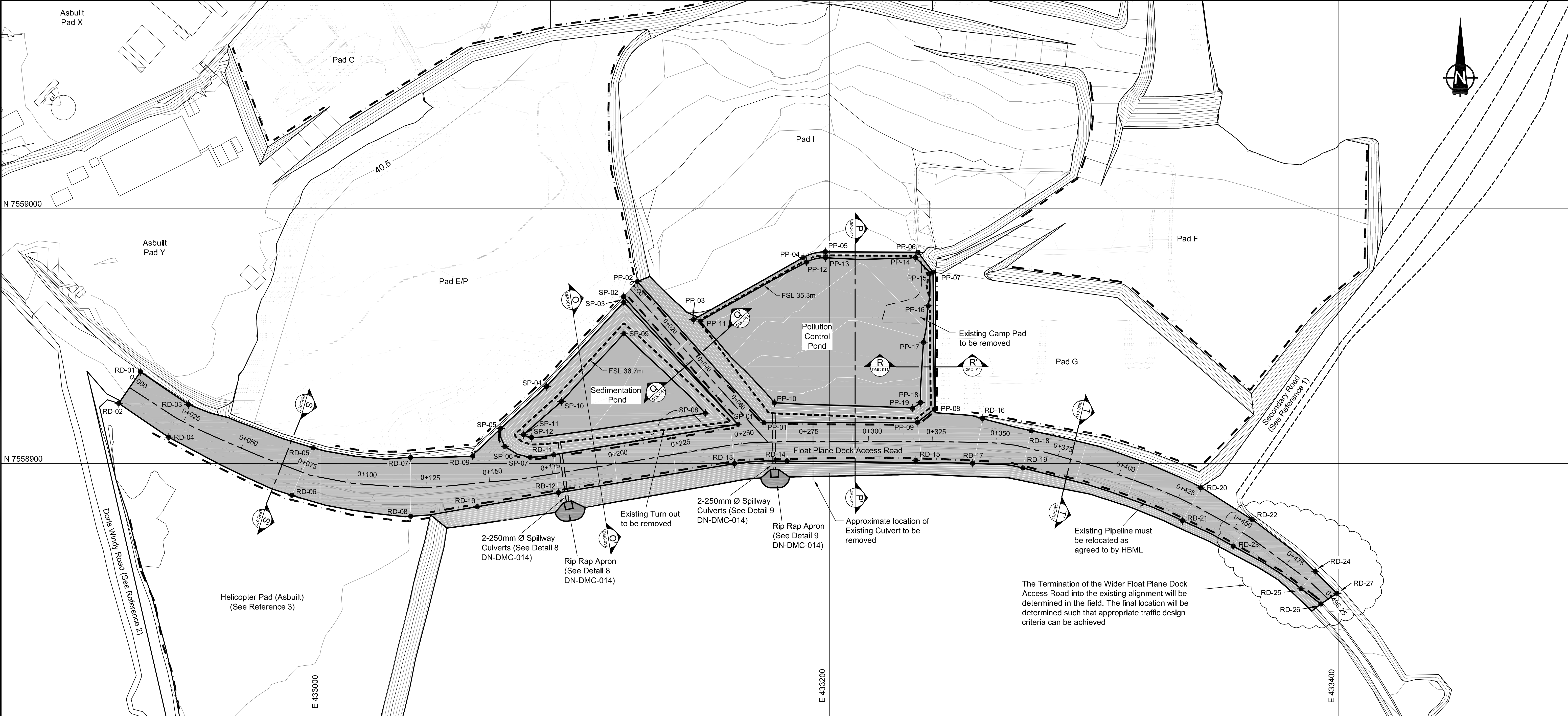
PAD J/H STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PJH-1	7,559,026.96	433,301.80	40.49
PJH-2	7,559,024.92	433,311.62	40.50
PJH-3	7,559,038.53	433,302.95	41.30
PJH-4	7,559,050.91	433,301.00	42.19
PJH-5	7,559,052.90	433,321.65	42.50
PJH-6	7,559,054.73	433,298.01	42.50
PJH-7	7,559,064.41	433,296.57	42.65
PJH-8	7,559,055.81	433,292.60	42.50
PJH-9	7,559,061.74	433,243.35	40.00
PJH-10	7,559,070.37	433,245.72	40.08
PJH-11	7,559,071.75	433,239.69	39.97
PJH-12	7,559,064.08	433,233.21	39.41
PJH-13	7,559,077.76	433,240.58	43.00
PJH-14	7,559,086.29	433,243.58	42.99
PJH-15	7,559,038.62	433,312.98	41.30
PJH-16	7,559,041.97	433,313.75	41.54
PJH-17	7,559,043.91	433,322.85	45.76
PJH-18	7,559,045.51	433,351.10	45.86
PJH-19	7,559,059.26	433,327.68	45.83
PJH-20	7,559,073.95	433,324.94	44.00
PJH-21	7,559,097.56	433,325.07	45.90
PJH-22	7,559,097.31	433,317.07	45.90
PJH-23	7,559,074.83	433,313.15	45.80
PJH-24	7,559,070.49	433,311.79	43.50
PJH-25	7,559,072.03	433,304.08	43.00
PJH-26	7,559,074.43	433,316.96	44.00
PJH-27	7,559,079.67	433,298.63	45.80
PJH-28	7,559,085.34	433,273.21	45.73
PJH-29	7,559,090.58	433,248.91	45.66
PJH-30	7,559,121.06	433,249.20	45.77
PJH-31	7,559,136.34	433,245.76	45.65
PJH-32	7,559,132.38	433,266.86	45.87
PJH-33	7,559,131.58	433,288.51	45.95
PJH-34	7,559,123.06	433,308.68	45.99
PJH-35	7,559,126.18	433,329.85	46.07
PJH-36	7,559,096.16	433,343.08	46.41
PJH-37	7,559,090.29	433,345.20	46.00

