



AGNICO EAGLE

MELIADINE GOLD PROJECT

Construction Summary (As-built) Report for Berm2, Channel3, Pond CP3, and Berm CP3

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DOCUMENT CONTROL

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EXECUTIVE SUMMARY

The construction of Berm2, Channel3, Pond CP3, and Berm CP3 took place at the Meliadine Gold Project, NU between August 2018 and January 2019. Agnico Eagle Mines Limited (AEM) performed all onsite construction management and quality control. This report summarizes the construction activities of each facility, the materials and equipment used, and any variations of the as-built structures from the design documents, as required by the Type "A" Water Licence (No. 2AM-MEL-1631) for this project.

Berm2 was designed to deflect non-contact water, from the catchment area to its west, from flowing into Pond CP3. Berm2 was constructed in general compliance with the design documents, in terms of dimensions and alignment. The bulk of the berm material; however, was changed from 300 mm minus overburden till to esker material as per an RFI submitted to the design engineer.

Channel3 was designed to intercept runoff from the catchment area for the proposed dry stack tailings storage facility (TSF). The side slopes of the channel and the width of the channel bottom vary from design documents. The side slopes range from 3.5H:1.0V to 1.8H:1.0V, though they were designed to be 2.5H:1.0V. The width of the channel bottom was designed to be 2.0 m from Station 0+200 to Station 0+640. However, the measured widths within this interval range from 0.8 m to 3.3 m, with an average of 1.73 m. This reduced width has been deemed acceptable.

The Channel3 berm was not constructed as shown on design drawings. However, to meet the original design intent of serving as Channel3 overflow protection, till from the excavation of Channel3 was placed against the slope of the access road and covered with 600 mm minus esker material (to serve as erosion protection).

Pond CP3 was designed to collect the runoff water from the TSF catchment and store that water on a short-term basis. The pond was constructed in general compliance with the design documents. It has a larger capacity than originally intended, which is beneficial for water management.

The associated downstream berm for Pond CP3 was designed to preserve permafrost in the original ground below the center of the berm. The final elevation of both Berm CP3 and the thermal cover is higher than originally designed. This additional height is beneficial to the berm's performance. Three ground temperature cables were installed in Berm CP3 to monitor berm performance and stability.

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ACRONYMS & ABBREVIATIONS

Acronyms/Abbreviations	Definitions
ABA	Acid-base Accounting
AEM	Agnico Eagle Mines Limited
ANFO	Ammonium Nitrate Fuel Oil
CAT	Caterpillar
CP	Collection Pond
GTC	Ground Temperature Cable
IDF	Inflow Design Flood
km	Kilometres
m	Metres
mm	Milimetres
QA	Quality Assurance
QC	Quality Control
RFI	Request for Information
TSF	Tailings Storage Facility

LIMITATIONS OF REPORT

This report and its contents are intended for the sole use of Agnico Eagle Mines Limited and their agents. Tetra Tech Canada Inc. (Tetra Tech) does not accept any responsibility for the accuracy of any of the data, the analysis, or the recommendations contained or referenced in the report when the report is used or relied upon by any Party other than Agnico Eagle Mines Limited, or for any Project other than the proposed development at the subject site. Any such unauthorized use of this report is at the sole risk of the user. Use of this document is subject to the Limitations on Use of this Document attached in the Appendix or Contractual Terms and Conditions executed by both parties.

1.0 INTRODUCTION

1.1 General

Tetra Tech Canada Inc. (Tetra Tech) was retained by Agnico Eagle Mines Limited (AEM) to prepare a construction summary (as-built) report for the water management facilities recently constructed at the Meliadine Gold Project (the Project) site. The Project site is located approximately 25 km north from Rankin Inlet, Nunavut on the peninsula between the east, south, and west basins of Meliadine Lake (63°01'23.8"N, 92°13'6.42"W). A general site plan for the project is shown in Figure 1.

This report summarizes construction activities of the water management facilities for the tailings storage facility (TSF): Berm2, Channel3, Pond CP3, and Berm CP3. Construction management and quality assurance for these facilities was performed by AEM Civil Construction, while several contractors provided the equipment and operators for the work including Inukshuk Contracting Ltd., Kivaliq Contractors Group, and Fournier et Fils. All surveying was conducted by Hamel Arpentage. Construction of the facilities occurred between August 2018 and January 2019.

This report, which is to be submitted to the Nunavut Water Board, is prepared to meet the requirements in the Type "A" Water Licence No. 2AM-MEL 1631 – Agnico Eagle Mines Limited for the Meliadine Gold Project (Part D, Item 3).

1.2 Related Documents

The supporting and related documents for this report are as follows:

- Design Report for CP3, CP4, CP Berms, Berm2, Channel3, and Channel4, Meliadine Project, Nunavut, AEM Document Number 6515-E-132-007-132-REP-017 (Tetra Tech 2018); and
- Geotechnical Specifications for Construction of CP3, CP4, Channel3, Channel4, and Berms, Meliadine Gold Project, NU, AEM Document Number 6515-E-132-007-132-SPT-006 (Tetra Tech 2018).

2.0 BERM2

2.1 Design Concept

Berm2 was designed to be a low permeability till berm located west of the TSF to deflect non-contact water (from the catchment area west of Berm2) from flowing into CP3, thereby reducing the overall water management requirements for the TSF area. Minor seepage through berm and its foundation into the TSF area is acceptable.

2.2 Construction Materials

Table 2-1 presents the materials that were used for the construction of Berm2.

Table 2-1: Materials Used for Berm2 Construction

Material	Volume
Esker Material (600 mm minus)	Not differentiated by survey
Esker Material (50 mm minus)	Not differentiated by survey
Selected Overburden Till Fill (300 mm Minus)	Not differentiated by survey
Total Volume of Berm2	9,492 m³

2.3 Construction Equipment

Table 2-2 presents the major equipment used during Berm2 construction.

Table 2-2: Equipment Used During Berm2 Construction

Haul Trucks	Excavators	Bulldozers	Loaders	Rollers
Komatsu HM400	CAT 349	Komatsu D65	Komatsu WA500	CAT CS563 Vibratory
Volvo 40t	CAT 308	CAT D6		
CAT 745	CAT 330			
Tandem				

2.4 Berm2 Construction Activities

Site preparation for Berm2 took place from August 9, 2018 to August 16, 2018. The berm toes were staked, and original ground was profiled by survey. An access road was constructed just off the Emulsion Road to Station 0+460.

Berm2 was completed between September 8, 2018 to September 25, 2018 and, apart from the material substitution discussed in Section 2.6, was generally constructed according to the guidelines and requirements stated in the design documents. All material was placed according to methods detailed in the Project Specifications (Tetra Tech 2018; AEM 2017). Specifically:

- 600 mm esker material was placed in controlled lifts not exceeding 900 mm;
- 50 mm minus screened esker material was placed in controlled lifts not exceeding 0.5 m; and
- Overburden till fill was placed in controlled lifts not exceeding 0.5 m.

Material was compacted with a 10-ton vibratory drum compactor to the satisfaction of AEM Civil Construction. Side slopes were shaped with an excavator after placement and compaction.

Photos 1 to 4 show Berm2 at various stages of construction. A 3-D as-built model of Berm2 is shown in Figure 2.

2.5 Quality Assurance and Quality Control

Construction management and quality assurance was performed solely by AEM Civil Construction.

Particle size analysis conducted on the 50 mm minus screened esker material (Appendix C) indicate that the material generally falls within specification for 50 mm minus granular material (AEM 2017), although slightly higher than specified proportions of particles sized between 2.00 mm and 12.5 mm were observed.

Particle size analysis conducted on the overburden till (Appendix C) indicate that the average proportion of fines (27.6%) are within the specified design range.

The 600 mm minus esker material was visually assessed by AEM Civil Construction as satisfactory for placement.

2.6 Variations from Design Documents

A Request for Information (RFI 6515-C-235-012-001) was issued to the design engineer (Tetra Tech) on August 9, 2018 to address material substitutions for Berm2. The design modifications resulting from this RFI are as follows:

- Bulk of berm material changed from 300 mm minus overburden till to 50 mm minus screened esker material;
- Overburden till fill placed on upstream side of berm only at a minimum 2 m width and 0.5 m height;
- Placement of till to be from Station 0+100 to Station 0+640; and
- Riprap layer omitted from the design.

A copy of the sketch of accepted modifications in response to the RFI is shown in Appendix B.

Immediately after completing construction of the access road to the berm; however, it was the decision of AEM Civil Construction to replace the 50 mm minus screened esker material with 600 mm minus esker material (the source of the 50 mm minus). As this material substitution occurred at the onset of berm construction, the change in material types was not differentiated by survey.

Table 2-3 summarizes the design and as-constructed dimensions of Berm2.

Table 2-3: Key Information and Design Parameters for Berm2

Item	Design Value	As-Built Values
Total Length (m)	616.0	650.0
Width of Berm Crest (m)	4.0	4.0
Side Slopes	2.5H:1.0V	3.5H:1.0V (min.) 2.5H:1.0V (max.)
Rip-Rap Thickness or Width (m)	1.0	Not included in revised design
Height of Berm (m)	1.5	1.5

The height and width of Berm2 are in general compliance with the design drawings. However, the side slopes of the berm are generally flatter than what was called for in the design. This is not expected to have a significant effect on the performance of the berm.

No survey data was provided for the placement of the overburden till due to server issues. The presence of this material to design specification is verified by the photograph record and anecdotal evidence.

3.0 CHANNEL3

3.1 Design Concept

Channel3 is designed to intercept runoff from the catchment area for the TSF. The intercepted runoff water will report to Pond CP3, where it will be pumped to former Lake H13, and then flow into Pond CP1 via Channel1.

3.2 Construction Materials

Table 3-1 presents the materials that were used for the construction of Channel3.

Table 3-1: Materials Used for Channel3 Construction

Material	Volume
Esker Material (600 mm minus)	Not differentiated by survey
Waste Rock (600 mm minus)	Not differentiated by survey
Selected Overburden Till Fill (300 mm Minus)	Not differentiated by survey
Rip-Rap	Not differentiated by survey
Non-Woven Geotextile (200 g/m ²)	Not differentiated by survey
Total Cut Volume from Existing Ground to bottom of Channel3	3,137 m³

3.3 Construction Equipment

Table 3-2 presents the major equipment used during Channel3 construction.

Table 3-2: Equipment Used During Channel3 Construction

Excavators	Haul Trucks	Loaders	Drills
Komatsu PC400	Komatsu HM400	Komatsu WA500	Sandvik DX800
Komatsu PC490	Volvo 40t		
CAT 308	CAT 745		
CAT 349			
CAT 352			

3.4 Channel3 Construction Activities

Site preparation for Channel3 took place on August 23, 2018. The channel alignment crests and toes were staked, and original ground was profiled by survey.

Mechanical excavation and geotextile and rip-rap placement was carried out from August 25, 2018 to September 6, 2018. Mechanical excavation (free-digging into the permafrost to design depth with an excavator)

was used from Station 0+000 to Station 0+500. Some ripping was required due to bedrock (from Station 0+200 to Station 0+250) and massive ice (at Station 0+275) encountered approximately 0.5 m below original ground surface. Most of the overburden till was disposed at the Tiriganiaq dump site. The remainder of the till was placed with an excavator against the CP3 Access Road to form the Channel3 berm. This till was then covered with 600 mm minus esker material for erosion protection.

Although attempts were made to effectively stage the excavation with the backfill and limit the amount of exposure, some degradation occurred to the ice-rich overburden soils. The downstream bank (against the access road) required rebuilding with a thaw-stable material (600 mm minus waste rock) between approximately Station 0+200 and Station 0+300. Overburden soils were completely removed in this section, geotextile was placed over the excavation, and 600 mm minus waste rock was placed and compacted to rebuild the bank.

Check dams and pumping were used for water management during the excavation and geotextile installation.

Geotextile was placed cross-width across the excavation by labourers. Each panel was overlapped by approximately one foot before placement of rip-rap. Rip-rap was placed with an excavator on top of the geotextile and lightly bucket-tamped into place.

Drill/blast excavation and geotextile and rip-rap placement took place from November 25, 2018 to December 8, 2018. Drill and blast operations were required to complete the excavation from approximately Station 0+500 to the tie-in at Pond CP3 (Station 0+640). Two blasts were conducted at the channel; one on November 25, 2018 and one on November 26, 2018. All overburden till material removed from this portion of the channel was placed as backfill on Berm CP3.

As with the mechanically excavated portion of the channel, geotextile was placed cross-width across the excavation by labourers, with each panel overlapped by approximately one foot before rip-rap placement. Rip-rap was placed with an excavator on top of the geotextile and lightly bucket-tamped into place.

Photos 5 to 11 show Channel3 at various stages of construction. A 3-D as-built model of Channel3 is shown on Figure 3.

3.5 Quality Assurance and Quality Control

All earthworks material not subject to particle size analysis (rip-rap) was visually assessed as satisfactory for placement by AEM Civil Construction.

3.6 Variations from Design Documents

Table 3-3 lists the key design parameters and as-constructed values for Channel3.

Table 3-3: Key Information and Design Parameters for Channel3

Item	Design Value	As-Built Values
Total Length (m)	636	656
Channel Bottom Width (m)	1.0 (upper section) 2.0 (lower section)	1.2 m to 2.4 m (upper section) 0.8 m to 3.3 m (lower section)
Side Slopes	2.5H:1.0V	1.8H:1.0V to 3.5H:1.0V
Rip-Rap Thickness or Width (m)	0.3	Not differentiated by survey

Table 3-3: Key Information and Design Parameters for Channel3

Item	Design Value	As-Built Values
Minimum Bottom Slope Gradient (%)	4.0 to 4.9 for upper section; 0.2 for lower section	5.3 for upper section; 0.4 for lower section
Maximum Depth of Channel Excavation (m)	3.6	3.0
Maximum Catchment Area (km ²)	1.0	1.0

Channel3 was designed to pass an extreme intensity flow under a 5-minute 1:100 return rainfall of 5.0 mm. The channel was generally constructed according to the guidelines and requirements stated in the design documents.

The width of the channel bottom from Station 0+200 to Station 0+640 is narrower (average 1.73 m) than designed (2.0 m) due to ground conditions encountered during construction. The narrower width is acceptable; however, the channel banks may overflow during a 1:100 Inflow Design Flood (IDF) event. Any overflow that may occur is to be protected by the presence of the Channel3 berm.

The side slopes of Channel3 vary, ranging from 3.5H:1.0V to 1.8H:1.0V. The slopes were designed to be 2.5H:1.0V. The flatter slopes are acceptable. The steeper slopes are expected to perform adequately but should be monitored and repaired if necessary.

Survey data is missing between approximately Station 0+465 to Station 0+485 and from Station 0+595 to the tie-in at Pond CP3 (Station 0+640) due to server issues.

Channel3 berm was not constructed as shown on design drawing 65-695-230-243. It was the opinion of AEM Civil Construction that incorporating a till core into the access road may compromise the stability of the road based on the ice-rich nature of the till excavated from the channel. To meet the original design intent of the till core (to serve as Channel3 overflow protection), till from the excavation of Channel3 was placed against the slope of the road and covered with 600 mm minus esker material to serve as erosion protection.

4.0 POND CP3

4.1 Design Concept

Pond CP3 was designed to collect the runoff water from the TSF catchment area and store this water on a short-term basis. The runoff water from Channel3 will be diverted into Pond CP3. The water collected in Pond CP3 will be actively pumped to former Lake H13, where it will then flow into Pond CP1 (via Channel1).

CP3 was designed to store 3/7 of 1 in 100 wet year freshet and to have the maximum operating water elevation under IDF be 2.0 m lower than the outlet elevation (the lowest ground surface elevation along the perimeter of the pond).

4.2 Construction Materials

Table 4-1 presents the materials that were used for the construction of Pond CP3.

Table 4-1: Materials Used for Pond CP3 Construction

Material	Volume
Blast Rock (600 mm minus; from excavation of the pond; placed as overburden protection around the pond)	5,613 m ³
Blast Rock (600 minus; from excavation of the pond; placed as ramp material)	Not differentiated by survey

4.3 Construction Equipment

Table 4-2 presents the major equipment used during CP3 construction.

Table 4-2: Equipment Used During CP3 Construction

Excavators	Haul Trucks	Loaders	Drills
CAT 349	Volvo 40t	Komatsu WA500	Sandvik DX800
CAT 352	CAT 745		

4.4 Pond CP3 Construction Activities

Site preparation for CP3 took place during October 2018. Snow was removed from the pond footprint with bulldozers. Exterior pond crests were staked, and original ground was profiled by survey. Decommissioned lakes B28 and B28A were found to have insufficient volumes of water, therefore, dewatering was not required.

Approximately 10,000 m³ of overburden was removed with an excavator between October 20, 2018 and October 26, 2018. This material was placed and compacted to form the base of Berm CP3.

Drill/blast excavation took place from November 26, 2018 to December 17, 2018. Test drilling was conducted in advance of production drill/blast to confirm frozen overburden conditions and bedrock depths. Drill and blast operations completed the remainder of the pond excavation. A total of eight blasts were conducted using both ANFO and emulsion explosive products. Excavators then sorted the blasted material and loaded it into haul trucks for placement as follows:

- Overburden was placed to form Berm CP3 or as part of the thermal cover for Pond CP3;
- Oversized material was removed to the Tiriganiaq dump site;
- Rock was placed as ramp material, used as overburden protection around the pond, and as erosion protection for Berm CP3; and
- The remainder of the blast rock was used to construct the northern section of the TSF access road, hauled for usage at CP4, or was temporarily stockpiled for future use.

Photos 12 to 20 show CP3 at various stages of construction. A 3-D as-built model of Pond CP3 is shown in Figure 4.

4.5 Variations from Design Documents

Key information and design parameters for CP3 are summarized in Table 4-3 along with the as-constructed values.

Table 4-3: Key Information and Design Parameters for Pond CP3

Item	Design Value	As-Built Values
Pond Bottom Elevation (m)	56.0	53.3 to 58.0 (avg. 55.0)
Approximate Pond Excavation Depth from Original Ground (m)	9.0 to 10.5	7.6 to 13.1 (avg. 10.9)
Maximum Water Elevation Under IDF (m)	63.0	63.0
Overburden Slopes	3.0H:1.0V	1.8H:1.0V to 2.7H:1.0V (avg. 2.2H:1V)
Bedrock/Overburden Contact Elevation (m)	Assumed 62.0	60.8 to 64.2
Minimum Thermal Protection Cover Thickness (m)	1.0	0.26 (min.) 1.22 (avg.)
Minimum Ramp Width (m)	7.0	8.5 to 20.6
Ramp Longitudinal Slope (%)	10.0	10.0
Safety Berm Height (m)	1.5	0.8 (min.) 2.5 (avg.)

Stage-storage for the pond is summarized in Table 4-4 and a stage-storage curve is depicted in Figure 6. The as-built capacity at the maximum water level (63.0 m) is 16,000 m³ more than the original design.

Table 4-4: Stage-Storage Capacity and Pond Surface Area with Elevations for CP3

Pond Elevation (m)	Pond Storage Volume (m ³)
53.0	0
53.5	1
54.0	52
54.5	382
55.0	1,039
55.5	2,173
56.0	3,927
56.5	5,944
57.0	8,079
57.5	10,331
58.0	12,712
58.5	15,202
59.0	17,798
59.5	20,506
60.0	23,353
60.5	26,352
61.0	29,534
61.5	32,961

Table 4-4: Stage-Storage Capacity and Pond Surface Area with Elevations for CP3

Pond Elevation (m)	Pond Storage Volume (m ³)
62.0	36,646
62.5	40,595
63.0	44,848

Pond CP3 was generally constructed according to design documents. The pond was excavated wider and deeper than specified in some areas, including just above the access ramp, resulting in a larger capacity. This will provide additional storage capacity to the pond.

Some areas of the pond have also been cut at a steeper slope in the overburden than designed. Minor sloughing may occur in the slopes; however, this is not expected to affect the functionality of the pond.

The contact between the bedrock and the overburden varies throughout the pond. The lowest point being 60.8 m. A portion of the overburden will be submerged as the water level reaches the specified maximum water level of 63.0 during the IDF event. The water in the pond should be pumped dry as per the design intent, after freshet and inflow events to prevent permafrost degradation of the bedrock and overburden and reduce sloughing of the pond slopes.

5.0 BERM CP3

5.1 Design Concept

Berm CP3 is the associated downstream berm for Pond CP3. Its purpose is to preserve permafrost in the original ground below the center of the berm.

5.2 Construction Materials

Table 5-1 presents the materials that were used for the construction of Berm CP3:

Table 5-1: Materials Used for Berm CP3 Construction

Material	Volume
Overburden Till (from the CP3 pond excavation)	38,615 m ³
Rip-Rap	9,246 m ³
Non-Woven Geotextile (200 g/m ²)	Not differentiated by survey

5.3 Construction Equipment

Table 5-2 presents the major equipment used during Berm CP3 construction.

Table 5-2: Equipment Used During CP3 Construction

Excavators	Haul Trucks	Bulldozers	Other
CAT 349	Volvo 40t	CAT D6	Sandvik DX800 drill
CAT 352	CAT 745	CAT D8	10-ton vibratory drum roller

5.4 Berm CP3 Construction Activities

Site preparation for Berm CP3 took place during October 2018. Snow was removed from the berm footprint with bulldozers. The berm toe-line and height of each lift were staked, and original ground was profiled by survey.

Berm CP3 construction was completed between October 20, 2018 and January 21, 2019. The berm was constructed entirely of material excavated from Pond CP3 and Channel3. Frozen overburden fill material was placed in controlled lifts as per the geotechnical specifications (Tetra Tech 2018). Overburden fill material was compacted with a 10-ton vibratory drum compactor to the satisfaction of AEM Civil Construction, and side slopes were shaped with an excavator after compaction.

Non-woven geotextile was placed on the downstream side of the berm after sloping was completed, as per specifications. Select blast rock from the Pond CP3 excavation was placed over the geotextile as rip-rap material by an excavator and bucket-tamped into place. Select blast rock was also placed over the entirety of the berm surface as erosion protection.

Photos 20 to 26 show Berm CP3 at various stages of construction. A 3-D as-built model of Berm CP3 is shown in Figure 5.

5.5 Instrumentation

Holes were drilled for three performance-monitoring ground temperature (thermistor) cables (GTCs) upon completion of Berm CP3 using a Sandvik DX800 drill. Four-inch diameter drill steel was inserted into the holes as casing. One GTC was installed in each hole on February 12, 2019 and the holes were then backfilled with sand. Locations of the GTCs are shown on the construction record drawings in Appendix D. Ground temperature profiles for each cable based on data from February 11, 2019 to May 2, 2019 are attached in Appendix E.

5.6 Variations from Design Documents

Key information and design parameters for Berm CP3 are summarized in Table 5-3 along with the as-constructed values.

Table 5-3: Key Information and Design Parameters for Berm CP3

Item	Design Value	As-Built Value
Total Berm Length (m)	320.0	308.0
Approximate Maximum Berm Height Above Original Ground (m)	3.0 (min.)	3.2 (min.)
	5.0 (max.)	6.2 (max.)
	4.2 (avg.)	4.9 (avg.)
Top Elevation of Till Core (m)	68.5	69.3
Berm Crest Elevation (m)	69.0	69.9

The final elevation of both Berm CP3 and the thermal cover was raised due to a larger than expected amount of overburden. The thicker fill will improve the thermal performance of the berm.

6.0 FILL MATERIALS GEOCHEMISTRY

Eight rock samples were collected from the Pond CP3 excavation for off-site independent testing to establish the material's potential for acid generation. Results of acid-base accounting (ABA) tests conducted on the samples indicate a low potential for acid generation of the Pond CP3 excavated material. Geochemical characterization of the solids also indicates low potential for arsenic leaching as concentrations are within project predictions, which indicated management was not required for arsenic leaching.

7.0 LONG TERM MONITORING

7.1 Purpose

Performance monitoring is an integral part of the operation of any water retention structure. Permafrost is expected to exist beneath the footprint of the CP3 pond and berm, and the design intent is to maintain the original permafrost foundation over the life of the berm. The performance of Berm CP3 will need to be monitored throughout its operating life. Long-term monitoring of Berm CP3 should include the following:

- Monitor thermal regime to confirm thermal prediction;
- Monitor settlement and movements of the berm;
- Conduct routine visual inspections of the berm; and
- Satisfy regulatory requirements for the berm performance monitoring.

7.2 Thermal Monitoring

The GTCs installed in Berm CP3 must be read on a regular basis. Tetra Tech recommends that the cables be read once per month during the first year, and then on a yearly basis during operation. The measured readings analyzed and reported in the annual inspection report. The reading frequency can be adjusted as required.

7.3 Routine Visual Inspection

Visual inspection and monitoring can provide early warning of many conditions that can contribute to structure failures and incidents. Monitoring and inspection during construction and operation may include, but not be limited to, settlement/movement monitoring, seepage monitoring, pond water level, and water quality monitoring if required.

AEM should regularly undertake a visual inspection of the ponds, channels, and berms, especially during spring and summer periods. AEM should contact Tetra Tech upon any noted water seepage through the berm, unusual settlement/deformation, or cracks. Any monitoring data should be sent to Tetra Tech for review and evaluation.

7.4 Formal Annual Inspection

An annual inspection, in accordance with Part I, items 14 and 15 of Water License No. 2AM-MEL1631 will be conducted by a qualified Geotechnical Engineer to document the performance of each structure. These visits should take place between the months of July and September of each year. The inspection shall be conducted in accordance with the Canadian Dam Safety Guideline. The specific tasks conducted during these visits should include: inspection of the upstream and downstream slopes for any sign of distress, inspection of the structure crest for any sign of transverse cracking, and inspection of the abutments and downstream toe for any evidence of seepage. The reports will be submitted to the Nunavut Water Board as per the water license requirements.

8.0 CLOSURE

We trust this report meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,
Agnico Eagle Mines Limited
Tetra Tech Canada Inc.



AEM DOCUMENT NUMBER 6515-695-230-REP-001
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Prepared by:

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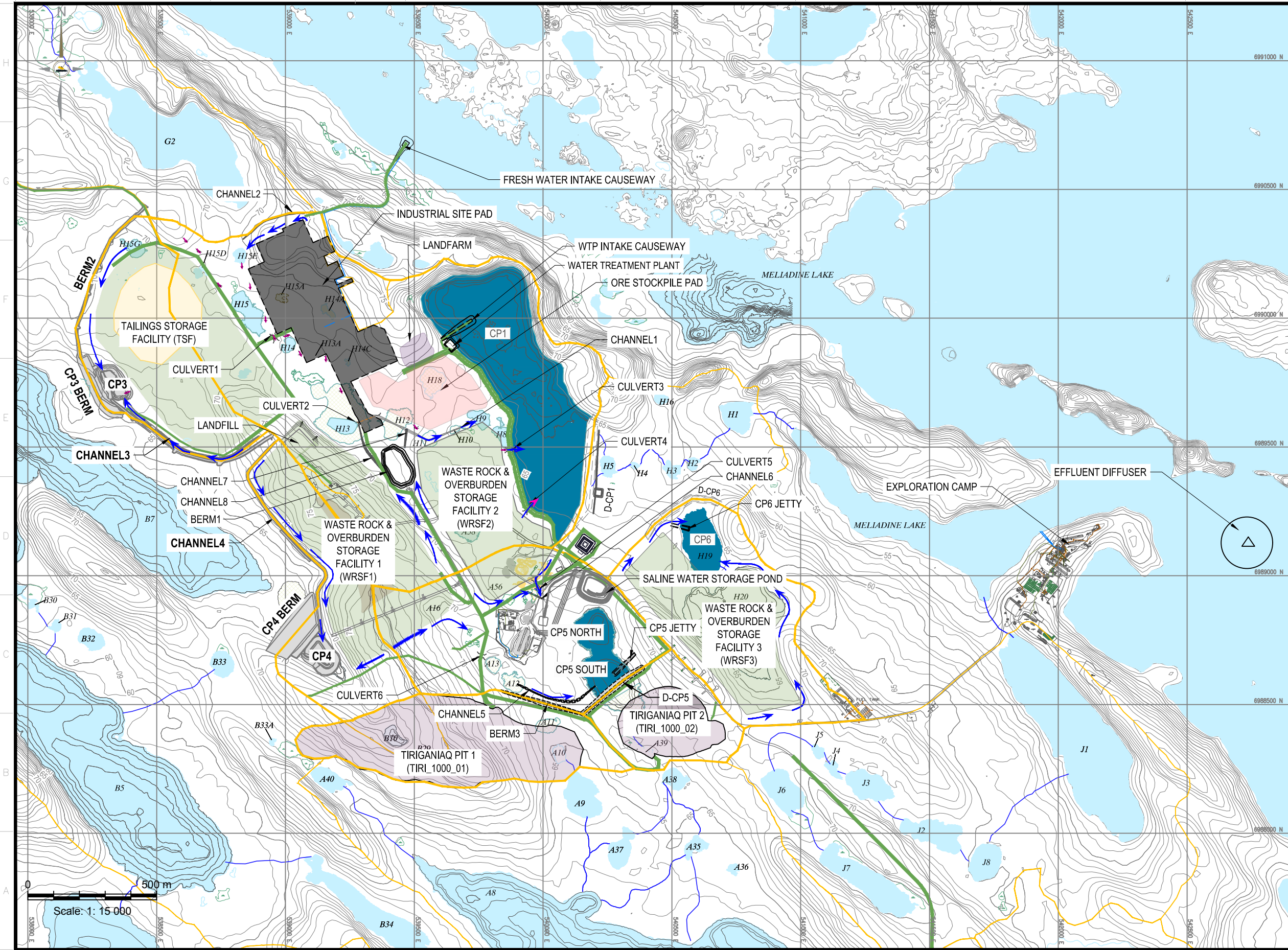
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Tetra Tech Canada Inc.
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REFERENCES

- Agnico Eagle Mines Limited (AEM). 2017. Technical Specifications for Civil Earthworks. Revision 3. June 2017. AEM Document Number 6515-GNS-014.
- Tetra Tech Canada Inc. (Tetra Tech). 2018. Design Report for CP3, CP4, CP Berms, Berm2, Channel3, and Channel4, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Limited June 2018. AEM Document Number 6515-E-132-007-132-REP-017. Tetra Tech Project Number ENG.EARC03076-02.

FIGURES

Figure 1	General Site Layout
Figure 2	3-D Model of Berm2
Figure 3	3-D Model of Channel3
Figure 4	3-D Model of Pond CP3
Figure 5	3-D Model of Berm CP3
Figure 6	CP3 Stage-Storage Curve



LEGEND

- CATCHMENT BOUNDARY
- SERVICE ROAD
- HAUL ROAD
- WATERBODY
- WATER COLLECTION POND
- DRAINED POND AREA
- OPEN PIT
- OVERBURDEN
- WASTE ROCK
- ORE
- TAILINGS
- INDUSTRIAL SITE PAD
- CONTACT WATER FLOW DIRECTION

AGNICO EAGLE

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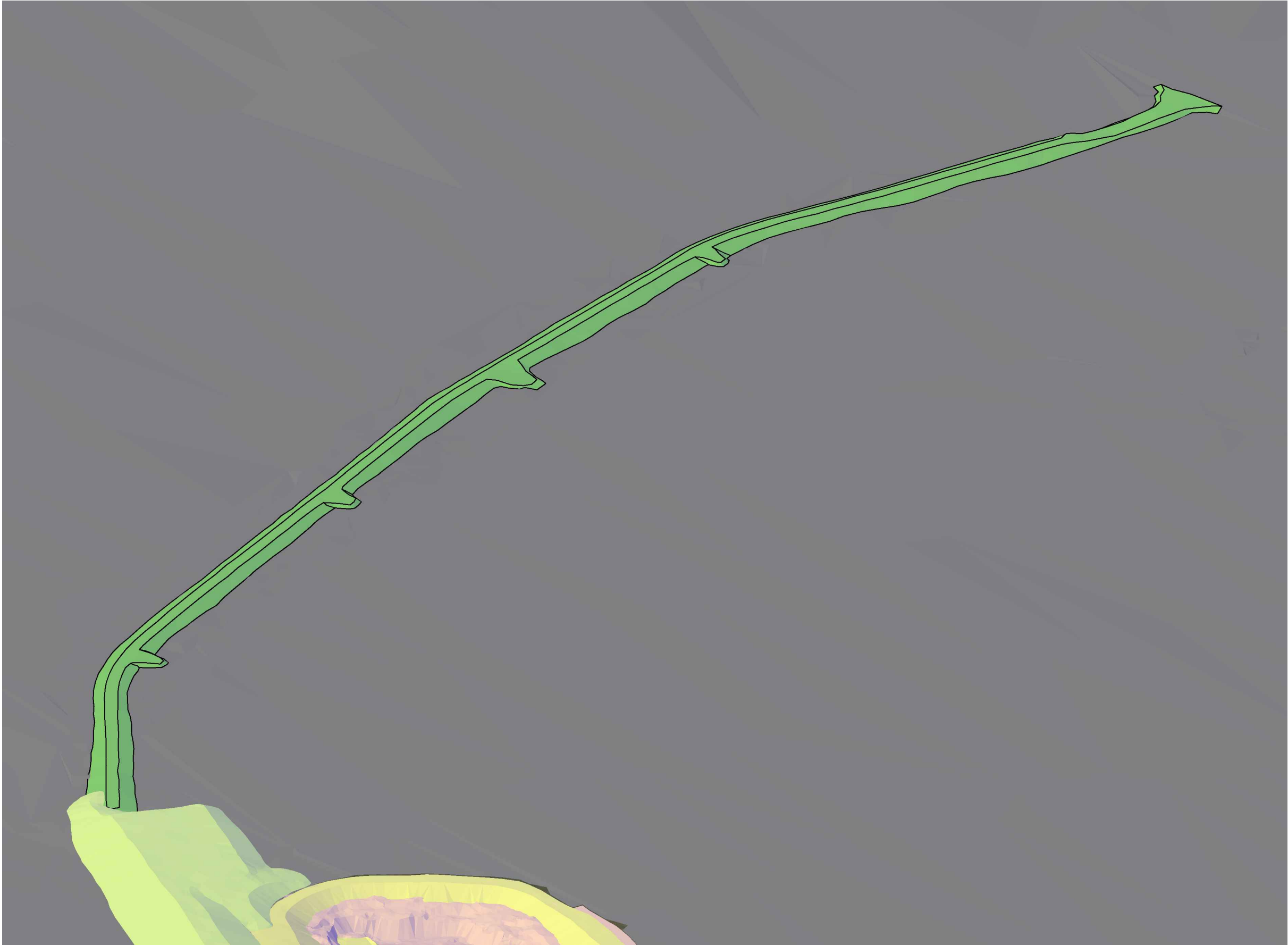
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APPROUVÉ PAR APPROVED BY			
No. PROJ. PROJECT NO.	6515		
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AGNICO EAGLE — MELIADINE GOLD PROJECT

FIGURE 1 — GENERAL SITE LAYOUT

ÉCHELLE/ SCALE	1:15000	FICHIER FILE	General Site Layout Plan .DWG
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FIGURE 2 - BERM2			

