Appendix 16

Meadowbank Collection Pond 23 Construction Summary Report

MEADOWBANK MINE WATER MANAGEMENT AND GEOTECHNICAL INFRASTRUCTURES

COLLECTION POND 23 – PHASE 1

CONSTRUCTION SUMMARY REPORT

Submitted by:
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March 23, 2026

Reviewed by:

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2026-03-23

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EXECUTIVE SUMMARY

Collection Pond 23, the area formerly referred to as Quarry 23, has been selected as the ultimate destination for water management of the North and South Cell Tailings Storage Facility at closure. Phase 1 of the construction of Collection Pond 23 was carried out from February 8 to June 1, 2025. The ultimate objective of Collection Pond 23 is to manage all runoff water from the Tailings Storage Facility via passive flow through a channel. Phase 1 of the project was executed in order to manage sufficient water storage for the 2025 open-water season.

The activities associated with Phase 1 construction of Collection Pond 23 included:

- Drilling and blasting of Phase 1 of Collection Pond 23
- Drilling and blasting of a channel from South Cell to Collection Pond 23
- Hauling of waste rock material generated from blasting activities
- Construction of a reclaim berm for filtering of process water during tailings deposition in South Cell

The Agnico Eagle Mine Operations team was responsible for management and construction activities associated with drilling, blasting, and hauling for Phase 1 of Collection Pond 23. Earthwork construction of the reclaim berm was completed by Kivalliq Contractor Group (KCG) and supervised by the Agnico Eagle Geotechnical team (Environment Department).

Revision List

Revision			Pages Revised	Comments
#	By	Date		
DRAFT	PO/MS	2025-11-29	All	For internal review
0	PG/RA/MP	2026-03-23	All	For issue

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SECTION 1.0 Introduction

The closure cover system for the North Cell and South Cell Tailings Storage Facility (TSF) includes a surface water management system that conveys water into drainage channels that drain towards a centralized collection point at Collection Pond 23 (CP23) via a passive spillway channel. The ultimate storage capacity of Collection Pond 23 must be completed by June 2028, in-line with the anticipated completion of the TSF capping construction.

To accommodate final tailings deposition in South Cell and to manage the required water storage capacity at closure, the existing Quarry 23 area required expansion prior to rockfill capping of the TSF. In addition, a channel from the South Cell area to Collection Pond 23 to safely convey flows associated with the Probable Maximum Flood (PMF) was required. The referenced feasibility design for this project is summarized in the Okane Consultants report dated August 1, 2025 (Project No. 948-249-007).

Phase 1 of the Collection Pond 23 expansion and associated channel construction at Meadowbank occurred between February 8 and June 1, 2025. The execution was a combined effort between Agnico Eagle's Mine Operations, Engineering, and Water Management & Geotechnical Engineering Department. Other departments such as Maintenance and E&I also provided valuable support for the completion of the project.

The major components of the Phase 1 of the Quarry 23 Project included:

- Drilling, blasting, and excavating Collection Pond 23;
- Drilling, blasting, and excavating the channel connecting the South Cell to Collection Pond 23; and
- Construction of the Reclaim Berm.

Prior to removal from Collection Pond 23, all blast-generated material was tested via drill hole sampling for ARD/ML potential. Other than leftover waste piles and some tailings during channel blasting, all waste rock generated was non-potentially acid generating (NPAG). Waste rock was hauled to various locations in the Meadowbank Complex to assist with road construction and progressive TSF capping.

1.1 Roles and Responsibilities

Initial bedrock sounding of the channel area was organized by the Environment Department and executed via Mine Operations drill and personnel.

The drill and blast patterns, along with the mining sequence of Collection Pond 23 Phase 1 and channel, were designed by Agnico Eagle's Engineering Department.

Execution and supervision of the construction activities were carried out by the Mine Operations Department of Agnico Eagle Meadowbank Complex.

The Engineering Design and Quality Assurance (QA) requirements for the Reclaim Berm were developed by the Water Management & Geotechnical Engineering Team within Agnico Eagle Meadowbank's Environmental Department. The Kivalliq Contractor Group (KCG) was contracted by Agnico Eagle to execute and oversee the construction of this structure.

Table 1 summarizes the general roles and responsibilities of each party involved in the construction of Collection Pond 23 Phase 1, channel, and reclaim berm. It also highlights the key companies and personnel who contributed to the various construction activities.

Table 1: Roles, Responsibilities and Key Personnel

Company	Role	Key Personnel	Position	Responsibility
Agnico Eagle Mines Limited (Agnico Eagle)	Owner	Laurier Allard Patrice Gagnon	Water Management & Geotechnical Engineering Coordinators	Owner's Representative
		Michelle Sadler Pablo Oliveira (EIT)	Geotechnical Engineers	QA/QC Representative & Survey
		Bruno Lessard Jerome Collard	Geotechnical Technicians	QA/QC & Survey
		Jeremy Kancir Jammy Beaudet Jean-Baptiste Bouchard Ryan Griswold	Mine Operations Supervisor	Field Supervisor
Kivalliq Contractor Group (KCG)	Contractor	Elliot Dessureault David Whittom	Project Manager	Contractor's Representative
		François Collard Claude Tremblay	Superintendent & Health and Safety Representative	Supervisor
		Louis Soucie	Field Supervisor	Field Supervisor
O'Kane Consultants	Consultant	Gillian Allen	Senior Engineer	Engineer & Project Manager
		Ismail Ouchebri	Senior Modeler	Design Consultant

1.2 Definitions of Terms Used in this Document

Table 2 presents the definitions of the terms used in this report.

Table 2: Definitions of Terms

Term	Definition
Agnico Eagle	Agnico Eagle Mines Limited, Owner.
As-Built Drawings	Document showing no new concept. It is the graphical representation of a built structure showing the real measurements and objects. It is an inventory of what was built for reference.
Consultant	Okane. Consultant group retained to develop closure design of the Meadowbank tailings storage facility (TSF).
Contractor	Kivalliq Contractor Group (KCG). On-site representative of the construction company contracted by the Owner to build a Reclaim Berm.
Coarse Filter	In the context of the Reclaim Berm construction, Coarse Filter (0-6") refer to the crush aggregate within specific gradation band (e.g., particles no greater than 6"). This material is placed onto the geotextile as a protective layer.
NPAG	A material that has been geochemically classified as being Non-Potentially Acid Generating.
ML	Material which has been geochemically classified as having the potential for Metal Leaching (ML) when in contact with water and air, as per the environmental testing program.
NML	Material which has been geochemically classified as being Non-Metal Leaching (NML) when in contact with water and air, as per the environmental testing program.
Owner	Agnico Eagle Mines Limited, Meadowbank Complex (Agnico Eagle).
Owner's Representative	Person(s) employed by the Owner to oversee the project works and the Owner's interests. The primary point of contact for the Contractor.

PAG	A material that has been geochemically classified as being Potentially Acid Generating.
Quality Assurance (QA)	A planned system of inspection and testing that document, to the satisfaction of the Owner, other stakeholders, and regulators that the Work complies with the design and Drawings. Quality Assurance forms a subset of the Quality Control program. Quality Assurance comprises inspections carried out during Quality Control and includes verifications, evaluations of materials and workmanship necessary to determine and document the quality of the constructed facility. Quality Assurance refers to the measures taken to assess whether the Contractor follows the design intent and Drawings.
Quality Control (QC)	A planned system of inspection, testing and documentation carried out during construction to ensure that the Work is being performed and completed in a manner that complies with the Drawings and Specifications.
Reclaim Berm	A structure constructed at the inlet of the spillway with the purpose of filtering tailings supernatant water and allow sediment-free water to reach the Collection Pond 23.
Channel	In the context of this report, channel is a structure built to provide the passive release of water from the South Cell (SC) TSF to Collection Pond 23 water management infrastructure.
Work	All activities associated with the construction of the Collection Pond 23 water management infrastructure.

1.3 As-Built Drawings

Table 3 presents the as-built drawings for the Collection Pond 23 project. The as-built drawings were prepared by the Water Management & Geotechnical Engineering Team of the Environment department of Agnico Eagle, Meadowbank Complex. The as-built drawings are included in Appendix C.

Table 3: List of As-Built Drawings

Drawing Number	Date	Rev	Drawing Title
001	2026-03-23	0	General Plan View As-Built
002	2026-03-23	0	2025 Collection Pond 23 Phase 1 As-Built
003	2026-03-23	0	South Cell Reclaim Berm & Channel As-Built

SECTION 2.0 Construction Activities and Schedule

This section outlines the schedule of the work completed during the Collection Pond 23 Phase 1 construction period, the material quantities, and a summary of the work performed.

2.1 Schedule

Phase 1 of the Collection Pond 23 construction activities was carried out between February 8 to June 1, 2025. The major components of the work included:

- Drilling, blasting, and excavating the Collection Pond 23;
- Drilling, blasting, and excavating the channel connecting South Cell to Collection Pond 23;
- Construction of the Reclaim Berm.

Table 4 outlines the general schedule for the Phase 1 of the Collection Pond 23 project.

Table 4: 2025 Collection Pond 23 Phase 1 Construction Schedule

Activity	Start	End
Signature Hole Blasting and Channel Investigation	February 8th 2025	February 8th 2025
Collection Pond 23 Drill and Blast	February 14th, 2025	May 9th, 2025
Channel Drill and Blast	March 29th, 2025	May 15th, 2025
Reclaim Berm Construction	May 20th, 2025	June 1st, 2025

2.2 Drill & Blast Designs

The drill and blast pattern designs were completed by the Engineering Department of the Agnico Eagle, Meadowbank Complex.

Table 5 outlines the drill and blast patterns, corresponding tonnage, and the dates each blast occurred. Detailed design plans are provided in Appendix B.

Table 5: List of Drill & Blast Patterns

Pattern	Date Blasted	Estimated Total Tonnage (t)	Comments
5128QYY02	2025-05-09	42,262	Temporary sump
5135QYY01	2025-02-14	10,680	
5135QYY03	2025-02-18	13,738	
5135QYY05	2025-02-24	45,505	
5135QYY07	2025-02-21	29,949	
5135QYY09	2025-02-21	29,132	
5135QYY11	2025-03-02	58,049	
5135QYY15	2025-03-03	47,938	
5135QYY17	2025-03-16	51,375	
5135QYY19	2025-03-16	52,408	
5135QYY25	2025-03-27	59,928	
5135QYY31	2025-03-24	47,611	
5135QYY33	2025-04-03	79,660	
5135QYY41	2025-03-24	29,357	
5140QYY22	2025-05-12	30,095	Channel
5140QYY24	2025-05-07	61,684	Channel
5140QYY26	2025-05-13	15,492	Channel
5140QYY28	2025-05-15	1,200	Channel
5142QYY02	2025-02-18	10,619	
5142QYY04	2025-02-18	10,619	
5142QYY32	2025-02-24	4,143	
5142QYY34	2025-04-03	22,656	
5149QYY01	2025-03-29	53,527	Channel
TOTAL		**807,627**	

2.3 Construction Summary

This section provides an overview of the main construction activities completed during the Phase 1 of the Collection Pond 23 project.

