SHEAR DIAMONDS LTD.

2011 ANNUAL GEOTECHNICAL INSPECTION JERICHO DIMOAND MINE, NT

REPORT

SEPTEMBER 2011
ISSUED FOR USE
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1.0 INTRODUCTION

EBA, A Tetra Tech Company (EBA) was retained by Shear Diamonds Ltd. (Shear) to complete the 2011 annual geotechnical inspection of earthworks infrastructure at the Jericho Diamond Mine (Jericho), NT.

This report presents the observations noted during the inspection and provides recommendations for each structure. It is understood that Shear will also use this report to fulfill the water licence requirement for an annual geotechnical inspection of the completed water retaining structures as specified in the Jericho Diamond Mine Water Licence NEWB1JER0410, Part G, Section 4.

The following structures were evaluated during the inspection:

- Processed Kimberlite Containment Area (PKCA) Dams and Dykes (West Dam, Divider Dyke A, East Dam, and Southeast Dam);
- C1 Diversion channel;
- Fuel Containment Facilities (Fuel tank farm, airstrip, and generator containment facilities);
- Landfill;
- Hazardous Waste Transfer Area; and
- Waste Rock Pile.

Areas evaluated during the 2011 inspection are shown in Figures 1 and 2.

2.0 INSPECTION METHODOLOGY

The inspection of all structures except the waste rock pile was completed by Mr. Gary Koop, P.Eng., of EBA, from July 15 to18 , 2011. The waste rock pile inspection was completed by Mr. Bill Horne, P.Eng of EBA on August 26, 2011. Each structure and its immediate surroundings were visually examined for signs of settlement, seepage, cracking, erosion, or other signs of distress. Observations made during the inspection were photographed and recorded. Photographs of the general condition of each site were also taken to track year by year changes to each structure.

Ground temperature data has been collected by the site personal at intervals throughout the year with the most recent set of readings obtained on July 14, 2011. The data plots were reviewed in conjunction with the observations from each site to develop any recommendations presented for each structure.

Some water elevations were measured as part of survey work completed on site, and were referenced where available. Where no water elevations were available, a qualitative assessment was made based on observation and comparison with records and photos from previous inspections.

Observations and measurements pertaining to each structure are discussed in the following sections. Site plans, photographs, and ground temperatures are presented in Appendices B through K.

The 2010 inspection was completed in September shortly after Shear's purchase of Jericho. This inspection was completed later in the year than the 2011 inspection. There was a significant amount of snow cover

during the 2010 inspection making comparison with the 2011 photo record difficult. Furthermore, ground temperature readings in 2010 were taken approximately two months later than the 2011 readings. As such, a comparison of the ground temperatures between the two inspections was not possible. The 2009 inspection, completed in mid-July, provided a better comparison with current inspection observations, and was therefore, used as the basis of evaluating changes in the site structures.

3.0 BACKGROUND INFORMATION

The Jericho Mine was operated by Tahera Diamond Corporation from 2006 through 2008. In early 2008, operations ceased due to financial difficulties. INAC assumed the care and maintenance of the mine in December 2008 until Shear purchased the facility in late August, 2010. At the time of the inspection, Shear had not yet resumed mining activities and was operating under care and maintenance.

During the past mine operation, fine processed kimberlite (PK) was deposited in a basin south of the main process plant as shown in Figure 2. The fine PK deposited in the eastern portion of the facility is contained by two dams, the East Dam and the Southeast Dam, and by one filter dyke, Divider Dyke A.

Fine PK effluent is filtered through Divider Dyke A into the western portion of the facility (Cells B and C). The West Dam and the surrounding high ground impound the water in the cells until it is discharged by pumping over the West Dam. At present, Cells B and C are only being used as polishing ponds for the facility. The construction of the dams and dykes was staged such that each structure was constructed as required. The current status of the dams and dykes is summarized in Table 1.