ECHELLE/ SCALE	N.T.S.	FICHIER FILE	3D Presentation.DWG
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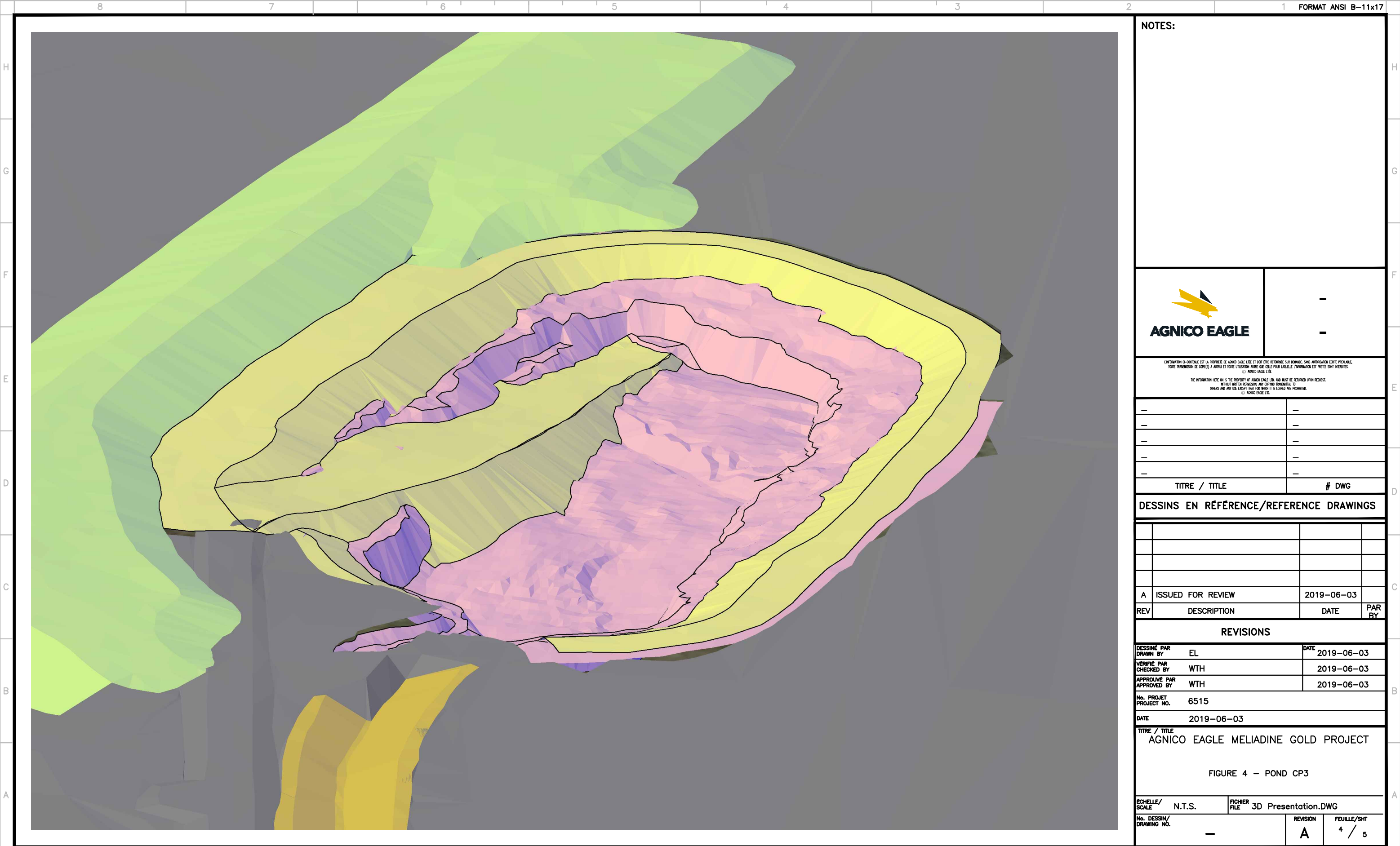
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DATE	2019-06-03

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FIGURE 3 - CHANNEL3

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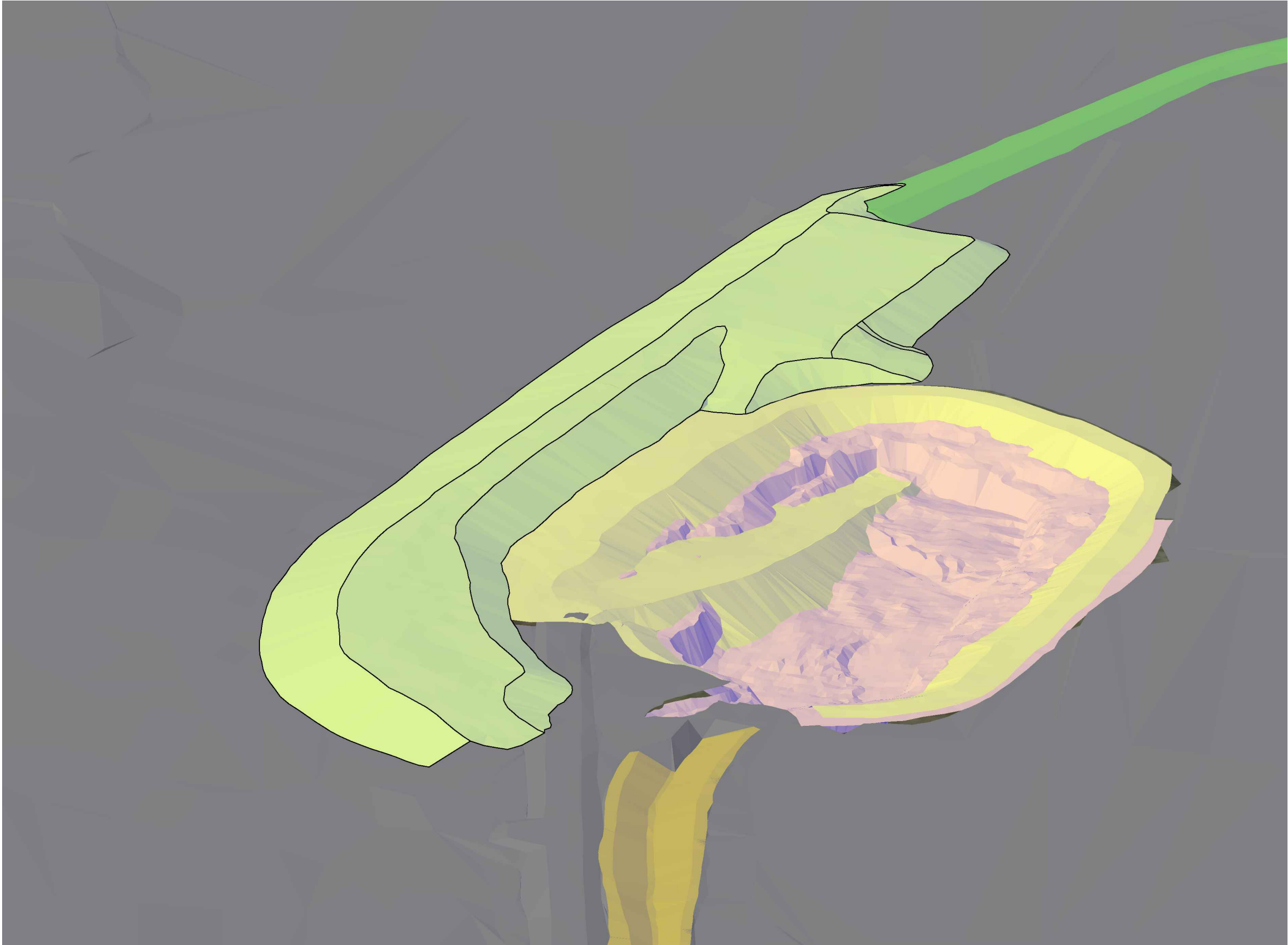
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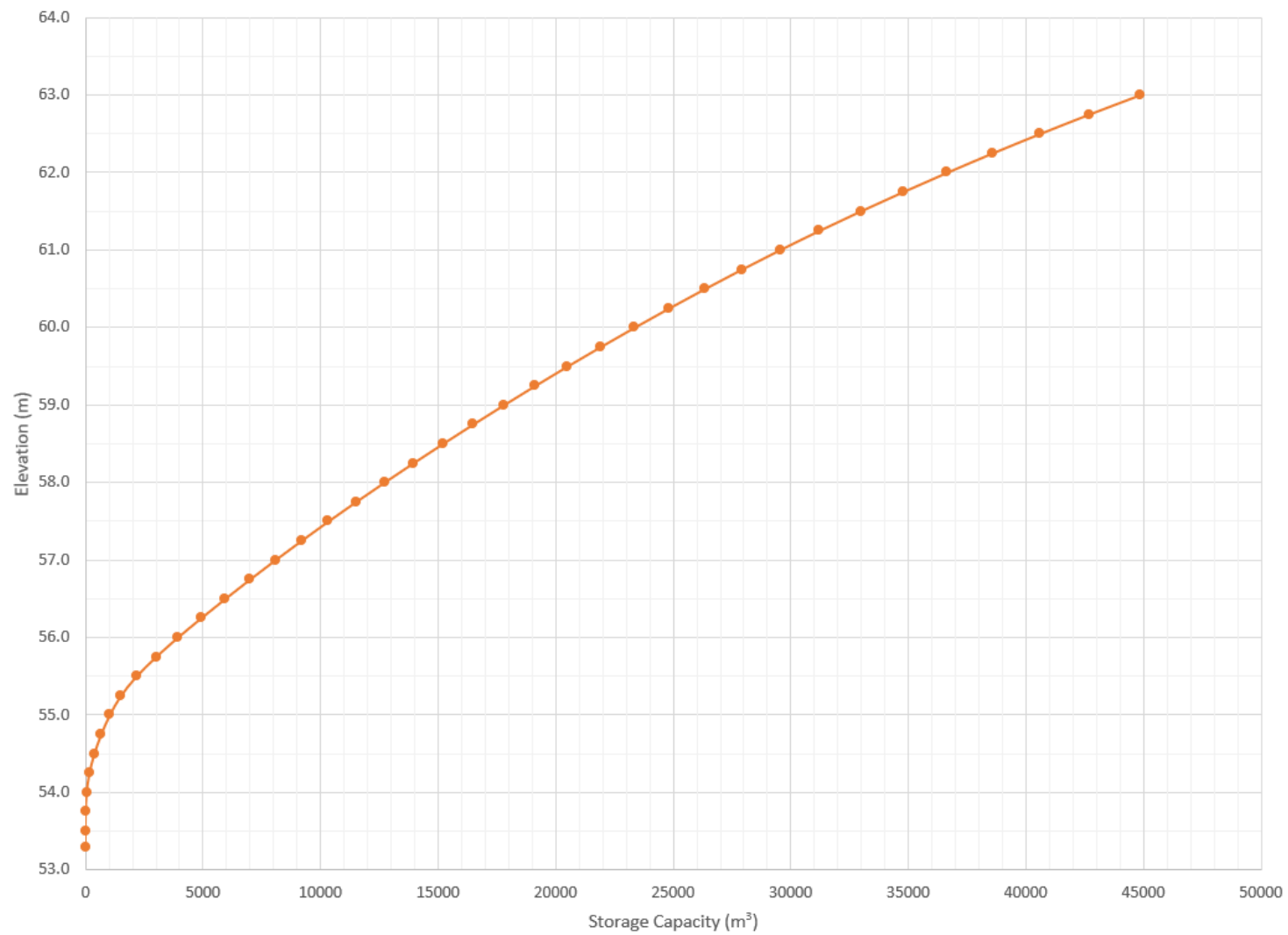
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DATE	2019-06-03		
TITRE / TITLE	AGNICO EAGLE MELIADINE GOLD PROJECT		
	FIGURE 5 - BERM CP3		

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LEGEND

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CLIENT



MELIADINE GOLD PROJECT

CP3 Stage-Storage Curve



PROJECT NO.
704-ENG.EARC03076-02

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DATE
June 7, 2019

Figure 6

STATUS
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PHOTOGRAPHS

Photo 1	09 11 2018 Berm2 - Sloping of upstream till; view from Station 0+520 facing down-chainage
Photo 2	09 13 2018 Berm2 - Compacted esker material; view from Station 0+240 facing down-chainage
Photo 3	09 15 2018 Berm2 - Unfinished downstream slope; view from Station 0+120 facing up-chainage
Photo 4	09 16 2018 Berm2 - Sloping downstream; view from Station 0+400 facing up-chainage
Photo 5	08 29 2018 Channel3 - Mechanical excavation; Station 0+090 facing down-chainage
Photo 6	08 29 2018 Channel3 - Poor bank conditions, ice, and bedrock; Station 1+320 facing up-chainage
Photo 7	09 03 2018 Channel3 - Rebuilt bank and channel construction; Station 1+185 facing up-chainage
Photo 8	11 24 2018 Channel3 - Loading holes Blast 1; Station 1+590 facing down-chainage
Photo 9	11 26 2018 Channel3 - Overburden removal from Blast 1; Station 1+600 facing down-chainage
Photo 10	11 30 2018 Channel3 - Geotextile placement; Station 1+580 facing up-chainage
Photo 11	11 30 2018 Channel3 - Rip-rap placement; Station 1+580 facing up-chainage
Photo 12	10 23 2018 Pond CP3 - Mechanical overburden excavation; view facing north
Photo 13	11 29 2018 Pond CP3 - Excavation after Blast 3
Photo 14	12 01 2018 Pond CP3 - Rock fragmentation after Blast 4
Photo 15	12 06 2018 Pond CP3 - Excavation after Blast 4; view facing northeast
Photo 16	12 07 2018 Pond CP3 - Excavation after Blast 4; view facing east
Photo 17	12 07 2018 Pond CP3 - Loading emulsion for Blast 5; view facing northeast
Photo 18	12 08 2018 Pond CP3 - Excavation after Blast 5; drone view
Photo 19	12 19 2018 Pond CP3 - Completed pond; view down-ramp facing north
Photo 20	02 16 2019 Pond CP3 - Completed snow fence; view facing southeast
Photo 21	11 25 2018 Berm CP3 - Pushing overburden lift; view facing northwest
Photo 22	11 27 2018 Berm CP3 - Placement of overburden Lift 2; view facing north
Photo 23	11 29 2018 Berm CP3 - Placement of upstream rock protection; view facing north
Photo 24	12 18 2018 Berm CP3 - Geotextile and placement of downstream rock protection; view facing south
Photo 25	12 19 2018 Berm CP3 - Final upstream slope; view facing north
Photo 26	01 22 2019 Berm CP3 - Completed downstream slope; view facing south

Photo 1: 09 11 2018 Berm2 - Sloping of upstream till; view from Station 0+520 facing down-chainage



Photo 2: 09 13 2018 Berm2 - Compacted esker material; view from Station 0+240 facing down-chainage

Photo 3: 09 15 2018 Berm2 - Unfinished downstream slope; view from Station 0+120 facing up-chainage



Photo 4: 09 16 2018 Berm2 - Sloping downstream; view from Station 0+400 facing up-chainage

Photo 5: 08 29 2018 Channel3 - Mechanical excavation; Station 0+090 facing down-chainage



Photo 6: 08 29 2018 Channel3 - Poor bank conditions, ice, and bedrock; Station 1+320 facing up-chainage

Photo 7: 09 03 2018 Channel3 - Rebuilt bank and channel construction; Station 1+185 facing up-chainage



Photo 8: 11 24 2018 Channel3 - Loading holes Blast 1; Station 1+590 facing down-chainage



Photo 9: 11 26 2018 Channel3 - Overburden removal from Blast 1; Station 1+600 facing down-chainage



Photo 10: 11 30 2018 Channel3 - Geotextile placement; Station 1+580 facing up-chainage



Photo 11: 11 30 2018 Channel3 - Rip rap placement; Station 1+580 facing up-chainage



Photo 12: 10 23 2018 Pond CP3 - Mechanical overburden excavation; view facing north



Photo 13: 11 29 2018 Pond CP3 - Excavation after Blast 3



Photo 14: 12 01 2018 Pond CP3 - Rock fragmentation after Blast 4



Photo 15: 12 06 2018 Pond CP3 - Excavation after Blast 4; view facing northeast



Photo 16: 12 07 2018 Pond CP3 - Excavation after Blast 4; view facing east



Photo 17: 12 07 2018 Pond CP3 - Loading emulsion for Blast 5; view facing northeast



Photo 18: 12 08 2018 Pond CP3 - Excavation after Blast 5; drone view



Photo 19: 12 19 2018 Pond CP3 - Completed pond; view down-ramp facing north



Photo 20: 02 16 2019 Pond CP3 - Completed snow fence; view facing southeast

Photo 21: 11 25 2018 Berm CP3 - Pushing overburden lift; view facing northwest



Photo 22: 11 27 2018 Berm CP3 - Placement of overburden Lift 2; view facing north



Photo 23: 11 29 2018 Berm CP3 - Placement of upstream rock protection; view facing north



Photo 24: 12 18 2018 Berm CP3 - Geotextile and placement of downstream rock protection; view facing south

Photo 25: 12 19 2018 Berm CP3 - Final upstream slope; view facing north



Photo 26: 01 22 2019 Berm CP3 - Completed downstream slope; view facing south

APPENDIX A

TETRA TECH'S LIMITATIONS ON USE OF THIS DOCUMENT

LIMITATIONS ON USE OF THIS DOCUMENT

GEOTECHNICAL

1.1 USE OF DOCUMENT AND OWNERSHIP

This document pertains to a specific site, a specific development, and a specific scope of work. The document may include plans, drawings, profiles and other supporting documents that collectively constitute the document (the "Professional Document").

The Professional Document is intended for the sole use of NELPCO Limited Partnership's (NELPCo) Client (the "Client") as specifically identified in the NELPCO Services Agreement or other Contractual Agreement entered into with the Client (either of which is termed the "Contract" herein). NELPCO does not accept any responsibility for the accuracy of any of the data, analyses, recommendations or other contents of the Professional Document when it is used or relied upon by any party other than the Client, unless authorized in writing by NELPCO.

Any unauthorized use of the Professional Document is at the sole risk of the user. NELPCO accepts no responsibility whatsoever for any loss or damage where such loss or damage is alleged to be or, is in fact, caused by the unauthorized use of the Professional Document.

Where NELPCO has expressly authorized the use of the Professional Document by a third party (an "Authorized Party"), consideration for such authorization is the Authorized Party's acceptance of these Limitations on Use of this Document as well as any limitations on liability contained in the Contract with the Client (all of which is collectively termed the "Limitations on Liability"). The Authorized Party should carefully review both these Limitations on Use of this Document and the Contract prior to making any use of the Professional Document. Any use made of the Professional Document by an Authorized Party constitutes the Authorized Party's express acceptance of, and agreement to, the Limitations on Liability.

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1.2 ALTERNATIVE DOCUMENT FORMAT

Where NELPCO submits electronic file and/or hard copy versions of the Professional Document or any drawings or other project-related documents and deliverables (collectively termed NELPCO's "Instruments of Professional Service"), only the signed and/or sealed versions shall be considered final. The original signed and/or sealed electronic file and/or hard copy version archived by NELPCO shall be deemed to be the original. NELPCO will archive a protected digital copy of the original signed and/or sealed version for a period of 10 years.

Both electronic file and/or hard copy versions of NELPCO's Instruments of Professional Service shall not, under any circumstances, be altered by any party except NELPCO. NELPCO's Instruments of Professional Service will be used only and exactly as submitted by NELPCO.

Electronic files submitted by NELPCO have been prepared and submitted using specific software and hardware systems. NELPCO makes no representation about the compatibility of these files with the Client's current or future software and hardware systems.

1.3 STANDARD OF CARE

Services performed by NELPCO for the Professional Document have been conducted in accordance with the Contract, in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practicing under similar conditions in the jurisdiction in which the services are provided. Professional judgment has been applied in developing the conclusions and/or recommendations provided in this Professional Document. No warranty or guarantee, express or implied, is made concerning the test results, comments, recommendations, or any other portion of the Professional Document.

If any error or omission is detected by the Client or an Authorized Party, the error or omission must be immediately brought to the attention of NELPCO.

1.4 DISCLOSURE OF INFORMATION BY CLIENT

The Client acknowledges that it has fully cooperated with NELPCO with respect to the provision of all available information on the past, present, and proposed conditions on the site, including historical information respecting the use of the site. The Client further acknowledges that in order for NELPCO to properly provide the services contracted for in the Contract, NELPCO has relied upon the Client with respect to both the full disclosure and accuracy of any such information.

1.5 INFORMATION PROVIDED TO NELPCO BY OTHERS

During the performance of the work and the preparation of this Professional Document, NELPCO may have relied on information provided by third parties other than the Client.

While NELPCO endeavours to verify the accuracy of such information, NELPCO accepts no responsibility for the accuracy or the reliability of such information even where inaccurate or unreliable information impacts any recommendations, design or other deliverables and causes the Client or an Authorized Party loss or damage.

1.6 GENERAL LIMITATIONS OF DOCUMENT

This Professional Document is based solely on the conditions presented and the data available to NELPCO at the time the data were collected in the field or gathered from available databases.

The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this document, at or on the development proposed as of the date of the Professional Document requires a supplementary exploration, investigation, and assessment.

NELPCO is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.

1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, NELPCO has not been retained to explore, address or consider and has not explored, addressed or considered any environmental or regulatory issues associated with development on the subject site.

1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems, methods and standards employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. NELPCO does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historical environment. NELPCO does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional exploration and review may be necessary.

1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

Construction activity can impact structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques, and construction sequence are known.

1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, and the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued satisfactory performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

1.16 DESIGN PARAMETERS

Bearing capacities for Limit States or Allowable Stress Design, strength/stiffness properties and similar geotechnical design parameters quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition used in this report. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions considered in this report in fact exist at the site.

1.17 SAMPLES

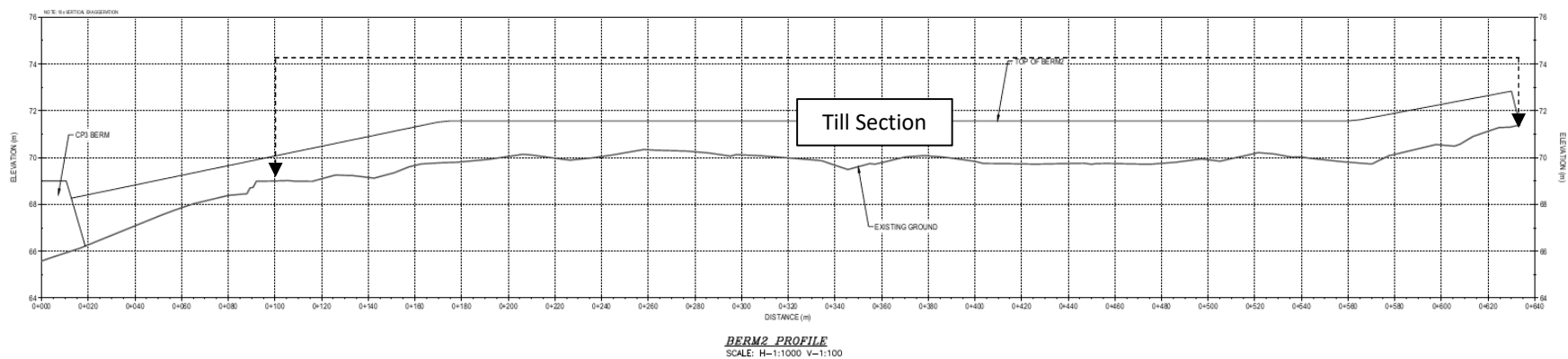
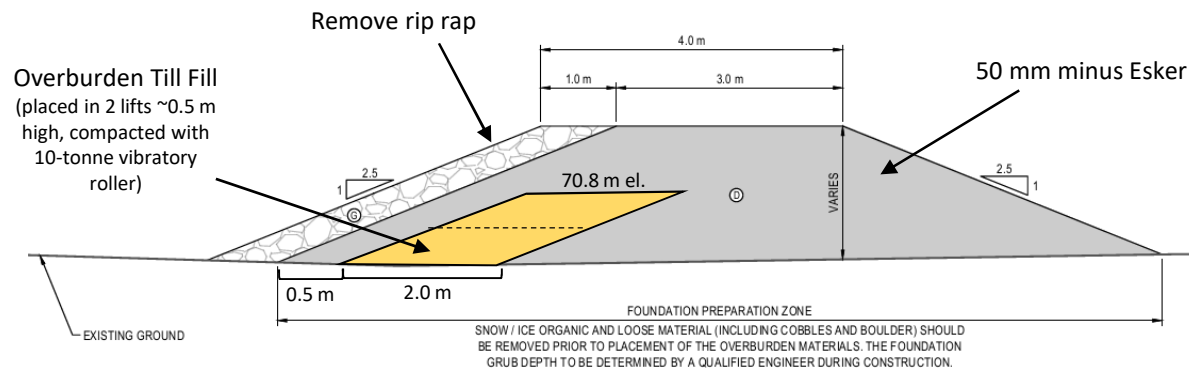
NELPCO will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.

1.18 APPLICABLE CODES, STANDARDS, GUIDELINES & BEST PRACTICE

This document has been prepared based on the applicable codes, standards, guidelines or best practice as identified in the report. Some mandated codes, standards and guidelines (such as ASTM, AASHTO Bridge Design/Construction Codes, Canadian Highway Bridge Design Code, National/Provincial Building Codes) are routinely updated and corrections made. NELPCO cannot predict nor be held liable for any such future changes, amendments, errors or omissions in these documents that may have a bearing on the assessment, design or analyses included in this report.

APPENDIX B

DESIGN MODIFICATION SKETCH FOR BERM2 (RFI 6515-C-235-012-001)



APPENDIX C

PARTICLE SIZE ANALYSIS RESULTS FOR 50 MM MATERIAL AND BATCH TILL

50 mm Minus Screener Reject - Meliadine North North

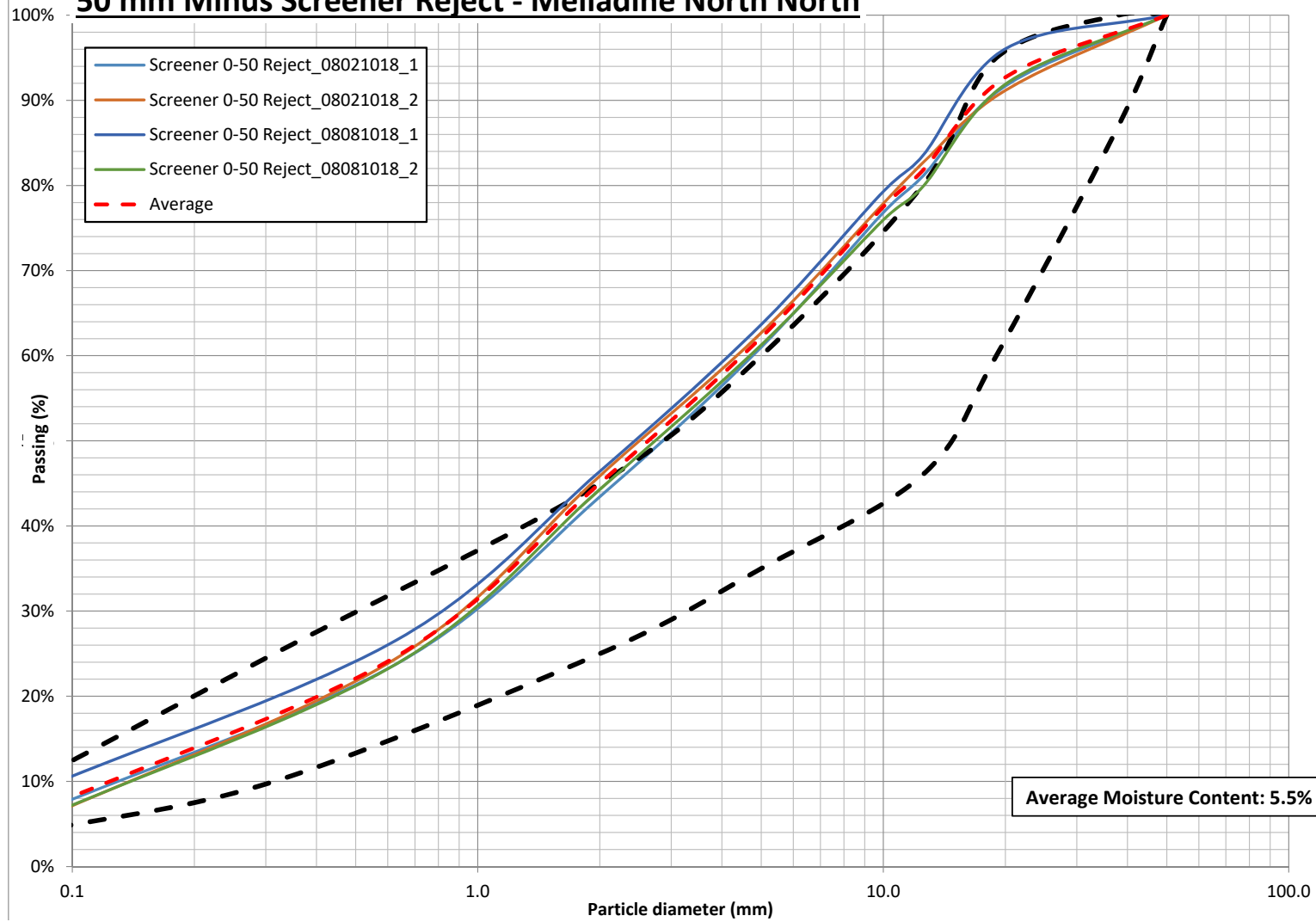


Table 1: Summary of Particle Size Analysis Results - 50 mm minus Screener Reject

No.	Sample ID	Sieve Size (mm)								Moisture Content
		50.00	20.00	12.50	10.00	5.00	2.00	0.63	0.08	
1	Screener 0-50 Reject_08021018_1	100.0%	91.7%	81.1%	76.8%	61.0%	43.4%	23.8%	6.0%	5.1%
2	Screener 0-50 Reject_08021018_2	100.0%	91.2%	82.6%	77.9%	62.7%	45.9%	24.5%	5.1%	5.2%
3	Screener 0-50 Reject_08081018_1	100.0%	96.1%	83.5%	79.3%	63.7%	46.4%	26.6%	8.7%	5.5%
4	Screener 0-50 Reject_08081018_2	100.0%	91.9%	79.8%	75.9%	61.3%	44.3%	23.8%	5.3%	6.1%
Average		100.0%	92.7%	81.7%	77.5%	62.2%	45.0%	24.7%	6.3%	5.5%

Batch Plant Till

Average Moisture Content: 12.1%

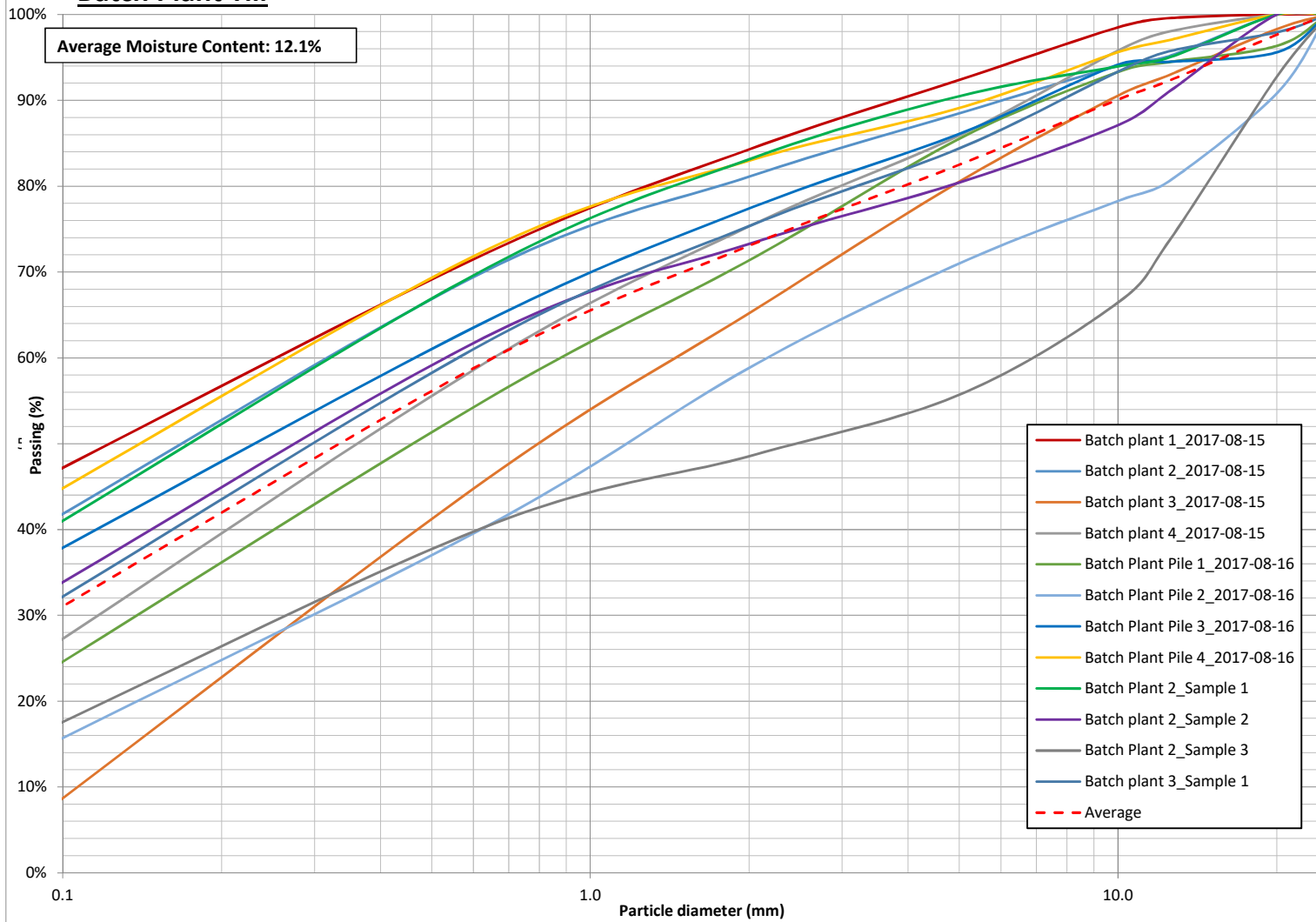
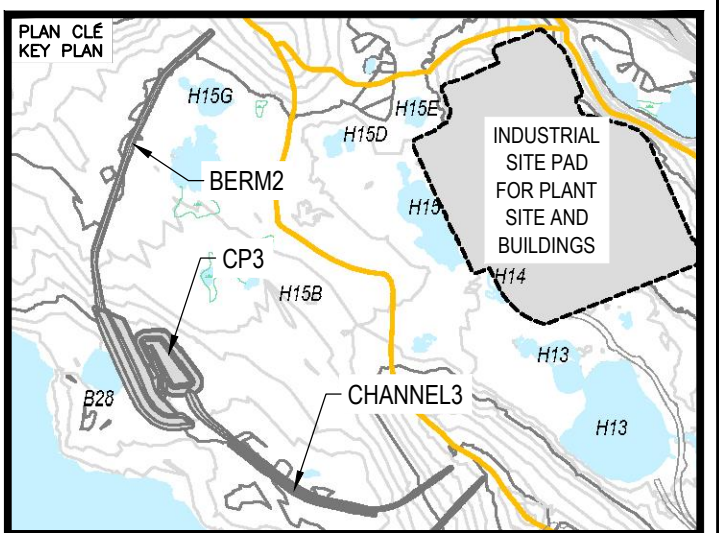
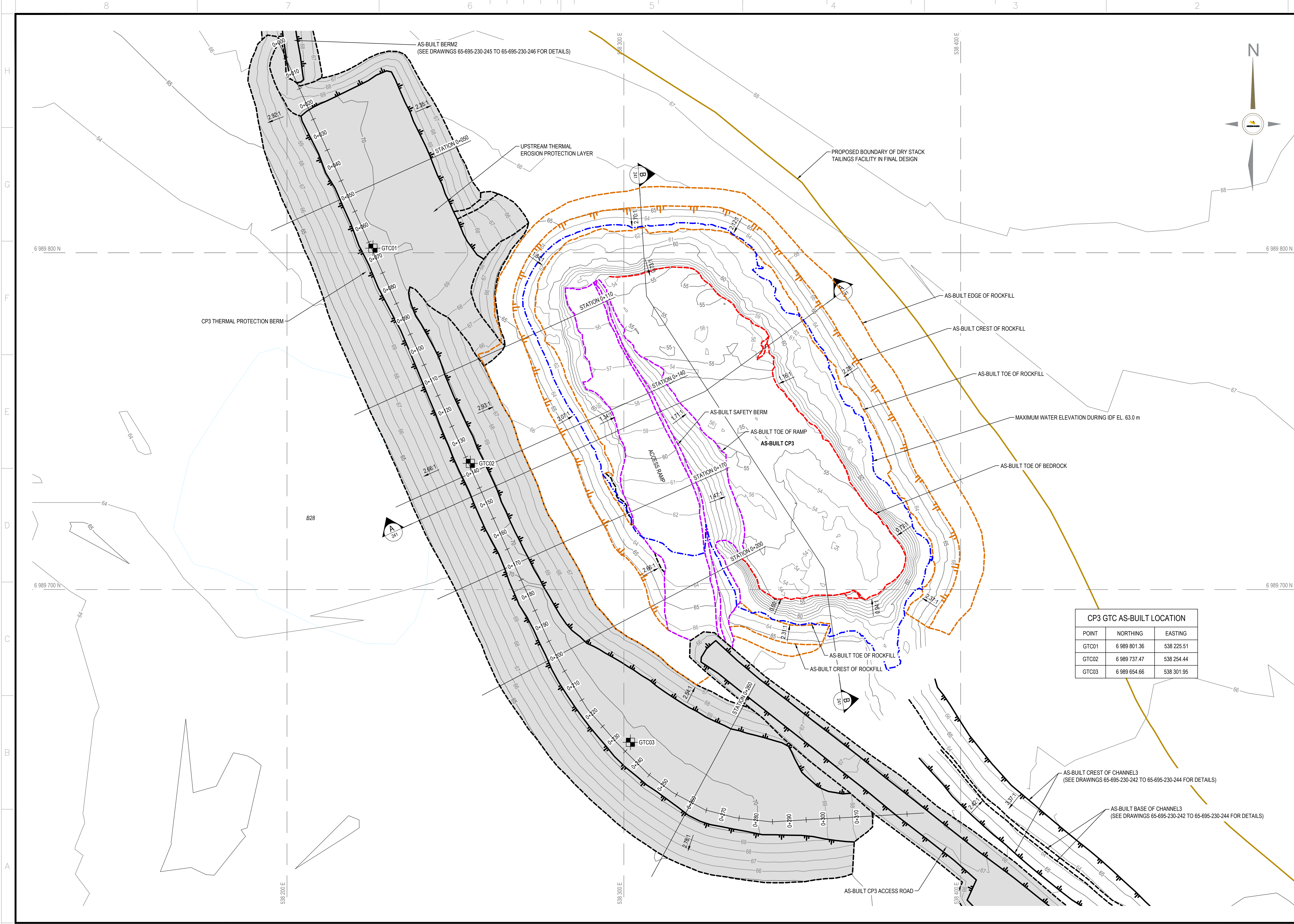


Table 2: Summary of Particle Size Analysis Results - Batch Plant Till

No.	Sample ID	Sieve Size (mm)								Moisture Content
		25.00	20.00	12.50	10.00	5.00	2.00	0.63	0.08	
1	Batch plant 1_2017-08-15	100.0%	100.0%	99.6%	98.5%	92.4%	84.3%	72.1%	44.2%	9.4%
2	Batch plant 2_2017-08-15	100.0%	100.0%	95.1%	94.0%	88.5%	81.1%	70.1%	38.3%	14.7%
3	Batch plant 3_2017-08-15	100.0%	98.3%	92.9%	90.5%	80.5%	65.2%	45.7%	4.2%	11.8%
4	Batch plant 4_2017-08-15	100.0%	100.0%	98.0%	95.8%	85.9%	75.3%	59.4%	23.4%	10.5%
5	Batch Plant Pile 1_2017-08-16	100.0%	96.3%	94.4%	93.3%	85.5%	71.4%	55.0%	20.9%	11.5%
6	Batch Plant Pile 2_2017-08-16	100.0%	90.8%	80.6%	78.3%	71.0%	58.8%	40.2%	12.8%	9.5%
7	Batch Plant Pile 3_2017-08-16	100.0%	95.6%	94.5%	94.1%	86.1%	77.4%	64.2%	34.7%	15.4%
8	Batch Plant Pile 4_2017-08-16	100.0%	100.0%	97.0%	95.6%	89.1%	83.0%	72.4%	41.4%	12.3%
9	Batch Plant 2_Sample 1	100.0%	100.0%	95.0%	93.9%	90.5%	83.1%	70.3%	37.4%	10.8%
10	Batch plant 2_Sample 2	100.0%	100.0%	91.0%	87.1%	80.4%	73.3%	62.4%	30.4%	14.0%
11	Batch Plant 2_Sample 3	100.0%	92.7%	73.6%	66.4%	55.7%	48.5%	40.3%	14.8%	13.1%
12	Batch plant 3_Sample 1	100.0%	97.9%	95.7%	93.3%	84.4%	75.4%	61.7%	28.6%	11.7%
Average		100.0%	97.6%	92.3%	90.1%	82.5%	73.1%	59.5%	27.6%	12.1%

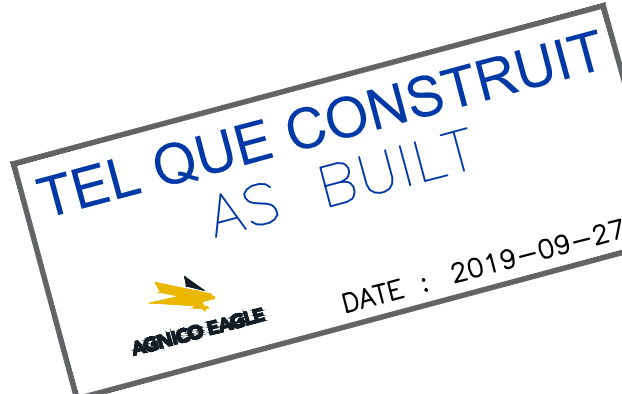
APPENDIX D

CONSTRUCTION RECORD DRAWINGS



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE WINTER 2018 / 2019
2. ASSUMED OPERATION SCHEDULE STARTING TO STORE WATER FROM FRESHET OF 2019.
3. POND DESIGN CAPACITY IS BASED ON STORING 3 OUT OF 7 FRESHET WATER UNDER 1:100 WET YEAR CONDITION.
4. THE MAXIMUM ALLOWABLE OPERATING WATER LEVEL IS AT ELEVATION 63.0 m UNDER THE DESIGN IDF CONDITION. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).