2.3.1 Collection Pond 23

The original preliminary requirement for the water storage capacity at Collection Pond 23 was 525,000 m^3, with an interim Phase 1 capacity required of 315,200 m^3. Due to ongoing feasibility design of the TSF closure at the time of Phase 1 construction initiation, the ultimate target for the water storage capacity in Collection Pond 23 was later modified to 305,000 m^3. This target volume and design storm event calculations are provided in Section 6.4 of the O'Kane Consultants report titled "Meadowbank Tailings Storage Facility – Closure Feasibility Level Design" dated January 22, 2026.

For Phase 1, an interim target of 140,000 m^3 was required to manage freshet runoff and process water from tailings deposition in the South Cell for summer 2025. An estimated 650,000 tons of in-situ rock removal was required to generate this volume. Standard blast design included 7 m-high walls and 8.5 m-wide catch benches. No pre-shear drilling was completed.

From February 14th, 2025 to mid-March 2025, drilling, blasting, and hauling activities took place in Collection Pond 23. The initial 7 m bench to floor 5142 was completed, with a main access ramp established in the southeast corner of the pond via a new southern access road adjacent to the airstrip. Routine drill, blast, and hauling activities continued for the 5135 floor. A small temporary sump to floor 5128 was blasted in order to manage water through the 2025 summer season, and allow Phase 2 to proceed in 2026 to a final floor of 5128.

2.3.2 Spillway

The spillway design basis was provided by Okane Consultants, and is included in Appendix A. Agnico Eagle proceeded with the detailed design and execution of the spillway. Peak flow estimates of 36.1 m^3/s yielded requirements for a spillway cross section of a minimum base width of 10 m and cross sectional area of 14 m^2. Due to constructability constraints, the designed minimum allowable working width in the spillway was 25 m to allow for safe passage of heavy equipment and excavator maneuverability.

The spillway was blasted in two stages, to floor 5149 and then final floor of 5140. Material was hauled via the Collection Pond 23 elevation 5135 floor, as well as a ramp created at the western side of the spillway. The final spillway is approximately 135 m at an overall grade of 4%, with an average width of 24.5 m. The spillway invert and outlet are 141.0 m and 138.0 m, respectively. The walls remain as a free face up to about 14 m at its maximum, with no catch benches.

A portion of the spillway was blasted over tailings material. A rockfill platform was constructed in these areas to allow sufficient drillhole depth. This contaminated material was placed in the South Cell Reclaim, within the footprint of the TSF.

2.3.3 Reclaim Berm

The reclaim berm is intended to be a temporary structure while tailings deposition is still occurring within the South Cell. Its design followed the same standard design for filtering berms, as completed at the main South Cell Reclaim in July 2023. The reclaim berm shall allow passage of tailings water, while retain tailings solids on the upstream. At the completion of tailings deposition within South Cell, the reclaim berm shall be removed and the area remediated, to ensure the spillway design invert and cross-sectional area are respected.

The reclaim berm was constructed following completion of the spillway. A 15.0 m-wide access road at El. 145 masl was constructed during the winter using NPAG waste rock from Collection Pond 23 development to establish a road for reclaim and tailings pipe placement. The road underwent compaction via 100T haul truck. For construction of the reclaim berm, a portion of this access road had to be temporarily removed in order to expose the natural ground and allow for key trench excavation and geotextile tie-in at the abutments. During removal of the road at the abutments, several large pockets of snow and oversized material greater than 1.5 m in diameter were encountered. This snow and material was removed to the approval of the Agnico Eagle geotechnical representative in order to ensure no opportunity for large void space development within the primary berm structure.

After the natural ground was exposed at the upstream abutments, it was scraped and prepared for geotextile placement. A key trench up to 0.5 m was dug via excavator. Geotextile was laid with segments overlapping by a minimum of 0.3 m. The geotextile was only placed up to an elevation of 143.8 masl due to limitations of the excavator reach. This does not pose concerns for the future performance of the structure as tailings and water is not planned to reach this elevation. A lift of 0.5 m of coarse filter material (0-6”) was placed over the geotextile and bucket-compacted.

In the main body of the structure, the foundation was prepared similarly to the abutments. At the upstream, the foundation was scraped and the rockfill slopes were prepared to 2H:1V. The key-in trench, geotextile installation, and coarse filter placement followed the same procedure as the abutments. After these steps were complete to El. 143.8 m, the rockfill road was raised back to El. 145 masl to accommodate the reclaim pipe and eventual tailings pipe. The downstream slope was placed and bucket compacted rockfill at 2H:1V slopes.

SECTION 3.0 QA/QC Program and Results

3.1 General

The QA/QC Program for Progressive Closure construction consists of two main components: the ARD-ML Sampling program and the QA/QC Inspection program. These programs ensure construction adheres to specifications and is properly documented.

The ARD-ML Sampling program confirms that the NPAG material used in construction does not generate acid rock drainage (ARD) or leach metals into the environment. Geochemical testing involves collecting construction material and analyzing it for ARD and Metal Leaching (ML). This testing, combined with historical geochemical data and Wenco records, validates that material stockpiles are NPAG. Samples were tested in the Meadowbank Assay Lab.

The QA/QC Inspection program ensures construction adheres to specifications through detailed inspections and documentation. QA/QC is conducted by Agnico Eagle Geotechnical personnel, involving daily surveys to ensure compliance with construction limits and grades, as well as frequent inspections of foundation conditions, fill placement, and compaction activities. The QA/QC Inspector's duties include:

- Conduct surveys, stake out, and compile material volumes.
- Inspect foundation surfaces before material placement to ensure safe material placement.
- Witness and document material placement and compaction activities.
- Ensure material gradation as per specification requirements (i.e., oversize material removed).
- Ensure proper material placement technique to minimize segregation.
- Perform periodic site visits for visual inspections and photo documentation.
- Collect information for the as-built construction report.
- Preparing as-built construction drawings.
- Collaborate with the Contractor and Owner's Representative to resolve issues.
- Storing photos, daily reports, and as-built information on the network.
- Construction photographs are presented in Appendix D.

3.2 ARD-ML Testing Results

The rockfill excavated from the Collection Pond 23 and channel were used and construction material for progressive closure activities in Meadowbank.

Throughout the Phase 1 of the project, samples of drill cuttings were collected from each blast pattern to ensure that only NPAG material was used for the constructing of permanent structures, such as the West Road raise and TSF cover. Samples from reject material to improve road conditions within Collection Pond 23 were also taken.

Six samples were identified as PAG during the project. All six PAG samples were from the crusher's rejected material used to improve the road conditions within Collection Pond 23. This reject material was removed from the Quarry floor in night shift of May 19th, 2025, and placed in the South Cell reclaim pond area. For the rockfill used as construction material, no samples were identified as PAG.

Appendix E presents the ARD-ML Sampling Results and the NPAG Determination Flowchart.

3.3 Blast Monitoring Results

The vibration induced by every blasts was monitored by Agnico Eagle using a Minimate IV blast monitor. Two different stations were installed to monitor vibrations for different purposes. The first station, FS50, was installed on a bedrock outcrop located in the vicinity of the Third Portage Lake to monitor the fish habitat. The second station, FS51, was positioned on the Saddle Dam 3 (SD3) infrastructure to monitor the vibration through the structure core and foundation.

Site-specific empirical vibration models were developed for both locations and presented in the memo provided to Fisheries and Oceans Canada (DFO) on December 17th, 2024. Vibration thresholds were determined to be 13 mm/s for FS50, limit for fish habitats, coming from the guidelines for blast monitoring by DFO and 50 mm/s for station FS51 coming from Agnico Eagle's Operations, Maintenance, and Surveillance (OMS) manual for tailings management structures.

All the blasting activities that occurred in 2025 at Collection Pond 23 were under these mentioned threshold, and are provided in the 2025 Blast Monitoring Report included in the 2025 Meadowbank Complex Annual Report.

3.4 Water Management

No free water was encountered during construction activities. Collection Pond 23 is located at a higher elevation than surrounding water bodies, and no perched inflows were anticipated. Winter activities also ensured frozen conditions for the entirety of the Phase 1 construction. Pumping infrastructure and sediment and erosion control measures were available throughout construction, if deemed necessary.

APPENDIX A – Okane Channel Design Memo

Saskatoon
112 - 112 Research Drive
Saskatoon, SK S7N 3R3
Canada
www.okaneconsultants.com

Memorandum

To: Erika Voyer – Mine Closure and Reclamation Lead, Agnico Eagle Mines Limited

From: Ismail Ouchebri, Senior Modeller

Our ref: 948-249-004 Rev0

Date: May 29, 2025

Re: **South Cell- Quarry 23 Spillway Design – Rev0**

Okane Consultants Inc. (Okane) has developed a feasibility level design for Agnico Eagles Mines Limited (AEM) for the tailings storage facility (TSF) cover system and water management for closure at the Meadowbank mine site. Water management during closure includes the use of Quarry 23 (Q23) as a collection pond for surface runoff from the TSF. To convey water from the TSF surface water management to the Q23 collection pond a spillway will be constructed. The following memorandum describes the design basis and design constraints for the spillway to the Q23 collection pond.

Closure Design

The cover system for the TSF North Cell and South Cell will include a surface water management system that conveys surface water into drainage channels that drain towards a collection pond (Q23). During the Closure Phase (2030 to 2050) water in the Q23 collection pond will be pumped to Portage Pit for treatment prior to discharge if required. Once in the Post-Closure Phase (after 2050), if water quality is acceptable, water will no longer be treated and a spillway connecting Q23 collection pond and Third Portage Lake will be established, and water will passively discharge. The existing Quarry 23 will be expanded by AEM to accommodate the surface water drainage from the TSF required for the Q23 collection pond.

South Cell to Quarry 23 Collection Pond Spillway Design

Design basis

Table 1 summarizes the proposed criteria, assumptions, and parameters, along with references to applicable regulations and guidelines for the South Cell-Q23 spillway design concept.