### PAD J/H STAKE-OUT TABLE

PAD J/H STAKE-OUT TABLE			
ID	Northing	Easting	Elevation (m)
PQ-1	7,559,131.39	433,319.74	49.43
PQ-2	7,559,129.58	433,310.31	49.35
PQ-3	7,559,136.68	433,294.61	49.28
PQ-4	7,559,138.75	433,269.81	49.12
PQ-5	7,559,139.99	433,251.34	48.99
PQ-6	7,559,147.66	433,249.88	49.04
PQ-7	7,559,167.67	433,252.02	49.19
PQ-8	7,559,171.97	433,269.13	49.34
PQ-9	7,559,174.14	433,281.23	49.45
PQ-10	7,559,174.27	433,288.92	49.50
PQ-11	7,559,172.34	433,298.58	49.56
PQ-12	7,559,167.34	433,307.69	49.59
PQ-13	7,559,159.20	433,314.65	49.58
PQ-14	7,559,151.90	433,328.68	49.63
PQ-15	7,559,142.45	433,329.17	49.57
PQ-16	7,559,152.65	433,303.31	49.46
PQ-17	7,559,145.11	433,299.32	49.38

										Original Drawings Stamped and Signed by Engineer										<div><b>SRK Consulting</b> Engineers and Scientists Vancouver</div>										<div><b>NEWMONT.</b> NORTH AMERICA</div>										Doris North 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LEGEND

New Ponds and Raised Road

Stake-out Point

Existing Buildings

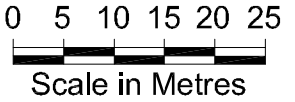
Safety Berms (See Typical Berm Barrier Options Detail on Dwg DN-DMC-03)

- REFERENCES
1.

For detailed road alignment refer to Engineering Drawings for the Doris North Secondary Road, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.
2.

For detailed road alignment refer to Engineering Drawings for the Doris-Windy All Weather Road, Doris Infrastructure Project, Nunavut, Canada, Revision 4, SRK 2010.
3.

For detailed design refer to Engineering drawings from the Laydown Pad Expansions, Doris North Project, Nunavut, Canada, Revision 0, SRK 2010.