L'INFORMATION CONTENUE EST LA PROPRIÉTÉ DE AGNICO EAGLE LTD. ET DOIT ÊTRE RETENUE SANS RÉPONSE, SANS AUTORIZATION ÉCRITE, POUR ÊTRE TRANSMISE À AUTRES. L'AGNICO EAGLE LTD. NE GARANTIT PAS L'EXACTITUDE DES DONNÉES FOURNIES. L'AGNICO EAGLE LTD. NE GARANTIT PAS L'EXACTITUDE DES DONNÉES FOURNIES. L'AGNICO EAGLE LTD. NE GARANTIT PAS L'EXACTITUDE DES DONNÉES FOURNIES.

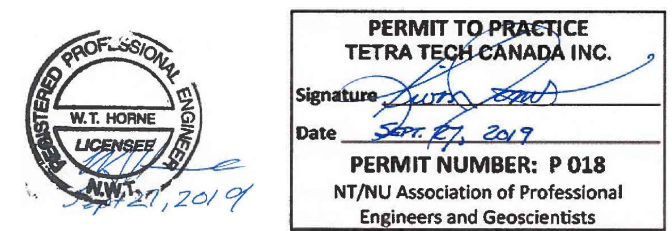
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A	2018-05-29	ISSUED FOR REVIEW	HX	WITH

REVISIONS



PERMIT TO PRACTICE
TETRA TECH CANADA INC.
Signature: [Signature]
Date: 2019-09-27
PERMIT NUMBER: P-018
NTNU Association of Professional Engineers and Geoscientists

TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

CP3 AND CP3 THERMAL PROTECTION BERM
LAYOUT PLAN

DESSINÉ PAR DRAWN BY	EL	DATE 2019-06-12
VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WITH	2019-06-12
ÉCHELLE SCALE	1:500	DATE 2019-06-12

NO. DESSIN
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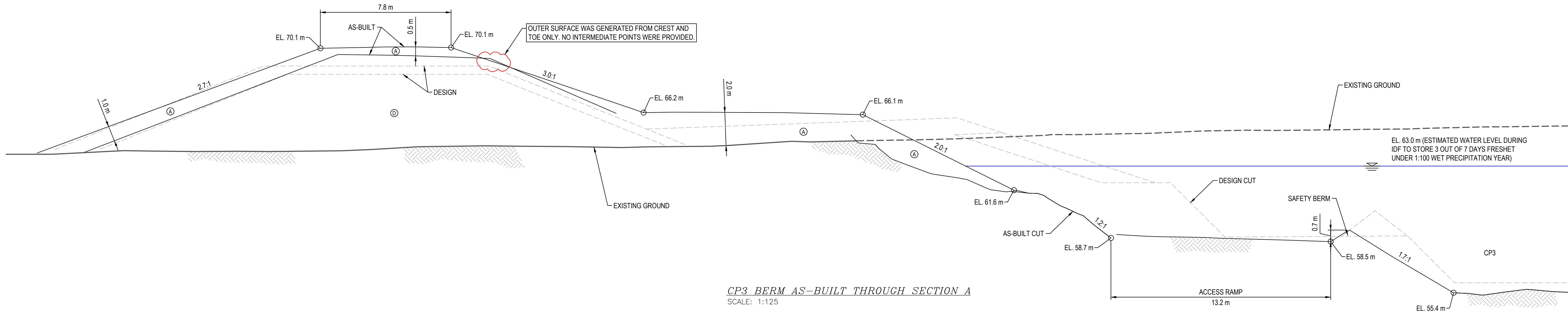
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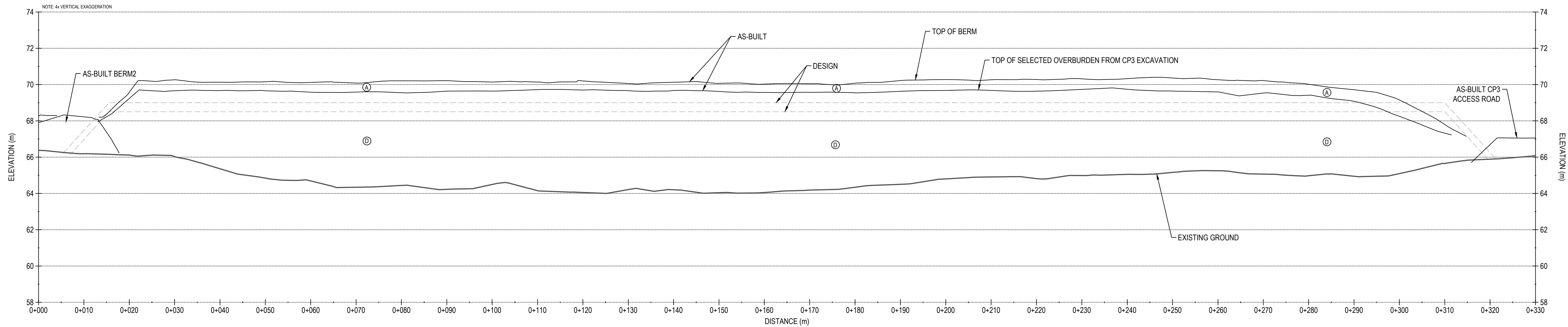
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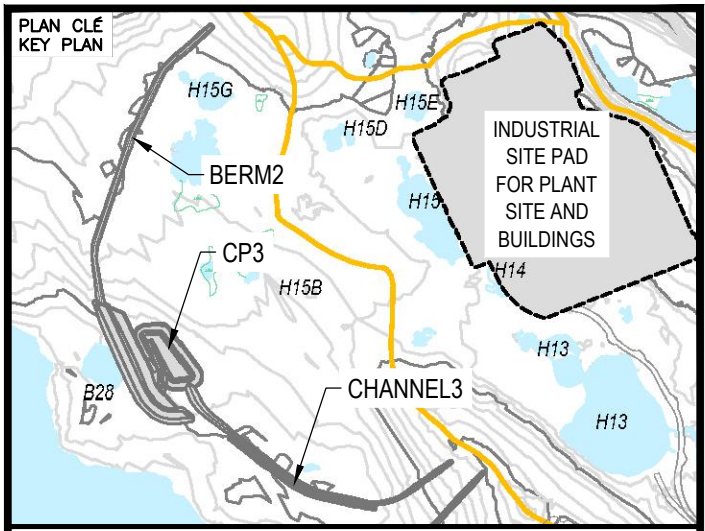
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CP3 BERM AS-BUILT THROUGH SECTION A
SCALE: 1:125



CP3 - BERM PROFILE
SCALE: H-1:500 V-1:125



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE WINTER 2018 / 2019
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LEGEND

- (A) CLEAN ROCKFILL FROM EXCAVATION (600 mm MINUS)
(D) OVERBURDEN FROM EXCAVATION (300 mm MINUS)



L'INFORMATION CONTENUE EST LA PROPRIÉTÉ DE L'AGNICO EAGLE LTD. ET NE DOIT ÊTRE RETENUE, SUR DEMANDE, SANS AUTORISATION ÉCRITE. PROJET: TEL QUE CONSTRUIT AS BUILT. L'INFORMATION CONTENUE EST LA PROPRIÉTÉ DE L'AGNICO EAGLE LTD. ET NE DOIT ÊTRE RETENUE, SUR DEMANDE, SANS AUTORISATION ÉCRITE. PROJET: TEL QUE CONSTRUIT AS BUILT.

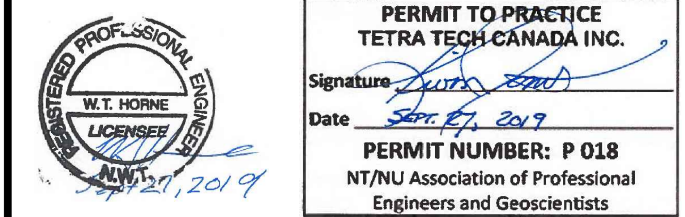
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A	2018-05-29	ISSUED FOR REVIEW	LOK	WITH	

REVISIONS



TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

CP3 TYPICAL SECTION AND
CP3 THERMAL PROTECTION BERM PROFILE

DESSINÉ PAR
DRAWN BY EL

DATE
2019-06-12

VERIFIÉ PAR
CHECKED BY HX

DATE
2019-06-12

APPROUVÉ PAR
APPROVED BY WTH

DATE
2019-06-12

ÉCHELLE
SCALE AS SHOWN

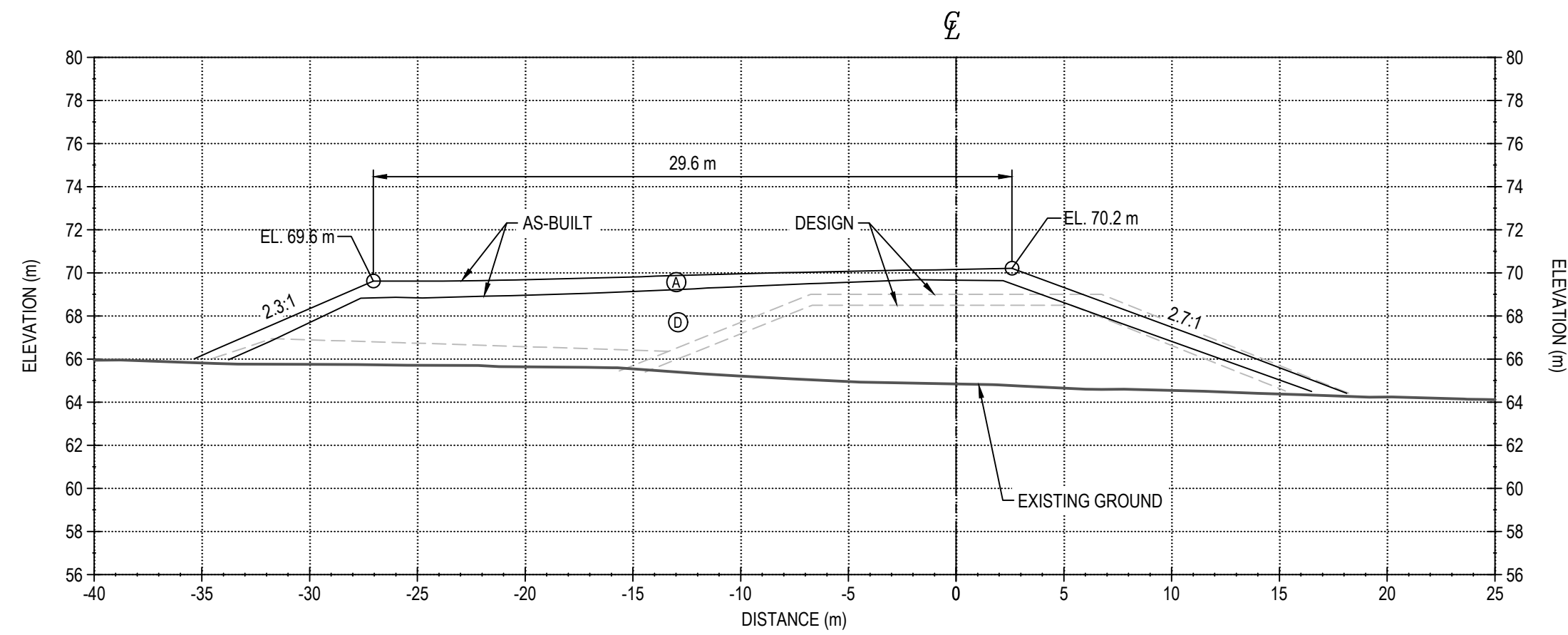
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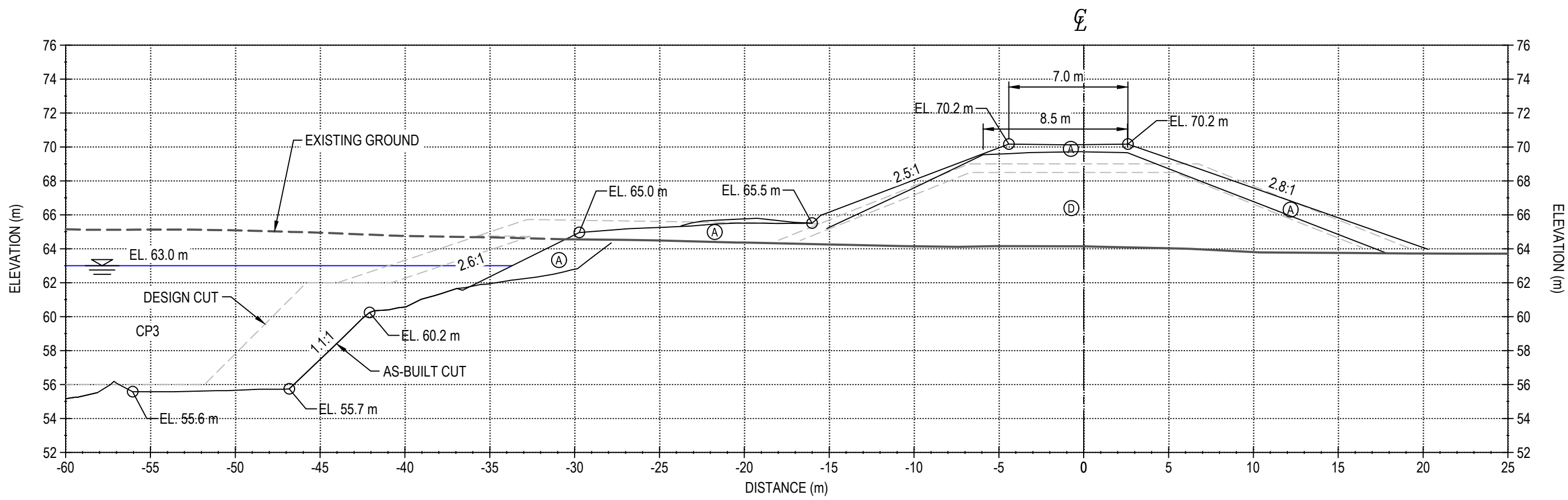
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PROJECT NO. 6515

REVISION
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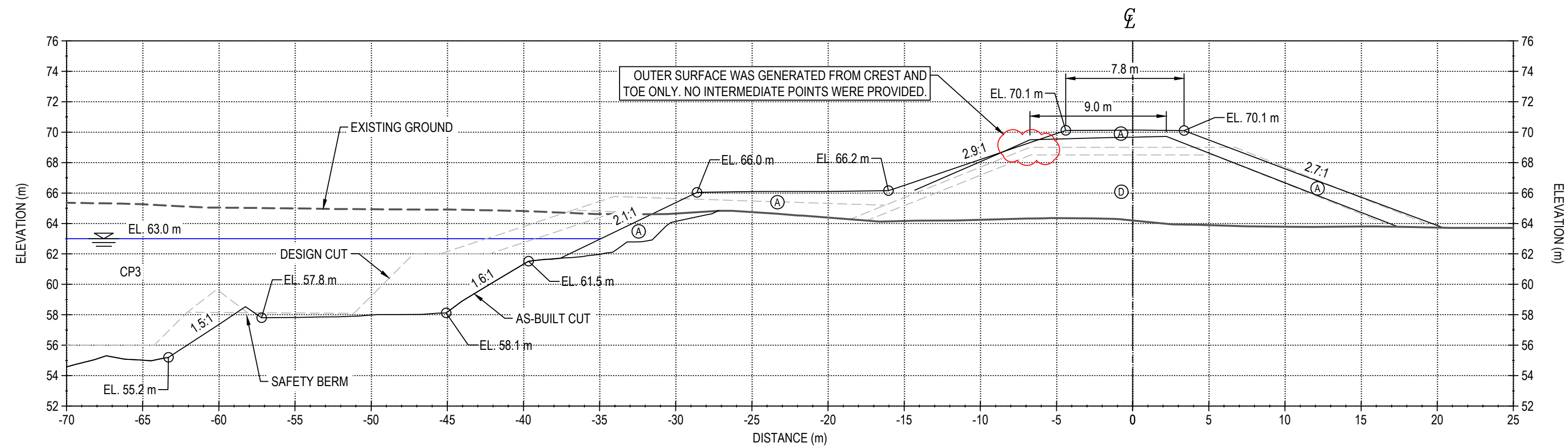
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2 / 13



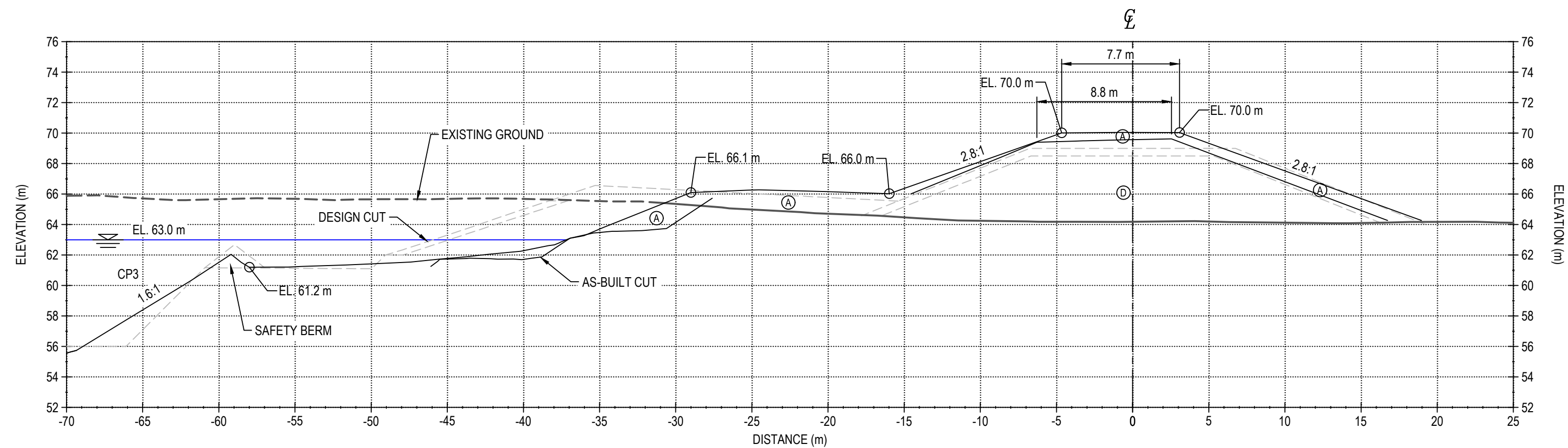
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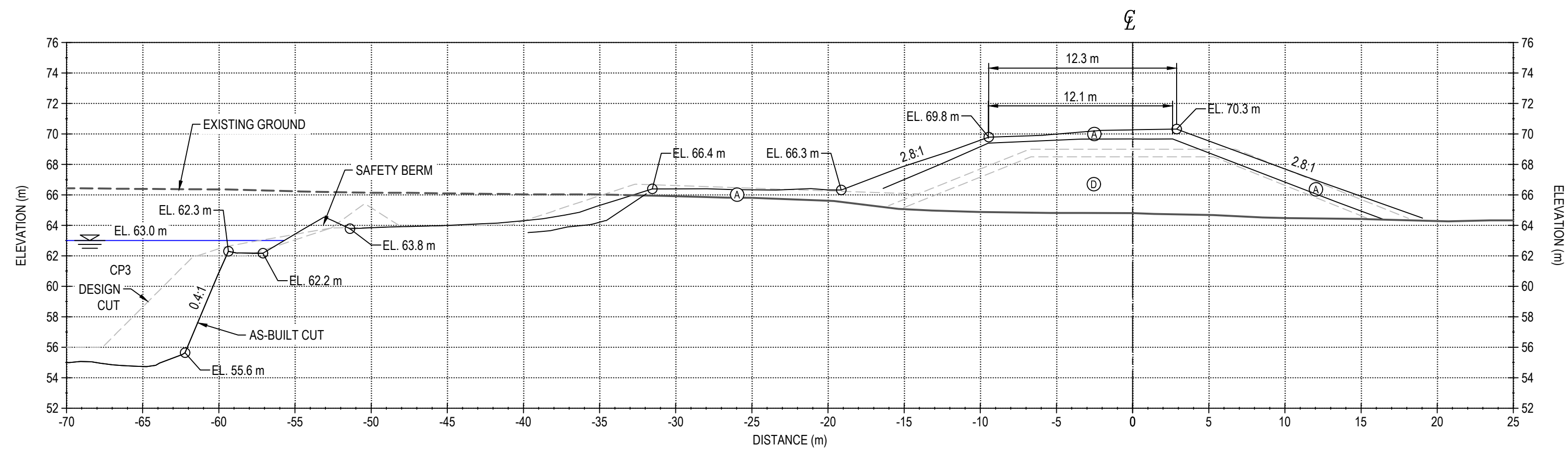
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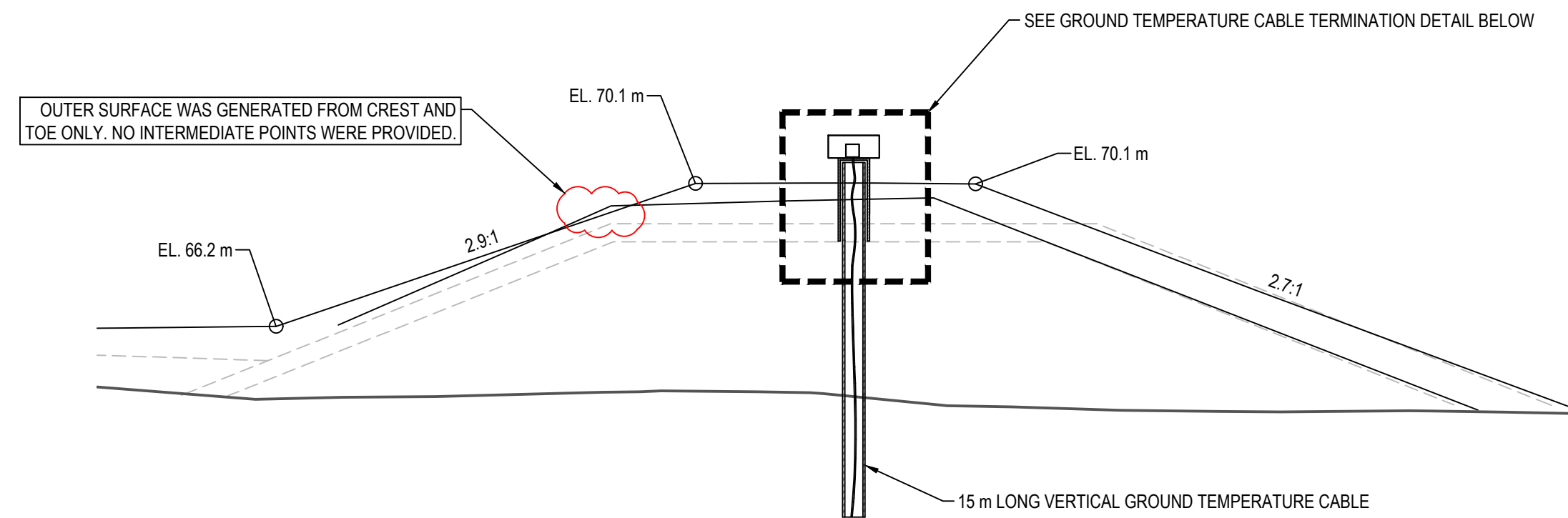
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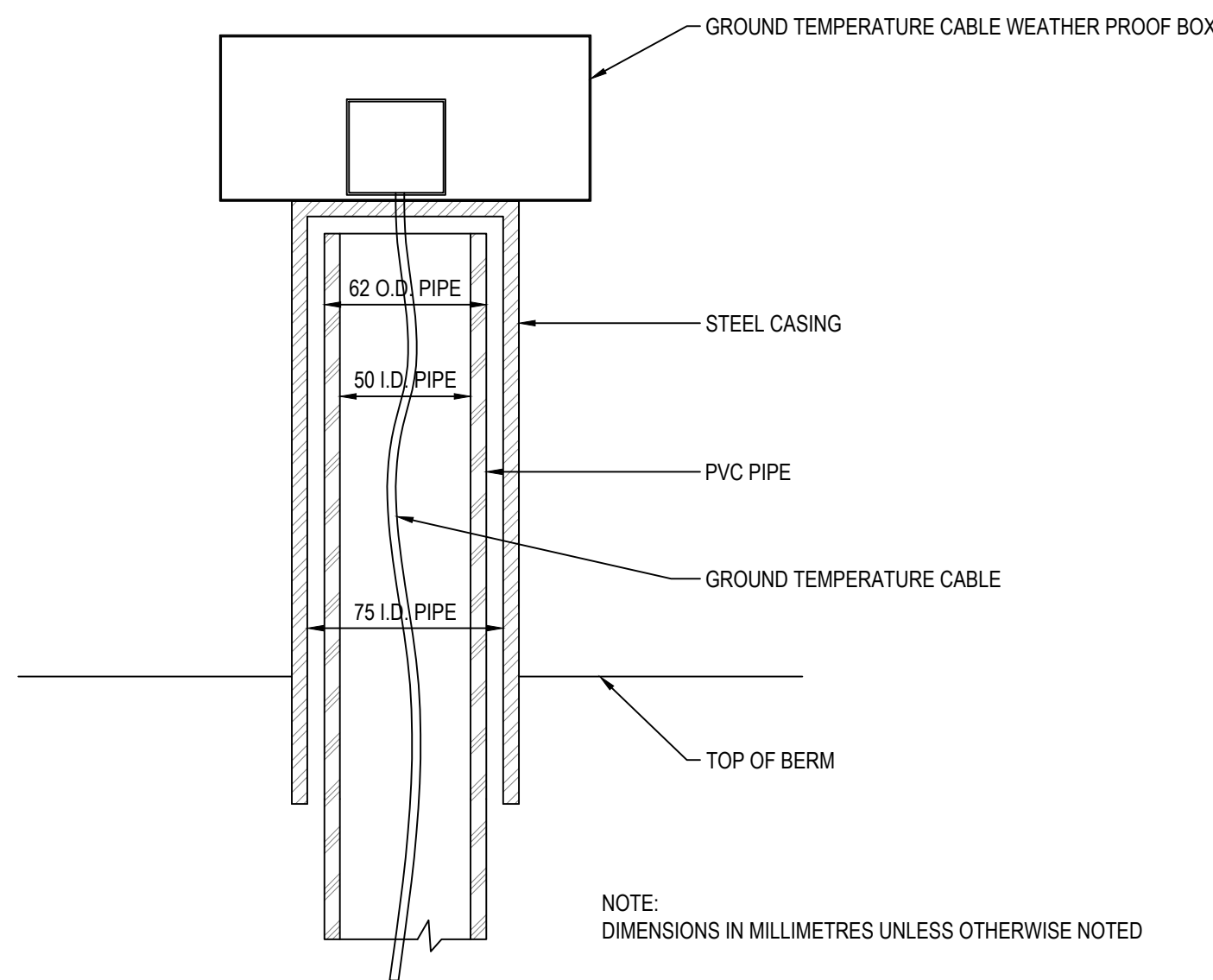
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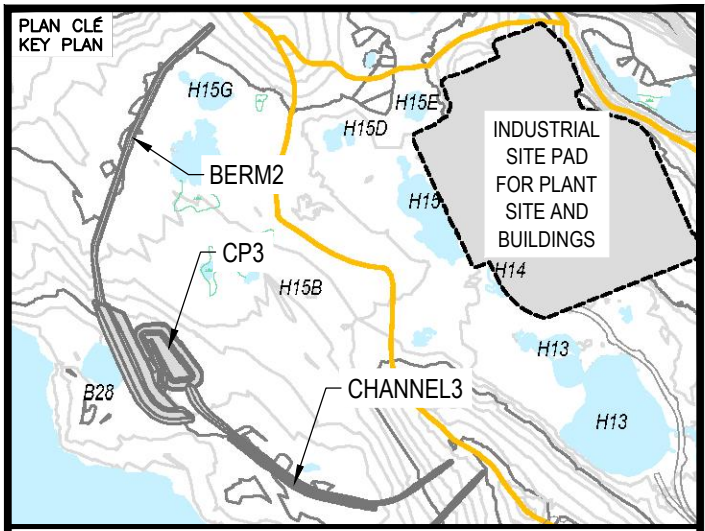


TYPICAL SECTION - CP3 BERM GROUND TEMPERATURE CABLE INSTALLATION



DETAIL - GROUND TEMPERATURE CABLE TERMINATION

SCALE: N.T.S.