Table 1: Proposed criteria, assumptions and Parameters for South Cell-Q23 spillway design

Infrastructure	Criterion, Assumptions, and Parameters	Value	Reference/ Comments
South Cell- Q23 Spillway	Design Event	Probable Maximum Flood (PMF)	Design event based on AEM (2021)[1] Design Guidelines (Principle III (f) section).
	Climate change projections	2080s 50th percentile (base case).	Site-specific climate change projections climate change WSP-Golder (2023)[2] and AEM (2021)[3] Design Guidelines.
	Drainage Slope	Minimum 0.5%	Minimum drainage slope consistent with Okane's design and design guidelines (Alberta Transportation 2022)[4].
	Lateral Slopes	Minimum 3H1V in soft soils 0.2 H1V (80°) in bedrock.	Lateral slope ensuring long term stability and AEM (2025) communication[5].
	Cover Slope	Minimum 0.5%	Minimum cover slope based on promoting drainage under minor differential settlement
	Freeboard	Minimum based on the larger of: • 0.3 m • Velocity head.	Minimum freeboard in consideration of previous design (WSP, 2023) and common practice.

Hydrology

The South Cell-Q23 collection pond spillway has been designed based on a Probable Maximum Flood (PMF), as it serves as a permanent water management infrastructure (AEM, 2021). In line with the

[1] AEM. 2021. RMMS Corporate Standard Water Management. July 2021.

[2] WSP-Golder, 2023. Meadowbank Tailings Storage Facility Closure design. November 2023.

[3] AEM (, 2021. RMMS Corporate Standard Water Management. July 2021.

[4] Alberta Transportation. 2022. Highway Geometric Design Guide. February 2022.

[5] Voyer E., 2025. Mine Closure and Reclamation Lead, AEM. FW: SC-Q23 Spillway. Email to Gillian Allen.17-02-2025.

recommendations of the AEM guideline (AEM, 2021), the governing design event was determined by comparing the effects of a summer rainfall event, a rainfall-dominated spring event, and a snowmelt-dominated spring event. The Probable Maximum Precipitation (PMP) summer rainfall event was chosen as it generates the highest peak flow, making it the most critical scenario for spillway design. The design hydrology was derived using the Soil Conservation Services method (USDA-NRCS 2010), a 24-hour storm distribution based on the intensity-duration-frequency data presented in Figure 1, a curve number value of 91 to represent the NAG isolation cover. The climate change projections (2080s, 50th percentile) were applied to the PMP.

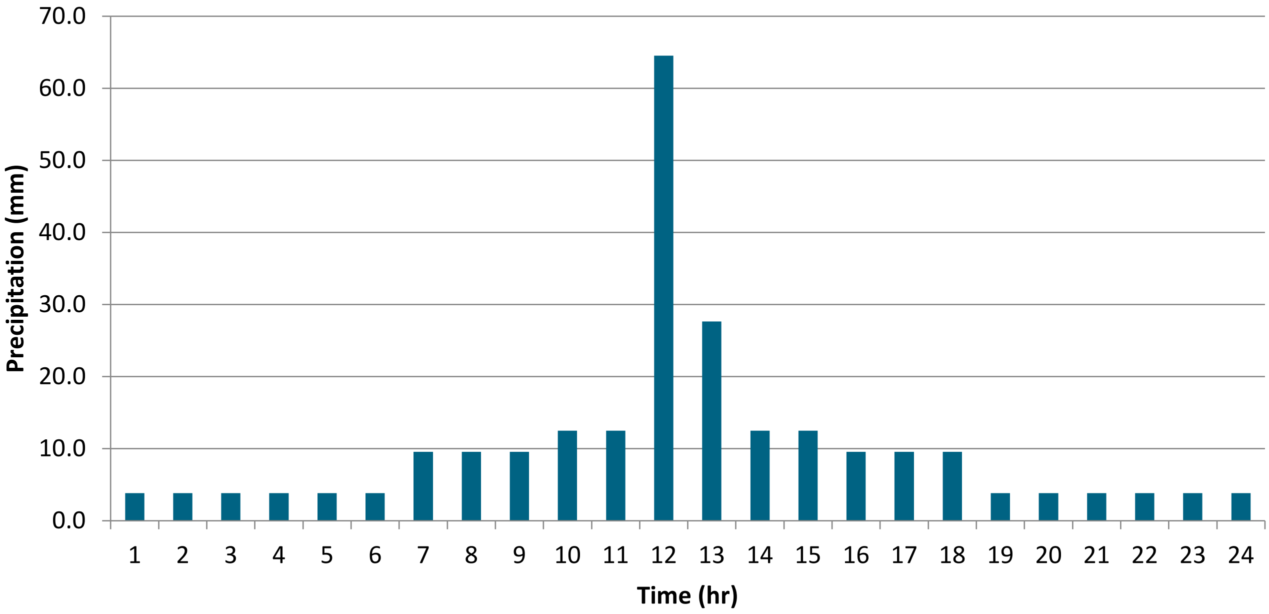


Figure 1: The 24-hour PMP rainstorm distribution (PMP= 245 mm)- Including Climate Change

The hydrology assessment was conducted to estimate the peak flow at the spillway using the HEC-HMS 4.4.1 model, developed by the US Bureau of Reclamation. Key model assumptions include:

- The 24-hour PMP (245 mm) rainstorm distribution, based on intensity-duration-frequency data.
- The SCS Curve Number loss method and the SCS Unit Hydrograph transform method for estimating direct runoff.

Watershed boundaries were delineated based on the cover system design. Both the North Cell and South Cell drain toward the Q23 collection pond. The tailings areas located outside the NCIS (North Cell Isolation System) were also included in the overall catchment. Additionally, the Portage RSF area reporting to the Q23 collection pond was also considered in the assessment. In these areas, rockfill will be placed to ensure that surface drainage is directed toward the Q23 collection pond.

Additionally, the watershed boundaries were extended to include the Portage Rock Storage Facilities (RSF1 and RSF2), whose drainage is directed to the North Cell and subsequently routed to Q23. As part of the design evaluation, Okane assessed whether runoff from RSF1 and RSF2 would be generated under the design event scenario.

Based on the assumed hydraulic conductivity of the RSF material, the infiltration rate under saturated conditions — which could occur during an extreme combined rainfall and snowmelt event — was estimated at approximately 1.5 millimetres per minute (mm/min). This infiltration rate was used to establish the threshold precipitation intensity above which surface runoff would occur. Any combined rainfall and snowmelt exceeding 1.5 mm/min would be expected to generate runoff.

For the design event, represented by the Probable Maximum Precipitation (PMP), the maximum precipitation intensity is estimated at 1.07 mm/hour, which is significantly below the infiltration capacity of the RSF. As a result, all incident water is expected to infiltrate into the rockfill, with no surface runoff being generated from RSF1 and RSF2. Instead, water will percolate slowly through the rockfill matrix as subsurface flow, which is delayed and does not contribute to peak flow during storm events.

Given this, Okane concluded that the flow generated from RSF1 and RSF2 under the design conditions does not influence the infrastructure design, as it does not affect the hydraulic sizing of the system. Consequently, the peak flows from these areas were not included in the design calculations.

Table 2 presents the watershed characteristics based on the defined boundaries, and Figure 2 illustrates the watershed layout. Table 3 summarizes the peak flow results at the Q23 spillway for the 24-hour PMP summer rainfall event.

Table 2: Design Watershed Characteristics

Watershed	Total Drainage Area (km^2)	Watershed Slope (%)	Longest Flow Path (km)	Assigned CN	Lag time (min)
North Cell	1.40	0.55	2.15	91	32.0
South Cell	0.65	0.50	1.20	91	21.0

CN= Curve Number.

Table 3: Probable Maximum Flood peak flow

Infrastructure	Peak Flow (m^3/s)- Including Climate Change
Sout Cell- Q23 Spillway	36.1

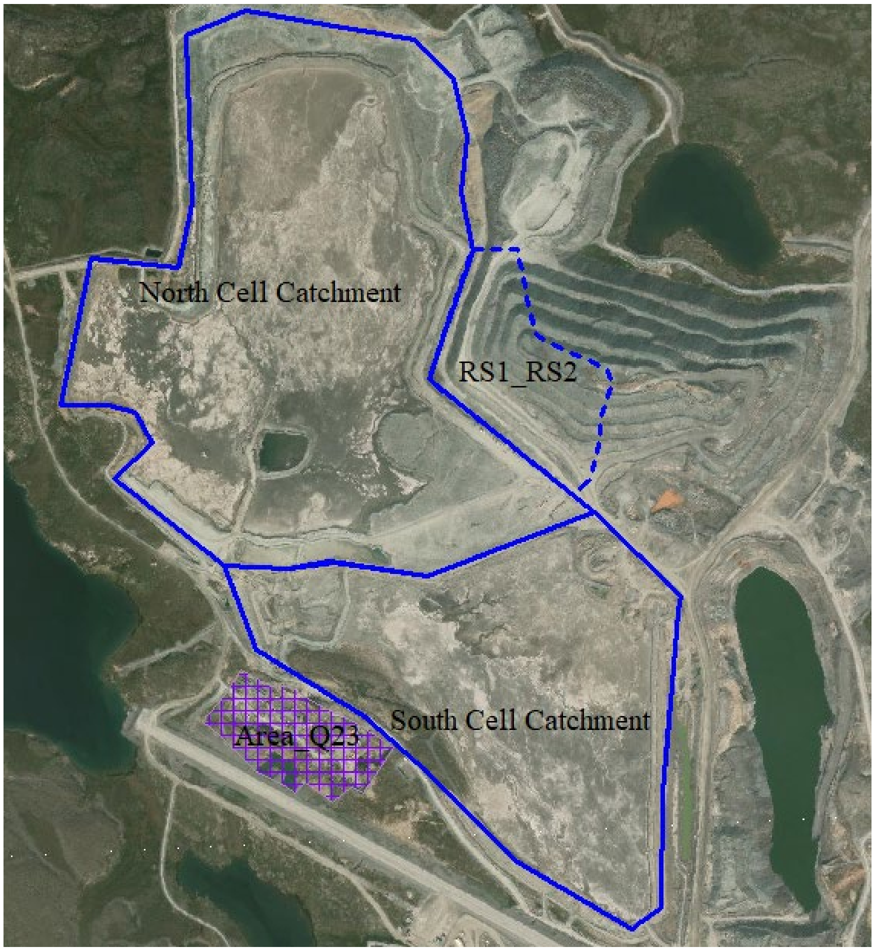


Figure 2: TSF Catchment boundaries

Hydraulics

The hydraulic design for the spillway was developed using HEC-RAS 6.4.1, created by the US Army Corps of Engineers. The model evaluates water levels, velocity, and shear forces at the spillway. Hydraulic conditions are assessed for the design flow.