Table 1: Summary of Jericho PKCA Structures

Structure	Status	Design Crest Elevation (m)	As-Built Crest Elevation (approx.) (m)	Comment
East Dam	Completed	524.5 Crest 523.5 Liner	524.5 Crest 523.5 Liner	A road was constructed on top of the East Dam. The crest of the road is approximately 527 m.
Southeast Dam	Completed	524.5 Crest 523.5 Liner	524.5 Crest 523.5 Liner	
Divider Dyke A	Partially Constructed	524	Varies – low point 521.5	
Divider Dyke B	Not in Place	524	-	
West Dam	Partially Constructed	528 Crest 524 Core	525 (min.) Crest 520 (min.) Core	
North Dam	Not in Place – upstream cofferdam constructed in 2007	528 Crest 524 Core	Coffer Dam Till 521 ROM Crest 522	Natural Ground of North Dam saddle 518.2 m (approx.)
Perimeter PK Containment Berm	Not in Place	528.5	–	

4.0 WEST DAM

The West Dam was inspected on July 15, 2011. The inspection summary, photographs, figures, and ground temperature profiles pertaining to the dam are provided in Appendix B.

Construction History

The West Dam is a frozen core dam located at the west end of the PKCA, as indicated in Figure B.1. The dam core comprises of frozen saturated gravel with secondary containment provided by a geosynthetic clay liner. The West Dam was partially constructed during the winters of 2005/2006 and 2006/2007. At the end of the 2007 season, the core had been constructed to its final elevation at the south abutment; however, the centre portion of the core was only constructed to approximately 520 m instead of its final design elevation of 524 m. The core was subsequently covered with a minimum of approximately 4 m of run-of-mine coarse rockfill for thermal protection.

Surface Water and Seepage

The water level upstream of the dam was 516.84 on July 20, 2011, and 514.26 on the downstream side. This equates to approximately 2.6 m of head across the dam. No discharge was occurring at the time of the inspection; however, a pump and associated piping were in place. Water, when discharged, is being released into a metal flume and against a geotextile lined energy dissipater. The discharge location and dissipater are shown in Photos B1 and B2, respectively.

The downstream slope and toe was inspected and no visible evidence of seepage was noted.

Ground Temperatures

Horizontal ground temperature cables have been installed through the dam core and downstream shell at base of the key trench, at original ground elevation, and at elevation 522 m. Ground temperature plots are presented in Appendix B. Measured core temperatures in July ranged from -5°C to -7°C at all elevations. Core temperatures are comparable to measurements made at the same time in 2009 although ground temperatures outside the core area fluctuated within 1°C of 2009 values. These measurements are well within design parameters and indicate that the core is appropriately insulated from the warmer summer temperatures.

Stability and Settlement

The dam crest was visually inspected and no sign of stress or instability was noted (Photo B3). Similarly, the upstream and downstream slopes are generally in good condition (Photo B4, B5). An area of settlement was noted on the downstream slope, near the south end of the dam (Photo B6, B7). This area was noted in 2009 and does not appear to have worsened from the previous inspection. The longitudinal cracking observed in 2009 also remains, but does not threaten the function of the dam.

Recommendations

Overall, the dam is performing within designed parameters and no remedial measures are required.

5.0 DIVIDER DYKE A

Divider Dyke A was inspected on July 15, 2011. The inspection summary, photographs, and figures pertaining to the dyke are provided in Appendix C.

Construction History

Divider Dyke A divides the PKCA into two areas, as shown in Figure C.1. Fine PK is deposited upstream (east) of Divider Dyke A and the area downstream of the Divider Dyke is used as a polishing pond for the PKCA water. The dyke consists of a sand and gravel filter zone, supported by rock fill superstructure. The filter is protected with a layer of rip-rap on the upstream side.

At present the divider dyke is only partially constructed. The rock fill superstructure crest is at the design elevation of 524 m; however, the filter zone of the dyke has only been brought to an elevation of 521.5 m. In the fall of 2007, localized migration of fines through the dyke was observed by Tahera. In an attempt to mitigate the migration of the fines, a layer of coarse PK was placed on the lower portion of the dyke face (Photo C5). The dyke has been performing well since this time up to and including the time of the inspection.

Surface Water and Seepage

The downstream water level on July 20 was 516.84 m. The downstream water appeared clear and no evidence of recent PK piping was observed. The water level upstream of the dyke was not available; however, the water edge was located well away from the upstream coarse PK. This appears similar to observations in 2009.

Stability and Settlement

The dyke crests and slopes appear to be in good condition with no signs of distress noted (Photos C1 through C4).