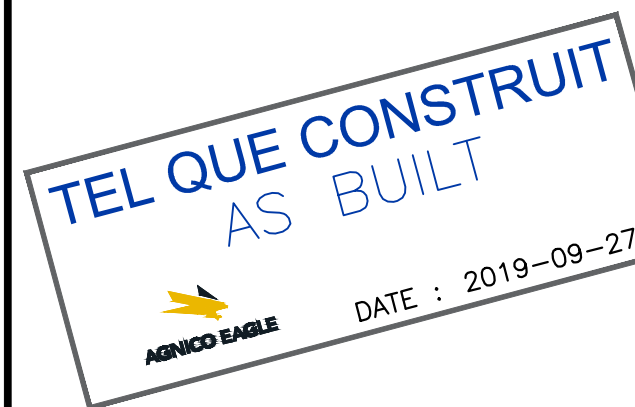


NOTES GÉNÉRALES / GENERAL NOTES

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LEGEND

- (A) CLEAN ROCKFILL FROM EXCAVATION (600 mm MINUS)
- (D) OVERBURDEN FROM EXCAVATION (300 mm MINUS)



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REVISIONS	
Signature	Date
Signature	Date
PERMIT NUMBER: P 018	
NTNU Association of Professional Engineers and Geoscientists	

TITLE / TITRE
AGNICO EAGLE MELIADINE GOLD PROJECT
CP3 AND CP3 THERMAL PROTECTION BERM SECTIONS

DESIGNED BY	EL	DATE	2019-06-12
CHECKED BY	HX	DATE	2019-06-12
APPROVED BY	WITH	DATE	2019-06-12
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NO. DESIGN PROJECT NO.	65-695-230-240
NO. PROJECT PROJECT NO.	6515
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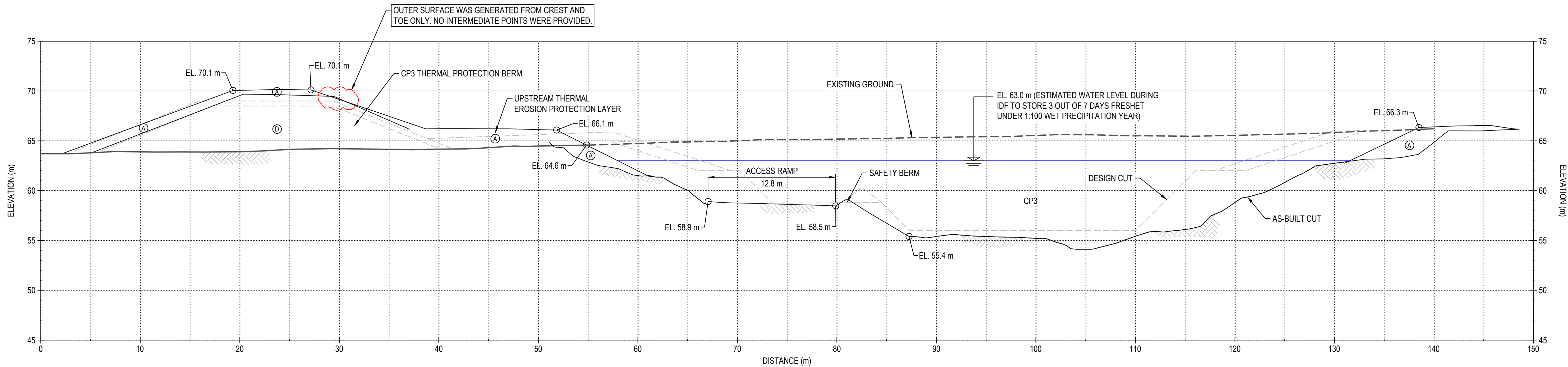
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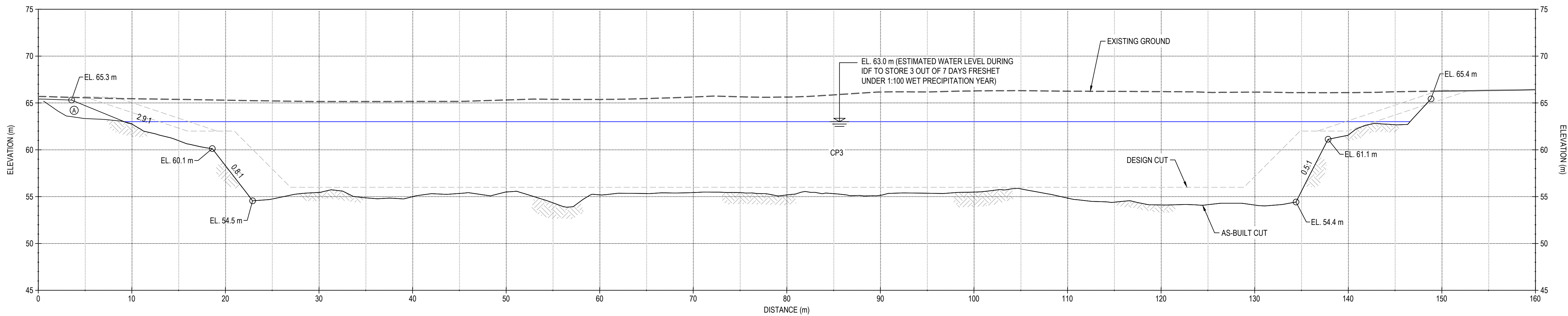
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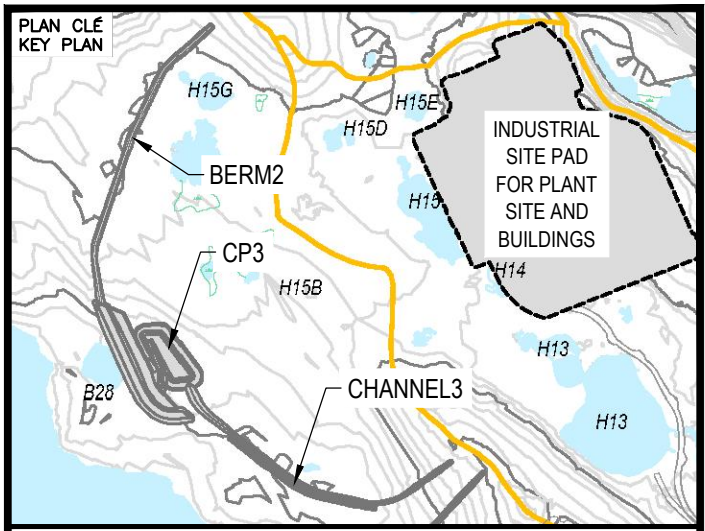
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SECTION B
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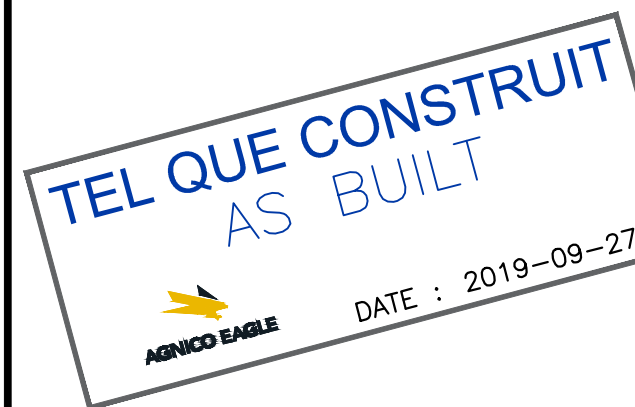


NOTES GÉNÉRALES / GENERAL NOTES

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5. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).

LEGEND

- (A) CLEAN ROCKFILL FROM EXCAVATION (600 mm MINUS)
- (B) OVERBURDEN FROM EXCAVATION (300 mm MINUS)



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PERMIT TO PRACTICE
TETRA TECH CANADA INC.

Signature: [Signature]

Date: [Date]

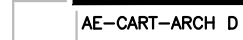
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NTNU Association of Professional Engineers and Geoscientists

TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

CP3
CROSS-SECTIONS

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VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WITH	2019-06-12
ÉCHELLE SCALE	1:250	DATE 2019-06-12
NO. DESIGN DRAWING NO. 65-695-230-241		
NO. PROJET PROJECT NO. 6515	REVISION 1	FEUILLE / SHEET 4 / 13



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).

TEL QUE CONSTRUIT
AS BUILT

DATE : 2019-09-12

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REV.	DATE	DESCRIPTION	PAR/BY	APP.	CLIENT	

REVISIONS



Signature [Signature]

Date Sept. 17, 2019

PERMIT NUMBER: P 018
NT/NU Association of Professional

Engineers and Geoscientists

TITRE / TITLE

AGNICO EAGLE MELIADINE GOLD PROJECT

CHANNEL3 AND CHANNEL3 BER LAYOUT PLAN

DESSINÉ PAR	DATE
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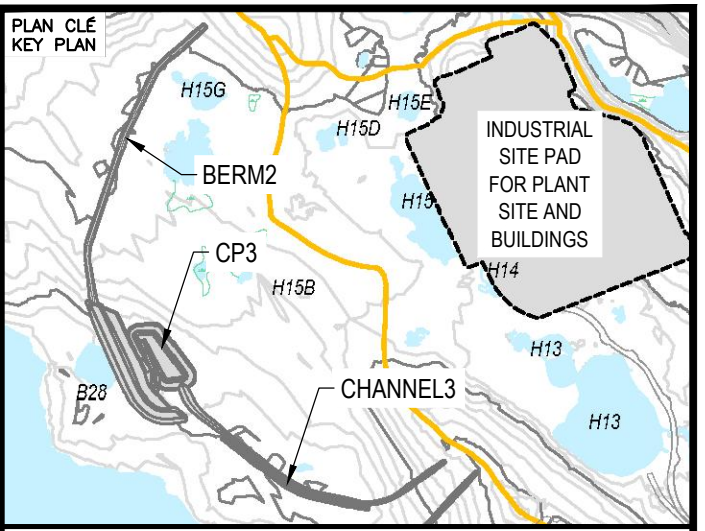
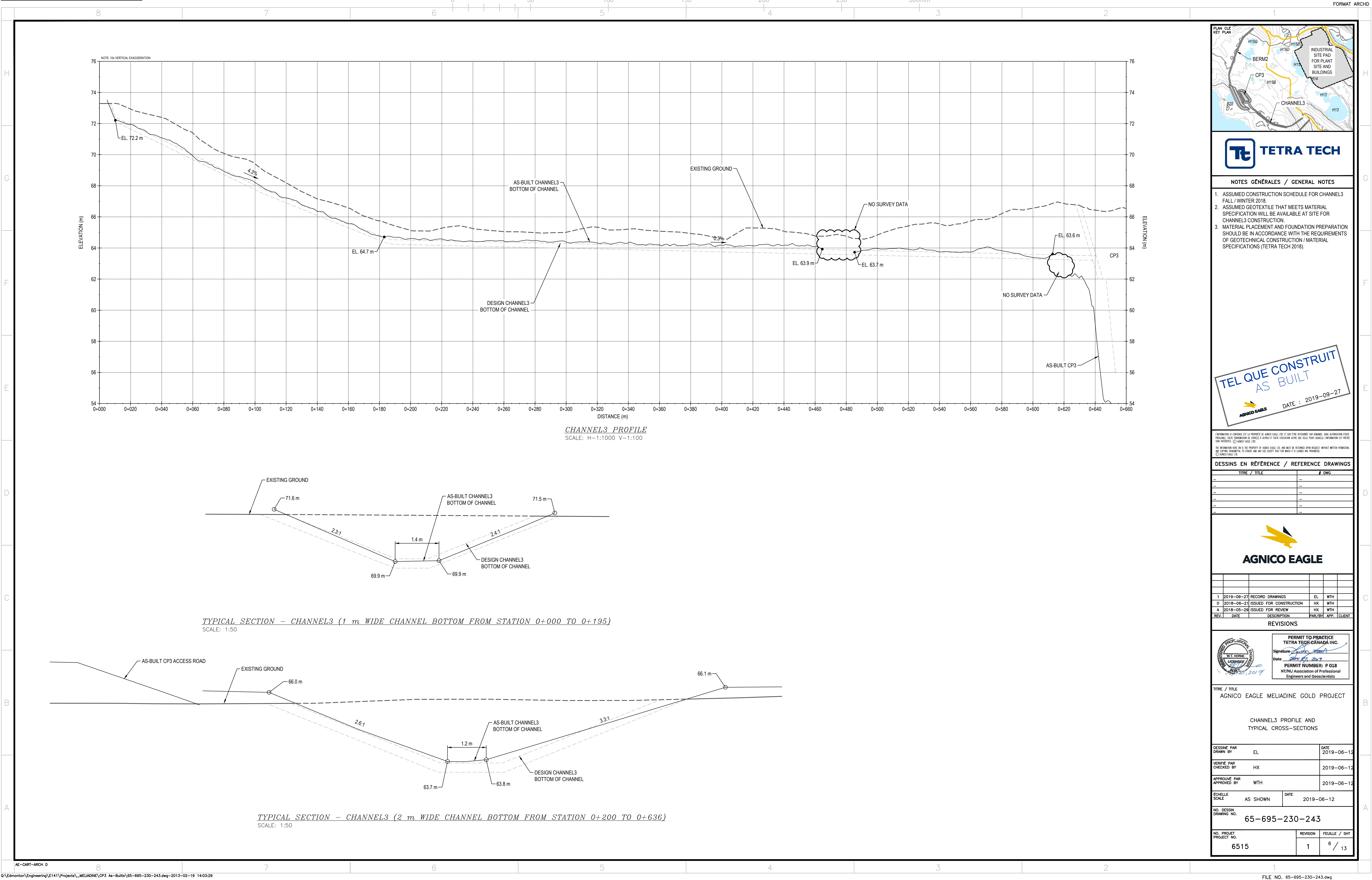
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65-695-230-242

NO. PROJ. PROJECT NO. 6515	REVISION	FEUILLE / SHT
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NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL 3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL 3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).



L'INFORMATION CONTENUE EST LA PROPRIÉTÉ DE AGNICO EAGLE LTD. ET DOIT ÊTRE RETENUE SANS AUTORIZATION ÉCRITE. PROHIBÉ DE LA REPRODUIRE OU D'UTILISER SANS L'APPROBATION DE AGNICO EAGLE LTD. (AGNICO EAGLE LTD. PROPRIÉTÉ).
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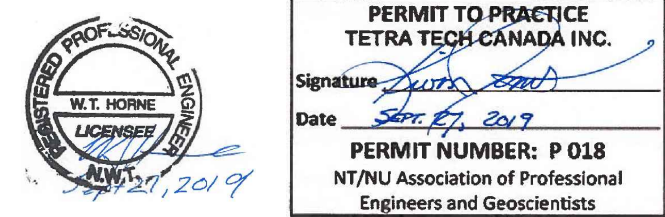
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REVISIONS



PERMIT TO PRACTICE
TETRA TECH CANADA INC.
Signature: *[Signature]*
Date: *2019-09-27*
PERMIT NUMBER: P-018
NTNU Association of Professional Engineers and Geoscientists

TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

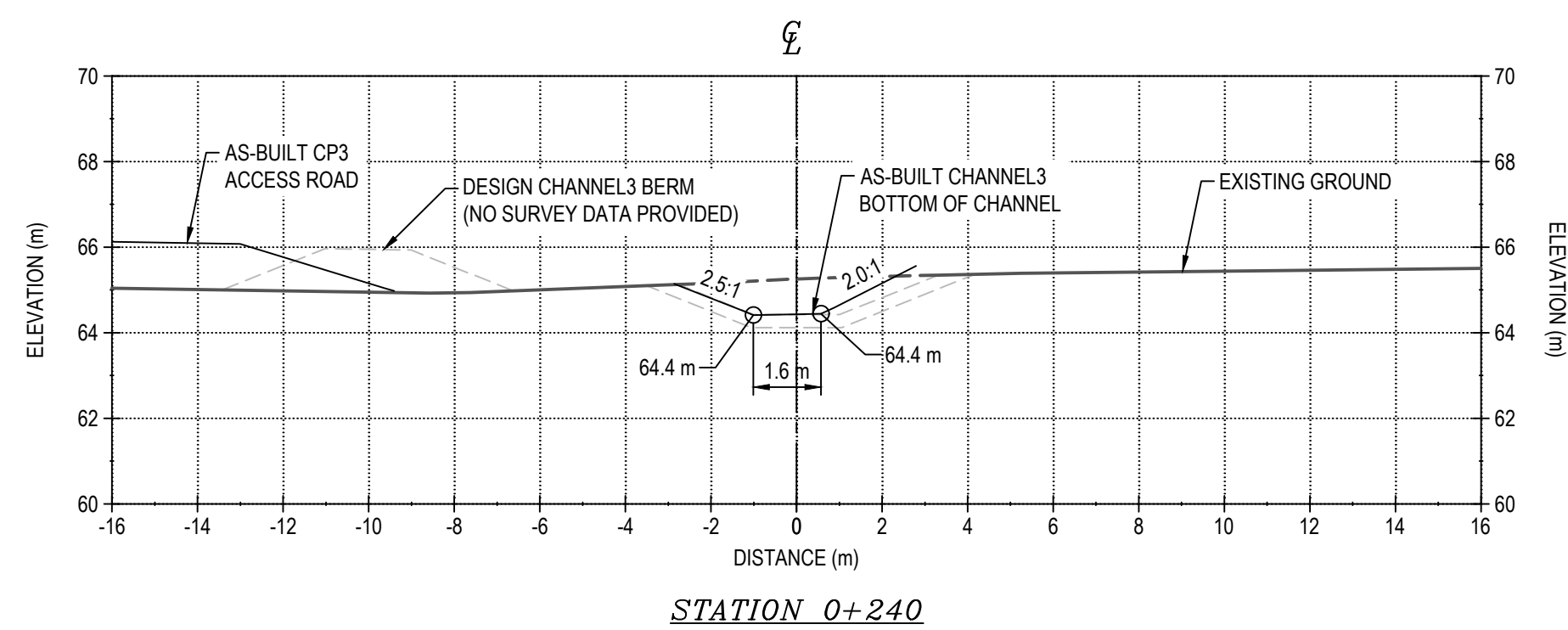
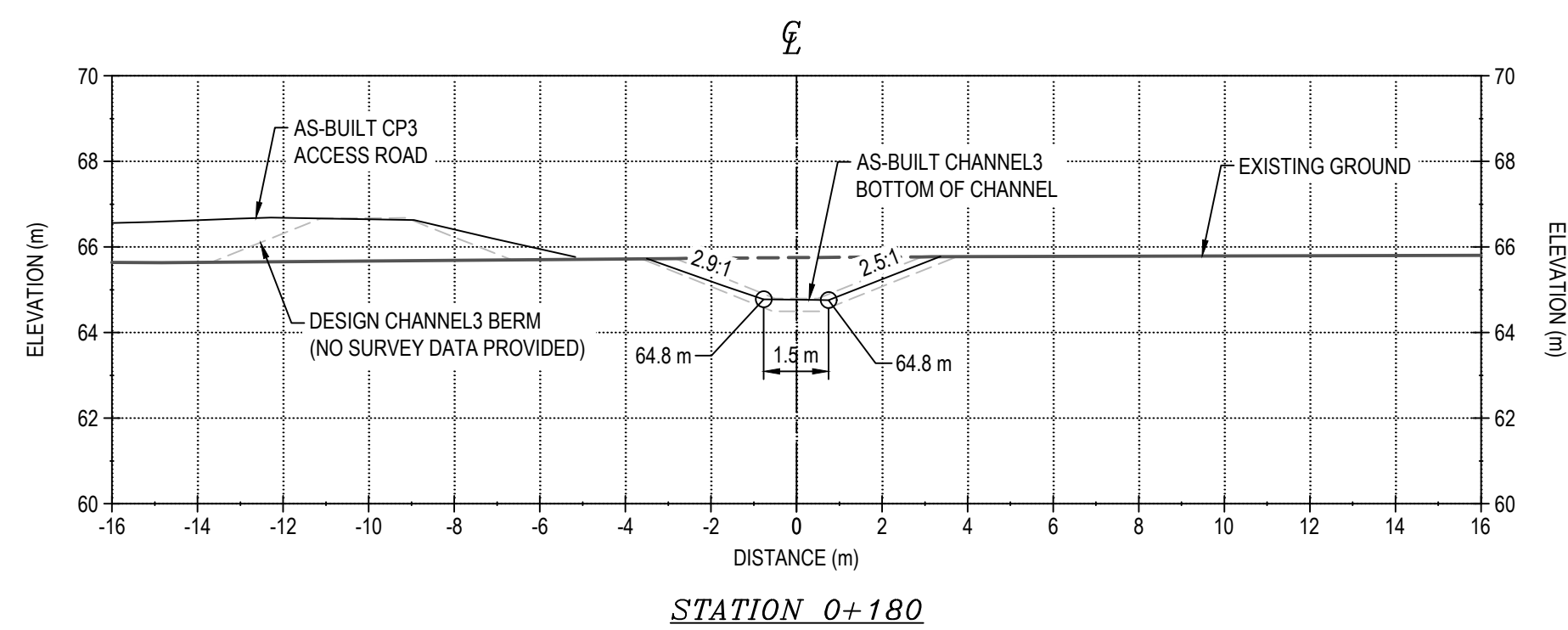
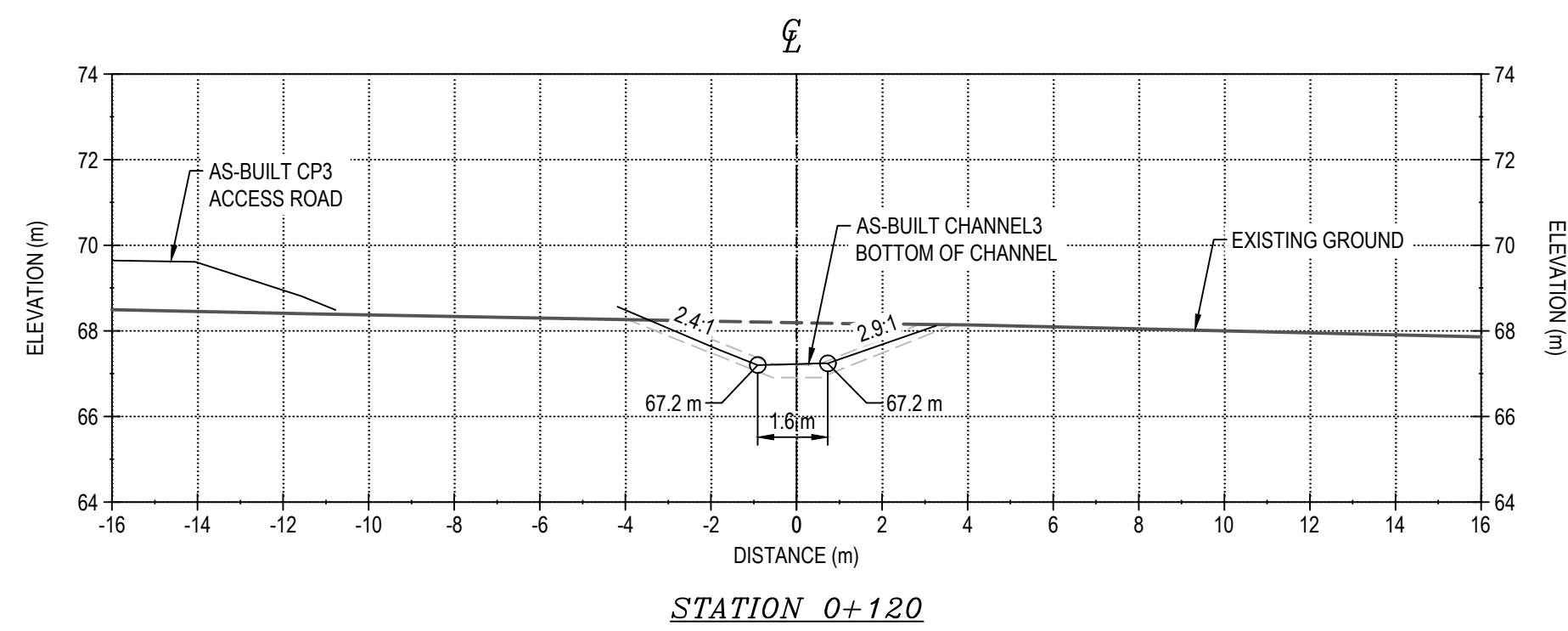
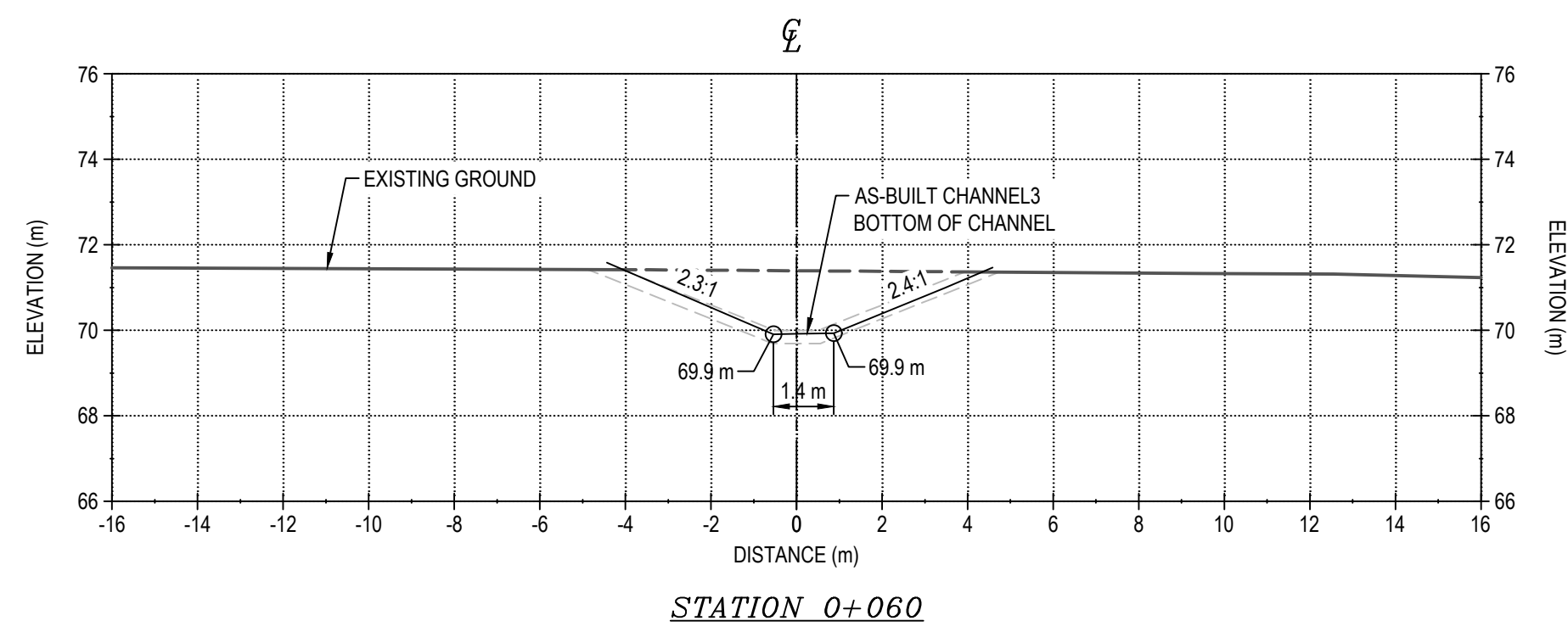
CHANNEL 3 PROFILE AND
TYPICAL CROSS-SECTIONS

DESSINÉ PAR DRAWN BY	EL	DATE 2019-06-12
VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WITH	2019-06-12

ÉCHELLE
SCALE AS SHOWN DATE 2019-06-12

NO. DESSIN
DRAWING NO. 65-695-230-243

NO. PROJET PROJECT NO.	REVISION	FEUILLE / SHEET
6515	1	6 / 13



1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).

TEL QUE CONSTRUIT
AS BUILT

 DATE : 2019-09-27

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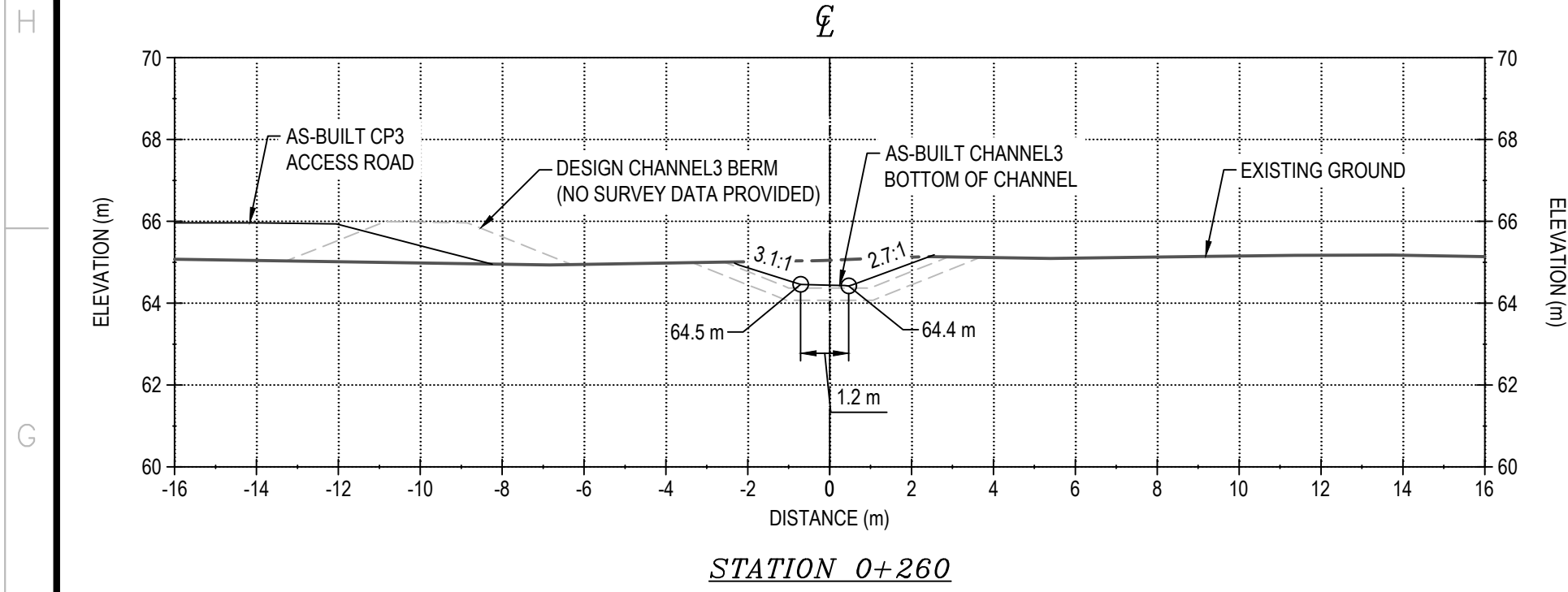
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A	2018-05-29	ISSUED FOR REVIEW	HX	WTH	
REV.	DATE	DESCRIPTION	PAR/BY	APP.	CLIENT

	PERMIT TO PRACTICE TETRA TECH CANADA INC.	
	Signature	<u>[Signature]</u>
	Date	<u>Sept. 27, 2019</u>
	PERMIT NUMBER: P 018 NT/NU Association of Professional Engineers and Geoscientists	

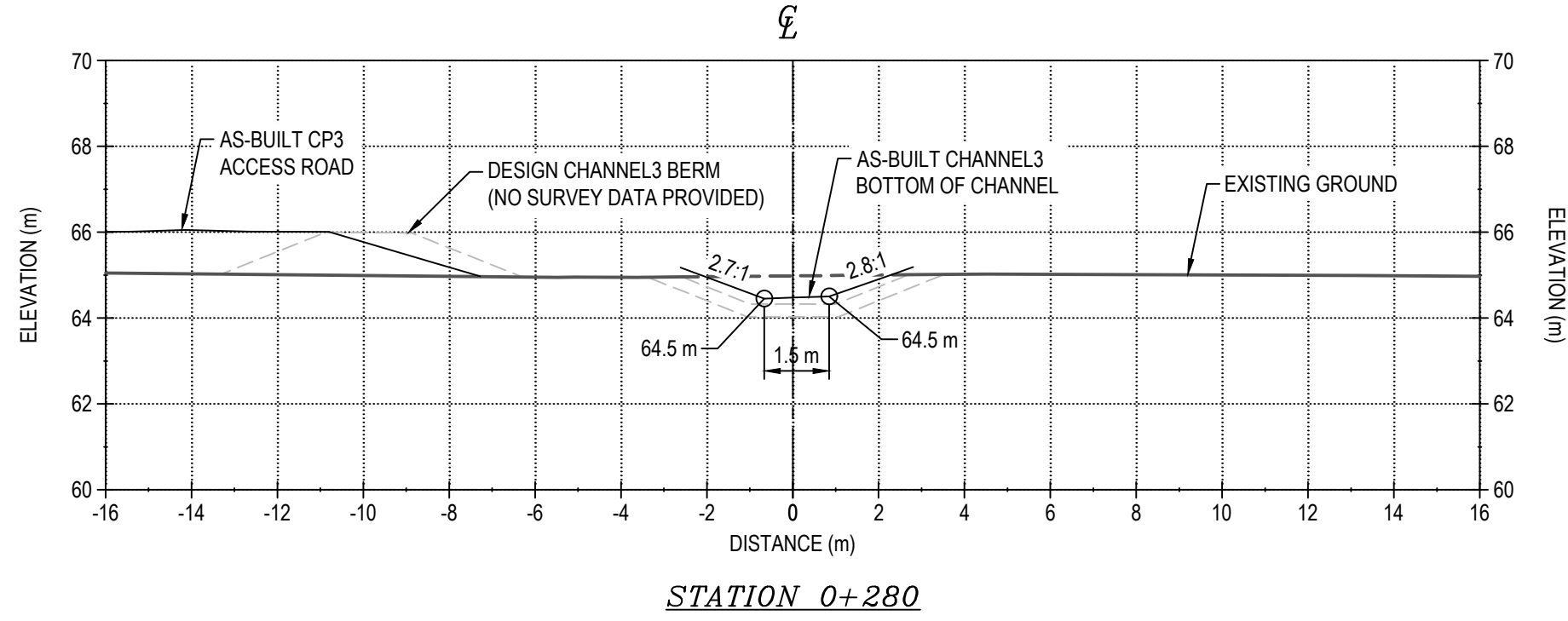
CHANNEL3 DESIGN SECTIONS
(1 of 3)

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VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WITH	2019-06-12
ÉCHELLE SCALE	1:150	DATE 2019-06-12

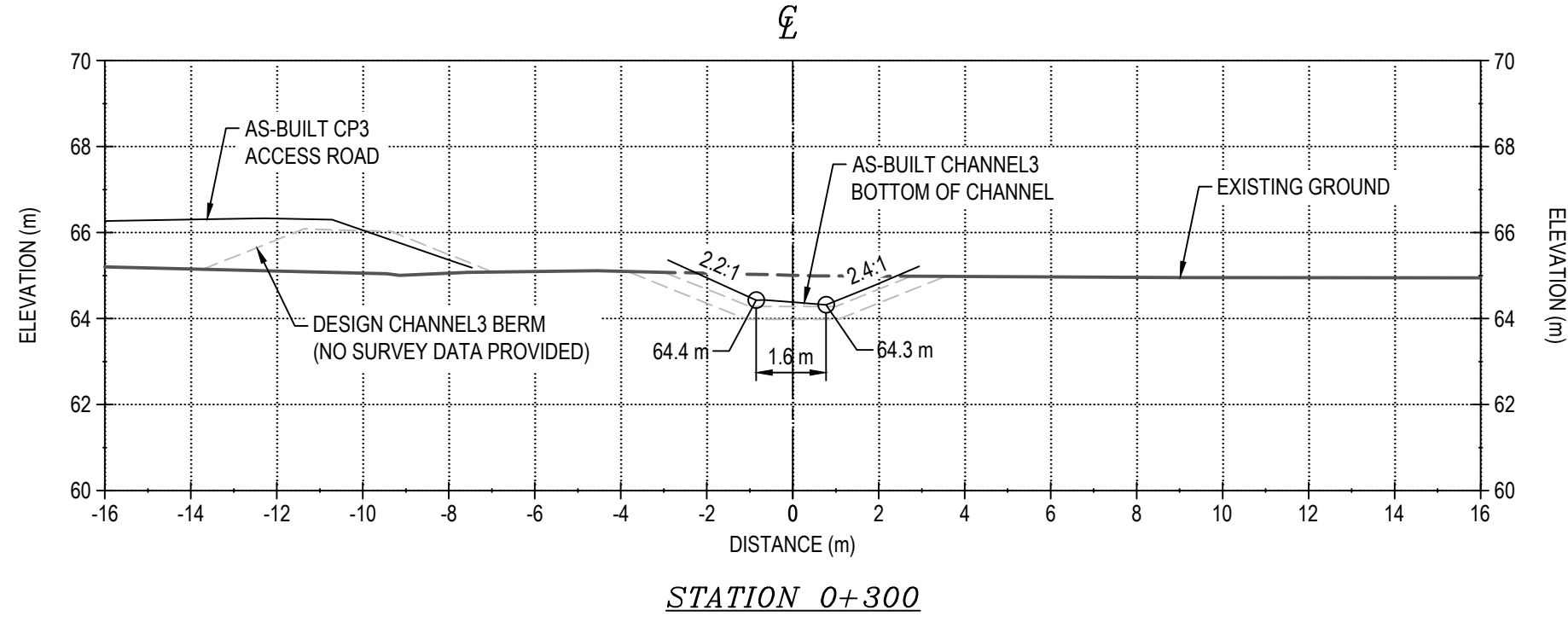
NO. PROJET PROJECT NO. 6515	REVISION	FEUILLE / SHT
	1	7 / 13