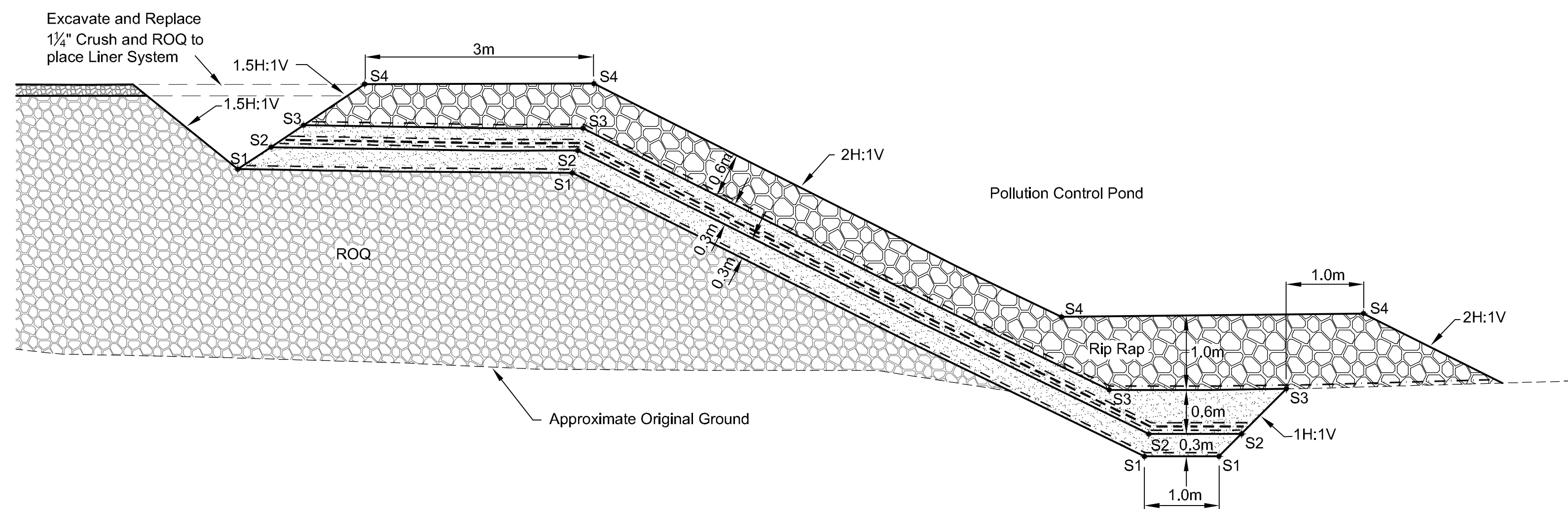
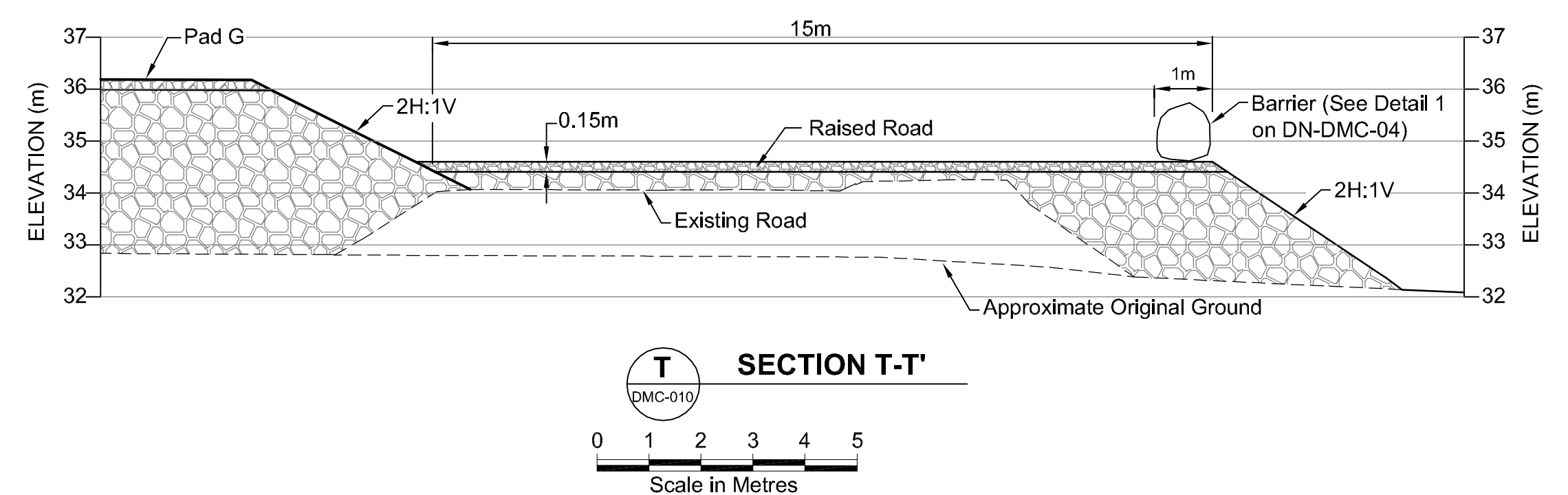