The spillway geometry was developed by integrating the design geometry with the existing tailings surface and the proposed Q23 expansion. AEM conducted a bedrock survey at the spillway location and estimated that the spillway will be entirely excavated in bedrock. AEM[6] also proposed a typical cross-section for the spillway, which considers their needs and construction constraints, and which Okane evaluated to ensure it meets the design criteria. Changes to the proposed cross-section should be reviewed by Okane to ensure that they meet design criteria.

[6] Voyer E., 2025. Mine Closure and Reclamation Lead, AEM. FW: SC-Q23 Spillway. Email to Gillian Allen.17-02-2025.

The cross-sectional shape of the spillway is typically trapezoidal, with lateral slopes of approximately 0.2H1V (80°) in the bedrock. The spillway base width is approximately 10 m along the channel length (approximately 138 m), the overall slope of the spillway invert is 1.5%.

Based on AEM's survey, the spillway is expected to be fully excavated in bedrock, ensuring that the entire water depth remains contained within the bedrock (1.4 m including the freeboard). In this case, no riprap is required. If the water flow exceeds the competent bedrock level on the lateral slopes, it must adhere to maximum slide slopes (Table 1) and be lined with geotextile and riprapped. Okane will be able to evaluate the required riprap in this case. Table 4 presents the spillway hydraulic design.

Table 4: South Cell- Q23 Spillway Design

Infrastructure	Design Event	Peak flow (m^3/s)	Side Slopes	Longitudinal Slope (%)	Base Width (m)	Depth including Freeboard (m)
South Cell-Q23 Spillway	PMP (Including CC)	36.1	0.2H1V (80°)	Inlet Invert: 141.54 m Outlet Invert: 139.50 m Slope 1.5%	10	1.4

PMP= probable maximum precipitation, CC= Climate Change.

Agnico has proposed[7] a spillway design with a width of 25 meters for operational reasons. Okane is comfortable with this proposed width, provided that the upstream and downstream elevations of the spillway remain unchanged.

It is important to highlight, however, that a proper transition between the internal channel and the spillway must be considered. According to standard hydraulic design practices, the transition should be designed with side slopes ranging between 1:3 and 1:5 (horizontal to vertical) to ensure smooth flow conditions and to minimize turbulence and energy losses.

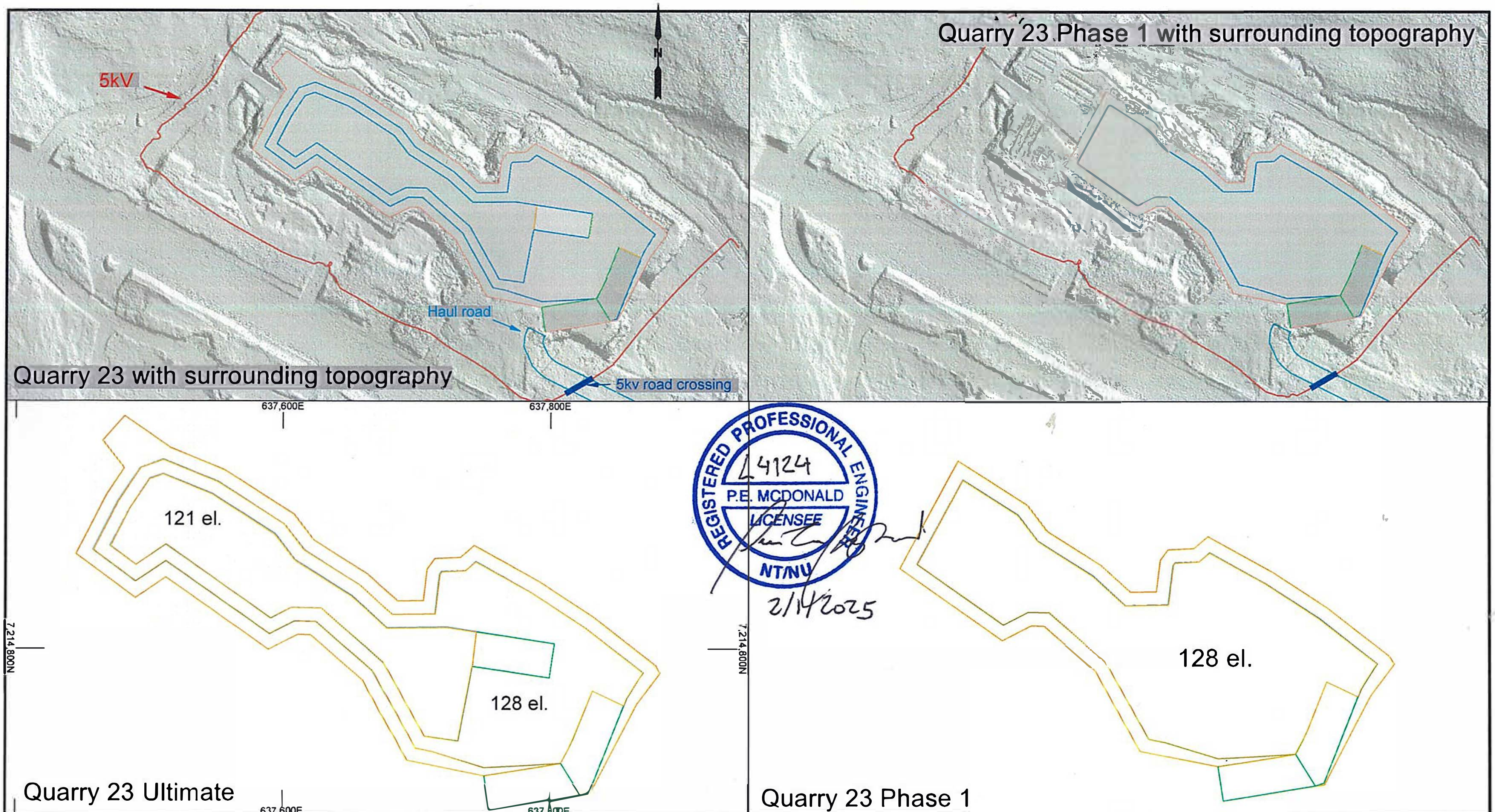
Alignment

Refer to Appendix A for channel alignment.

We trust information provided in this memorandum is satisfactory for your requirements. Please do not hesitate to contact me at +1 450-368-6158 or iouchebri@okaneconsultants.com should you have any questions or comments.

[7] Gagnon P., 2025. Water Management & Geotechnical Engineering Coordinator, AEM. RE: Q23 Spillway Design DRAFT FOR DISCUSSION. Email to Gillian Allen.01-04-2025.

APPENDIX B – Design Drawings



Quarry 23 capacity: 542,804m³ to elevation 139
Quarry 23 Phase 1 capacity: 364,359m³ to elevation 139
See Sign-off document for additional details

DRAWN BY: Loui Blaquière	DATE: 2025/01/13
SURVEY CHECK	DATE
GEOLOGY CHECK	DATE
ENGINEERING CHECK	DATE
MODIFIED BY	DATE

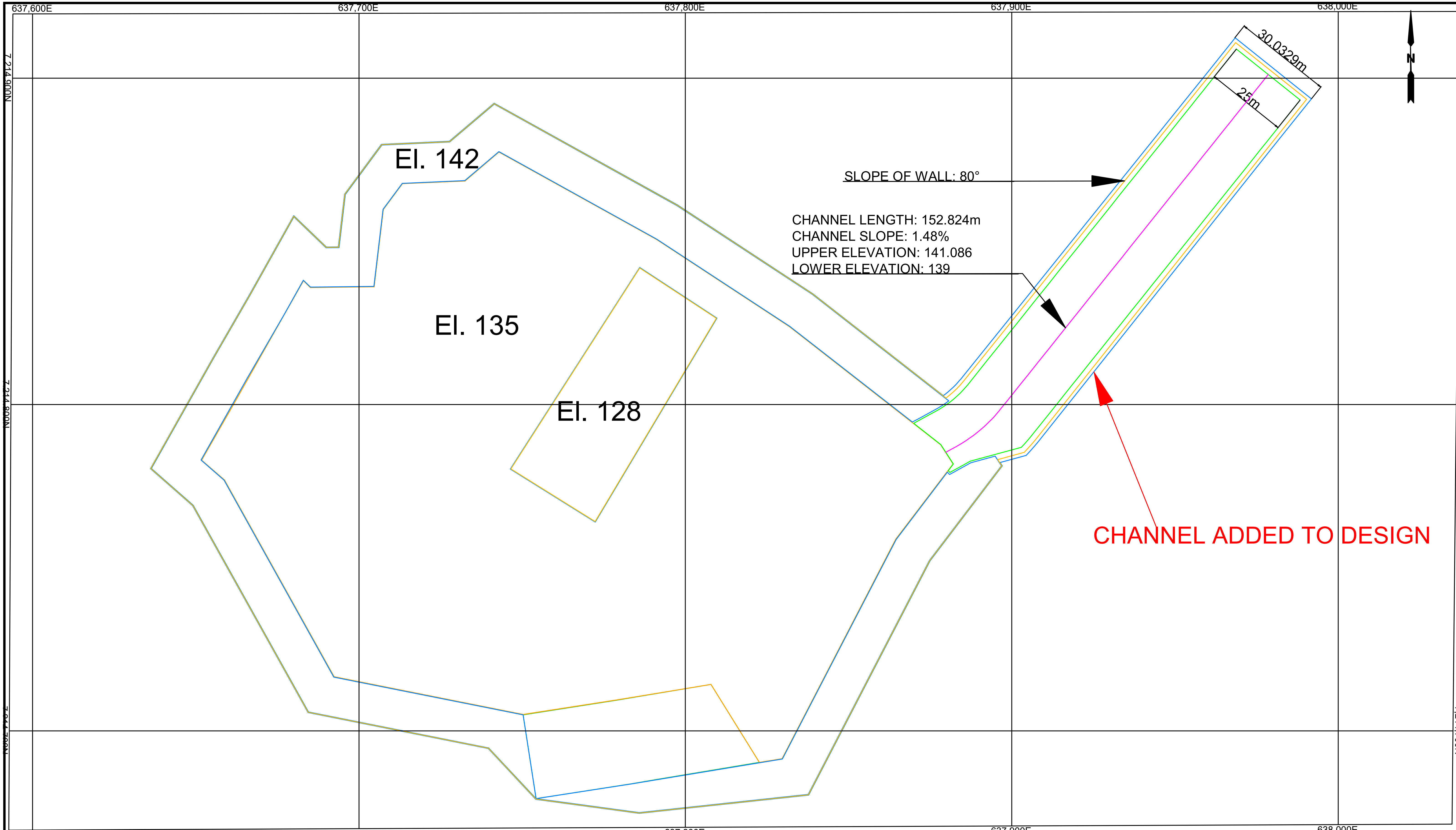
AGNICO EAGLE

MEADOWBANK DIVISION
ENGINEERING
QUARRY 23 DESIGN

Progressive closure

SCALE	DATE	FILE
N.T.S.	2025/01/13	.DWG

\cambfs01\GROUPS\Engineering\04-MineEng\06-PLANNING\06-PIT & DUMPS DESIGNS\MEADOWBANK\ Quarry 23