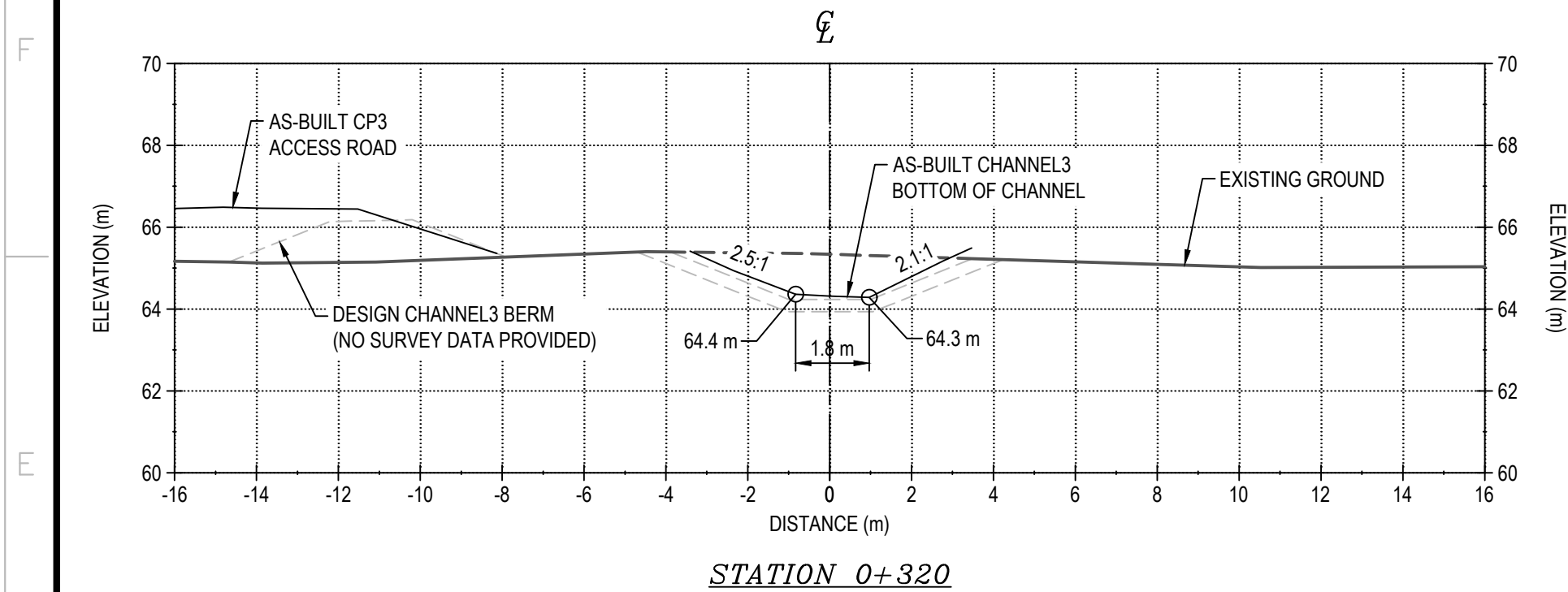
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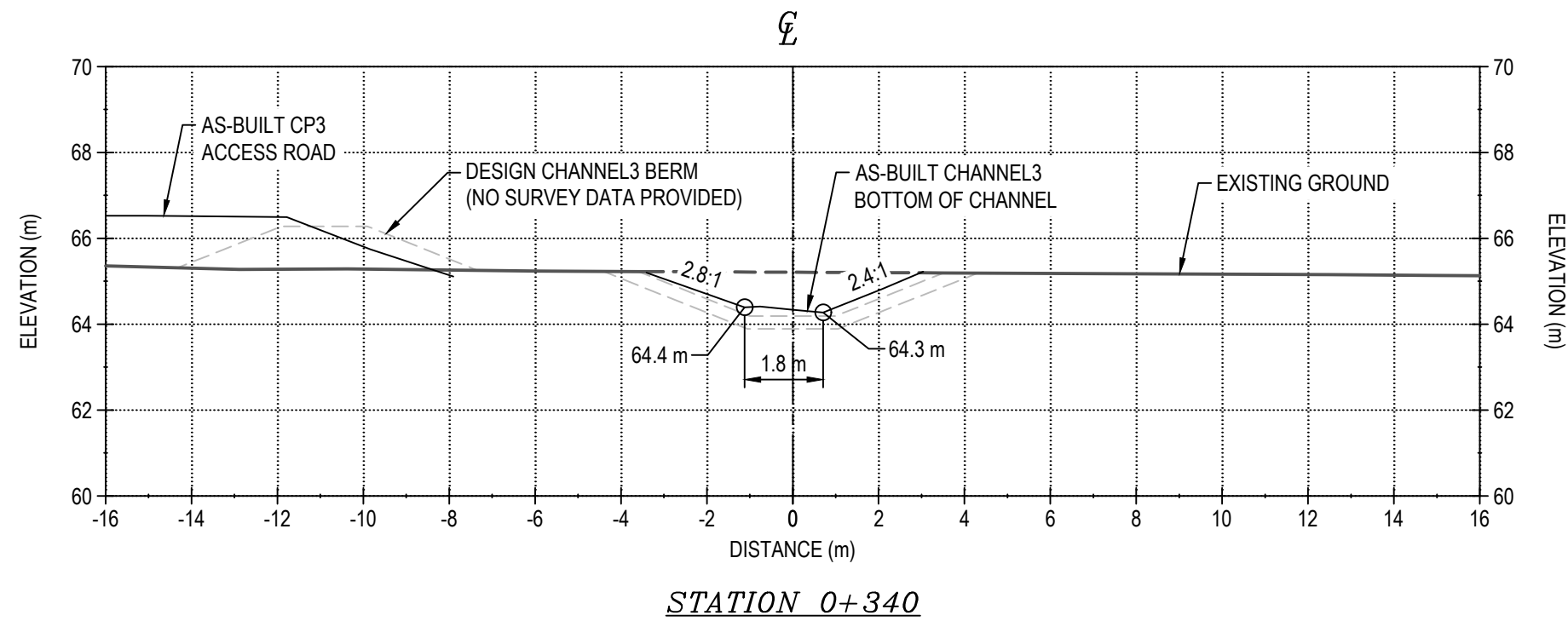
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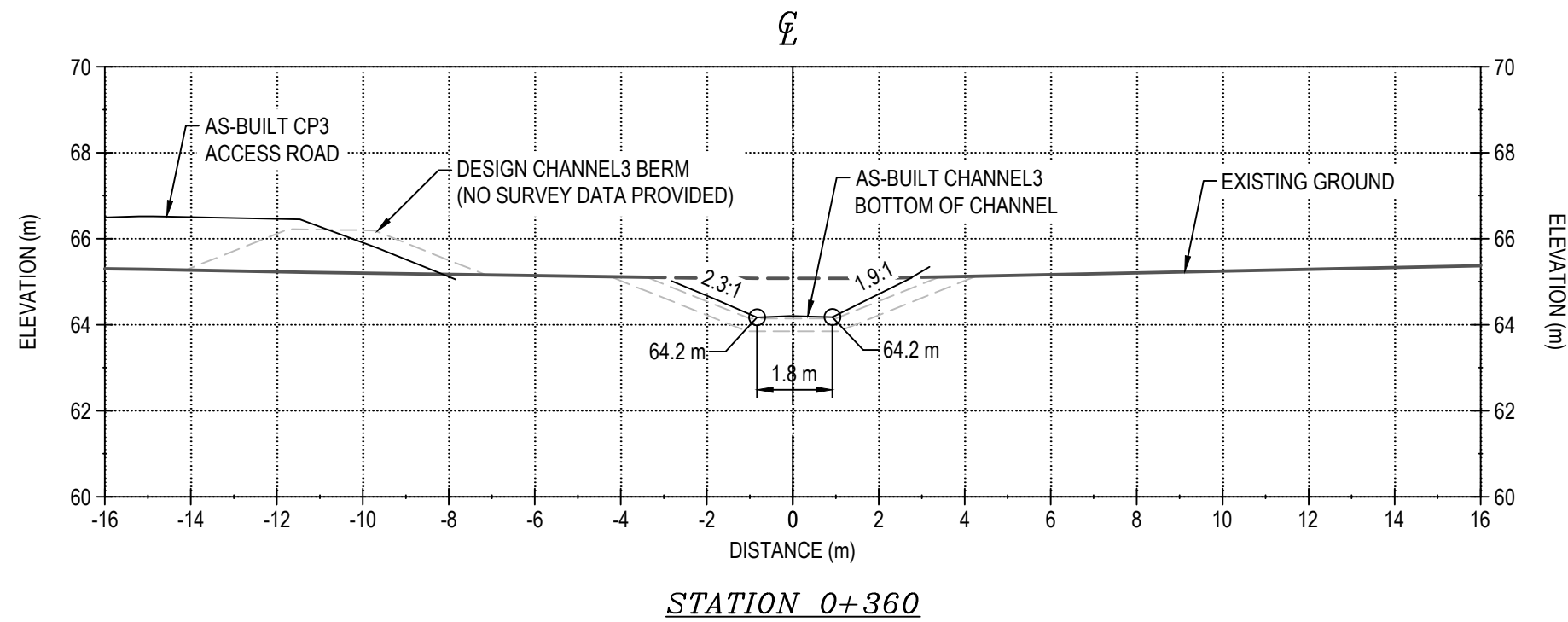
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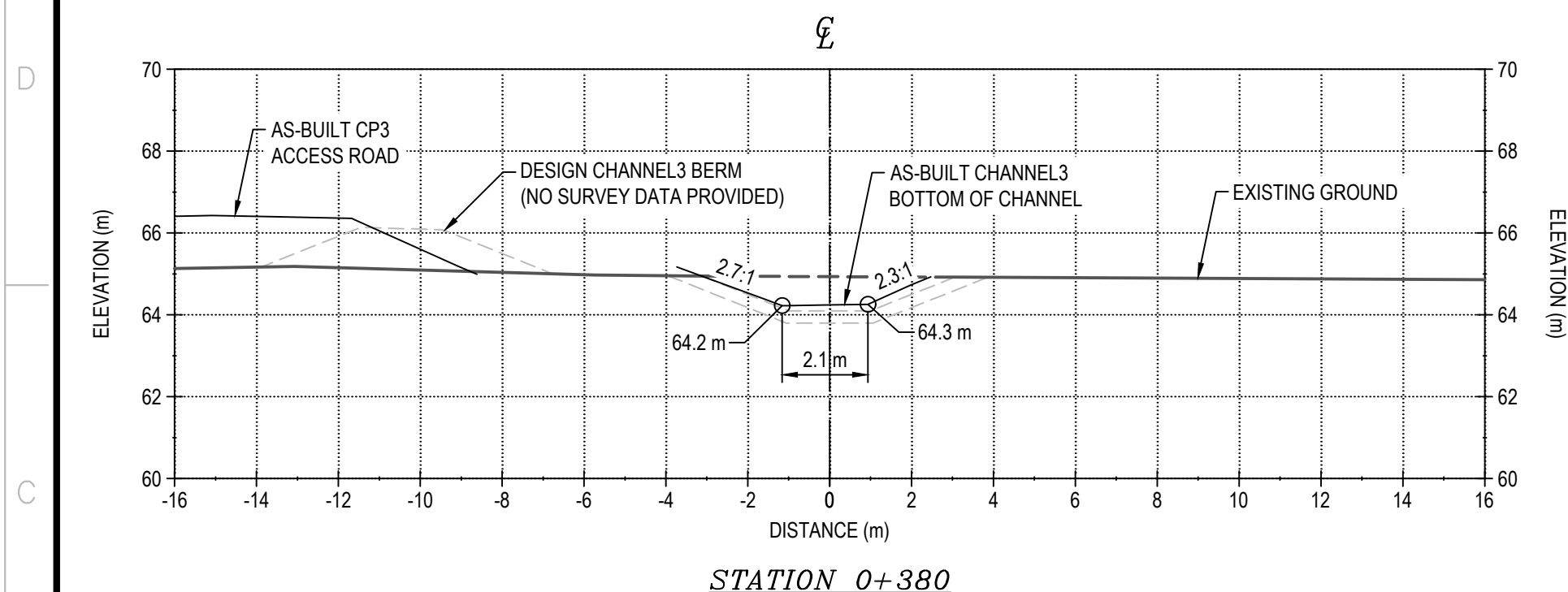
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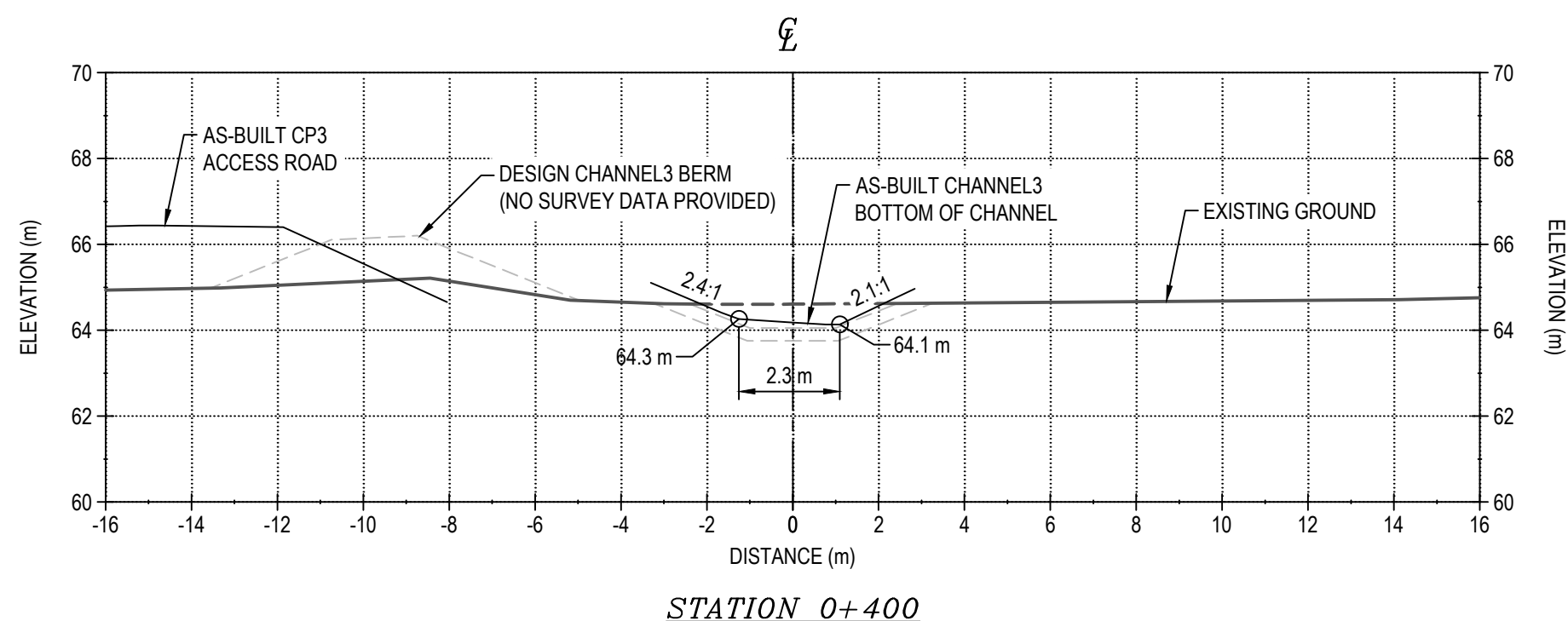
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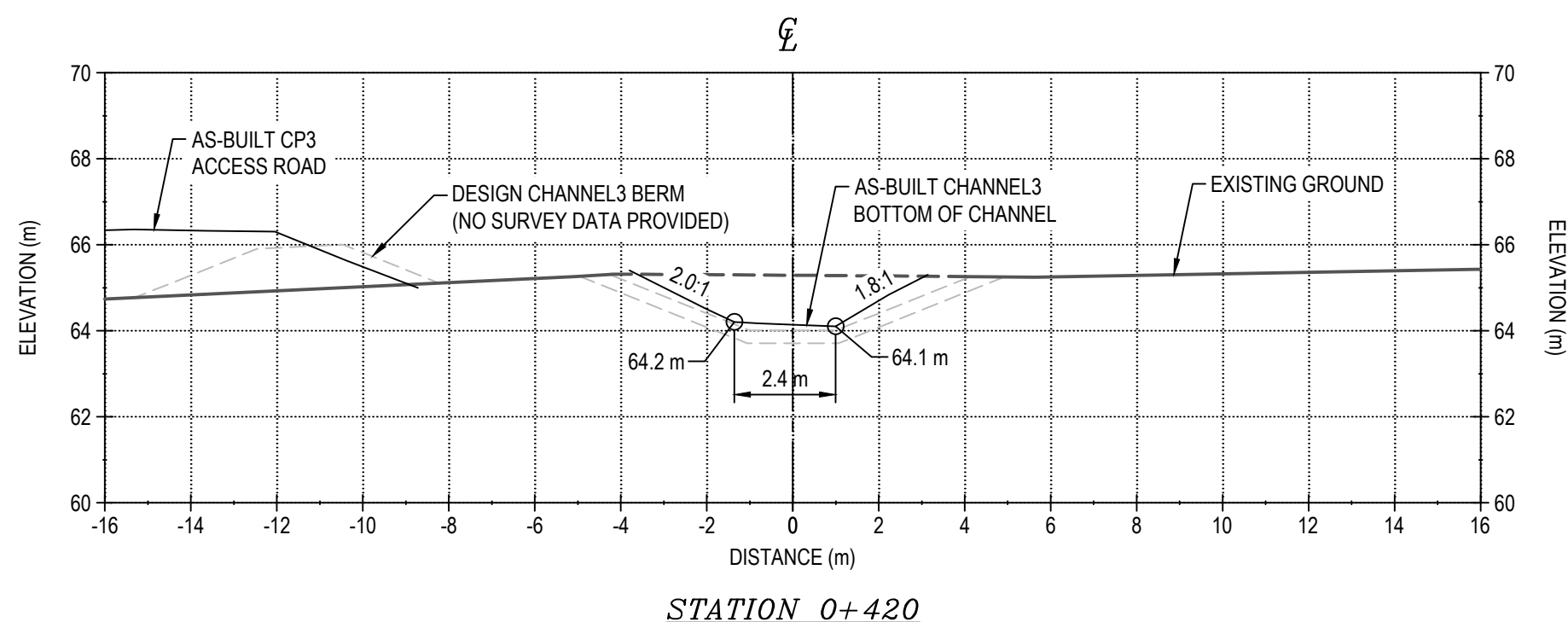
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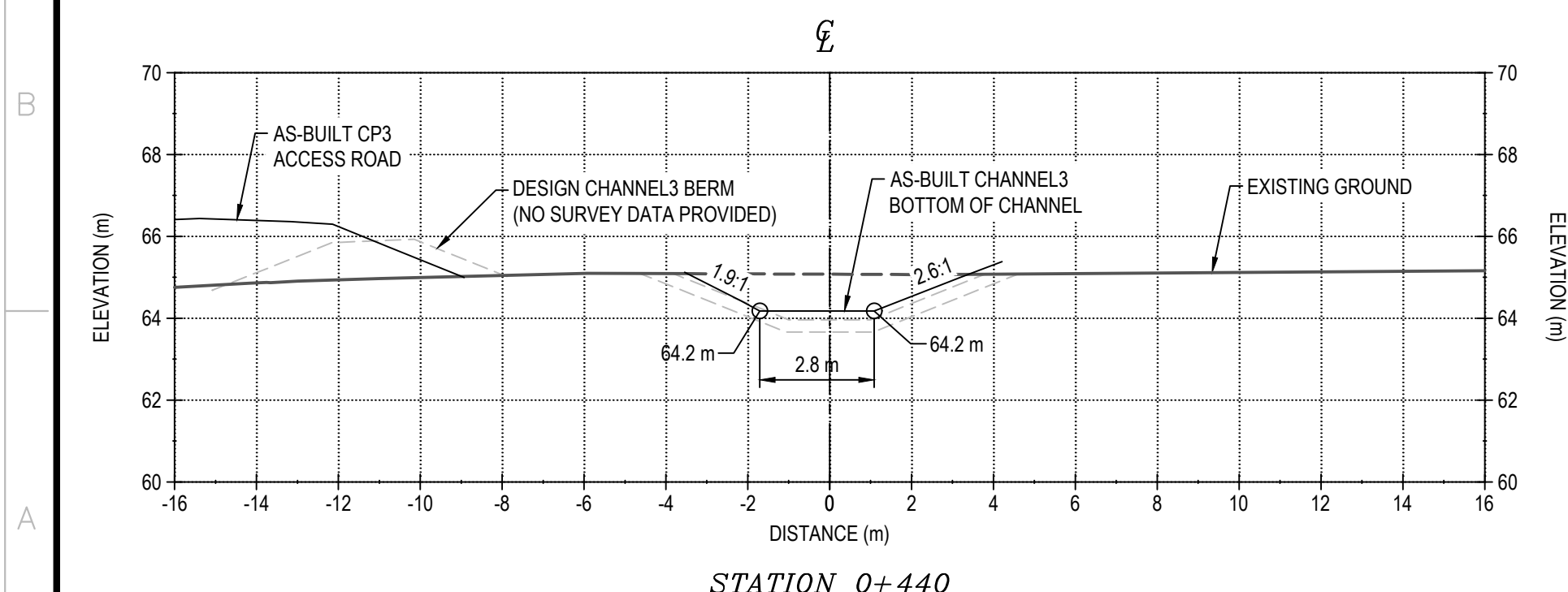
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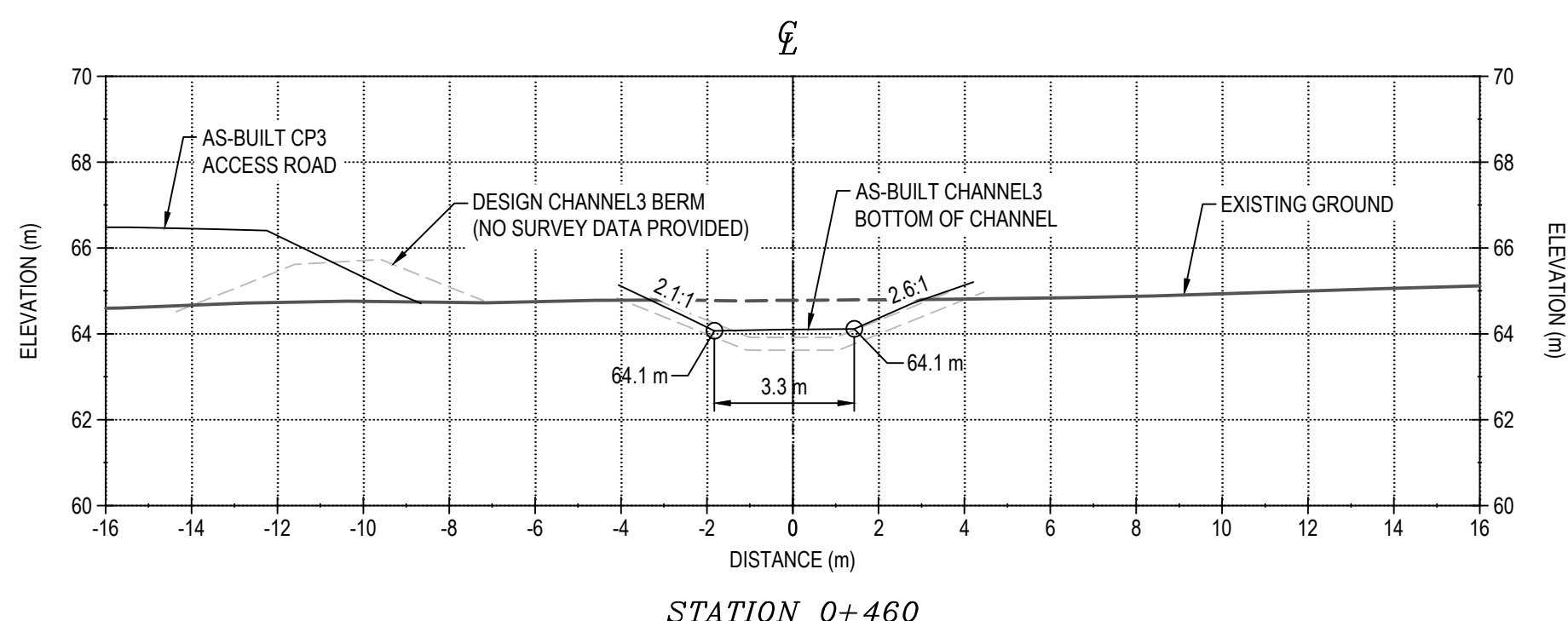
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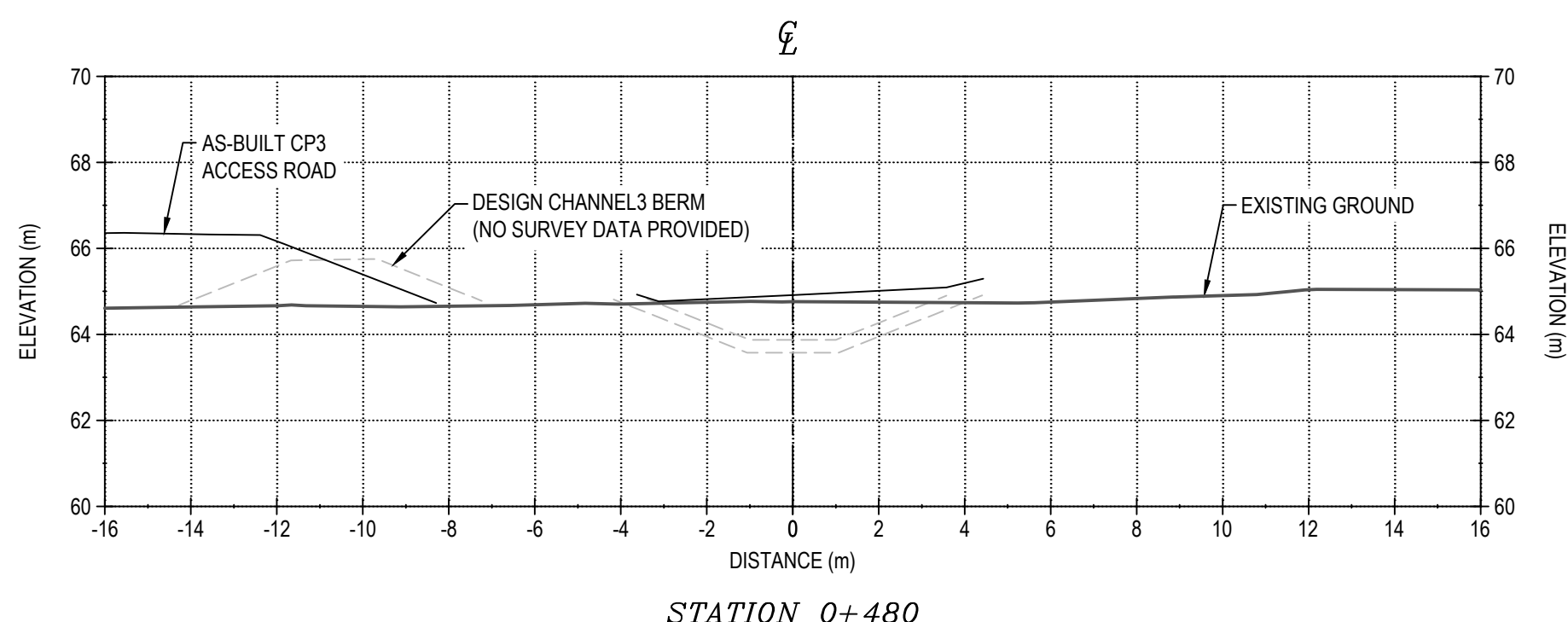
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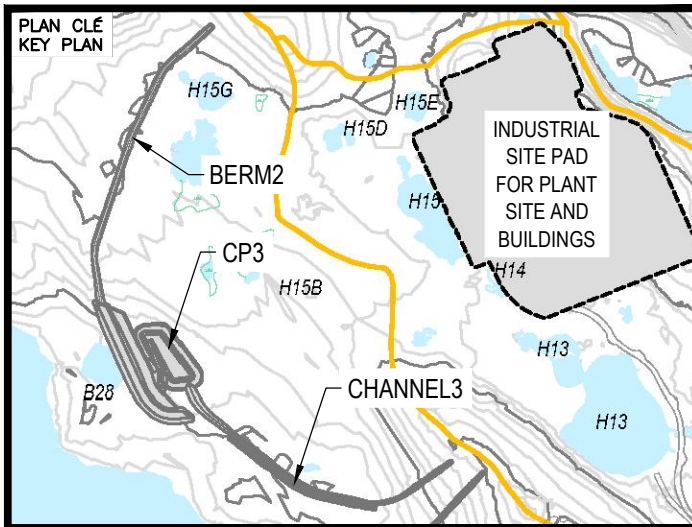
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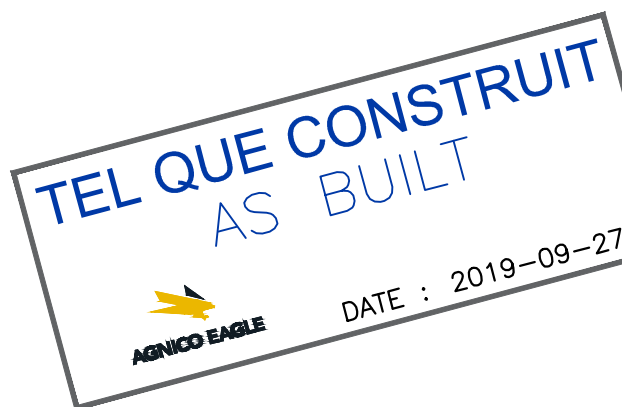


STATION 0+480



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).



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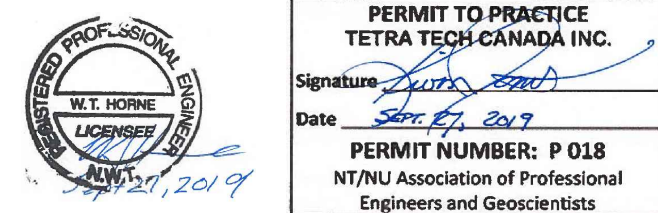
DESSINS EN RÉFÉRENCE / REFERENCE DRAWINGS

TITRE / TITLE	# DWG



REV.	DATE	DESCRIPTION	PAR/REV.	APP.	CLIENT
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0	2018-06-21	ISSUED FOR CONSTRUCTION	HX	WTH	
1	2018-05-29	ISSUED FOR REVIEW	LOK	WTH	