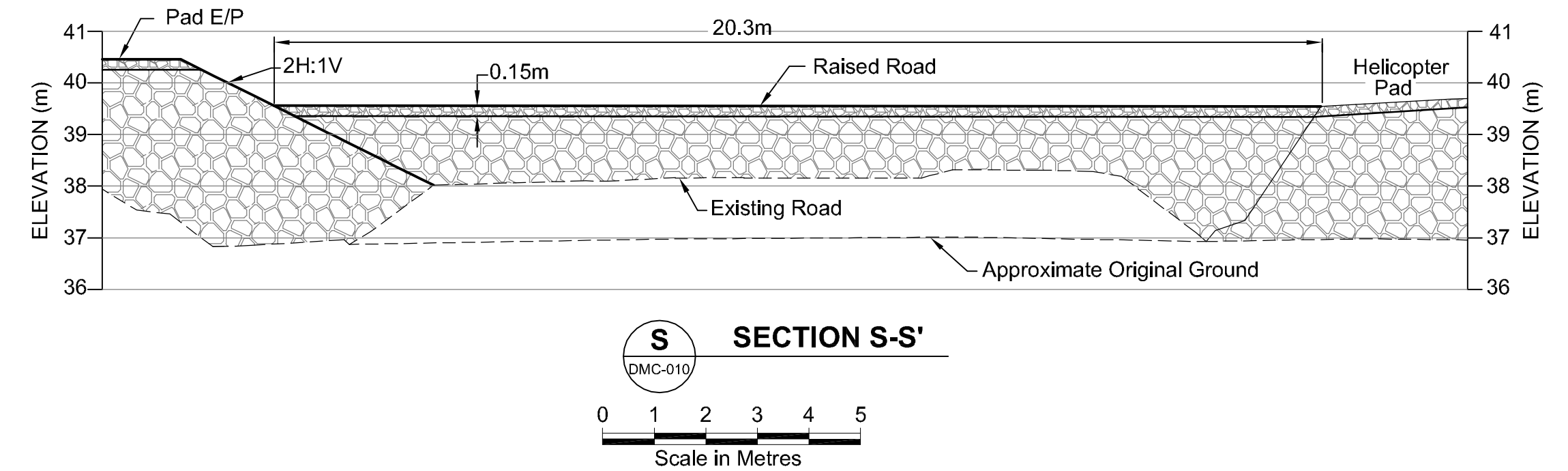
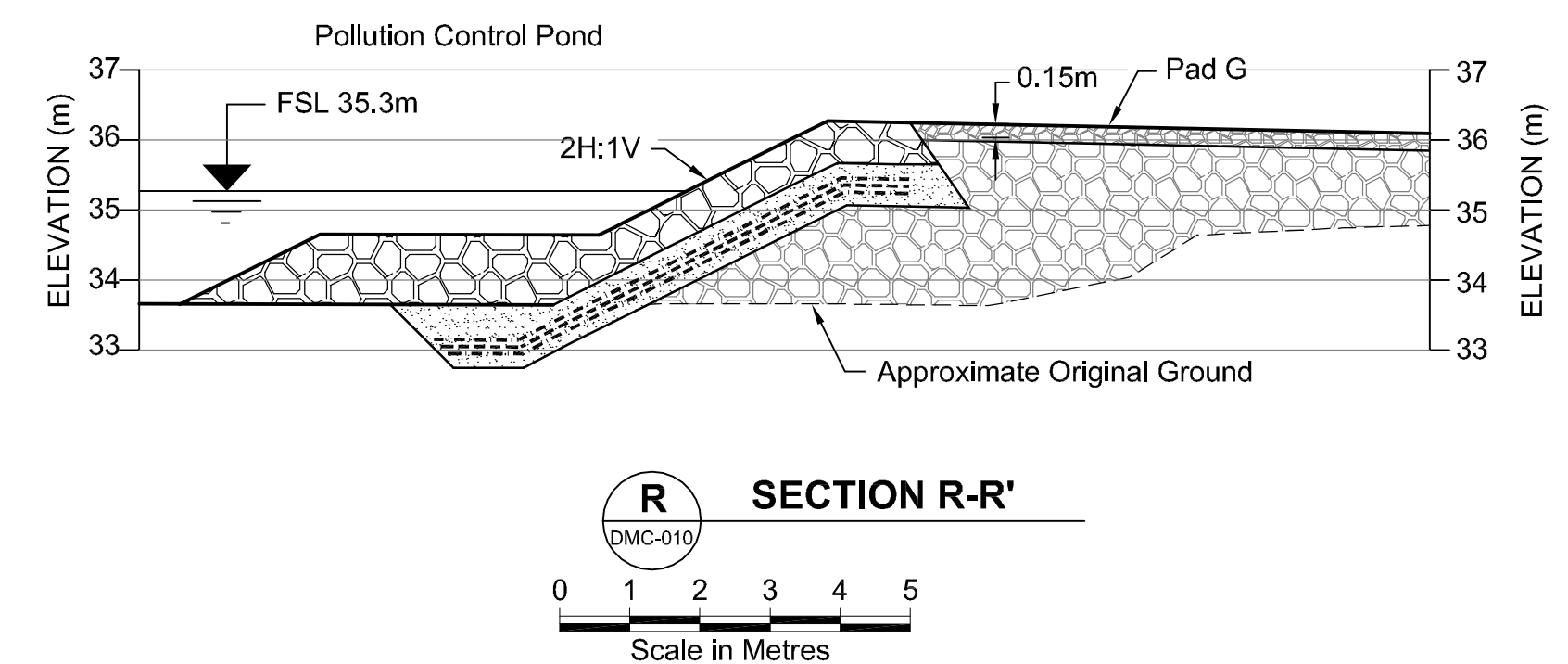