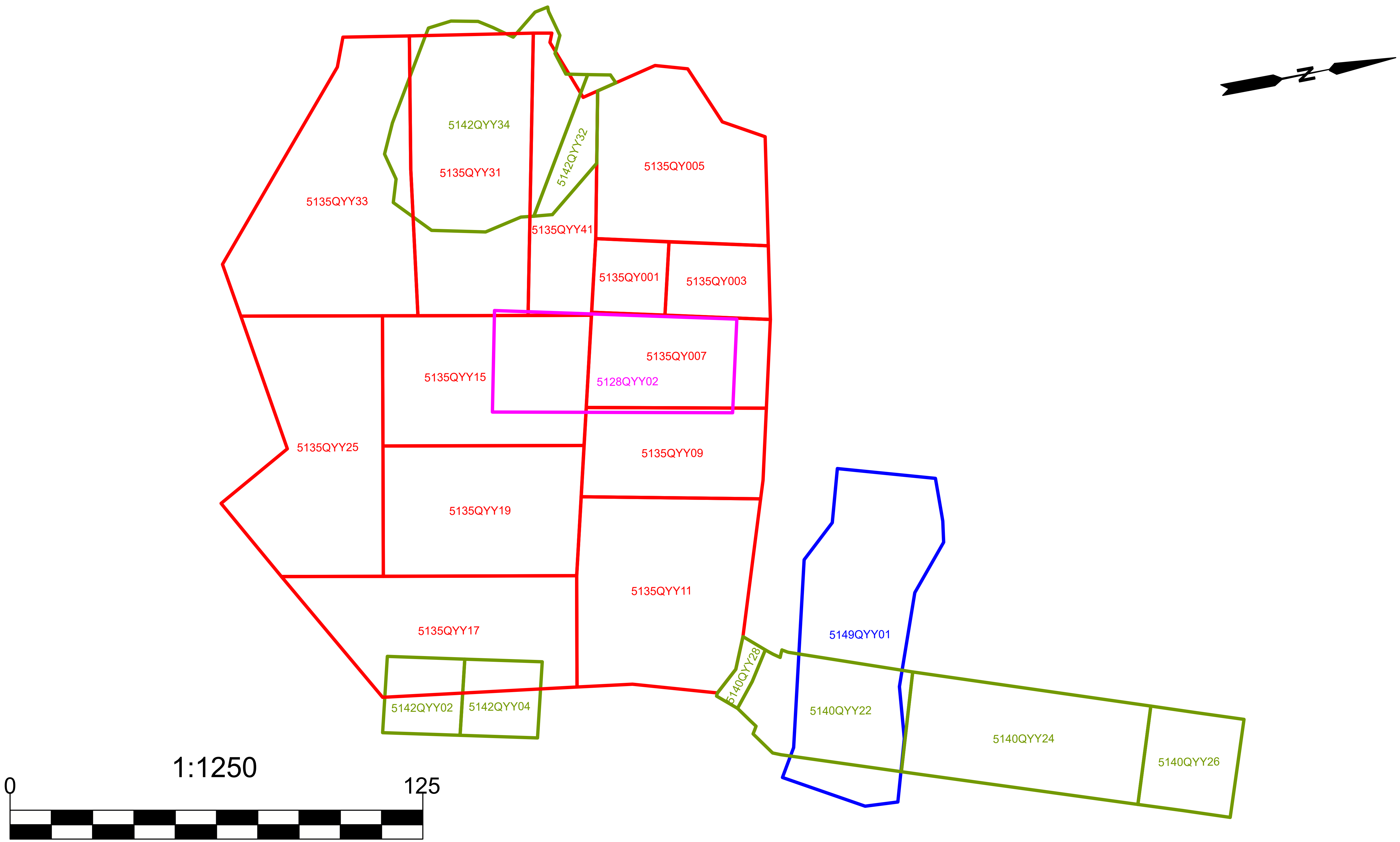


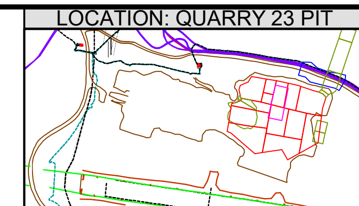
DRAWN BY	DATE
MAXIME DAVIAULT	4-1-2025
SURVEY CHECK	DATE
GEOLOGY CHECK	DATE
ENGINEERING CHECK	DATE
MODIFIED BY	DATE
ENGINEERING APPROVAL	DATE

AGNICO EAGLE
MEADOWBANK

MEADOWBANK DIVISION
ENGINEERING
QUARRY 23 DESIGN
PH1 WITH CHANNEL
MEADOWBANK

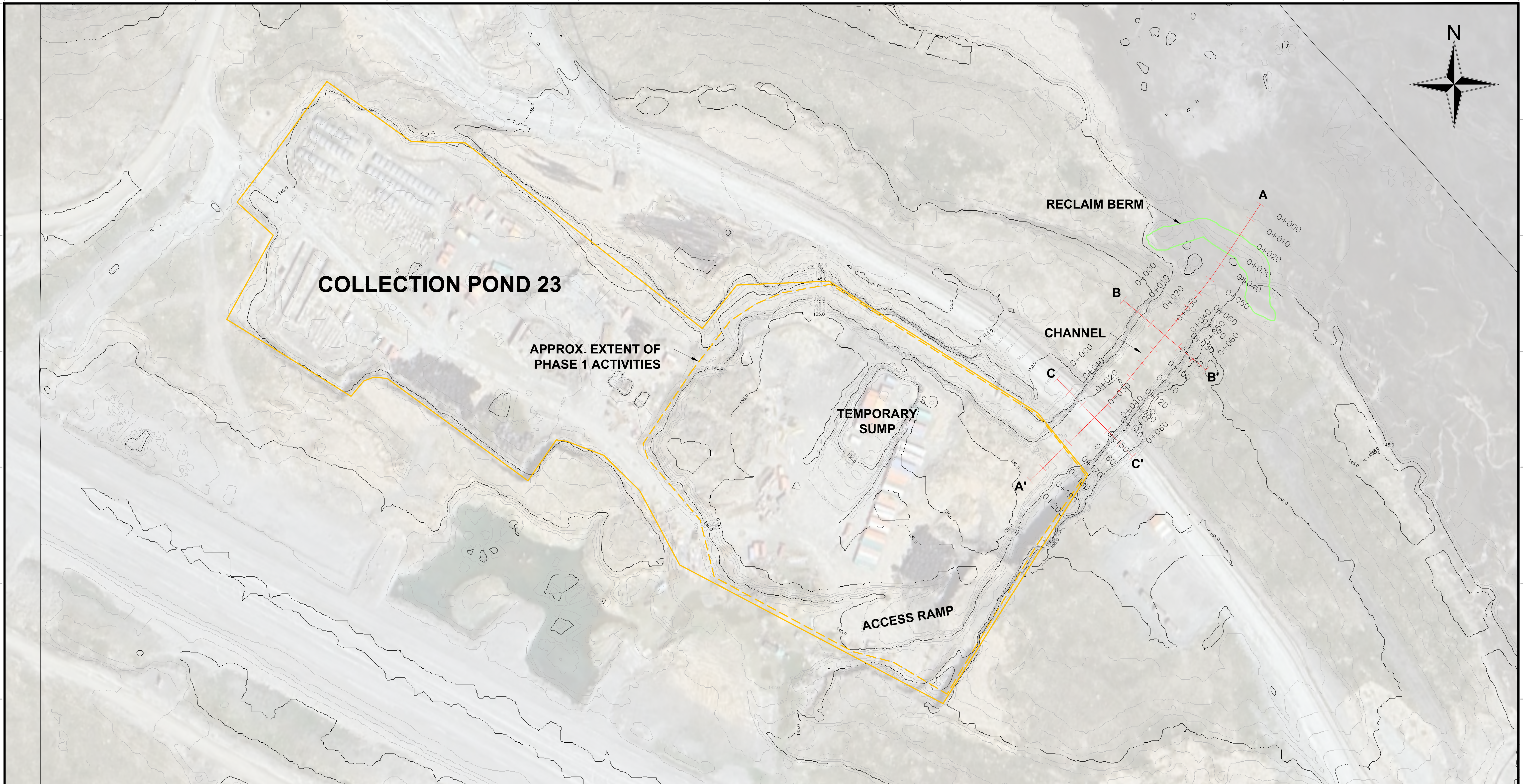
SCALE	DATE	FILE
1:1054.32		.DWG



LOCATION: QUARRY 23 PIT	DRAWN BY ADRIEN G.	DATE 12/4/2025		MEADOWBANK DIVISION
	SURVEY CHECK	DATE	AGNICO EAGLE MEADOWBANK	ENGINEERING
	GEOLOGY CHECK	DATE		DRILL & BLAST
	ENGINEERING CHECK	DATE		BLAST MONITORING MAP
	MODIFIED BY	DATE		QUARRY 23
	ENGINEERING APPROVAL	DATE		SCALE N.T.S. / DATE / FILE

APPENDIX C – As-Built Drawings

FORMAT ANSI B – LANDSCAPE



NOTES:
1. ALL ELEVATIONS AND COORDINATES ARE IN METERS UNLESS OTHERWISE SPECIFIED.
2. DATE OF SATELLITE IMAGERY: JULY 25, 2024.
3. TOPOGRAPHIC DRONE SURVEY PERFORMED MAY 25, 2025.