Recommendations

Overall the dyke is in good condition and no remedial action is required.

6.0 EAST DAM

The East Dam was inspected on July 15, 2011. The inspection summary, photographs, figures, and ground temperature plots pertaining to the dam are provided in Appendix D.

Construction History

The East Dam is an impervious geomembrane lined dam at the east end of the PKCA, as shown in Figure D.1. The dam was constructed during the 2005/2006 winter season. The liner is keyed into saturated sand and gravel till permafrost and bedrock. The upstream shell of the dam was constructed of a thick till layer and a coarse PK layer. Fine PK was then deposited off the upstream face of the dam. A layer of rockfill was placed on the dam crest in the summer 2006 above the liner elevation allowing equipment to travel over the dam during the construction of the Southeast Dam.

Surface Water and Seepage

Fine PK has been discharged against the upstream face of the dam. As such no water was impounded against the dam at the time of the inspection. Some localized ponding was observed along the downstream toe (Photo D4); however, this is the result of water pooling in a local topographic low and not an indication of seepage through the dam.

Ground Temperatures

Two ground temperature cables were installed in the key trench, at the base of the geomembrane liner. Temperatures at the base of the key trench were between -2.6°C and -4.9°C during the inspection. These measurements are within 1°C of the values measured at the same time in 2009.

Stability and Settlement

The dam crest and slopes were visually inspected (Photos D1 to D3). No signs of distress or instability were noted.

Recommendations

The East Dam is performing within designed parameters and no remedial action is required.

7.0 SOUTHEAST DAM

The East Dam was inspected on July 15, 2011. The inspection summary, photographs, figures, and ground temperature plots pertaining to the dam are provided in Appendix E.

Construction History

The Southeast Dam is an impervious geo-membrane lined dam at the east end of the PKCA. The dam was constructed during the 2006/2007 winter season. The liner is keyed into saturated sand and gravel till permafrost and bedrock. The upstream shell of the dam was constructed of a thick till layer and a coarse PK layer. Fine PK was then deposited off the upstream face of the dam. No water was impounded against the dam at the time of the inspection.

Surface Water and Seepage

Similar to the East Dam, Fine PK has been discharged against the upstream face of the dam. As such, no water was impounded against the dam at the time of the inspection. One area of localized ponding was observed along the downstream toe; however, this is an area of natural ponding and not an indication of any seepage through the dam (Photo E6). No other standing water was observed along the dam toe.

Ground Temperatures

Three ground temperature cables are installed in the key trench at the base of the geo-membrane liner. The summer 2011 measurements ranged from -4.9 to -6.5°C in the base of the key trench. This is approximately 1 to 3°C cooler than ground temperature measurements taken at the same time in 2009.

Stability and Settlement

No areas of instability or settlement were noted along the dam crest (Photo E1), or upstream or downstream slopes (Photos E2 and E3). Two localized erosion areas on the upstream face were observed where PK was formerly discharged over the dam crest (Photos E4 and E5). The scour is approximately 1 to 1.5 m deep and is isolated to two locations along the lower reaches of the upstream slope. The scour is isolated to the coarse PK cover and does not impact on dam performance.

Recommendations

Overall, the Southeast Dam is performing well and no remedial action is required at this time.

8.0 CI DIVERSION

The C1 Diversion was inspected on July 15, 2011. The inspection summary, photographs, and figures, pertaining to the dam are provided in Appendix F.

Construction History

The C1 Diversion was constructed to divert water from the natural C1 stream around the Jericho open pit, as shown in Figure F.1. It consists of a cut-off area lined with a HDPE liner keyed into bedrock; a rock cut channel (Reach A), a transition zone to natural ground (Reach B), and two till berms that contain the diverted stream (Reach C), which redirects the water back to the original stream bed. The rock cut was constructed in the summer of 2005 and the cut-off and berms were constructed in the winter 2005/2006. Water first flowed through the diversion in the spring of 2006.

Surface Water and Seepage

Some water was flowing through the channel at the time of the inspection. The volume was relatively small; however, it was sufficient to generate flow through the inlet structure, Reach A and Reach C. Water in Reach B was flowing through the riprap cover and was not observed.

No seepage was noted along the north berm in Reach C. Water was flowing through the reach as intended.