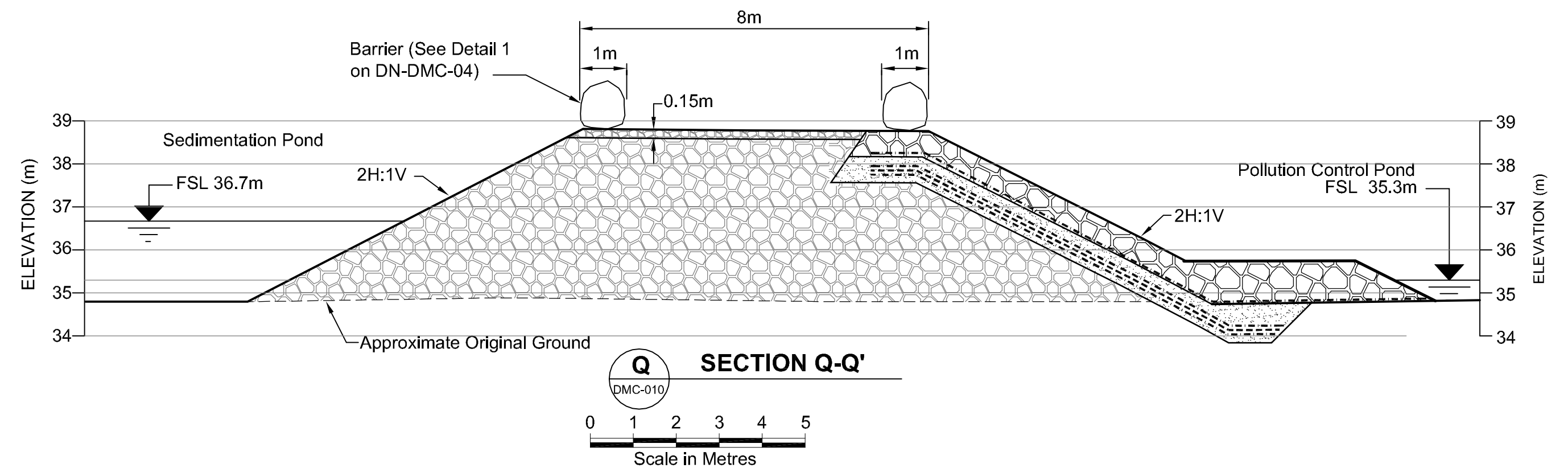