AGNICO EAGLE

DRAWN BY	DATE
M. SADLER	2025-11-30
REVIEWER	REVIEW DATE
P. GAGNON	2026-03-23

DWG. 001

REV / REV BY	DATE
IFR / MS	2025-11-30
AB / PG	2026-03-23

MEADOWBANK DIVISION

ENVIRONMENT & CRITICAL INFRASTRUCTURE

2025 COLLECTION POND 23 - PHASE 1

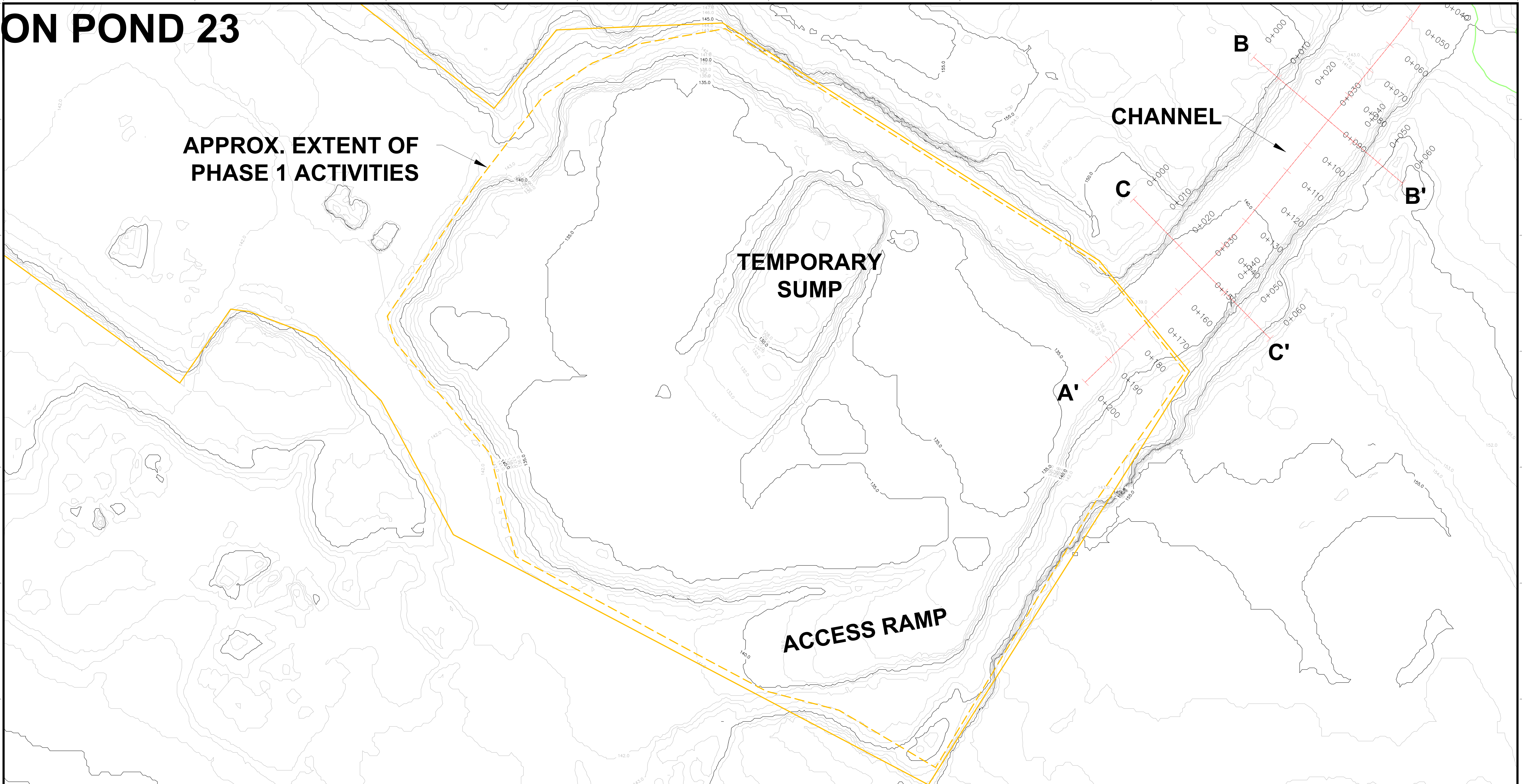
GENERAL PLAN VIEW

AS-BUILT

SCALE	DATE	FILE
N.T.S.	2026-03-23	.DWG

C:\Users\MICHEL~1.SAD\AppData\Local\Temp\AcPublish_20448\CP23 2025 PHASE 1-AS BUILT.dwg, 21 Mar 2026

FORMAT ANSI B – LANDSCAPE



NOTES:

1. ALL ELEVATIONS AND COORDINATES ARE IN METERS UNLESS OTHERWISE SPECIFIED.
2. TOPOGRAPHIC DRONE SURVEY PERFORMED MAY 25, 2025.
3. WALL SCALING PERFORMED AS REQUIRED AND DIRECTED BY AGNICO EAGLE OPERATIONS SUPERVISOR.

AGNICO EAGLE

DRAWN BY	DATE
M. SADLER	2025-11-30
REVIEWER	REVIEW DATE
P. GAGNON	2026-03-23

DWG. 002

REV / REV BY	DATE
IFR / MS	2025-11-30
AB / PG	2026-03-23

MEADOWBANK DIVISION

ENVIRONMENT & CRITICAL INFRASTRUCTURE

2025 COLLECTION POND 23 - PHASE 1

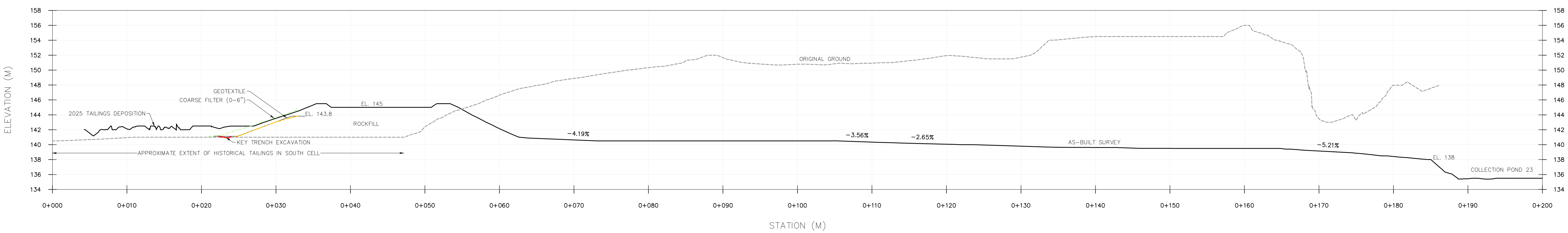
AS-BUILT

SCALE	DATE	FILE
N.T.S.	2026-03-23	.DWG

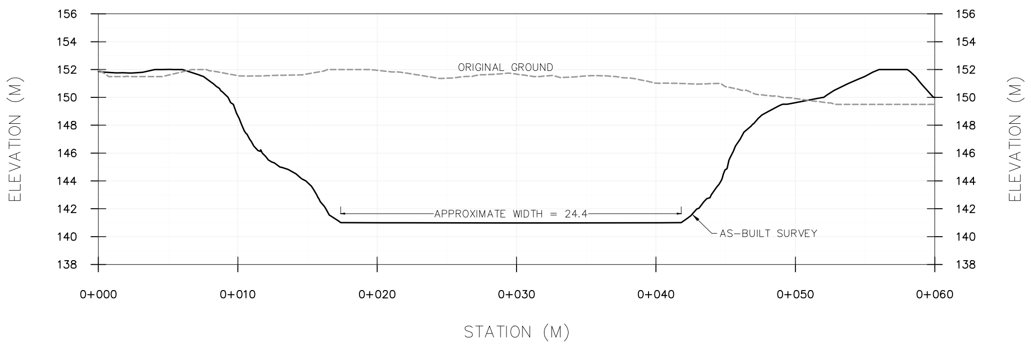
C:\Users\MICHEL~1.SAD\AppData\Local\Temp\AcPublish_20449\CP23 2025 PHASE 1-AS BUILT.dwg, 21 Mar 2026

FORMAT ANSI B – LANDSCAPE

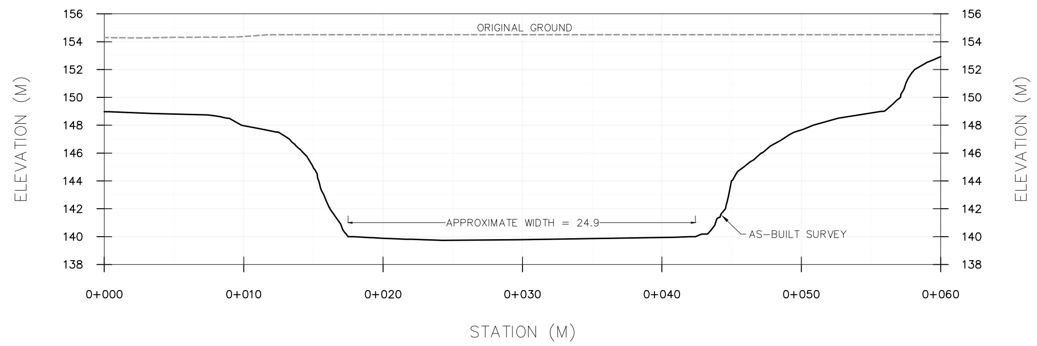
SECTION A – SOUTH CELL TO CP23 CHANNEL AND RECLAIM BERM



SECTION B – SOUTH CELL TO CP23 CHANNEL – 0+090



SECTION C – SOUTH CELL TO CP23 CHANNEL – 0+150



NOTES:

1. ALL ELEVATIONS AND COORDINATES ARE IN METERS UNLESS OTHERWISE SPECIFIED.
2. ORIGINAL GROUND SURVEY DATED OCTOBER 7, 2023.
3. SPILLWAY AS–BUILT DRONE SURVEY DATED MAY 25, 2025. FINAL RECLAIM BERM AND UPSTREAM TAILINGS DEPOSITION DRONE SURVEY DATED OCTOBER 4, 2025.
4. ONLY NPAG MATERIAL APPROVED BY GEOLOGY WAS USED FOR CONSTRUCTION.
5. SNOW REMOVAL AND FOUNDATION PREPARATION WAS APPROVED BY THE GEOTECHNICAL FIELD REPRESENTATIVE.
6. QA/QC AND SURVEY WAS CARRIED OUT BY AN AGNICO EAGLE SITE REPRESENTATIVES.
7. STAGGERED EQUIPMENT TRAFFICKING WAS USED FOR ROCKFILL COMPACTION. COARSE FILTER WAS PLACED AND COMPACTED VIA BUCKET COMPACTION.
8. GEOTEXILE USED WAS SOLENO NON–WOVEN NEEDLED GEOTEXTILE TX–170.