The culvert under Carat Lake Road is damaged at the inlet and outlet (Photo F5 and F7); however, it appears that the inlet has damage has been trimmed from the 2009 inspection. Though some work has been completed, the existing inlet could still provide flow impedance in high-flow cases.

Stability and Settlement

Cracking and settlement was observed in the Reach A inlet fill pad, consistent with observations made in 2009 (Photos F1 and F2). However, some of the cracking noted in 2009 appears to have self-healed over the last two years. A portion of the riprap on the east side of the Reach A inlet is sloughing near the channel edge (Photo F3). At present this is not impacting flow through the area; however the area should be monitored during subsequent inspections.

The rock cut in Reach A and the Reach B transition area performing well with no significant areas of instability noted (Photo F4 and F6).

Longitudinal cracking was observed at several locations in the Reach C north berm. This is likely a result of settlement of the till material used to construct the berm core (Photo F10). The cracking does not appear to be impacting the performance of the berm, but should continue to be monitored.

Recommendations

Overall, the C1 Diversion is performing as intended. The culvert inlet may be restricting flows somewhat. The culvert should be further trimmed to remove all flow barriers. This could be done during the normal maintenance period next summer, or sooner if flow backups are observed. The following areas should be monitored during subsequent inspections:

- Settlement in Reach A inlet fill pad,
- Sloughing near channel in Reach A inlet; and
- Cracking in north berm (Reach C).

9.0 TANK FARM

The tank farm was inspected on July 16, 2011. The inspection summary, photographs, and figures, pertaining to the tank farm are provided in Appendix G.

9.1 GENERAL

The primary tank farm for the site is located at the Jericho plant site area. The tank farm is currently divided into two sections: Phase 1 and Phase 2. Phase 1 was constructed in winter 2004/2005 during the construction of the Jericho plant site. Phase 2 was constructed between May and October 2005. Phase 1 consists of eight 500,000 litre tanks, and Phase 2 consists of four 1,500,000 litre tanks.

Both tank farm containment areas are lined with a 60 mil high density polyethylene (HDPE) liner for secondary containment. EBA understands that the base of the Phase 1 area was constructed of frozen esker fill while the base of the Phase 2 tank farm was constructed of run-of-mine rockfill.

9.2 PHASE 1 AREA

Surface Water and Seepage

Ponded water was observed in the northern portion of the Phase 1 area during the inspection. The approximate limits are shown in Figure G.1. A 2-inch pump was set up in the ponded area; however, it was not operating at the time of the inspection. A slight sheen was observed on the ponded water at several locations.

Stained soil was observed throughout the Phase 1 area (Photo G2). The staining appears consistent with observations made by EBA during the last several inspections and is not considered to be related to a recent release.

Stability and Settlement

Significant settlement (up to 390 mm since being monitored) was observed in the Phase 1 tank farm during the first two years of operation. The majority of the settlement appears to have occurred between the spring of 2005 and September 2006. No survey of the berm has been completed since July 2007; however, a comparison of photo records indicates that no significant changes appear to have occurred since 2009.

The settlement of the tanks in the Phase 1 tank farm varied between 5 mm and 40 mm between September 2006 and July 2007. During the 2011 inspection, gaps ranging from 25 to 50 mm were observed beneath tanks 3, 4, and 5, between the tank underside and tank farm base (Photo G4). The settlement does not appear to have worsened from the 2010 inspection.

Berm settlement was observed in the northeast corner of the tank farm (Photo G3) as shown in Figure G.1. The settlement does not appear to be impacting the performance of the structure, however, if the liner crest at any point is lower than the design elevation, the capacity of the containment facility will be reduced.

An area of exposed liner was noted on the west berm adjacent to Tank 1. The exposed liner was located near the berm crest, but does not appear to be impacting on the berm performance.

A 600 mm level was used to check the approximate list of the tanks in the facility. The survey showed similar lists to those measured in 2010; however, the method used is not considered accurate enough for a direct comparison to be made.

Recommendations

As stated in previous inspection reports, EBA recommends that the tank manufacturer be retained to evaluate the integrity of the Phase 1 tanks before they are put back in service. That being said, the tank farm appears to be performing adequately given the low volume of fuel stored on site at this time.