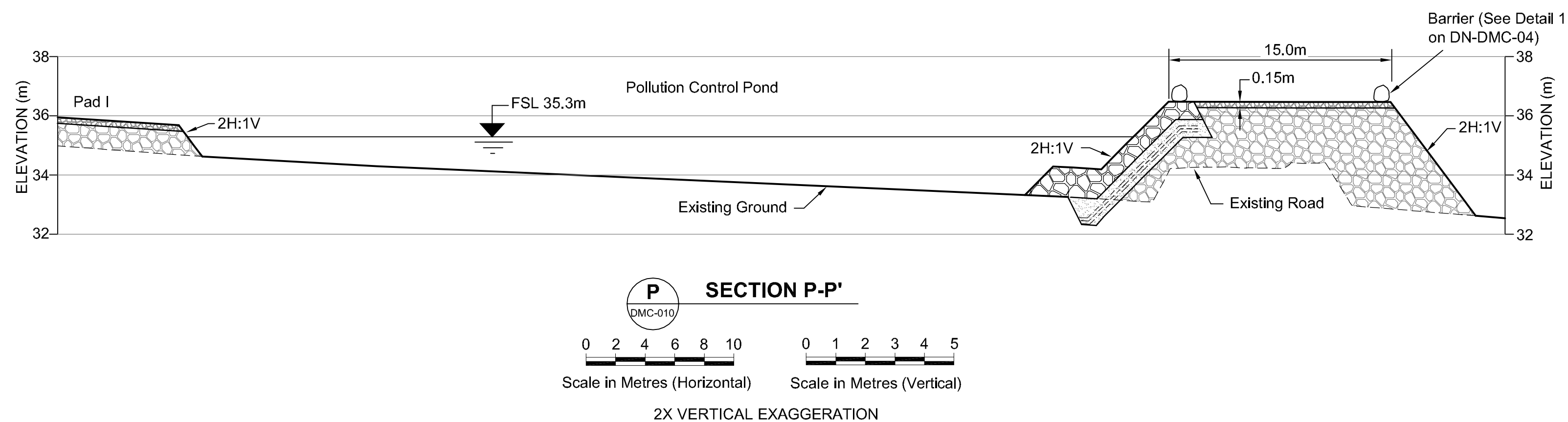
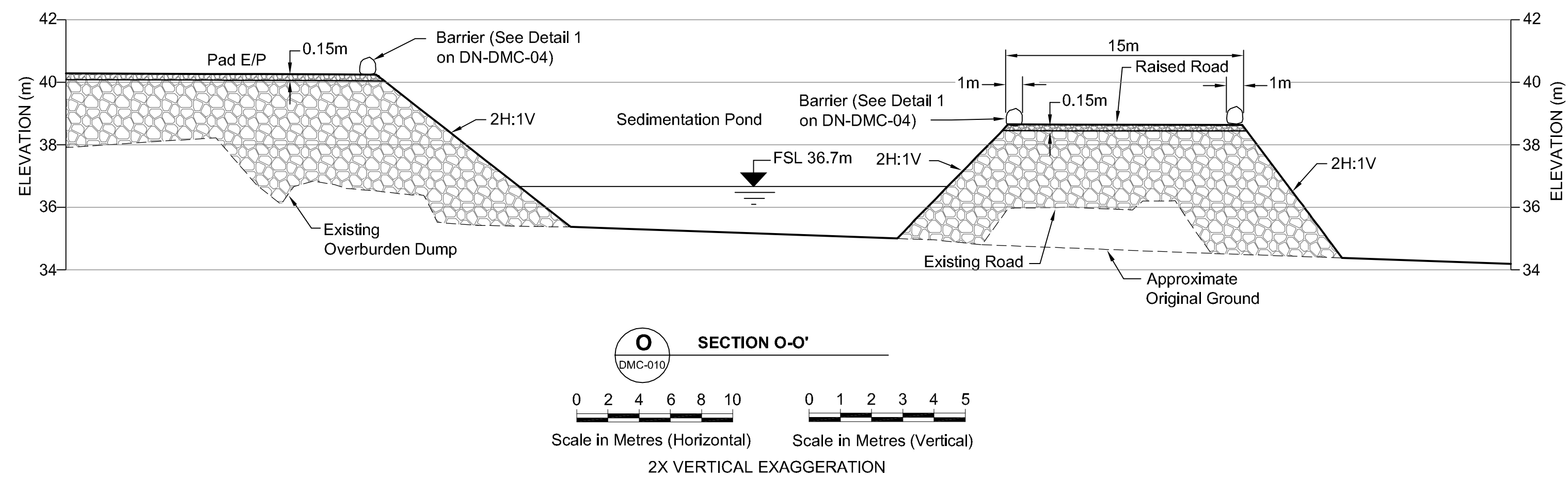