AGNICO EAGLE

DRAWN BY	DATE
M. SADLER	2025-11-30
REVIEWER	REVIEW DATE
P. GAGNON	2026-03-23

DWG. 003

REV / REV BY	DATE
IFR / MS	2025-11-30
AB / PG	2026-03-23

MEADOWBANK DIVISION

ENVIRONMENT & CRITICAL INFRASTRUCTURE

2025 COLLECTION POND 23 - PHASE 1

SOUTH CELL RECLAIM BERM & CHANNEL

AS-BUILT

SCALE	DATE	FILE
N.T.S.	2026-03-23	.DWG

C:\Users\MICHEL~1.SAD\AppData\Local\Temp\AcPublish_20450\CP23 2025 PHASE 1–AS BUILT.dwg, 21 Mar 2026

APPENDIX D – Construction Photographs

D.1 – Collection Pond 23 Construction Photographs

Photo 1.1	
	East wall of collection pond; signature blast
Photo 1.2	
	West wall of collection pond; signature blast

Photo 1.3	
	Mucking of 5142 blast material

Photo 1.4	
	Routine drilling and mucking activities in the western sector

Photo 1.5	
	Clean floor of 5142 prior to blasting to 5135

Photo 1.6	
	Progress of routine mucking activities, looking southeast

Photo 1.7	
	Progress of routine mucking activities, looking southwest

Photo 1.8	
	Progress of 5135 floor

Photo 1.9	
	Drilling of 5128 temporary sump

Photo 1.10	
	Completed 5128 sump with safety berms in place

Photo 1.11	
	Status of southern access ramp, looking south

Photo 1.12	
	Status of southern access ramp, looking north

Photo 1.13	
	Completed Phase 1, looking northeast and 5135 floor

D.2 – Spillway Construction Photographs

<table>
<tr><td rowspan="2">Photo 2.1</td><td>
</td></tr>
<tr><td>Ramp from west created to 5149 to access spillway footprint</td></tr>
<tr><td rowspan="2">Photo 2.2</td><td>
</td></tr>
<tr><td>Survey preparation of drill patterns in spillway on 5149 bench</td></tr>
</table>

Photo 2.3	
	Blast to 5140 floor
Photo 2.4	
	Mucking of 5140 material and creation of temporary drilling pad

Photo 2.5	
	Progress of mucking 5149 floor

Photo 2.6	
	Blast to 5140, progressive mucking

Photo 2.7	
	Completed mucking of spillway to 5140 floor, looking south towards CP23
Photo 2.8	
	Completed mucking of spillway to 5140 floor, looking north towards South Cell

Photo 2.9	
	Completed spillway, looking south from temporary reclaim berm

D.3 – Reclaim Berm Construction Photographs

Photo 3.1	
	Beginning of removal of access road at El. 145
Photo 3.2	
	Exposure of natural ground for abutment tie-in and discovery of snow-rich material. Snow was removed prior to proceeding.

Photo 3.3	
	Preparation of upstream foundation in tailings
Photo 3.4	
	Preparation of upstream foundation in tailings

Photo 3.5	
	Preparation of key trench at east abutment
Photo 3.6	
	Condition of key trench excavation in tailings near east abutment

Photo 3.7	
	Preparation of east abutment tie-in to natural ground. Removal of snow.
Photo 3.8	
	Sloping of rockfill material in preparation for geotextile placement at east abutment

Photo 3.9	
	Excavation of key trench at east abutment
Photo 3.10	
	Preparation and survey markings for key trench excavation upstream

Photo 3.11	
	Geotextile placement on natural ground at east abutment
Photo 3.12	
	Coarse filter placed over geotextile near eastern slope

Photo 3.13	
	Completed placement and sloping of coarse filter near east abutment during road re-establishment to El. 145 masl
Photo 3.14	
	Preparation of upstream rockfill slope towards west abutment

Photo 3.15	
	Continuation of geotextile placement and key trench excavation at upstream slope
Photo 3.16	
	Non-compliant snow removal at west abutment

Photo 3.17	
	Exposure of natural ground at west abutment and slope preparation for geotextile
Photo 3.18	
	Geotextile placement at west abutment

Photo 3.19	
	Coarse filter placement at west abutment
Photo 3.20	
	Rockfill slope preparation and key trench excavation continuing past west abutment

Photo 3.21	
	Geotextile placement on upstream slope, near west abutment
Photo 3.22	
	Continuation of coarse filter placement over geotextile

Photo 3.23	 Completion of coarse filter placement and sloping
Photo 3.24	 Completion of coarse filter placement and sloping

Photo 3.25	
	Survey markings indicating continuation of road cover at El. 145 masl
Photo 3.26	
	Preparation of downstream slope within spillway

Photo 3.27	
	Progress of first lift of rockfill on downstream slope
Photo 3.28	
	Progress of first lift of rockfill on downstream slope

Photo 3.29	
	Progress of second lift of rockfill on downstream slope
Photo 3.30	
	Completed downstream slope

Photo 3.31	
	Completed downstream slope
Photo 3.32	
	Completed upstream slope

APPENDIX E – ARD-ML Sampling Results and NPAG Determination Flowchart

2025 Quarry 23 - Laboratory Results

Sample ID	Results							Comments
	C (%)	S (%)	NP	MPA	NPR	Wt (kg)	Conclusion	
BH-24B	0.845	0.03	70.42	0.94	75.12	14.58	NPAG	Spillway channel investigation grab sample
5135QYY01-1304	1.13	0.05	94.17	1.56	60.27	4.58	NPAG	
5135QYY01-1	0.983	0.036	81.92	1.13	72.82	6.82	NPAG	
5142QYY02-1001	1.039	0.03	86.59	0.94	92.36	0.00	NPAG	
5142QYY02-1005	0.246	0.03	20.50	0.94	21.87	0.00	NPAG	
QY5142QYY04-1404	0.8	0.03	66.67	0.94	71.12	0.00	NPAG	
QY5142QYY04-1005	0.787	0.03	65.59	0.94	69.96	0.00	NPAG	
QY5142QYY04-1001	0.722	0.03	60.17	0.94	64.18	0.00	NPAG	
QY5135QYY07-1404	1.197	0.03	99.76	0.94	106.41	0.00	NPAG	
QY5135QYY07-1412	1.06	0.03	88.34	0.94	94.23	0	NPAG	
QY5135QYY07-1408	1.181	0.03	98.42	0.94	104.98	0	NPAG	
QY5135QYY09-1001	1.288	0.03	107.34	0.94	114.50	0	NPAG	
QY5135QYY09-1009	0.958	0.03	79.84	0.94	85.16	0	NPAG	
QY5135QYY09-1005	1.021	0.03	85.09	0.94	90.76	0	NPAG	
QY5135QYY07-1001	0.966	0.03	80.51	0.94	85.87	0	NPAG	
QY5135QYY07-1005	1.119	0.03	93.26	0.94	99.47	0	NPAG	
QY5135QYY07-1009	0.95	0.03	79.17	0.94	84.45	0	NPAG	
QY5135QYY09-1408	1.544	0.03	128.68	0.94	137.25	0	NPAG	
QY5135QYY03-1001	1.169	0.03	97.42	0.94	103.92	0	NPAG	
QY5135QYY03-1404	1.213	0.03	101.09	0.94	107.83	0	NPAG	
QY5135QYY09-1404	1.509	0.03	125.76	0.94	134.14	0	NPAG	
QY5135QYY03-1005	1.123	0.03	93.59	0.94	99.83	0	NPAG	
QY5135QYY05-1805	1.148	0.03	95.67	0.94	102.05	0	NPAG	
QY5135QYY05-1801	1.02	0.196	85.01	6.13	13.88	0	NPAG	
QY5142QYY02-1404	1.391	0.03	115.92	0.94	123.65	0	NPAG	
QY5135QYY05-1005	1.125	0.03	93.76	0.94	100.01	0	NPAG	
QY5135QYY05-1408	1.123	0.033	93.59	1.03	90.75	0	NPAG	
QY513QYY09-1412	1.153	0.03	96.09	0.94	102.50	0	NPAG	
QY513QYY09-1412 Split PrepDup	1.166	0.03	97.17	0.94	103.65	0	NPAG	
QY5135QYY11-1009	1.084	0.03	90.34	0.94	96.36	0	NPAG	
QY5135QYY11-1009 Split PrepDup	1.12	0.03	93.34	0.94	99.56	0	NPAG	
QY5135QYY11-1001	1.691	0.03	140.93	0.94	150.32	0	NPAG	
QY5135QYY11-1005	1.104	0.051	92.01	1.59	57.73	0	NPAG	
QY5135QYY05-1009	0.989	0.03	82.42	0.94	87.92	0	NPAG	
QY5135QYY15-1001	0.848	0.03	70.67	0.94	75.38	0	NPAG	
QY5135QYY15-1408	0.768	0.03	64.00	0.94	68.27	0	NPAG	
QY5135QYY15-1005	0.879	0.057	73.25	1.78	41.13	0	NPAG	
QY5135QYY15-1404	0.751	0.03	62.59	0.94	66.76	0	NPAG	
QY5135QYY15-1009	0.483	0.03	40.25	0.94	42.94	0	NPAG	
QY5135QYY11-1404	1.015	0.03	84.59	0.94	90.23	0	NPAG	
QY5135QYY11-1805	1.011	0.03	84.26	0.94	89.87	0	NPAG	
QY5135QYY11-1801	0.996	0.03	83.01	0.94	88.54	0	NPAG	