A survey of the berm crest should be completed, if the Phase 1 area will be used to store additional fuel, to evaluate if the liner crest elevation has been lowered. A survey of the tanks should also be completed to accurately assess the list angles.

9.3 PHASE 2 AREA

Surface Water and Seepage

No significant water was present in the Phase 2 area during the inspection (Photo G5). Slight staining was observed at isolated locations within the facility.

Settlement

Minimal vertical movement of the tanks in Phase 2 was measured between September 2006 and July 2007; however, settlement under the edge of Tank 9 is apparent and has been noted during previous inspections. The gap between the Tank 9 bottom and the soil under the tank is approximately 50 mm (Photo G6). This is approximately 15 mm larger than the 2010 inspection measurements.

The berms around the Phase 2 area appear to be in good condition. No areas of settlement or other distress were noted.

A small portion of exposed liner, approximately 200 mm by 600 mm, was noted on the berm crest near Tank 9. The exposed liner does not impact on berm performance or function.

Recommendations

The tanks and containment area appear to be functioning adequately. No remedial action is required at this time.

The settlement under Tank 9 should be monitored during subsequent geotechnical inspections.

10.0 GENERATOR TANK CONTAINMENT AREA

The generator tank containment area was inspected on July 18, 2011. The inspection summary, photographs, and figures, pertaining to the dam are provided in Appendix H.

One 64,000 litre fuel tank is located adjacent to the generator area at the plant site (Photo H1), as shown in Figure H.1. The tank is contained by a lined berm containment area covered with a layer of crushed gravel.

Stability and Settlement

The perimeter berms were inspected and no signs of instability or distress were noted. Similarly, the tank appears to be in good condition and functioning adequately.

A low spot in the berm was observed, consistent with previous inspections (photo H2).

Recommendations

It is recommended that the as-built information be reviewed to determine at what elevation the liner is located. These low areas should be hand excavated to determine the liner elevation in this area. The capacity of the secondary containment should be reviewed if the liner is lower than the designed/as-built configuration.

11.0 AIRSTRIP TANK CONTAINMENT AREA

The airstrip tank containment area was inspected on July 16, 2011. The inspection summary, photographs, and figures, pertaining to the containment area are provided in Appendix I.

Inspection Observations

Two 64,000 litre fuel tanks are located adjacent to the airstrip apron within a lined containment area (Photo I1). The secondary containment liner is covered with a layer of crushed gravel. No signs of berm instability were found though some exposure of the HPDE liner was present during the inspection.

It is understood that a fuel spill occurred within the containment area in the winter of 2006/2007. Stained soil is present inside the base of the containment area (Photo I2 and I3). The current inspection deals only with the geotechnical issues of the containment area and does not address the fuel spill.

Recommendations

The containment area appears to be performing satisfactorily. No remedial action is required at this time.

12.0 LANDFILL

The landfill was inspected on July 16, 2011. The inspection summary, photographs, and figures pertaining to the landfill are provided in Appendix J.

Inspection Observations

Waste generated during mine operations was landfilled in a designated area within the till dump area. Metal debris is separated and stockpiled in a separate area. Domestic wastes are burned in the open and placed into a small pit, which during active mining operations, was capped with till from the open pit mine. Food wastes are not placed in the landfill; rather they are processed in a designated incinerator. Surface and subsurface drainage from the landfill area flows towards the open pit.

The landfill had been backfilled at the time of the inspection no surface debris was observed (Photo J1). No sign of surface instability was noted. It is understood that the landfill is not currently being used.

Recommendations

The landfill appears to be performing adequately. No remedial action is required at this time.

13.0 HAZARDOUS WASTE TRANSFER AREA

The hazardous waste transfer area (HWTA) was inspection on July 17, 2011. The inspection summary, photographs, and figures pertaining to the HWTA are provided in Appendix K.

Construction History

The HWTA is a HDPE lined facility used for the temporary storage of hazardous waste prior to disposal or transfer off site. The facility was constructed by the previous mine ownership; however, details pertaining to the facility construction are limited.

The facility comprises two cells separated by a center berm. Site observation indicates the liner system comprises a HDPE liner placed between layers of nonwoven geotextile. The containment berms appear to be constructed of esker and crush material.