REVISIONS



TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

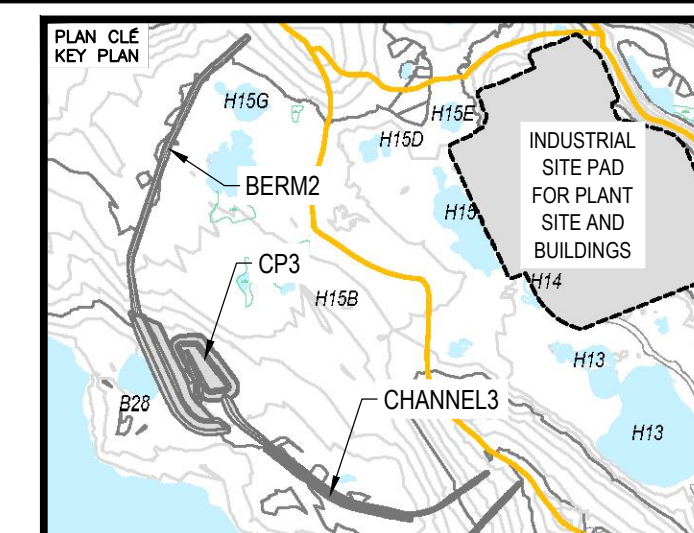
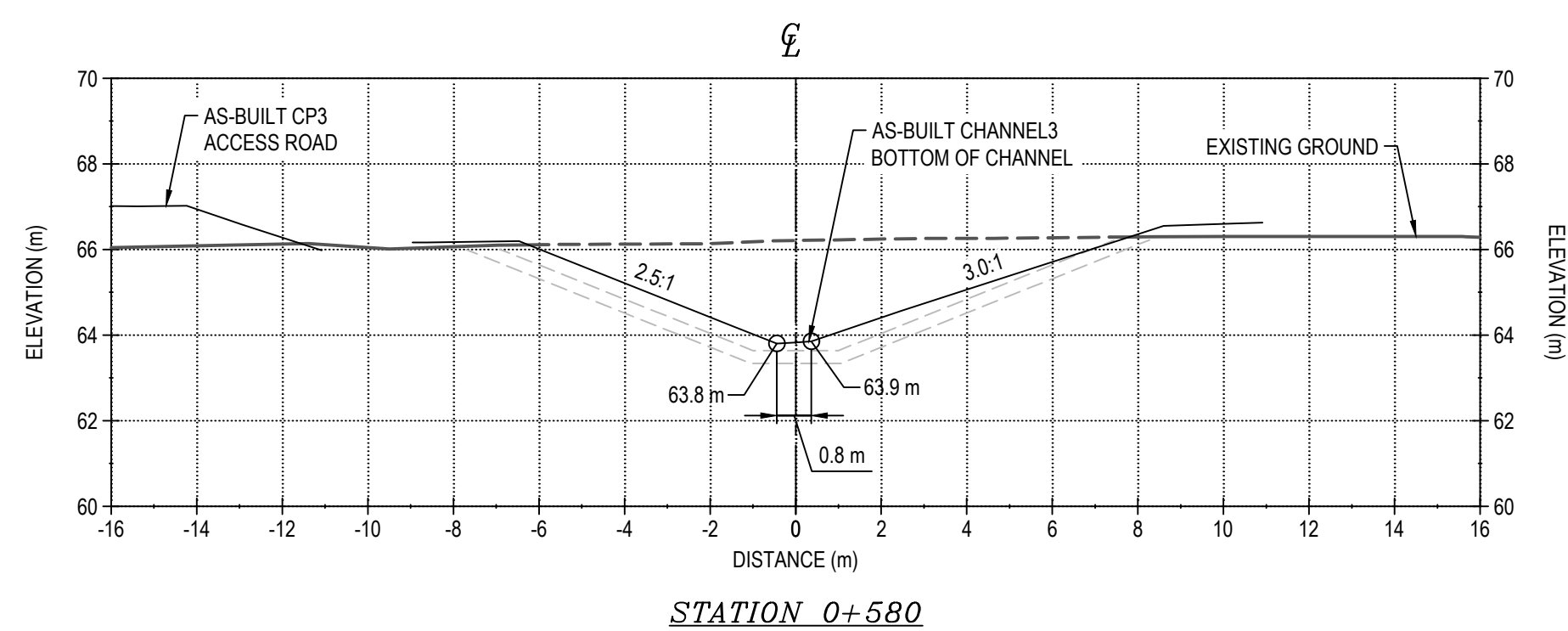
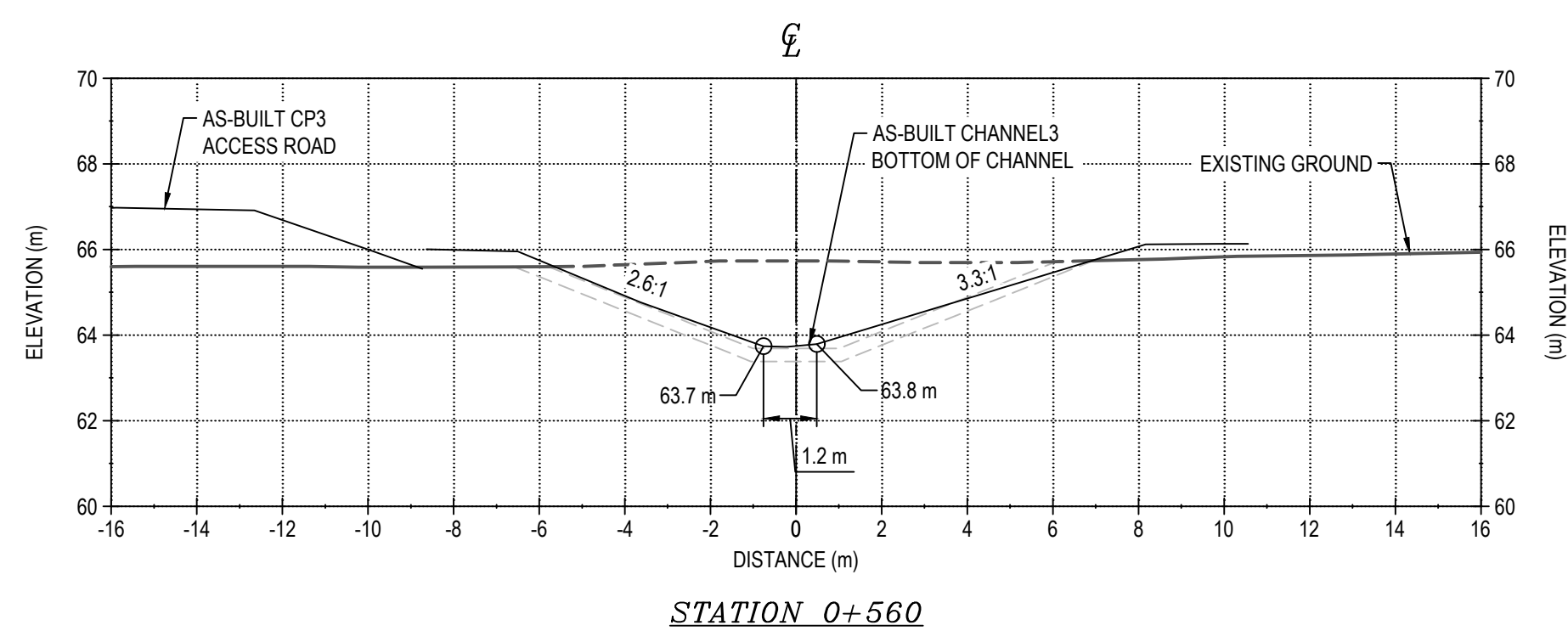
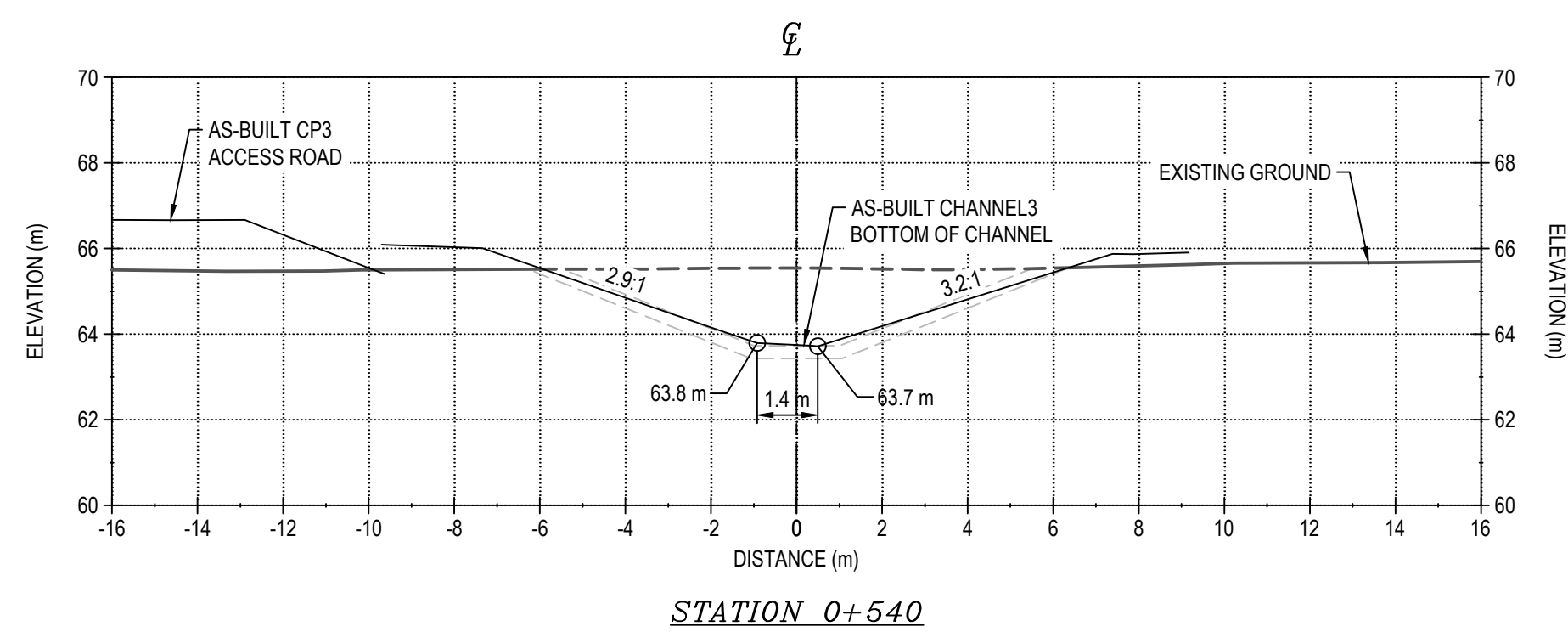
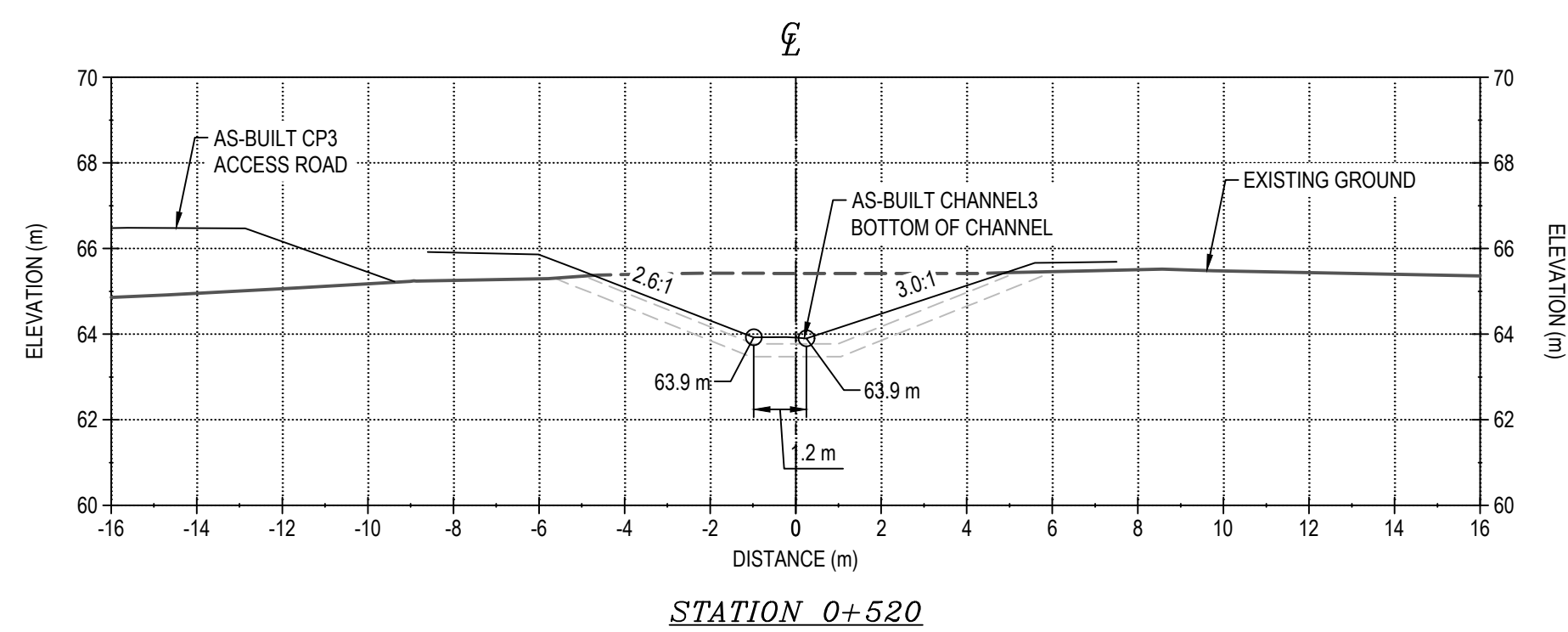
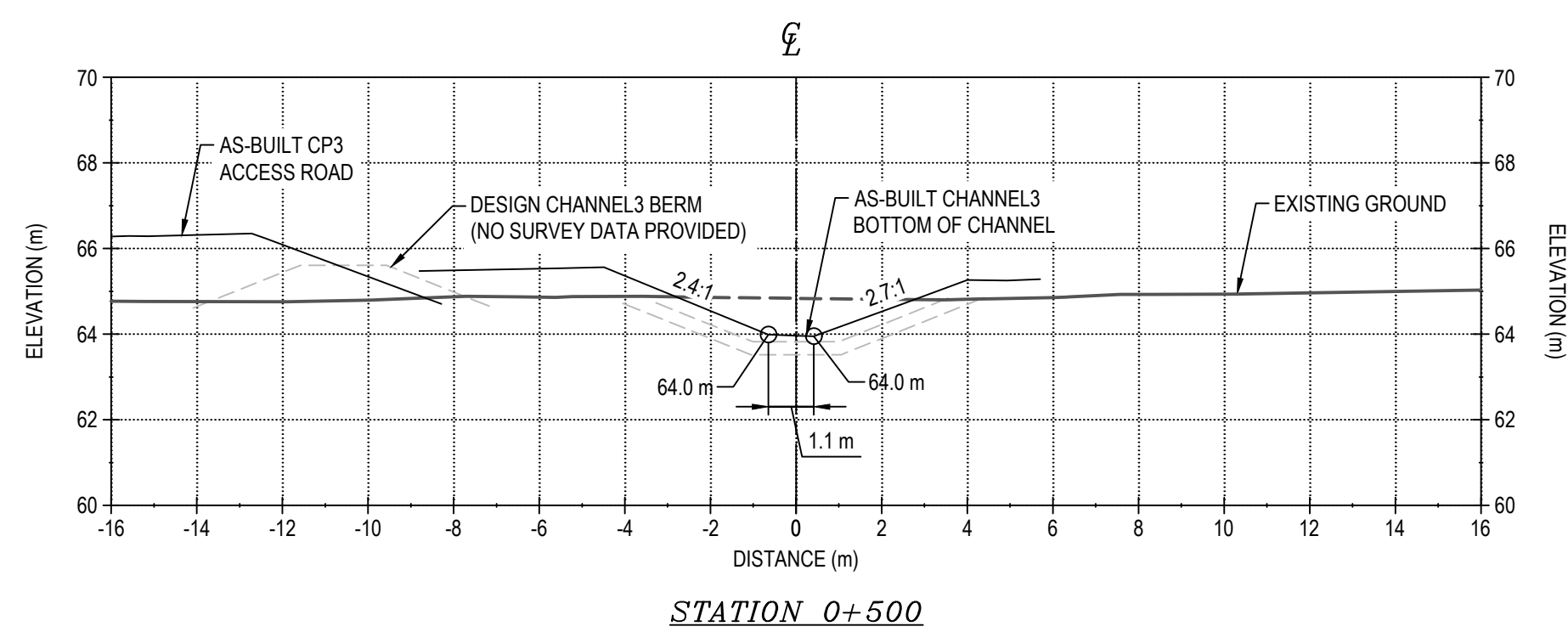
CHANNEL3 DESIGN SECTIONS
(2 of 3)

DESSINE PAR DRAWN BY	EL	DATE 2019-06-12
VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WTH	2019-06-12

ÉCHELLE
SCALE 1:150

NO. DESSIN
DRAWING NO. 65-695-230-244-2

NO. PROJET PROJECT NO.	REVISION	FEUILLE / SHEET
6515	1	8 / 13



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).

TEL QUE CONSTRUIT
AS BUILT

 DATE : 2019-09-27

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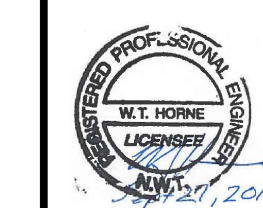
DESSINS EN RÉFÉRENCE / REFERENCE DRAWINGS

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AGNICO EAGLE

[illegible]

REVISIONS



PERMIT TO PRACTICE
TETRA TECH CANADA INC.
Signature [Signature]
Date Sept. 27, 2019
PERMIT NUMBER: P 018
NT/NU Association of Professional
Engineers and Geoscientists

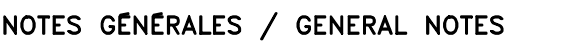
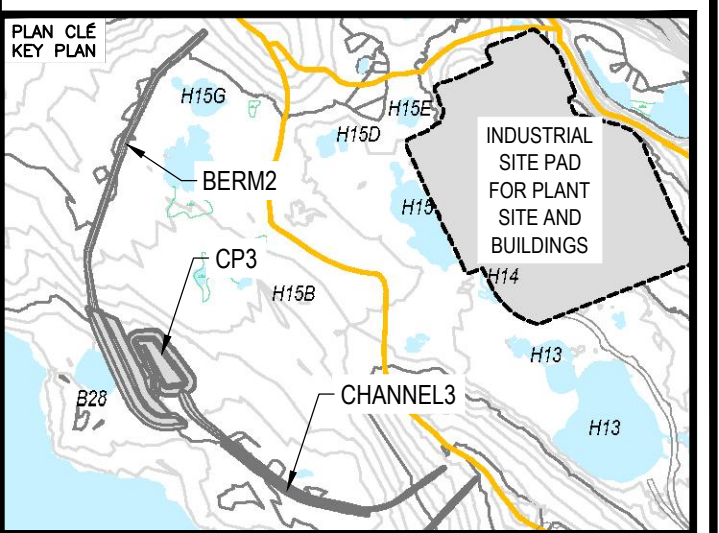
TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

CHANNEL3 DESIGN SECTIONS
(3 of 3)

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VÉRIFIÉ PAR CHECKED BY	HX	DATE	2019-06-12
APPROUVÉ PAR APPROVED BY	WTH	DATE	2019-06-12
ÉCHELLE SCALE	1:150	DATE	2019-06-12

NO. DESSIN
DRAWING NO. 65-695-230-244-3

NO. PROJET PROJECT NO. 6515	REVISION	FEUILLE / 5
	1	9 / 13



- TEL QUE CONSTRUIT
AS BUILT
-  DATE : 2019-09-27

DESSINS EN RÉFÉRENCE / REFERENCE DRAWINGS



AGNICO EAGLE

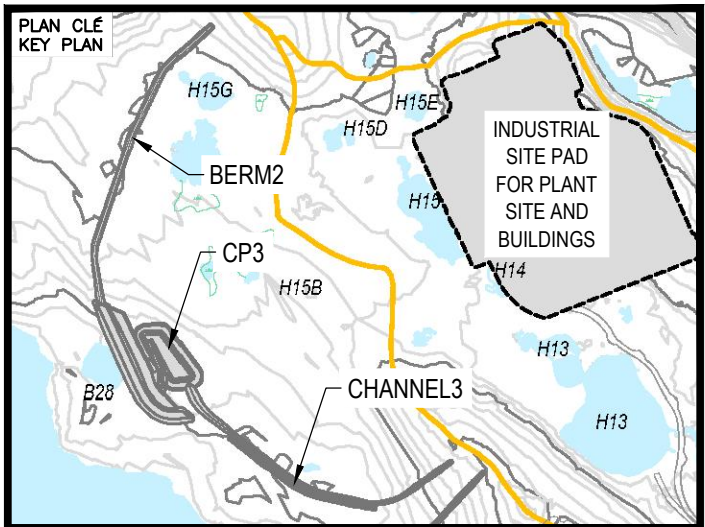
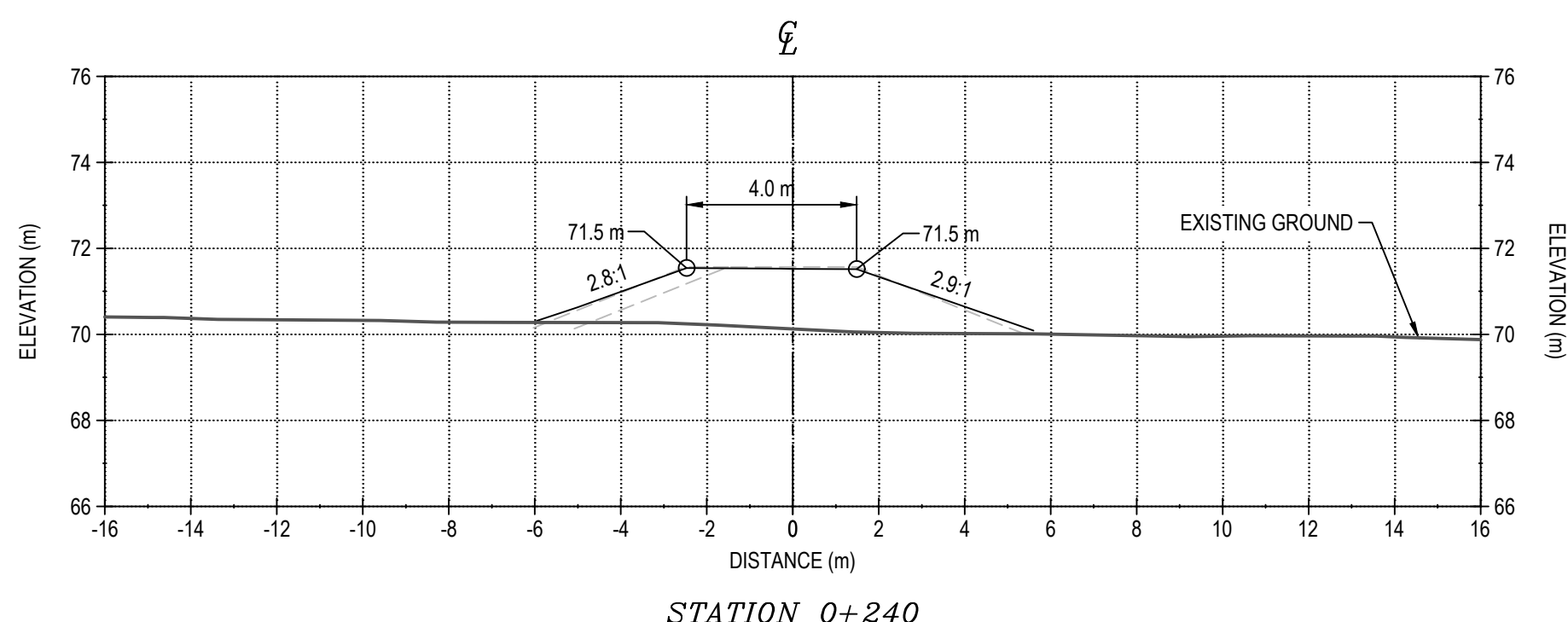
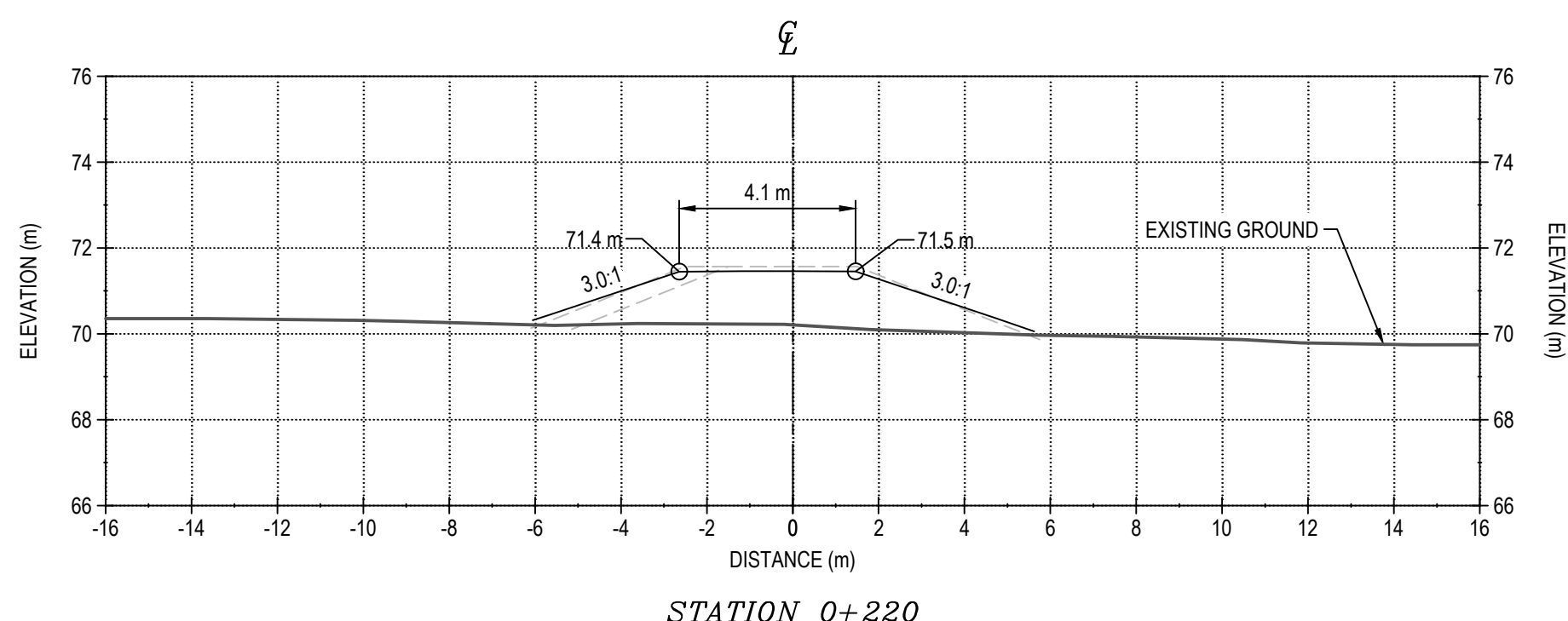
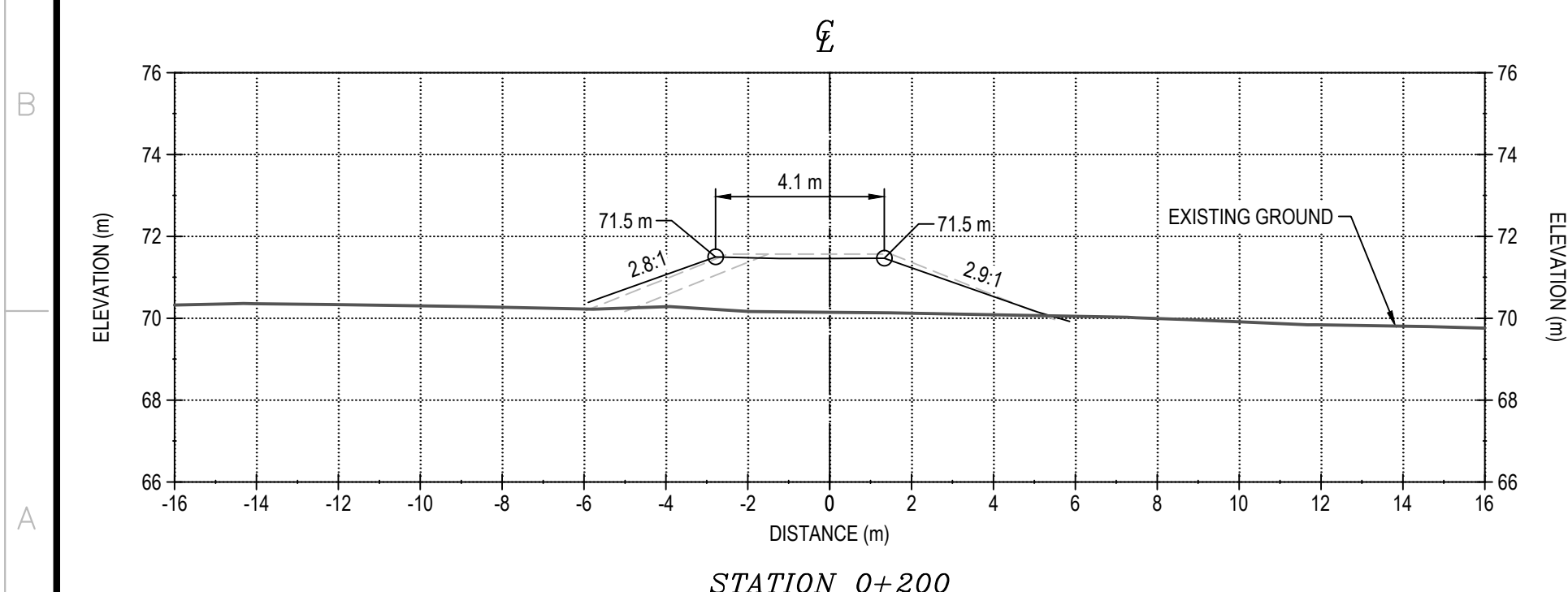
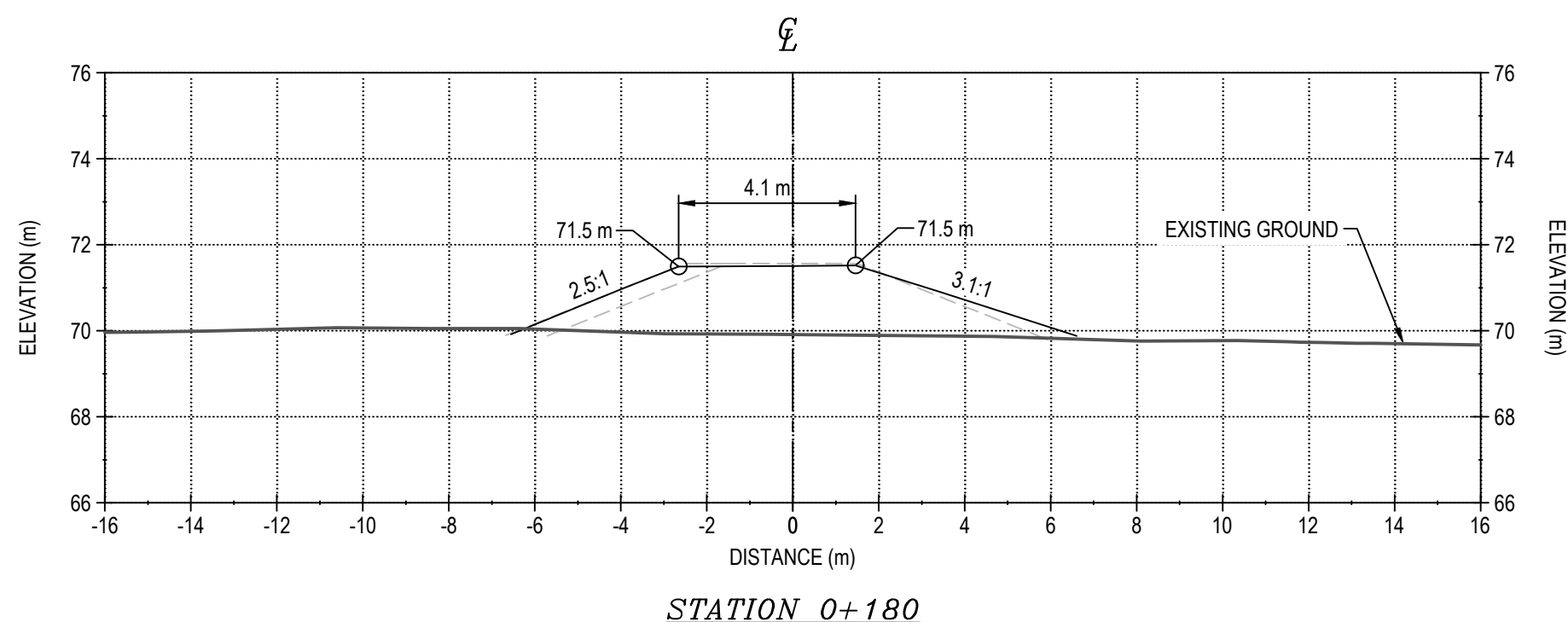
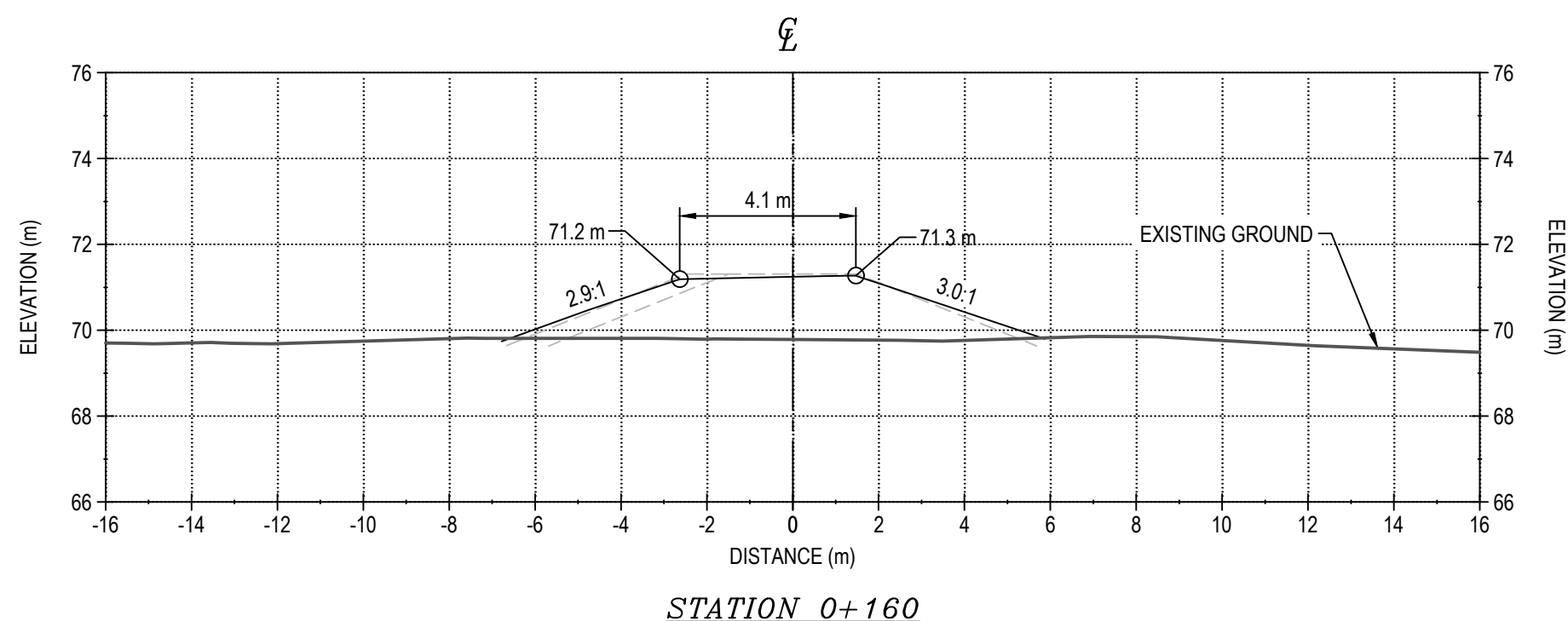
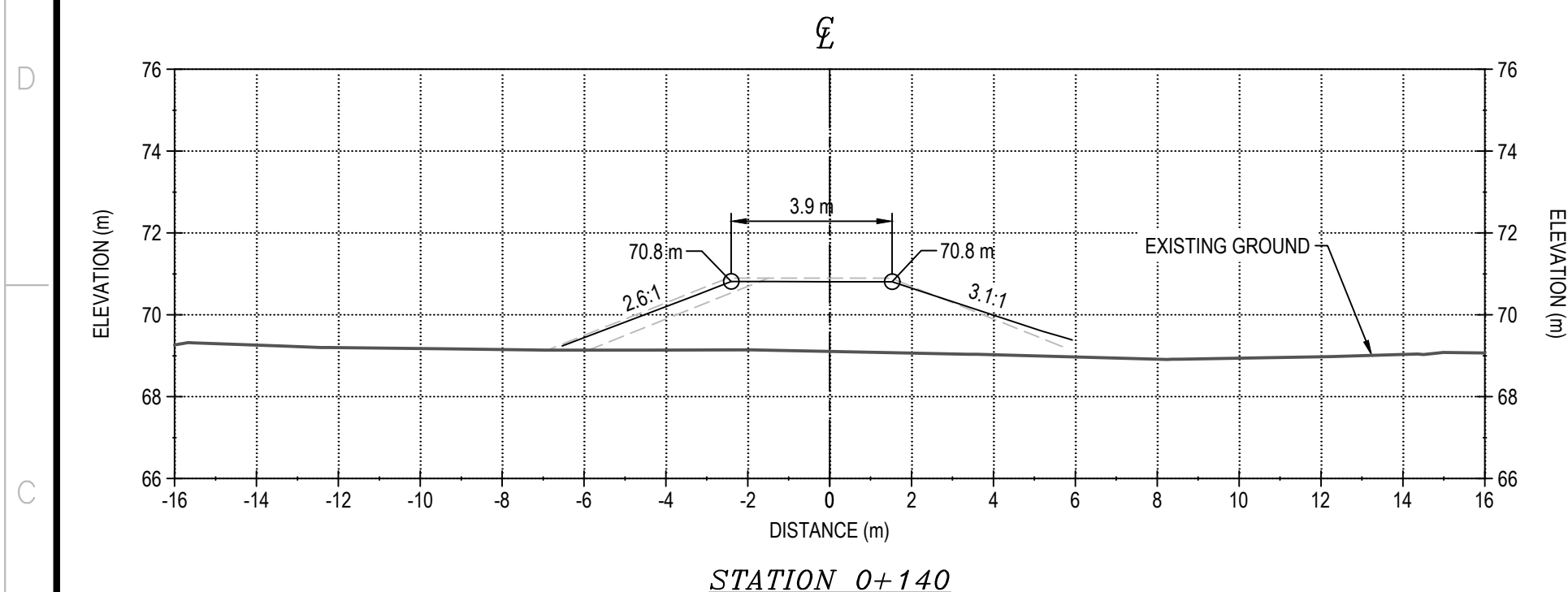
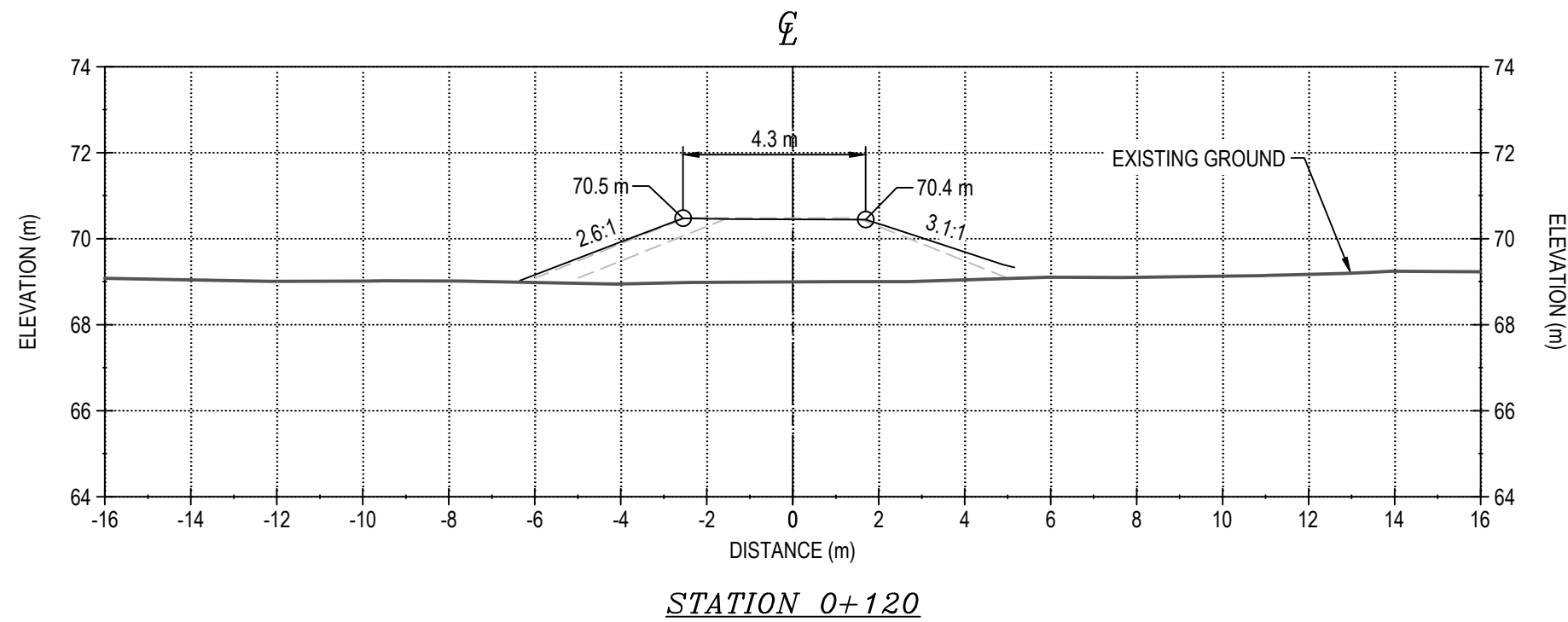
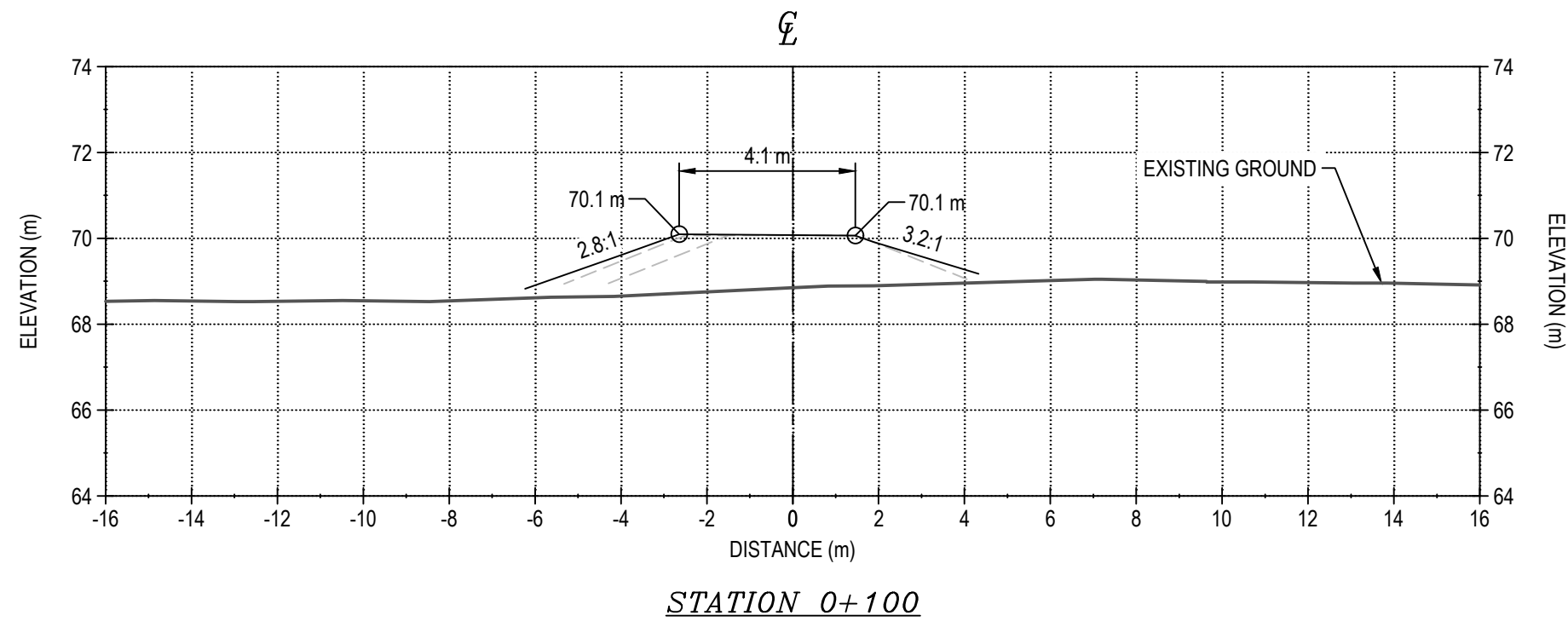
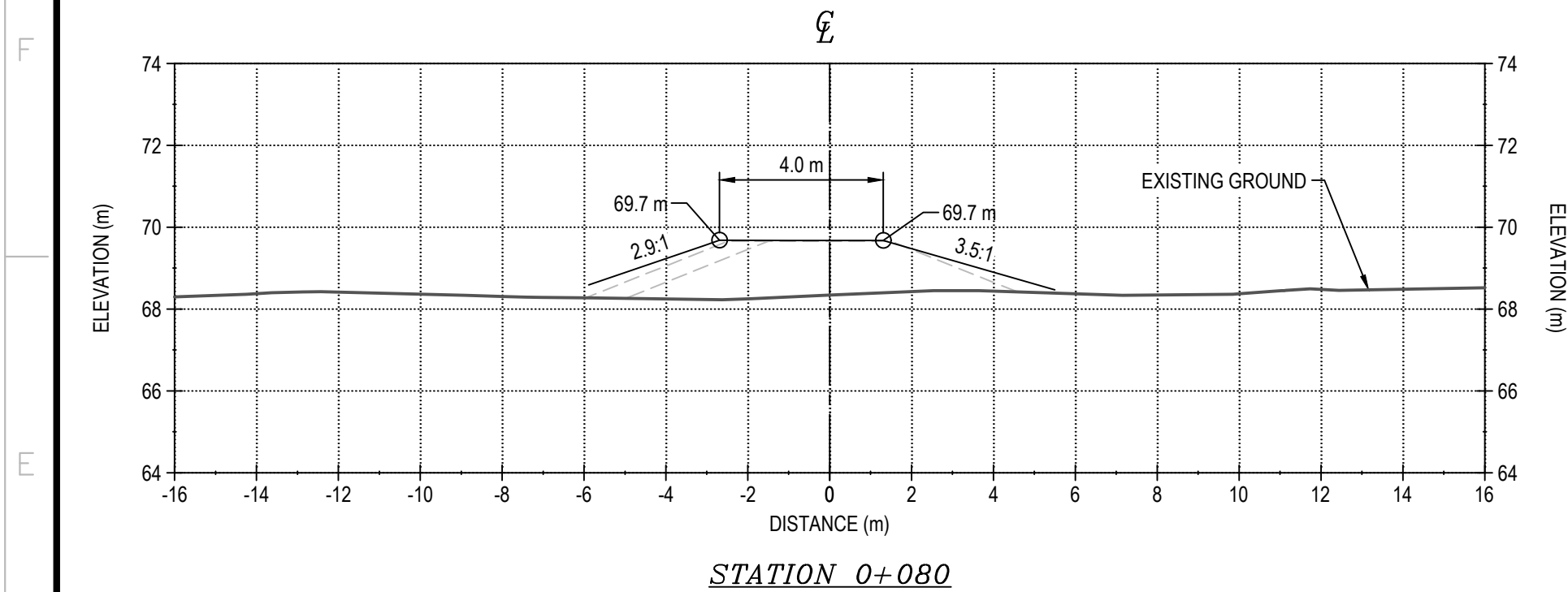
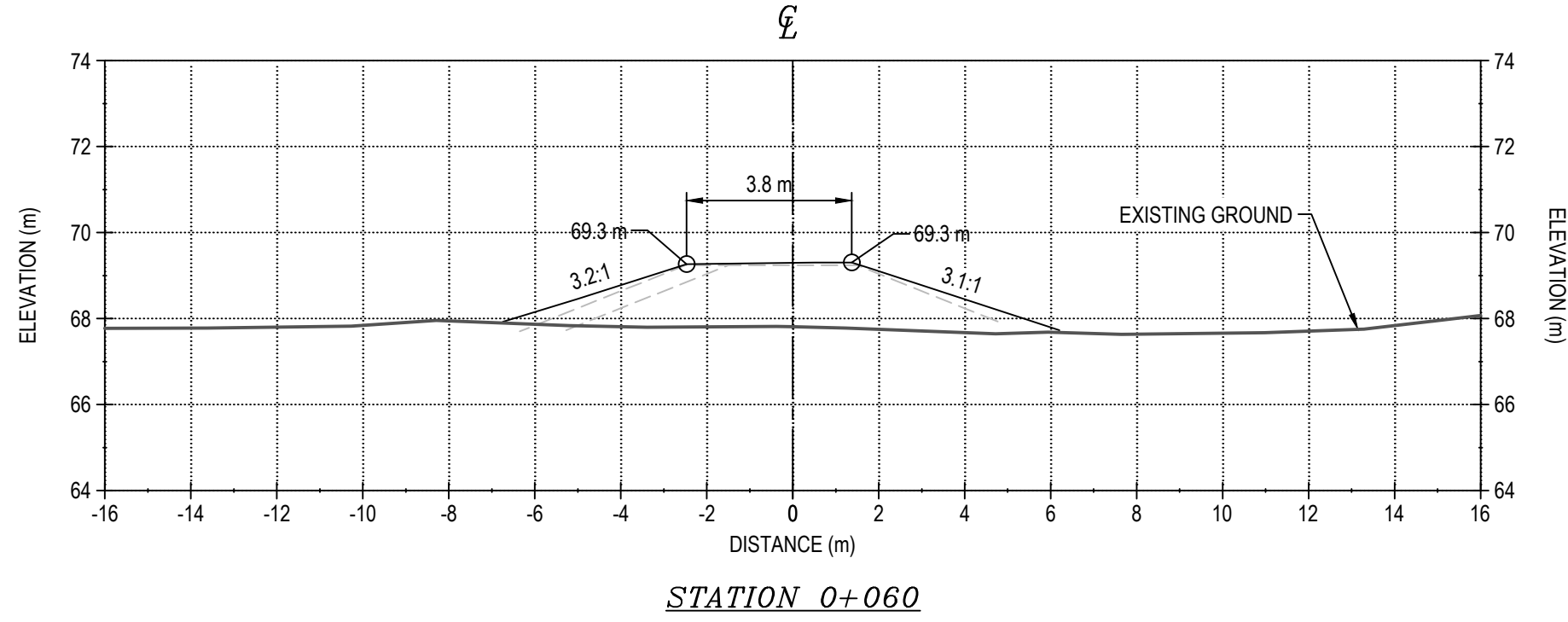
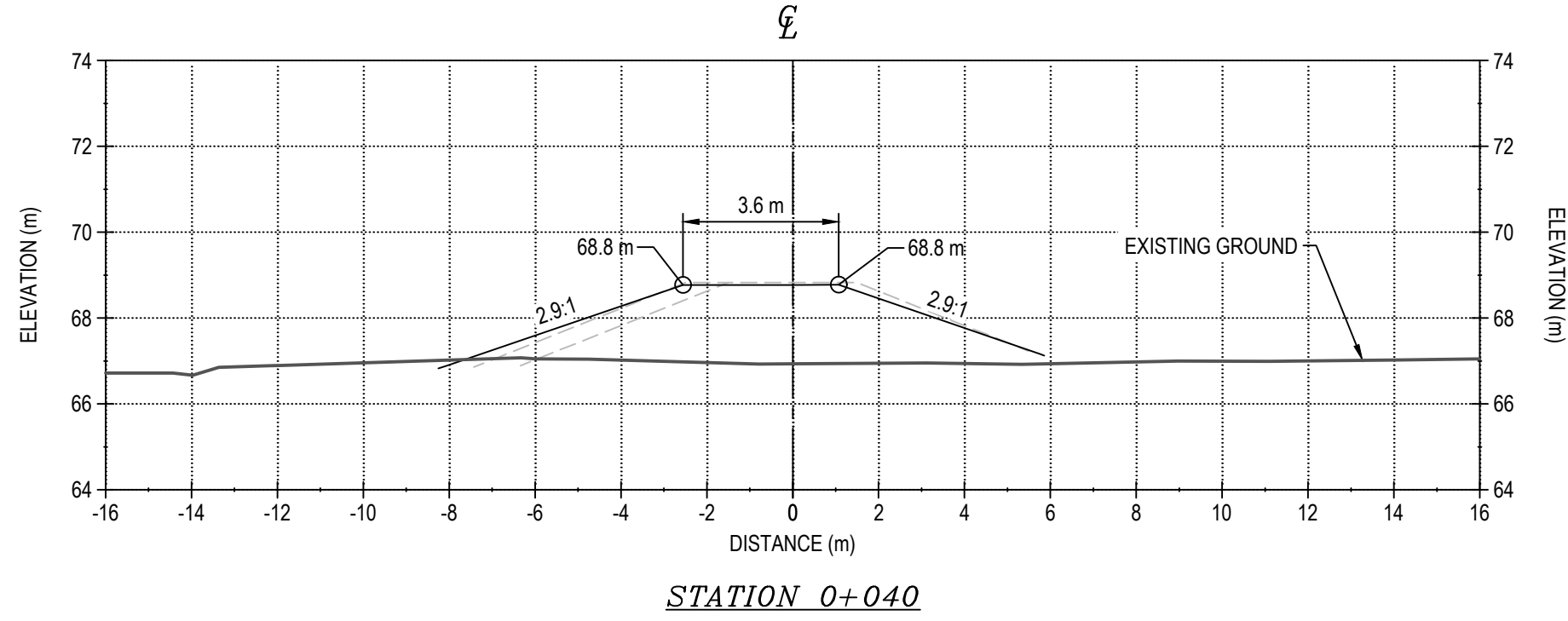
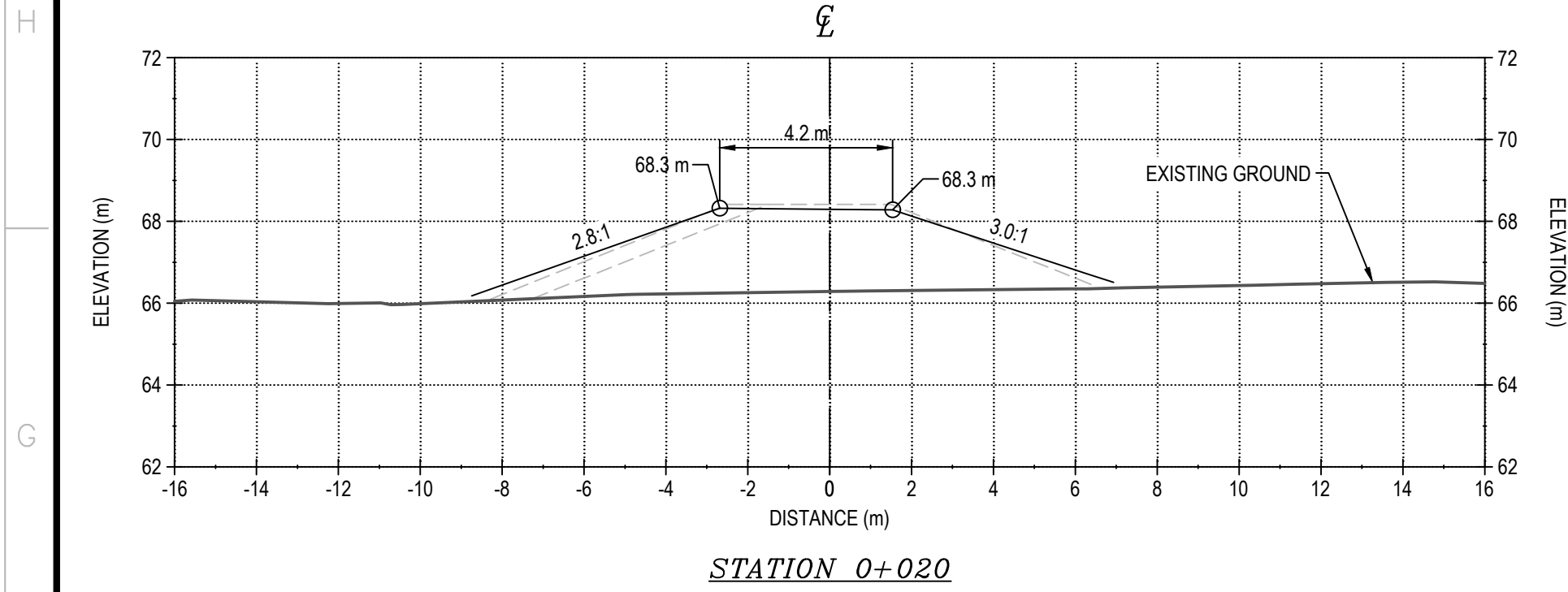
	REVISIONS
	PERMIT TO PRACTICE TETRA TECH CANADA INC. Signature: <u>[Signature]</u> Date: <u>Sept. 27, 2019</u> PERMIT NUMBER: P 018 NTNU Association of Professional Engineers and Geoscientists