Sample ID	Results							Comments
	C (%)	S (%)	NP	MPA	NPR	Wt (kg)	Conclusion	
QY5135QYY15-1805	1.185	0.03	98.76	0.94	105.34	0	NPAG	
QY5135QYY15-1801	1.057	0.03	88.09	0.94	93.96	0	NPAG	
QY5135QYY11-1809	0.986	0.03	82.17	0.94	87.65	0	NPAG	
QY5142QYY34-1404	0.728	0.03	60.67	0.94	64.72	5.25	NPAG	
QY5142QYY34-1801	0.793	0.03	66.09	0.94	70.49	3.42	NPAG	
QY5142QYY34-1809	0.842	0.03	70.17	0.94	74.85	2.96	NPAG	
QY5142QYY34-2204	1.057	0.03	88.09	0.94	93.96	4.01	NPAG	
QY5142QYY34-1001	0.84	0.03	70.00	0.94	74.67	3.66	NPAG	
QY5142QYY34-1408	1.056	0.03	88.01	0.94	93.87	3.99	NPAG	
QY5142QYY34-1805	0.867	0.03	72.25	0.94	77.07	4	NPAG	
QY5135QYY17-1801	0.918	0.043	76.51	1.34	56.93	0	NPAG	
QY5135QYY17-2204	1.056	0.059	88.01	1.84	47.73	0	NPAG	
QY5135QYY17-2208	1.045	0.063	87.09	1.97	44.24	0	NPAG	
QY5135QYY17-2601	0.786	0.03	65.50	0.94	69.87	0	NPAG	
QY5135QYY17-2605	0.778	0.03	64.84	0.94	69.16	0	NPAG	
QY5135QYY19-1404	0.981	0.03	81.76	0.94	87.21	5.37	NPAG	
QY5135QYY19-1809	0.819	0.03	68.25	0.94	72.80	5	NPAG	
QY5135QYY19-1408	0.83	0.03	69.17	0.94	73.78	3.96	NPAG	
QY5135QYY19-1005	1.519	0.03	126.59	0.94	135.03	2.49	NPAG	
QY5135QYY19-1009	1.485	0.03	123.76	0.94	132.01	4.63	NPAG	
QY5135QYY19-2208	1.446	0.03	120.51	0.94	128.54	4.41	NPAG	
QY5135QYY19-1805	0.661	0.03	55.09	0.94	58.76	4.79	NPAG	
QY5135QYY19-1801	0.678	0.03	56.50	0.94	60.27	3.36	NPAG	
QY5135QYY25-1408	1.508	0.03	125.68	0.94	134.05	3.61	NPAG	
QY5135QYY19-2204	1.54	0.03	128.34	0.94	136.90	4.51	NPAG	
Q23-W-FD-02	0.934	0.03	77.84	0.94	83.03	2.35	NPAG	Free dump grab samples from far west end of Q23. Dumped there during initial floor clean up
Q23-W-FD-01	1.17	0.03	97.51	0.94	104.01	6.66	NPAG	Free dump grab samples from far west end of Q23. Dumped there during initial floor clean up
QY5135QYY25-1009	1.276	0.03	106.34	0.94	113.43	3.52	NPAG	
QY5135QYY25-1404	1.033	0.03	86.09	0.94	91.83	5.21	NPAG	
QY5135QYY25-1412	0.877	0.03	73.09	0.94	77.96	4.3	NPAG	
QY5135QYY25-1001	1.424	0.03	118.67	0.94	126.59	2.32	NPAG	
QY5135QYY25-1805	1.404	0.03	117.01	0.94	124.81	5.1	NPAG	
QY5135QYY25-1005	1.449	0.03	120.76	0.94	128.81	2.99	NPAG	
QY5135QYY25-1801	1.077	0.03	89.76	0.94	95.74	3.79	NPAG	
QY5135QYY31-2204	1.253	0.03	104.42	0.94	111.39	5.97	NPAG	
QY5135QYY31-2601	1.051	0.03	87.59	0.94	93.43	3.02	NPAG	
QY5135QYY31-2605	1.242	0.03	103.51	0.94	110.41	3.13	NPAG	
QY5135QYY31-1805	1.085	0.05	90.42	1.56	57.87	2.01	NPAG	
QY5135QYY25-1809	1.094	0.03	91.17	0.94	97.25	6.45	NPAG	
QY5135QYY25-1813	1.238	0.03	103.17	0.94	110.05	5.17	NPAG	
QY5135QYY41-1005	1.271	0.03	105.92	0.94	112.99	3.12	NPAG	
QY5135QYY41-1801	0.811	0.03	67.59	0.94	72.09	5.72	NPAG	
QY5135QYY41-1404	0.903	0.03	75.26	0.94	80.27	3.42	NPAG	
QY5135QYY41-1001	1.086	0.03	90.51	0.94	96.54	1.93	NPAG	

Sample ID	Results							Comments
	C (%)	S (%)	NP	MPA	NPR	Wt (kg)	Conclusion	
QY5135QYY41-1805	1.14	0.03	95.01	0.94	101.34	4.17	NPAG	
QY5135QYY25-1017	1.246	0.032	103.84	1.00	103.84	4.33	NPAG	
QY5149QYY01-1420	1.122	0.03	93.51	0.94	99.74	0	NPAG	
QY5135QYY33-1805	1.359	0.127	113.26	3.97	28.54	0	NPAG	
QY5149QYY01-1009	1.373	0.03	114.42	0.94	122.05	0	NPAG	
QY5149QYY01-1005	1.348	0.03	112.34	0.94	119.83	0	NPAG	
QY5135QYY33-2605	1.481	0.03	123.42	0.94	131.65	0	NPAG	
QY5149QYY01-1412	0.848	0.03	70.67	0.94	75.38	0	NPAG	
QY5135QYY33-2204	1.146	0.605	95.51	18.91	5.05	0	NPAG	
QY5135QYY33-1801	1.554	0.03	129.51	0.94	138.14	0	NPAG	
QY5149QYY01-1404	0.903	0.03	75.26	0.94	80.27	0	NPAG	
QY5149QYY01-1408	0.945	0.03	78.76	0.94	84.01	0	NPAG	
QY5135QYY33-2208	1.161	0.596	96.76	18.63	5.19	0	NPAG	
QY5149QYY01-1001	1.352	0.03	112.67	0.94	120.19	0	NPAG	
QY5135QYY33-1809	1.009	0.03	84.09	0.94	89.69	0	NPAG	
QY5135QYY31-1404	1.544	0.03	128.68	0.94	137.25	0	NPAG	
QY5135QYY41-2601	0.824	0.03	68.67	0.94	73.25	0	NPAG	
QY5135QYY33-2601	1.587	0.265	132.26	8.28	15.97	0	NPAG	
QY5149QYY01-1416	1.042	0.044	86.84	1.38	63.16	0	NPAG	
QY5135QYY33-1005	1.193	0.03	99.42	0.94	106.05	2.58	NPAG	
QY5135QYY33-1412	1.086	0.03	90.51	0.94	96.54	2.02	NPAG	
QY5135QYY33-1001	1.222	0.03	101.84	0.94	108.63	3.54	NPAG	
QY5135QYY33-1013	1.047	0.03	87.26	0.94	93.07	4.01	NPAG	
QY5135QYY33-1404	1.14	0.03	95.01	0.94	101.34	1.85	NPAG	
QY5135QYY33-1009	1.043	0.03	86.92	0.94	92.72	2.44	NPAG	
QY5135QYY33-1408	1.182	0.03	98.51	0.94	105.07	3.02	NPAG	
QY5135QYY33-1416	1.124	0.03	93.67	0.94	99.92	2.66	NPAG	
QY5140QYY24-2104A	0.961	0.03	80.09	0.94	85.43	4.75	NPAG	
QY5140QYY24-2505A	0.881	0.03	73.42	0.94	78.32	3.95	NPAG	
QY5140QYY24-1304A	0.881	0.03	73.42	0.94	78.32	4.36	NPAG	
QY5140QYY24-1701A	1.056	0.03	88.01	0.94	93.87	1.75	NPAG	
QY5140QYY24-2501A	0.833	0.03	69.42	0.94	74.05	1.6	NPAG	
QY5140QYY24-1705A	1.118	0.03	93.17	0.94	99.38	4.18	NPAG	
QY5140QYY24-1A	1.07	0.03	89.17	0.94	95.12	2.78	NPAG	
QY5140QYY24-2505B	0.839	0.03	69.92	0.94	74.58	1.66	NPAG	
QY5140QYY24-1304B	0.898	0.03	74.84	0.94	79.83	3.12	NPAG	
QY5140QYY24-1B	0.934	0.03	77.84	0.94	83.03	4.26	NPAG	
QY5140QYY24-2104B	0.975	0.03	81.26	0.94	86.67	4.12	NPAG	
QY5140QYY24-2501B	0.932	0.03	77.67	0.94	82.85	5.83	NPAG	
QY5140QYY24-1701B	0.911	0.03	75.92	0.94	80.98	4.12	NPAG	
QY5140QYY24-1705B	0.924	0.03	77.01	0.94	82.14	4.86	NPAG	
QY5128QYY02-1	1.063	0.054	88.59	1.69	52.50	0	NPAG	
QY5128QYY02-1304	0.98	0.03	81.67	0.94	87.12	0	NPAG	
QY5128QYY02-2505	0.93	0.03	77.51	0.94	82.67	0	NPAG	

Sample ID	Results							Comments
	C (%)	S (%)	NP	MPA	NPR	Wt (kg)	Conclusion	
QY5140QYY22-1005	1.115	0.03	92.92	0.94	99.12	0	NPAG	
QY5140QYY22-1001	1.222	0.03	101.84	0.94	108.63	0	NPAG	
QY5140QYY22-1801	1.31	0.031	109.17	0.97	112.70	0	NPAG	
QY5140QYY22-1404	1.444	0.03	120.34	0.94	128.36	0	NPAG	
QY5140QYY22-1805	1.446	0.03	120.51	0.94	128.54	0	NPAG	
QY5140QYY26-1003	1.541	0.03	128.43	0.94	136.99	0	NPAG	
QY5140QYY26-1001	1.291	0.03	107.59	0.94	114.76	0	NPAG	
Q-23-01 (Reject)	0.224	1.382	18.67	43.19	0.43	3.22	PAG	Reject used to improve Quarry 23 road conditions. To be removed.
Q-23-02 (Reject)	0.225	1.457	18.75	45.53	0.41	4.26	PAG	Reject used to improve Quarry 23 road conditions. To be removed.
Q-23-03 (Reject)	0.454	0.877	37.84	27.41	1.38	4.27	PAG	Reject used to improve Quarry 23 road conditions. To be removed.
Q-23-04 (Reject)	0.484	0.735	40.34	22.97	1.76	3.3	PAG	Reject used to improve Quarry 23 road conditions. To be removed.
Q-23-05 (Reject)	0.381	0.689	31.75	21.53	1.47	3.51	PAG	Reject used to improve Quarry 23 road conditions. To be removed.
Q-23-06 (Reject)	0.317	0.701	26.42	21.91	1.21	4.47	PAG	Reject used to improve Quarry 23 road conditions. To be removed.

ROCK CODES: 88 = PAG 99 = NAG