Containerized debris is contained in both cells. A significant amount of hydrocarbon impacted soils have also been stored in the northeast corner of the west cell. EBA understands that this contaminated soil is from former spills occurring around the airstrip.

Surface Water and Seepage

No water was present in the facility at the time of the inspection (Photos K1 and K2); however, there are several perforations in the liner which would prevent the complete containment in the facility.

Stability and Settlement

The berms appear to generally be in good condition with no areas of instability noted. There is a low spot on the central berm, near the southwest end as shown in Figure K.1. The settlement does not appear to be impacting the performance of the structure, however, if the liner crest at any point is lower than the design elevation, the capacity of the containment facility will be reduced.

Several liner perforations were noted inside the facility at 0.3 m above the liner base or higher (Photo K3 and K4). A large perforation was noted on the southwest berm at the location shown in Figure K.1. A patch was placed over the hole; however, the patch was not welded and is therefore ineffective. The liner base is covered and was not inspected; however, previous observations during freshet indicate that there is some water impoundment on the liner base. This suggests the liner base may be reasonably intact or at the very least in better condition than the berm side slopes.

Recommendations

There are several perforations in the HWTA which will prevent containment in the event of a spill inside the facility. However the observed perforations are part way up the side of the berm, and not in the floor of the HWTA. EBA recommends that an evaluation of the HWTA be completed, including an assessment of the liner system integrity, and remedial actions implemented.

Prior to remedial actions being implemented, the facility should be monitored regularly for signs of seepage or other instability. Snow should be removed from the facility prior to freshet to reduce potential impoundment inside the HWTA.

14.0 WASTE ROCK STORAGE AREAS

The waste rock piles were inspected by Mr. Bill Horne, P.Eng, during a site visit on August 26, 2011. The inspection report and photos pertaining to the waste rock piles are provided in Appendix L.

Inspection Observations

The crest and slopes of the waste rock piles were observed for signs of instability or distress and none were noted. Localized areas of settlement were observed in Waste Dump Site 1; however, these do not appear to be impacting pile performance.

Recommendations

Overall, the waste rock piles appear to be in good condition and no remedial action is required at this time.

Observations made during the August inspection reflect conditions at a finite period in time. Regular inspections of the waste rock pile should be included as part of the regular geotechnical inspections being completed by Shear, particularly once mining resumes and active waste rock placement is occurring.

The August inspection could only comment on the existing pile configuration. The current waste rock pile should be evaluated to verify its compliance with the waste rock pile design.

15.0 CLOSURE

We trust this report meets your present requirements. Should you have any questions or comments, please contact the undersigned at your convenience.