BERM2
LAYOUT PLAN

NO. DESSIN
DRAWING NO. 65-695-230-245

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NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).



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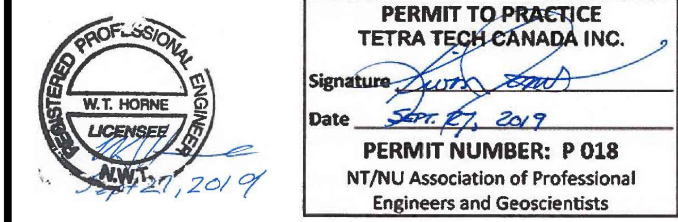
DESSINS EN RÉFÉRENCE / REFERENCE DRAWINGS

TITRE / TITLE	# DWG



REV.	DATE	DESCRIPTION	PAR/REV.	APP.	CLIENT
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0	2018-06-21	ISSUED FOR CONSTRUCTION	HX	WITH	
1	2018-05-29	ISSUED FOR REVIEW	LOK	WITH	

REVISIONS



PERMIT TO PRACTICE
TETRA TECH CANADA INC.
Signature: [Signature]
Date: 2019-09-27
PERMIT NUMBER: P-018
NTNU Association of Professional Engineers and Geoscientists

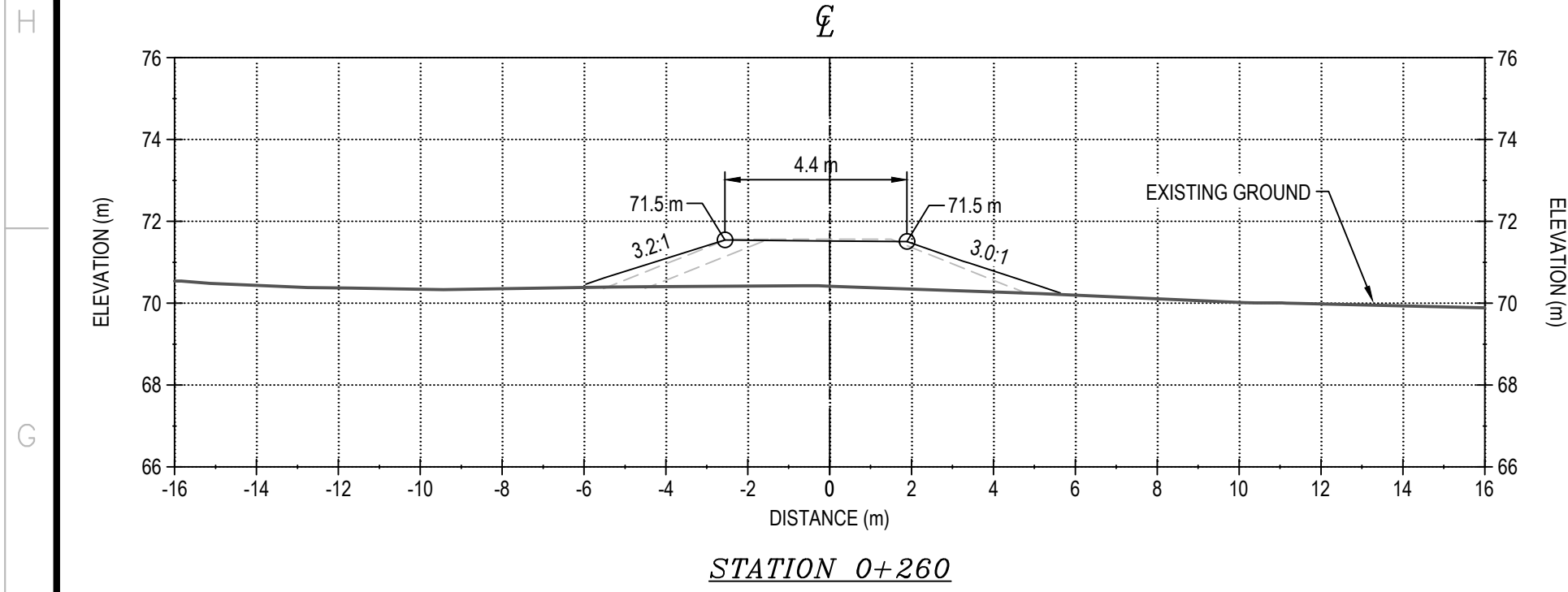
TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

BERM2 DESIGN SECTIONS
(1 of 3)

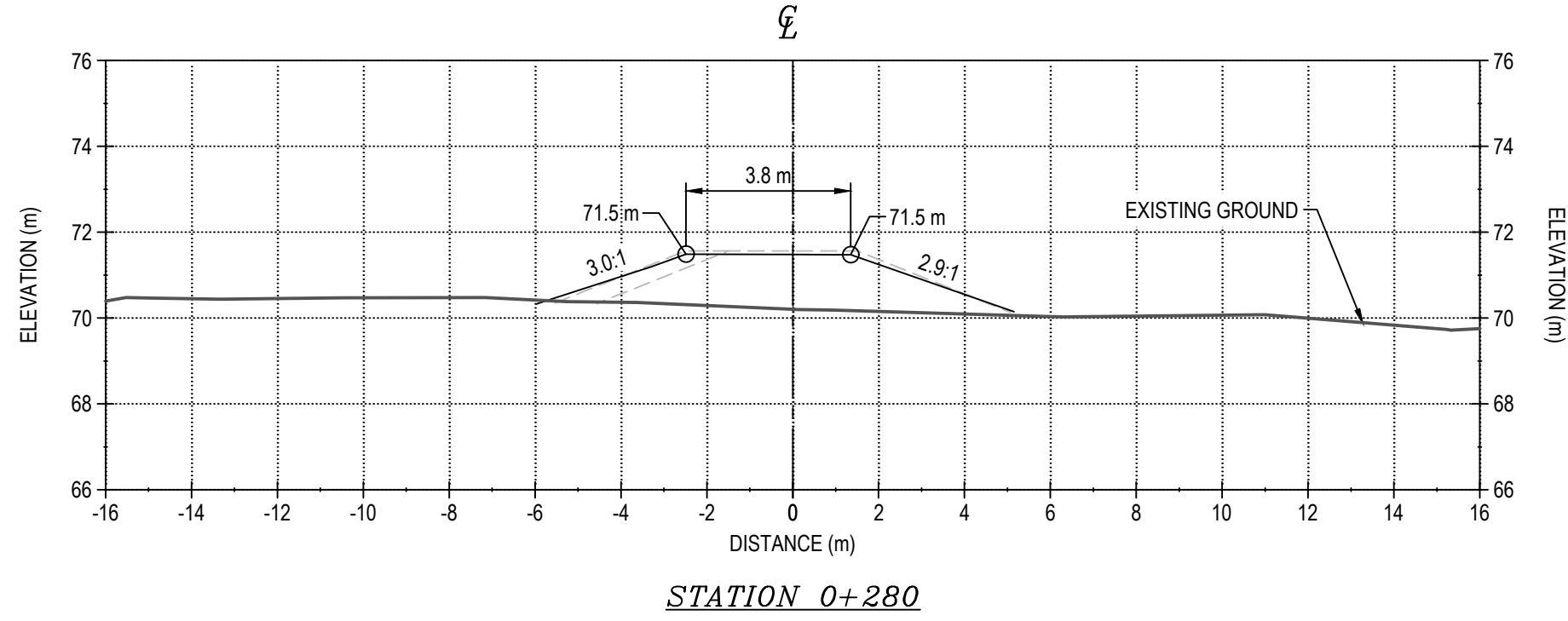
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VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WITH	2019-06-12
ÉCHELLE SCALE	1:150	DATE 2019-06-12

NO. DESSIN
DRAWING NO. 65-695-230-246-1

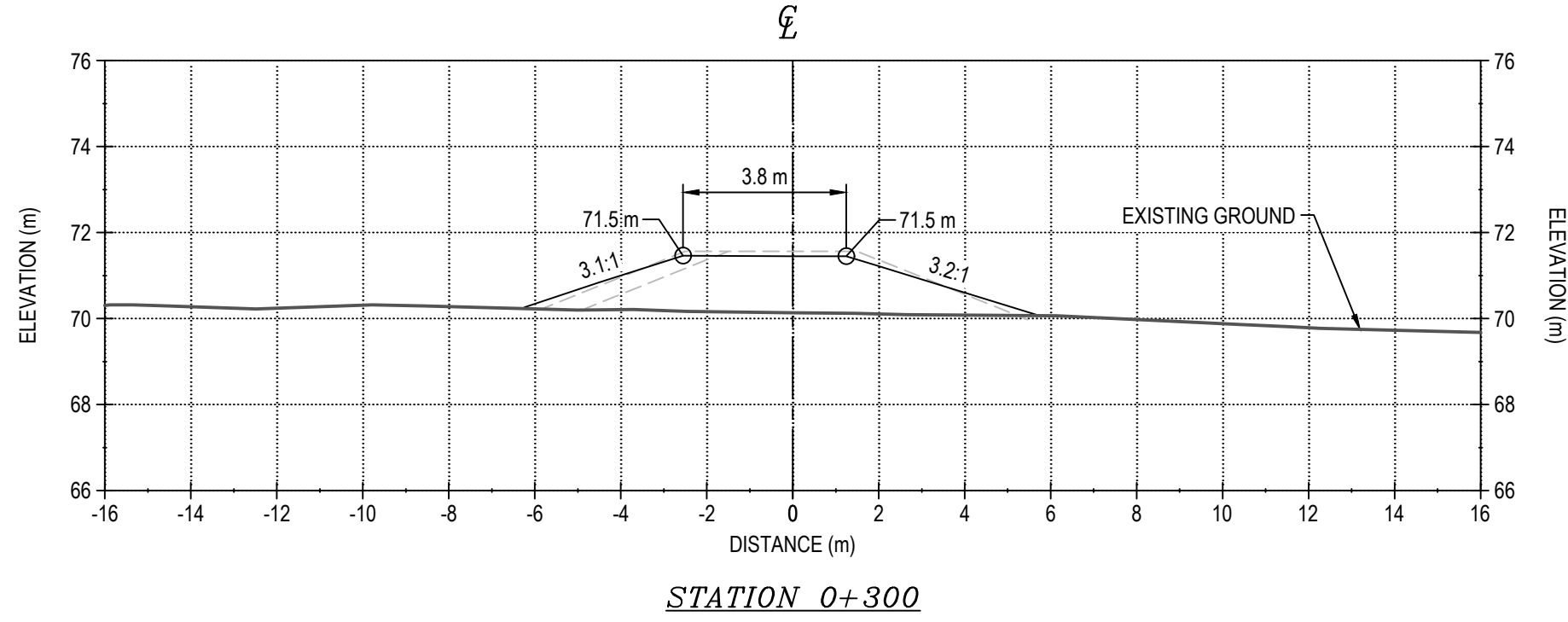
NO. PROJET PROJECT NO.	REVISION	FEUILLE / SHEET
6515	1	11 / 13