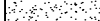
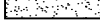
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<b>LEGEND</b>			
	¾" Crushed Material	----	Existing ground surface
	1 ¼" Crushed Material	-----	Textured 60 mil HDPE Liner
	Run of Quarry Material	- · - · -	12 oz. Non-woven Geotextile
	Rip Rap	S1 +	Surface Stake Out Points

**7** **TYPICAL DETAIL 7 - POLLUTION CONTROL POND**  
**TYPICAL SURFACE REFERENCE SECTION**

0 0.5 1 1.5 2 2.5  
 Scale in Metres

[illegible]

Original Drawings  
Stamped and  
Signed by Engineer

PROFESSIONAL ENGINEERS STAMP

Original Drawings  
Stamped and  
Signed by Engineer



**SRK Consulting**  
Engineers and Scientists  
Vancouver

DESIGN: MK	DRAWN: MDDS	REVIEWED: EMR
CHECKED: LW	APPROVED: EMR	DATE: Sept. 29,

FILE NAME: DN-DMC-010.dwg



HOPE BAY MINING LIMITED

SRK JOB NO.: 1CH008 027	SRK DWG NO.: DN-DMC-011
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SRK DWG NO.: DN-DMC-011

DORIS NORTH PROJECT

DRAWING TITLE:

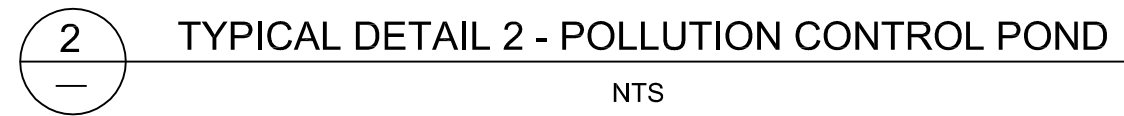
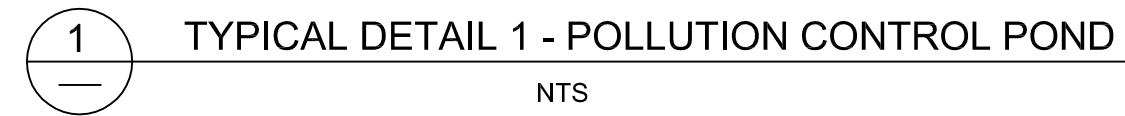
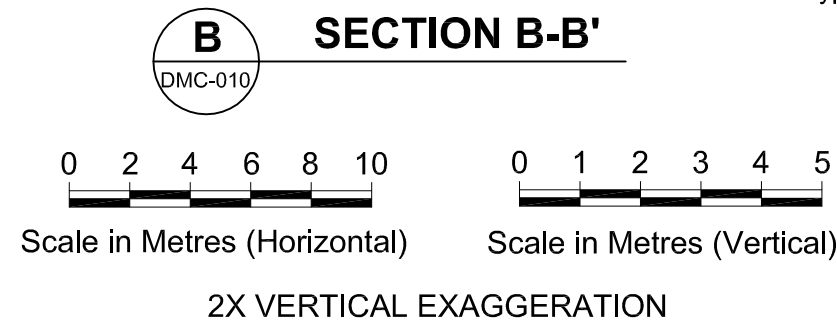
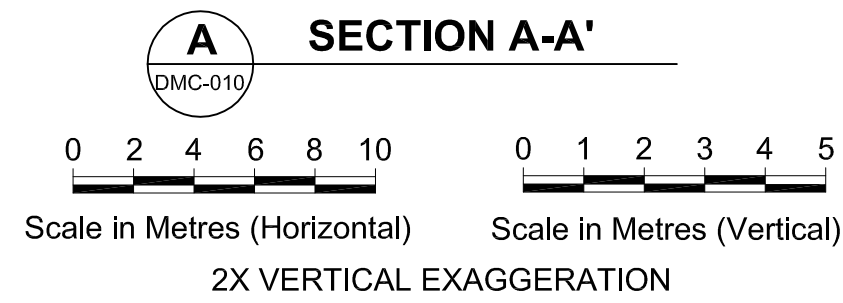
### Sedimentation and Pollution Control Ponds Sections




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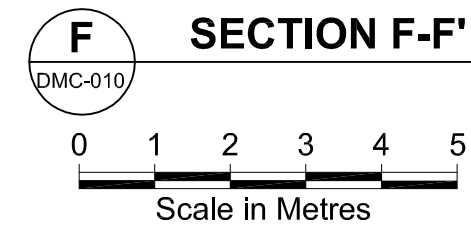
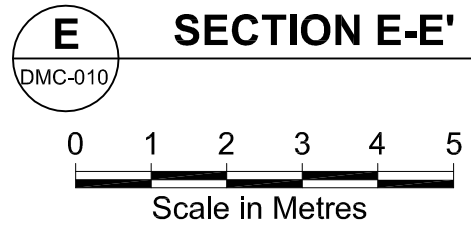
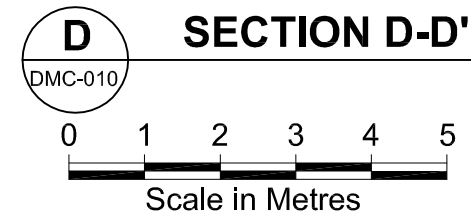
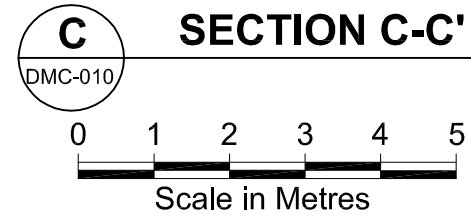
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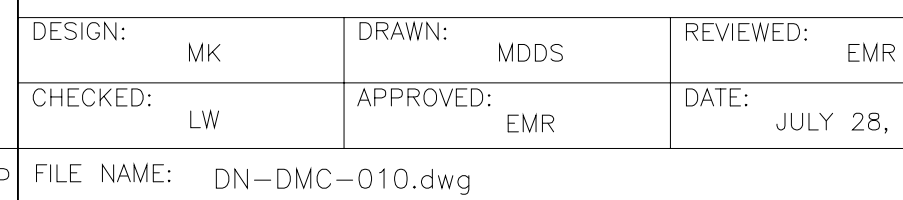




	3/4" Crushed Material	----	Existing ground surface
	1 1/4" Crushed Material	-----	Textured HDPE Liner
	Run of Quarry Material	-----	12 oz. Non-woven Geotextile

[illegible]

PROFESSIONAL ENGINEERS STAMP



HOPE BAY MINING LIMITED

SRK JOB NO.: 1CH008 027

DRAWING TITLE:

## Sedimentation and Pollution Control Ponds Sections

NEMONT DRAWING NO.

SHEET  
12 OF 15

REVISION NO.
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