Sincerely,
EBA, A Tetra Tech Company

FOR

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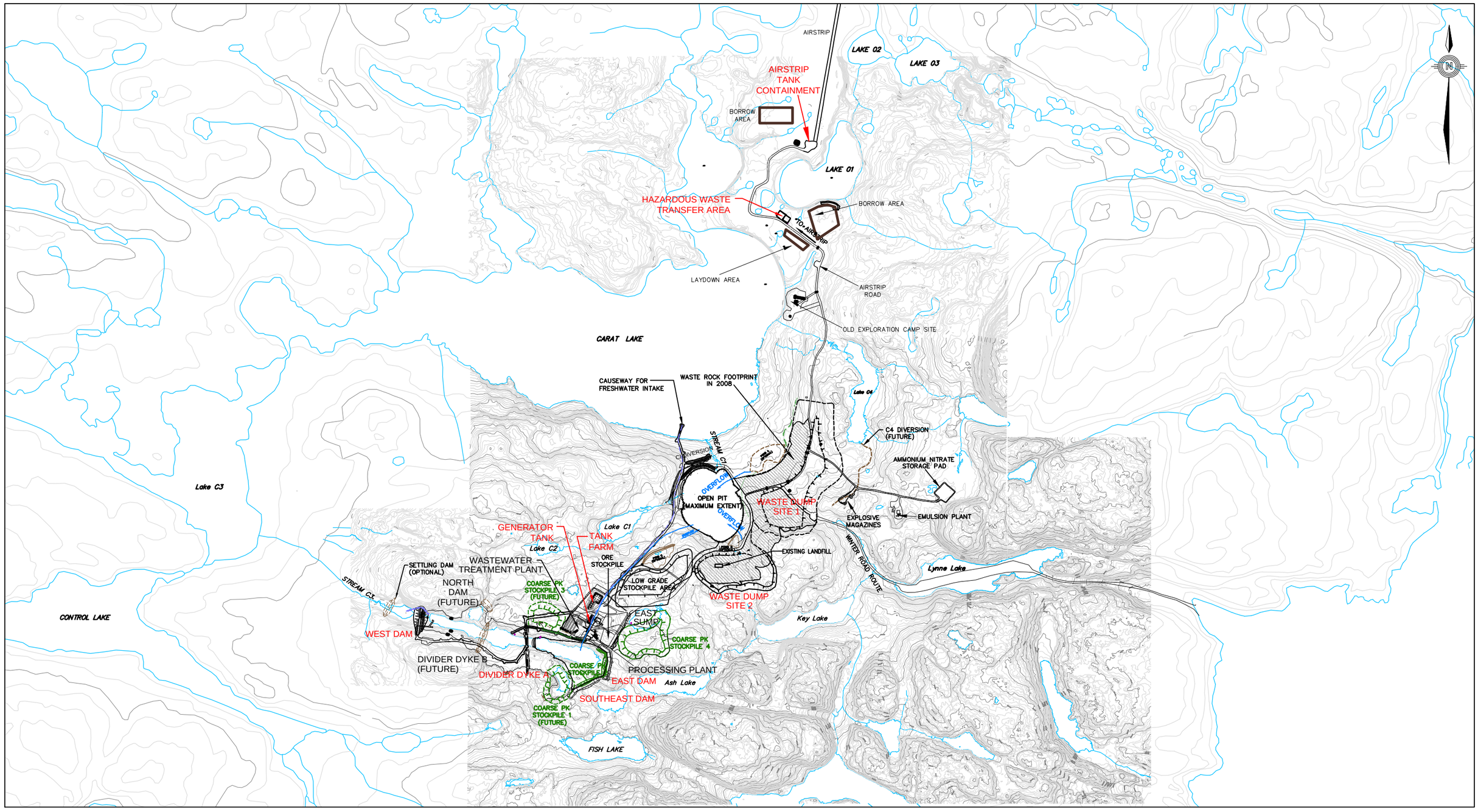
PERMIT TO PRACTICE
EBA ENGINEERING CONSULTANTS LTD.
Signature
Date SEPT. 15, 2011
PERMIT NUMBER: P 018
NWT/NU Association of Professional Engineers and Geoscientists

FIGURES



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Scale: 1: 25 000 (metres)

NOTES
- LAYOUTS ARE APPROXIMATE, AND MAY NOY REFLECT ACTUAL SIZE AND LOCATIONS.
- FOOTPRINTS OF WASTE ROCK PILES, COARSE PK STOCKPILES, AND ORE STOCKPILES ARE SHOWN IN MAXIMUM LIMITS, ACTUALY FOOTPRINTS MAY VARY.
- AREAS INSPECTED ARE HIGHLIGHTED IN RED

STATUS
ISSUED FOR REVIEW

CLIENT
shear diamonds

2011 ANNUAL GEOTECHNICAL INSPECTION JERICHO DIAMOND MINE, NU

GENERAL SITE PLAN

eba A TETRA TECH COMPANY

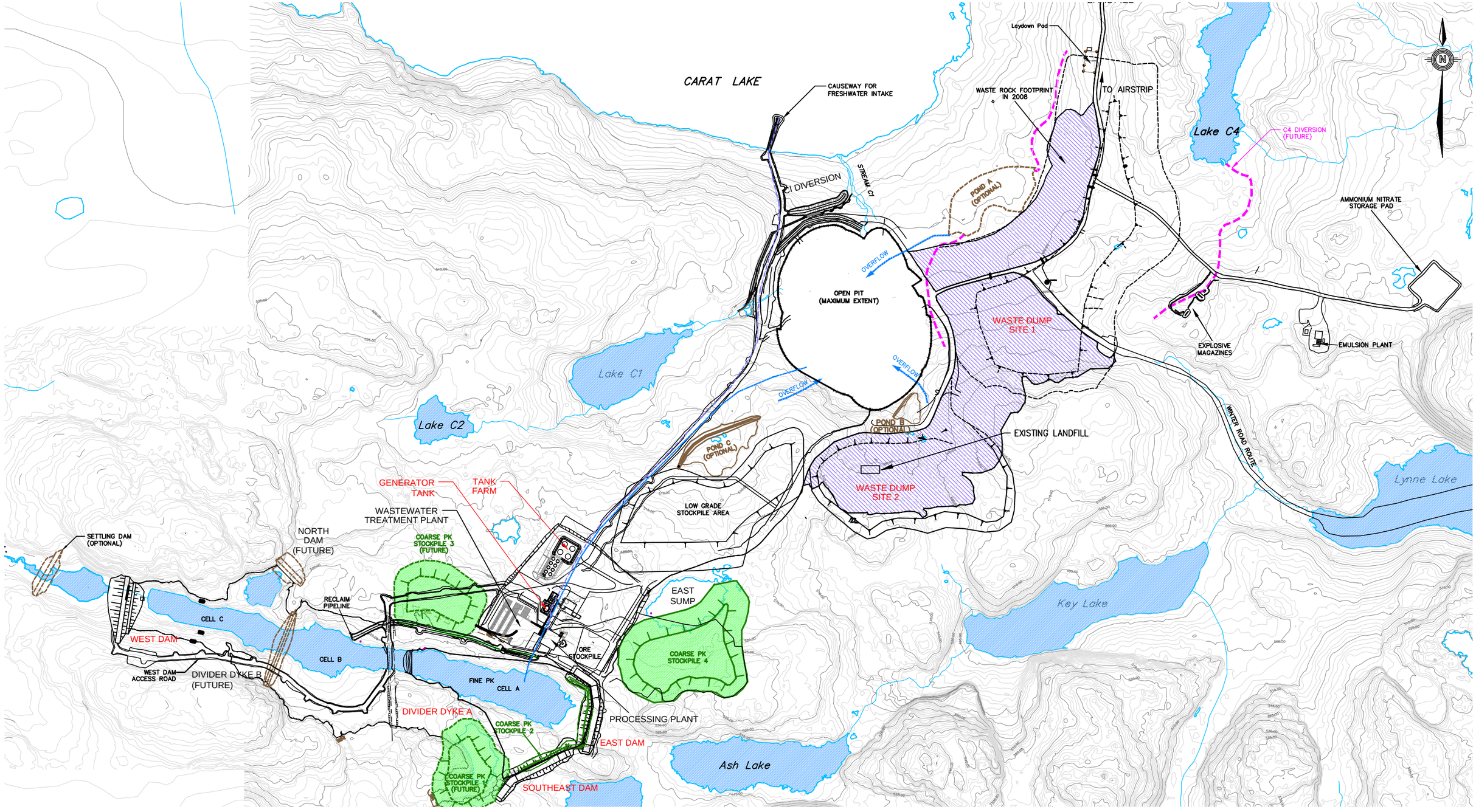
PROJECT NO.	DWN	CKD	REV
E14101140	EP	GDK	0

OFFICE: EBA-EDM

DATE: September, 2011

Figure 1

C:\Users\ernest.palczewski\Desktop\Figure 2_Site infrastructure.dwg [FIGURE 2] September 13, 2011 - 1:51:16 pm (BY: PALCZEWSKI, ERNEST)



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NOTES
- LAYOUTS ARE APPROXIMATE, AND MAY NOT REFLECT ACTUAL SIZE AND LOCATIONS.
- FOOTPRINTS OF WASTE ROCK PILES, COARSE PK STOCKPILES, AND ORE STOCKPILES ARE SHOWN IN MAXIMUM LIMITS, ACTUAL FOOTPRINTS MAY VARY.
- INSPECTED AREAS HIGHLIGHTED IN RED

STATUS
ISSUED FOR REVIEW

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2011 ANNUAL GEOTECHNICAL INSPECTION JERICHO DIAMOND MINE, NU

SITE INFRASTRUCTURE

eba A TETRA TECH COMPANY

PROJECT NO.	DWN	CKD	REV
E14101140	EP	GDK	0

OFFICE: EBA-EDM
DATE: September, 2011

Figure 2

APPENDIX A

APPENDIX A GENERAL CONDITIONS