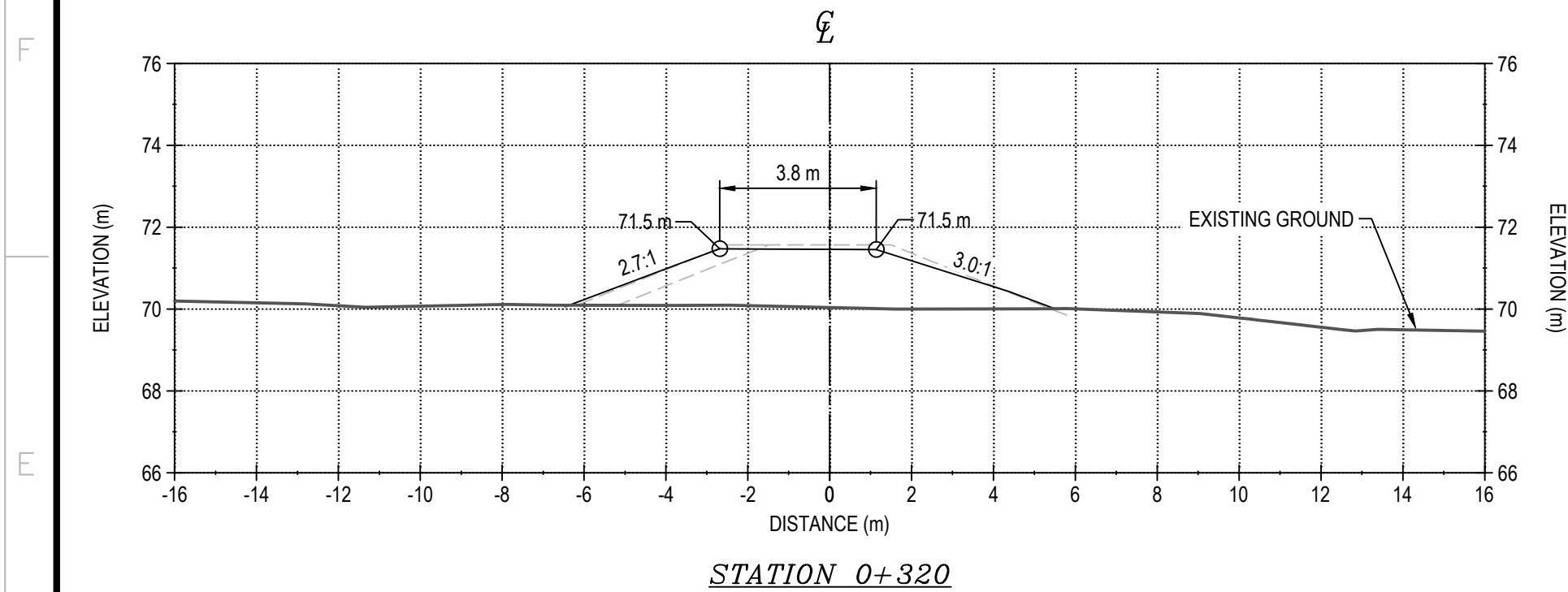
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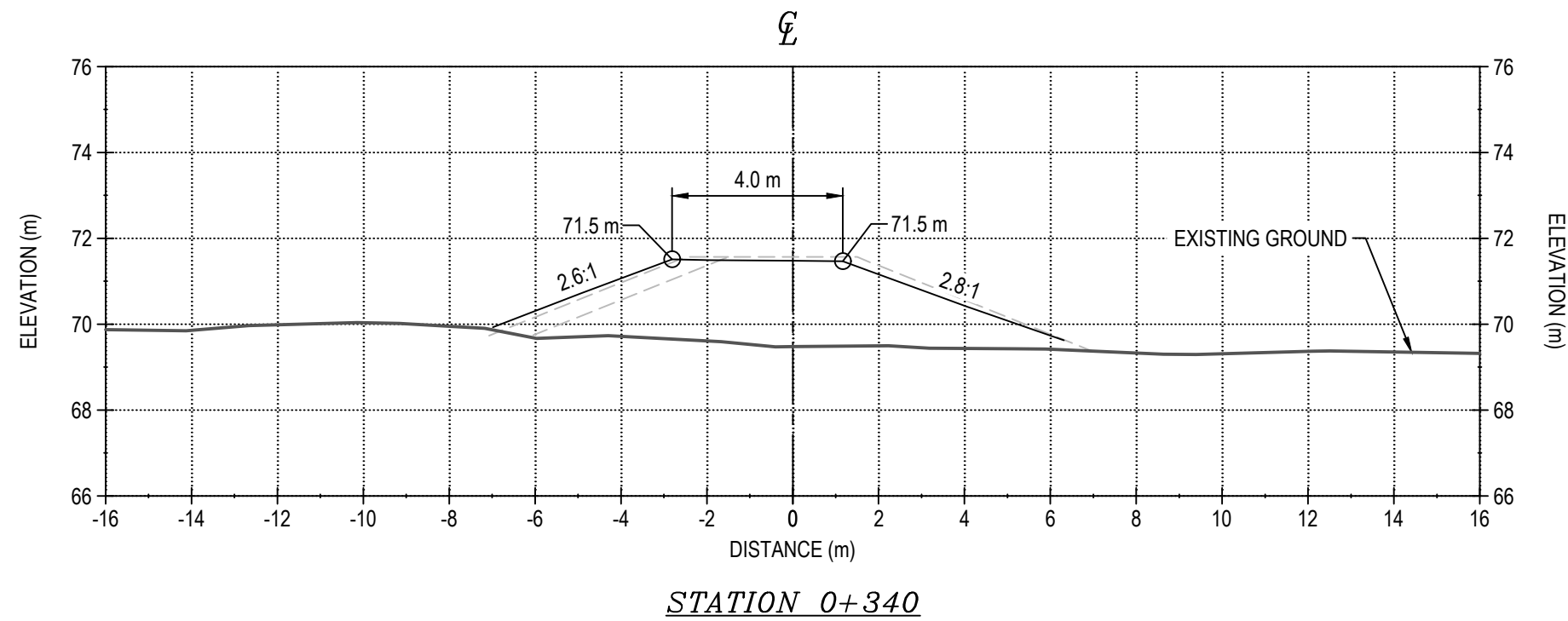
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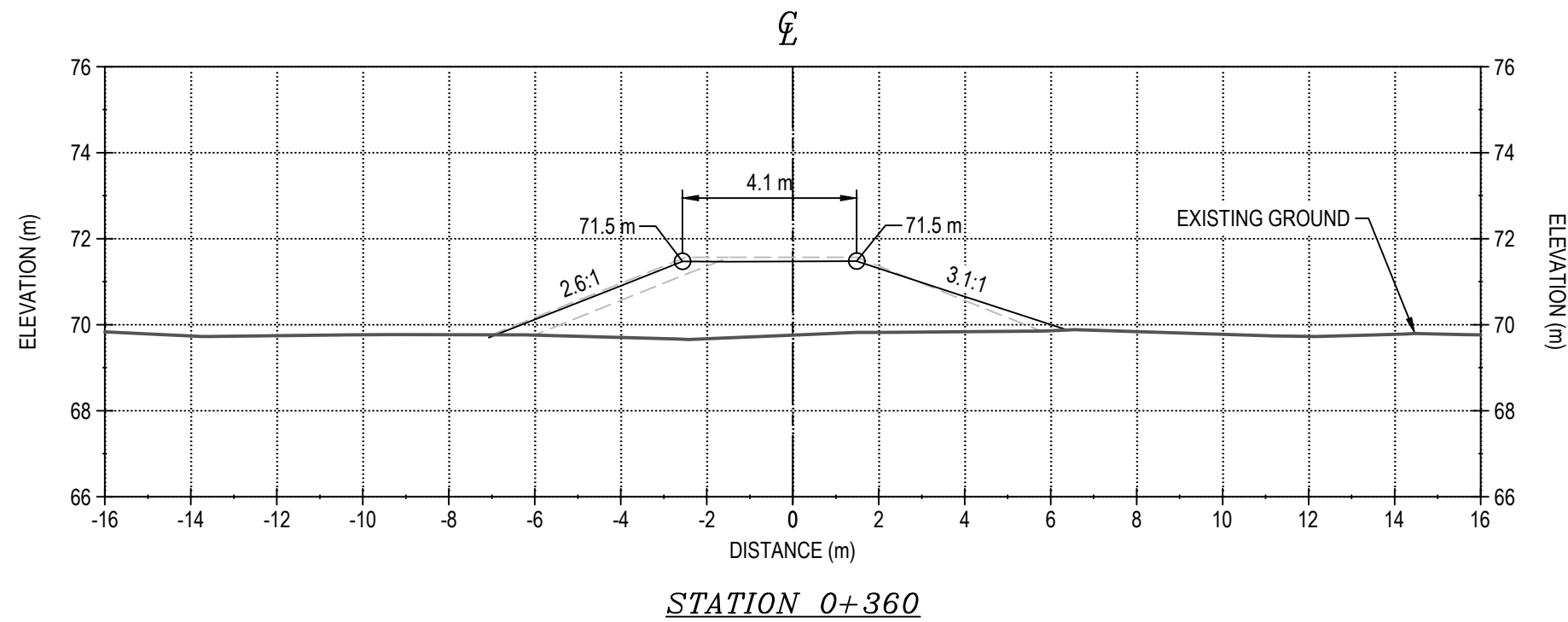
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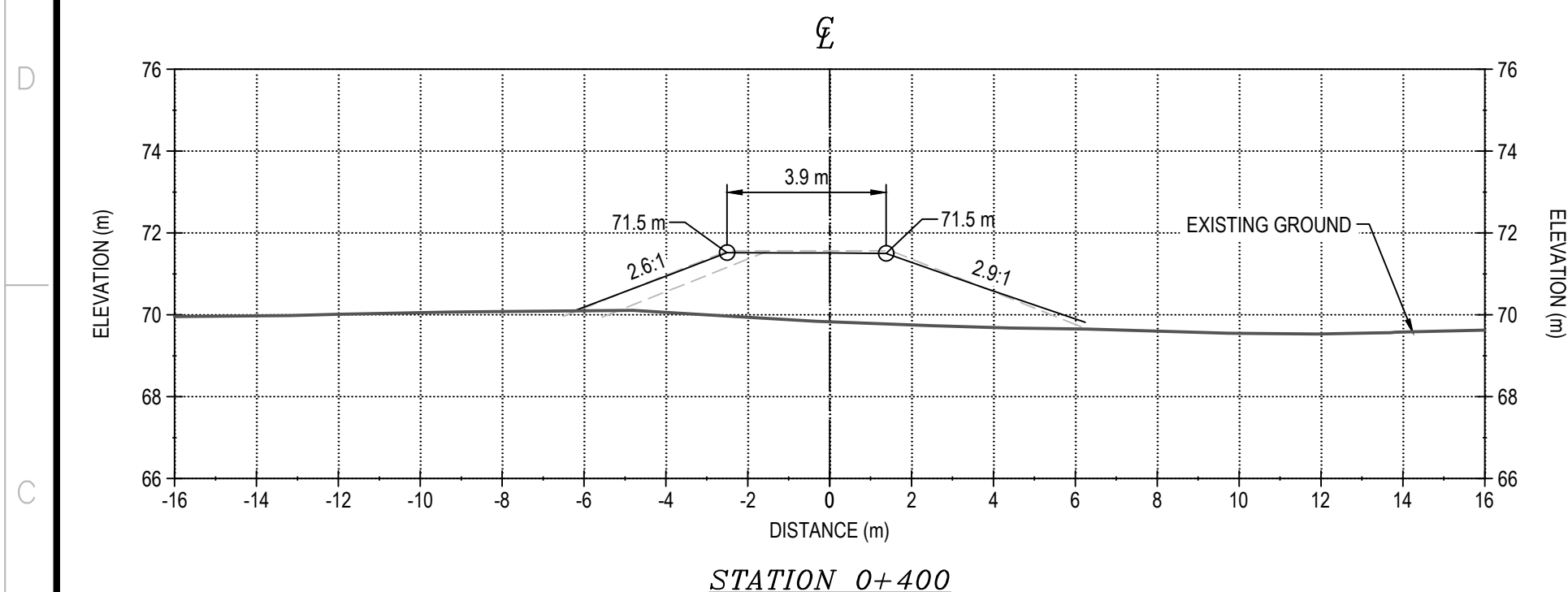
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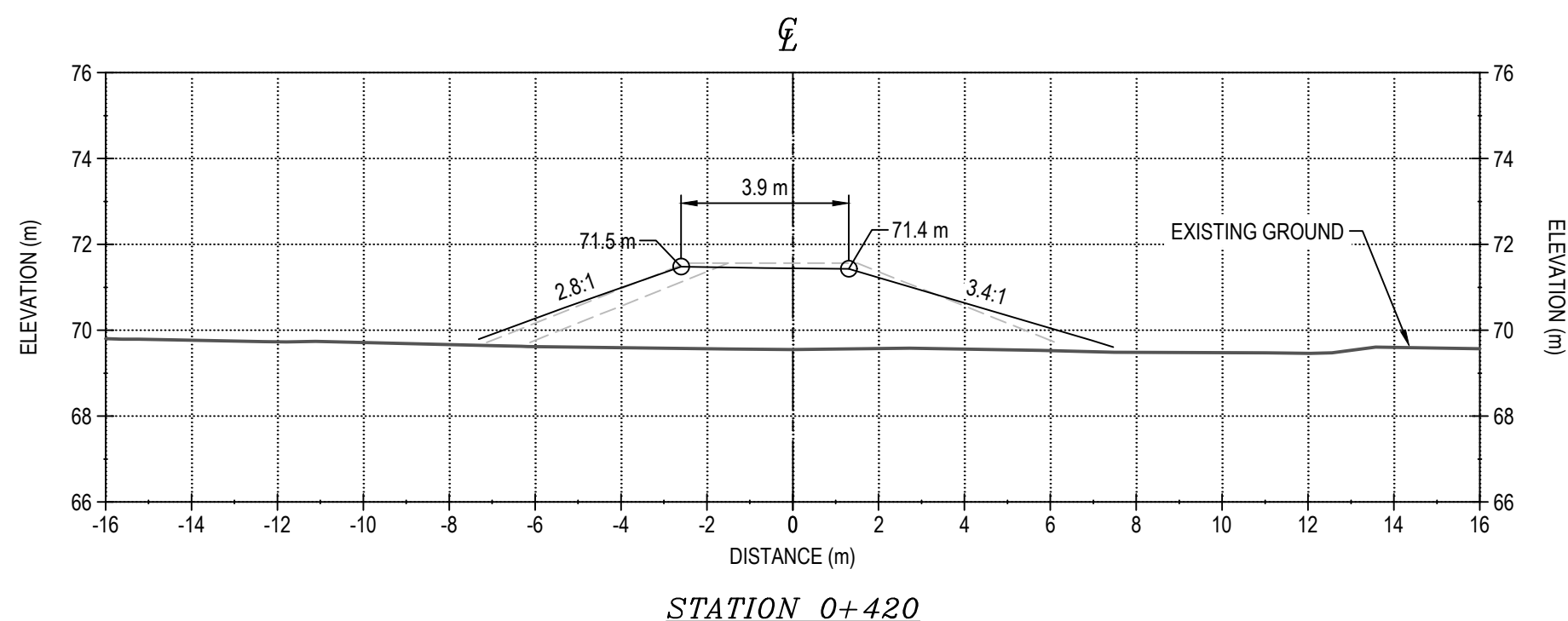
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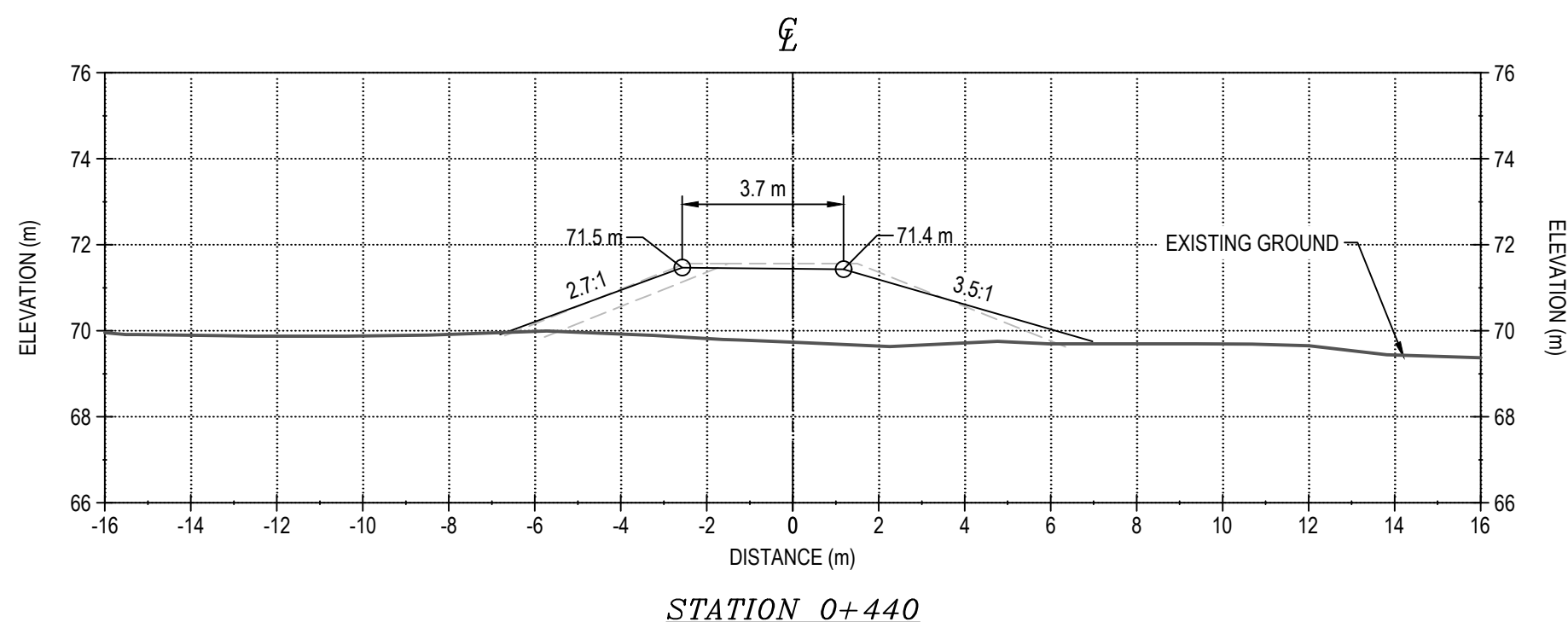
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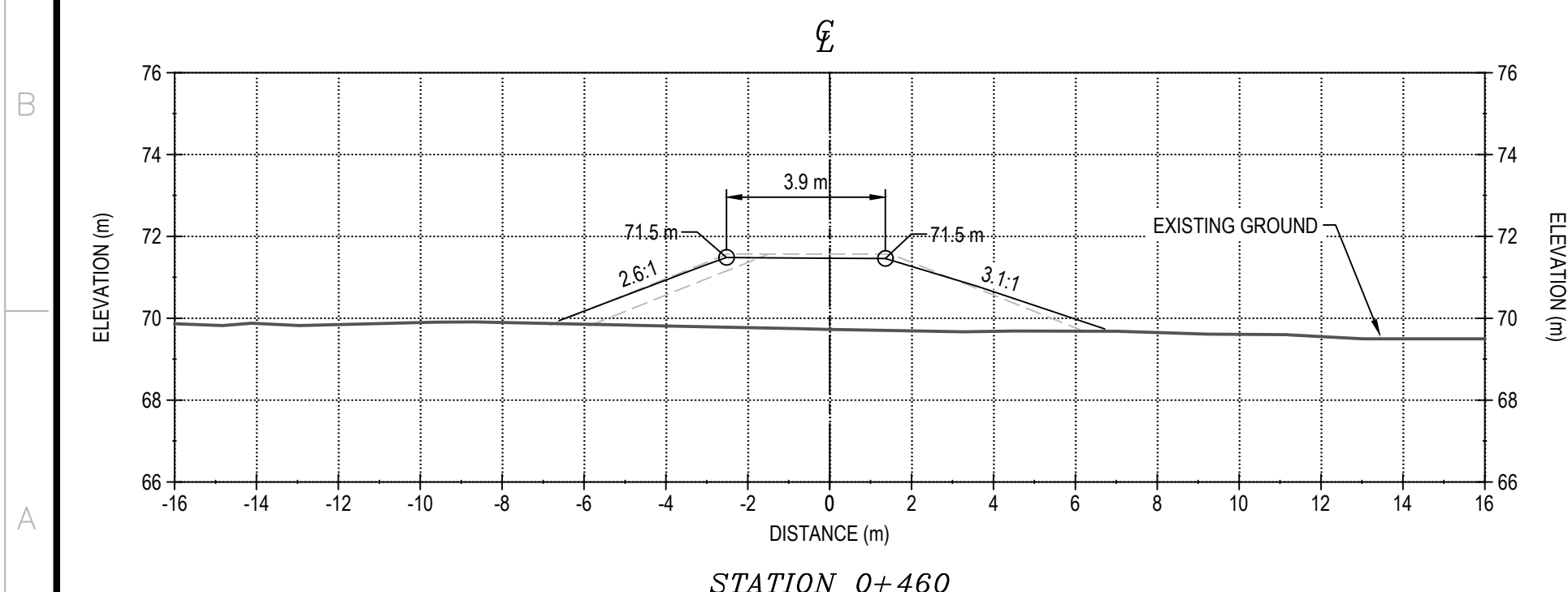
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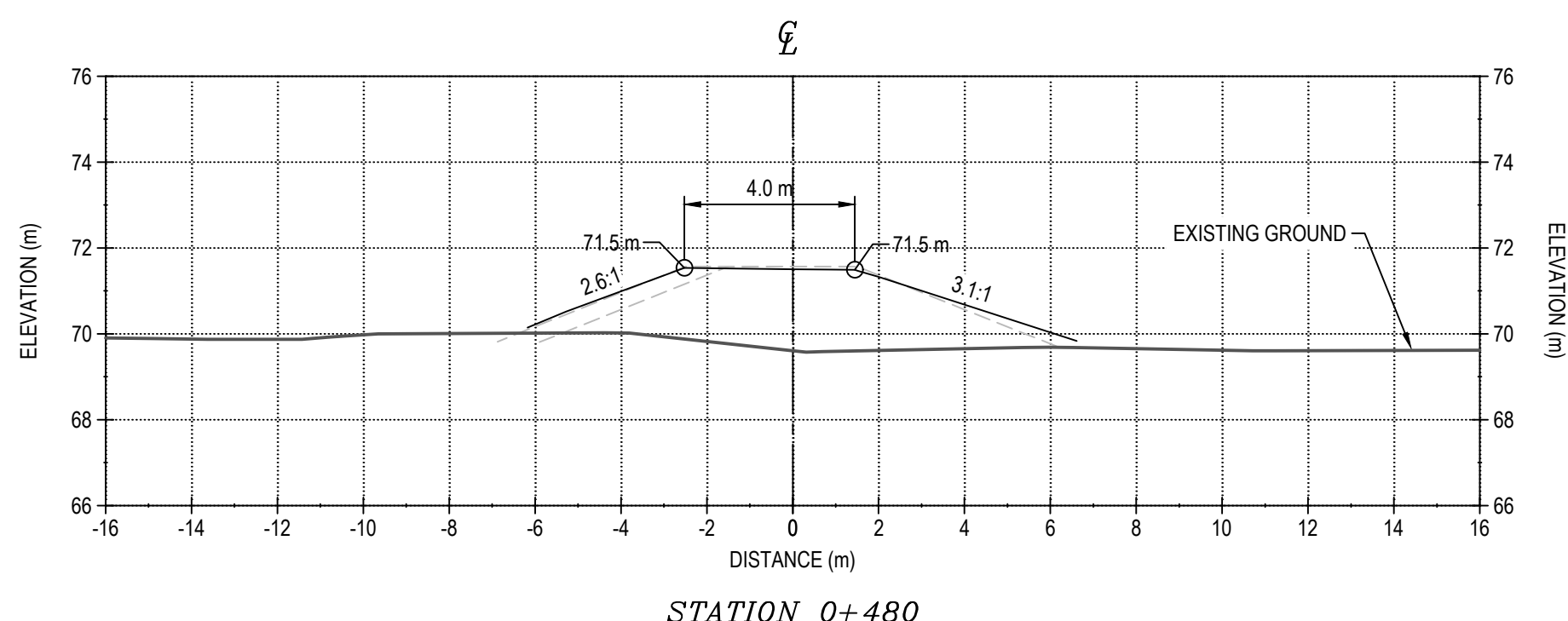
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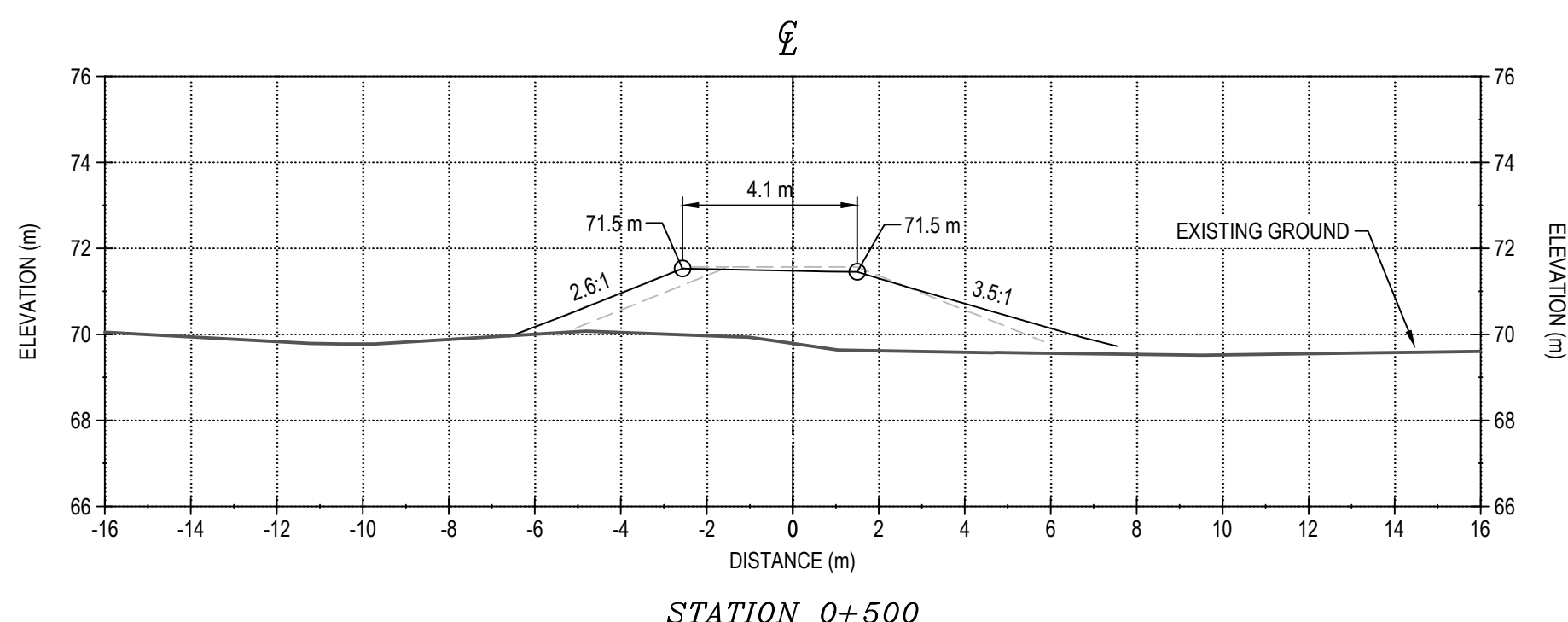
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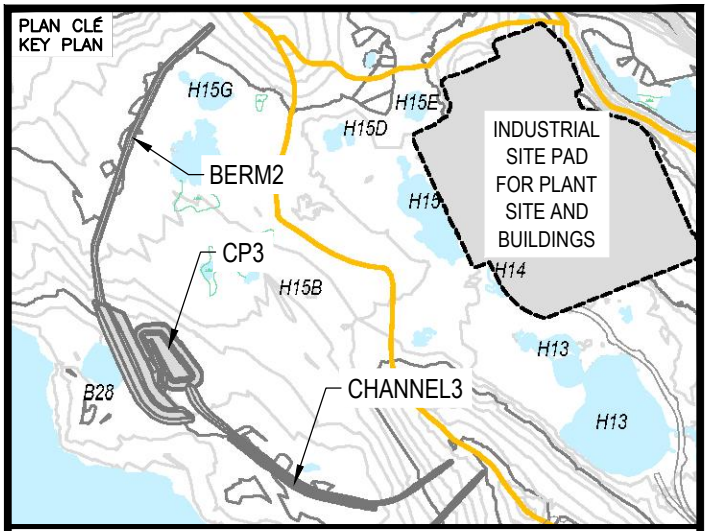
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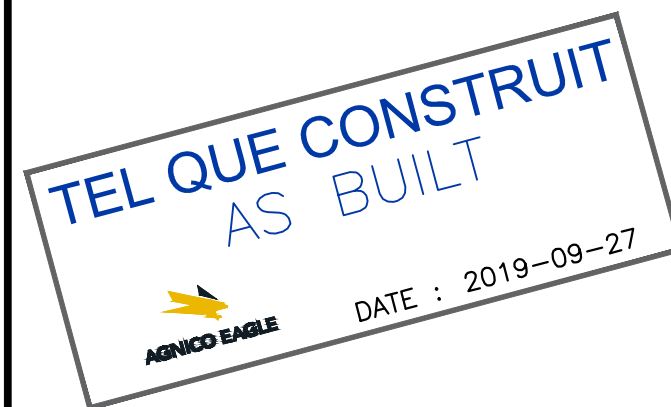


STATION 0+500



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTEXTILE THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).



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DESSINS EN RÉFÉRENCE / REFERENCE DRAWINGS

TITRE / TITLE	# DWG
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REV.	DATE	DESCRIPTION	PAR/REV.	APP.	CLIENT
1	2019-09-27	RECORD DRAWINGS	EL	WITH	
0	2018-06-21	ISSUED FOR CONSTRUCTION	HX	WITH	
1	2018-05-29	ISSUED FOR REVIEW	LOK	WITH	

REVISIONS



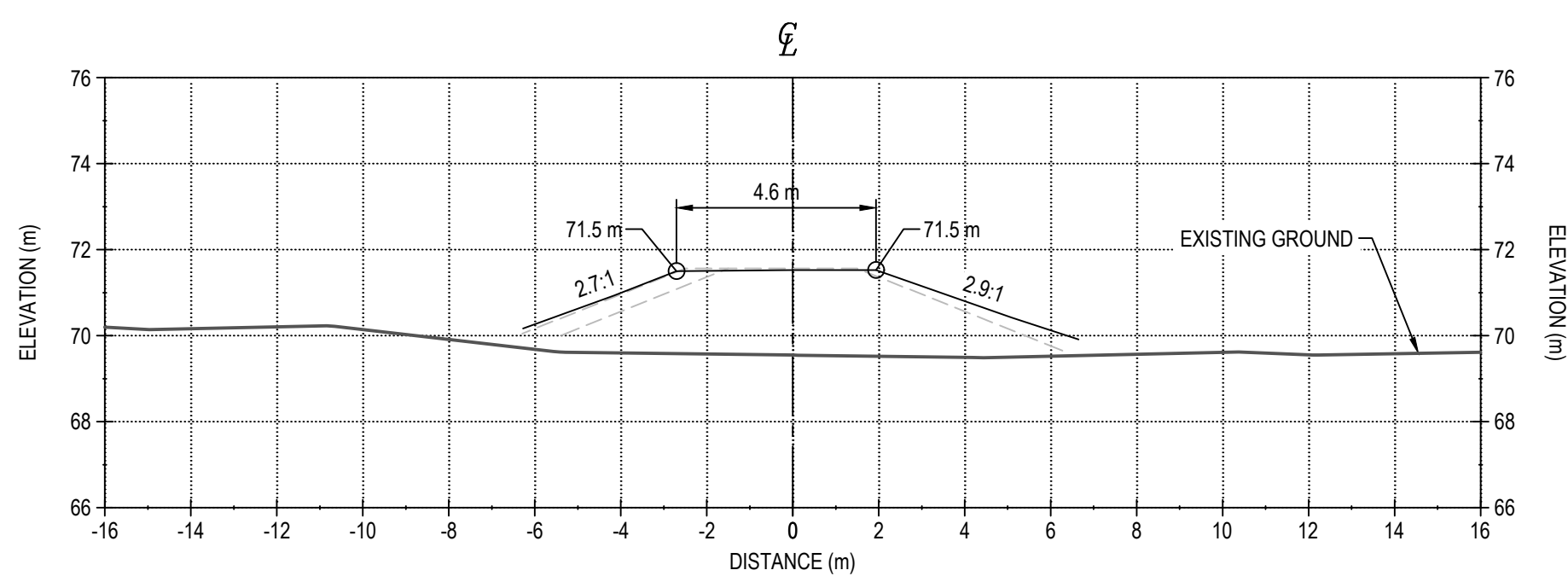
TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

BERM2 DESIGN SECTIONS
(2 of 3)

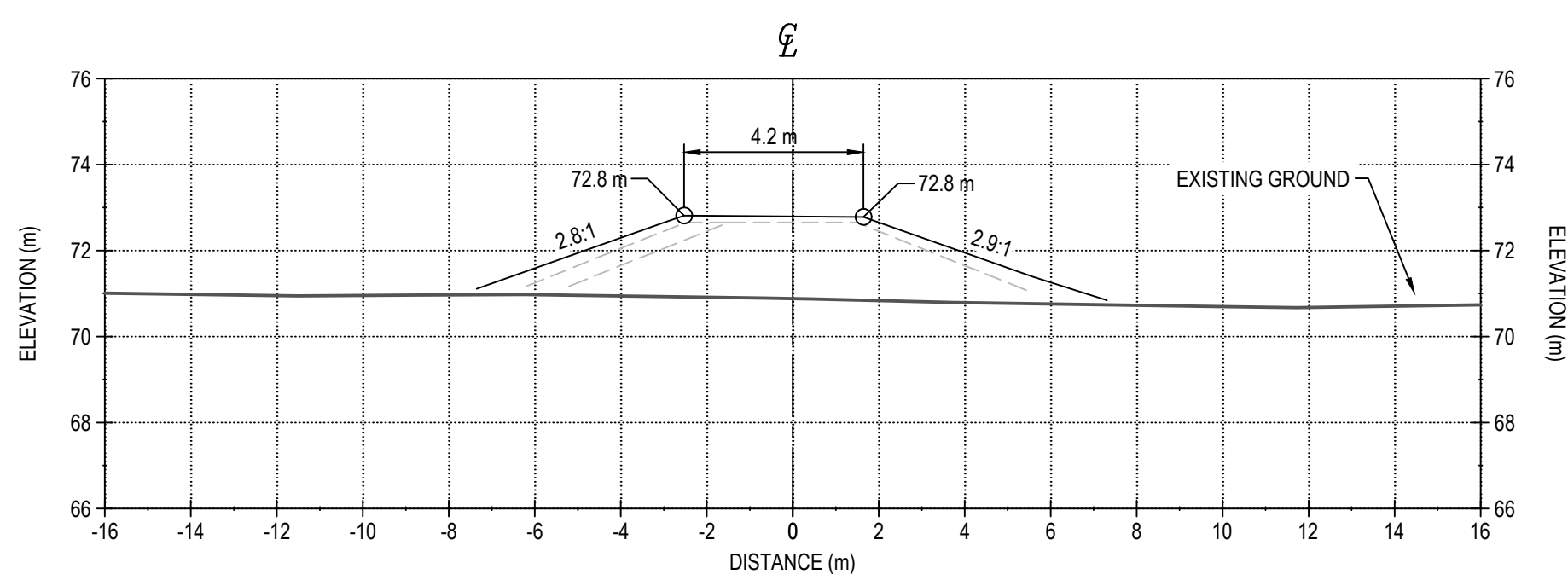
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VÉRIFIÉ PAR CHECKED BY	HX	2019-06-12
APPROUVÉ PAR APPROVED BY	WITH	2019-06-12
ÉCHELLE SCALE	1:150	DATE 2019-06-12

NO. DESSIN
DRAWING NO. 65-695-230-246-2

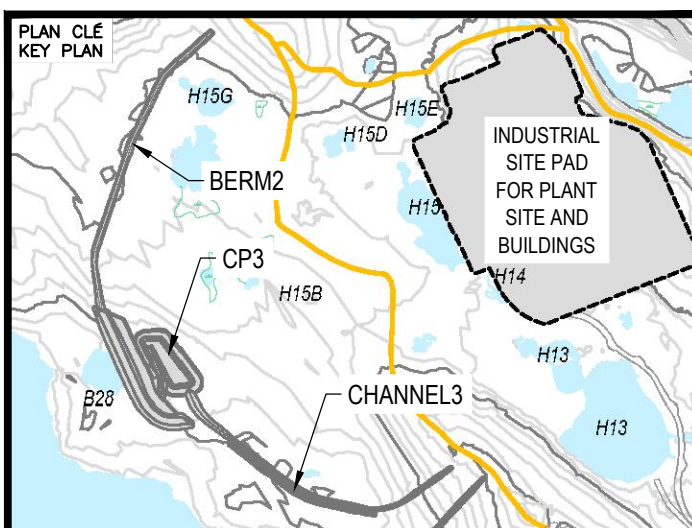
NO. PROJET PROJECT NO.	REVISION	FEUILLE / SHEET
6515	1	12 / 13



STATION 0+560



STATION 0+620



NOTES GÉNÉRALES / GENERAL NOTES

1. ASSUMED CONSTRUCTION SCHEDULE FOR CHANNEL3 FALL / WINTER 2018.
2. ASSUMED GEOTECHNICAL THAT MEETS MATERIAL SPECIFICATION WILL BE AVAILABLE AT SITE FOR CHANNEL3 CONSTRUCTION.
3. MATERIAL PLACEMENT AND FOUNDATION PREPARATION SHOULD BE IN ACCORDANCE WITH THE REQUIREMENTS OF GEOTECHNICAL CONSTRUCTION / MATERIAL SPECIFICATIONS (TETRA TECH 2018).

TEL QUE CONSTRUIT
AS BUILT

DATE : 2018

L'INFORMATION CI-CONTENUE EST LA PROPRIÉTÉ DE AMNCO EAGLE LTD. ET DOIT ÊTRE RETOURNÉE SUR DEMANDE, SANS AUTORISATION ÉCRITE PRÉALABLE. TOUTE TRANSMISSION DE CETTES INFORMATIONS À AUTRI (ET TOUTE UTILISATION AUTRE QUE CELLE POUR LAQUELLE L'INFORMATION EST PRÉSENTÉE) EST INTERDITE. © AMNCO EAGLE LTD.

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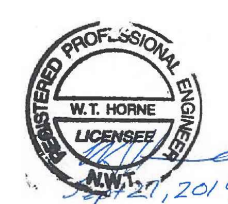
DESSINS EN RÉFÉRENCE / REFERENCE DRAWINGS

TITRE / TITLE	# DWG

**AGNICO EAGLE**

1	2019-09-27	RECORD DRAWINGS	EL	WTH	
0	2018-06-21	ISSUED FOR CONSTRUCTION	HX	WTH	
A	2018-05-29	ISSUED FOR REVIEW	HX	WTH	
REV.	DATE	DESCRIPTION	PAR/BY	APP.	CLIENT

REVISIONS



PERMIT TO PRACTICE
TETRA TECH CANADA INC.

TETRA TECH CANADA INC
Signature: _____

Signature: [Signature]
Date: May 23, 2018

PERMIT NUMBER: P 018

NT/NU Association of Professional
Engineers and Geoscientists

TITRE / TITLE
AGNICO EAGLE MELIADINE GOLD PROJECT

BERM2 DESIGN SECTIONS
(3 of 3)

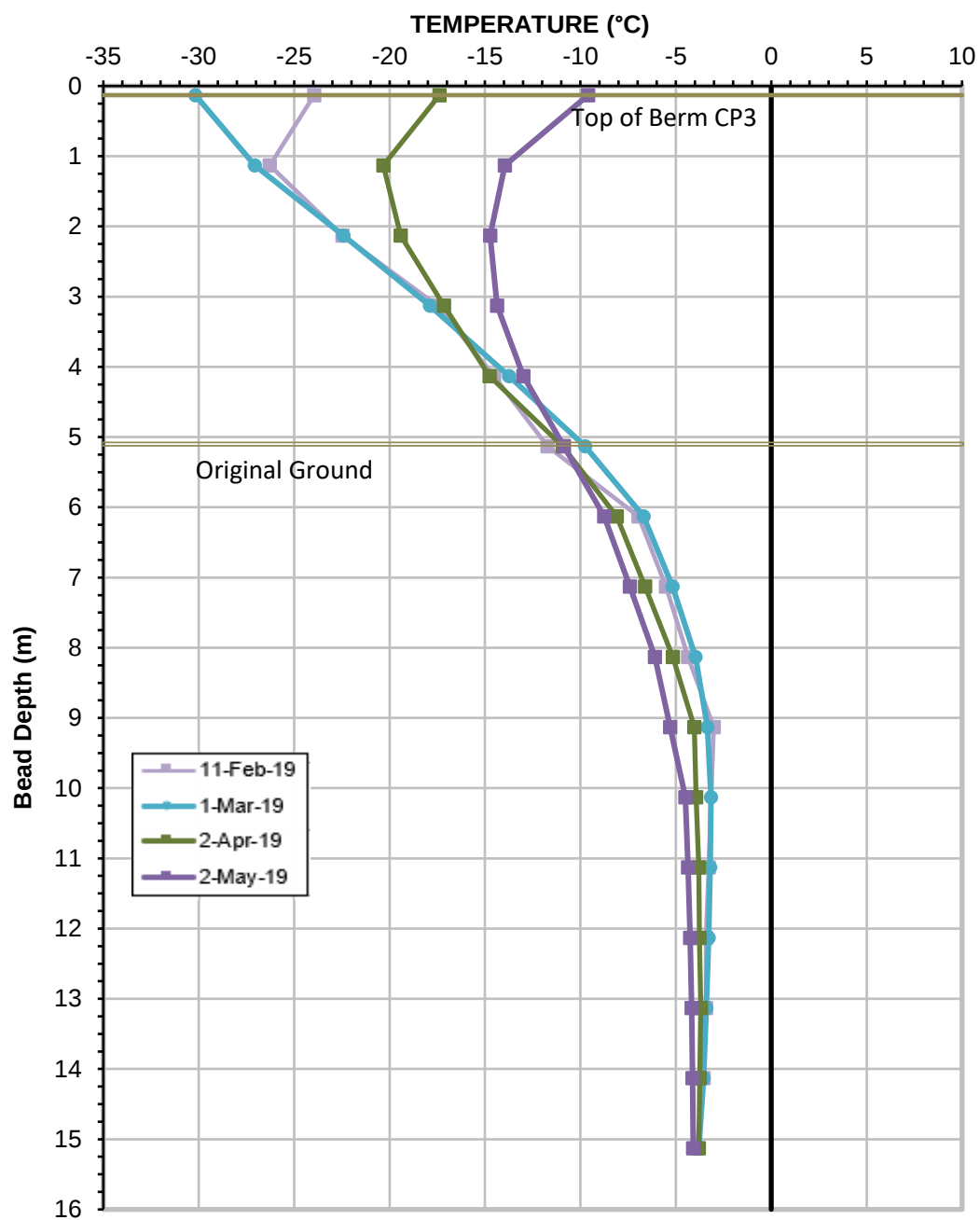
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ÉCHELLE SCALE	1:150	DATE	2019-06-12

NO. DESSIN
DRAWING NO. 65-695-230-246-3

NO. PROJET PROJECT NO. 6515	REVISION	FEUILLE / SHT
	1	13 / 13

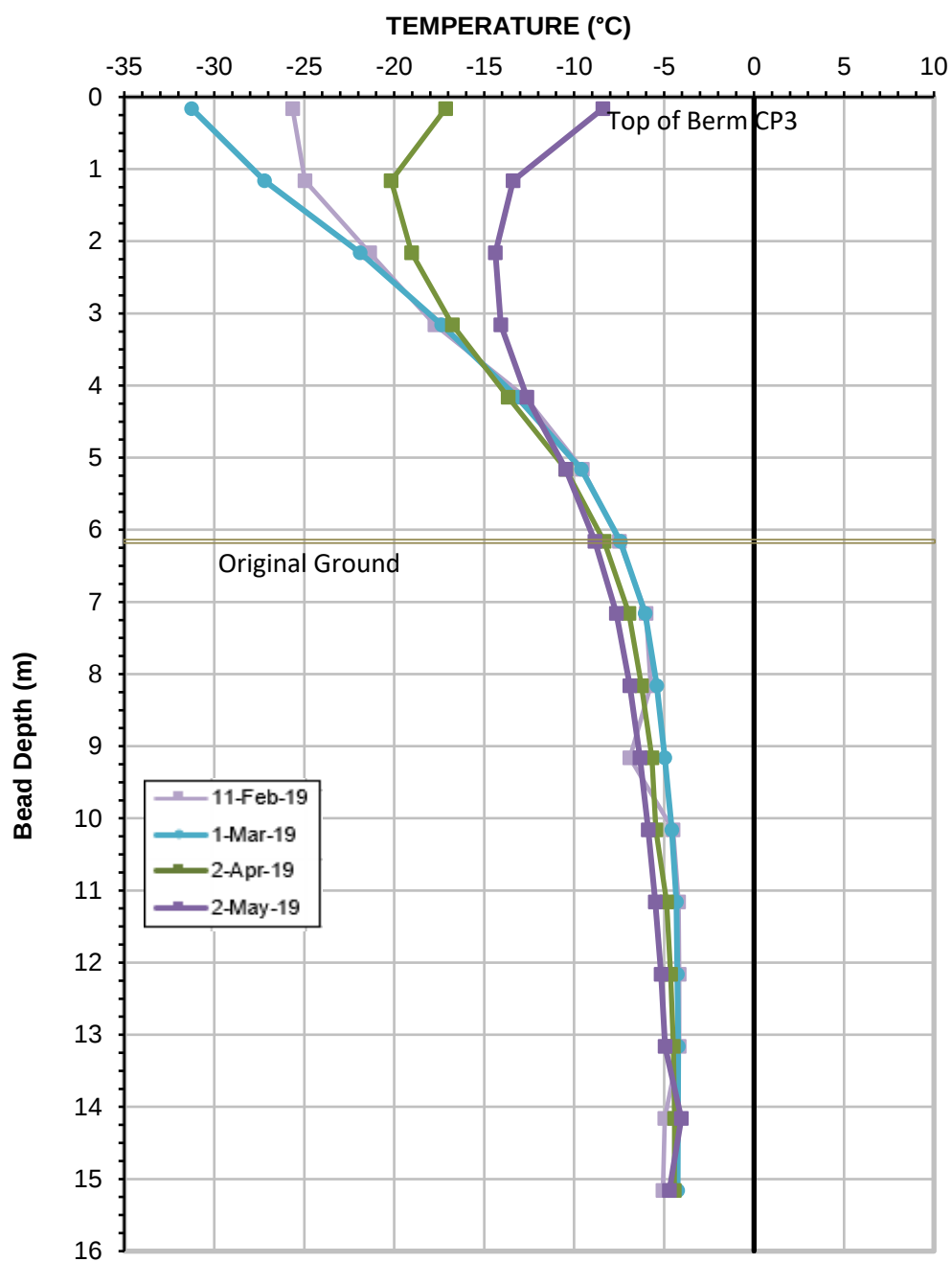
APPENDIX E

GROUND TEMPERATURE PROFILES FOR BERM CP3



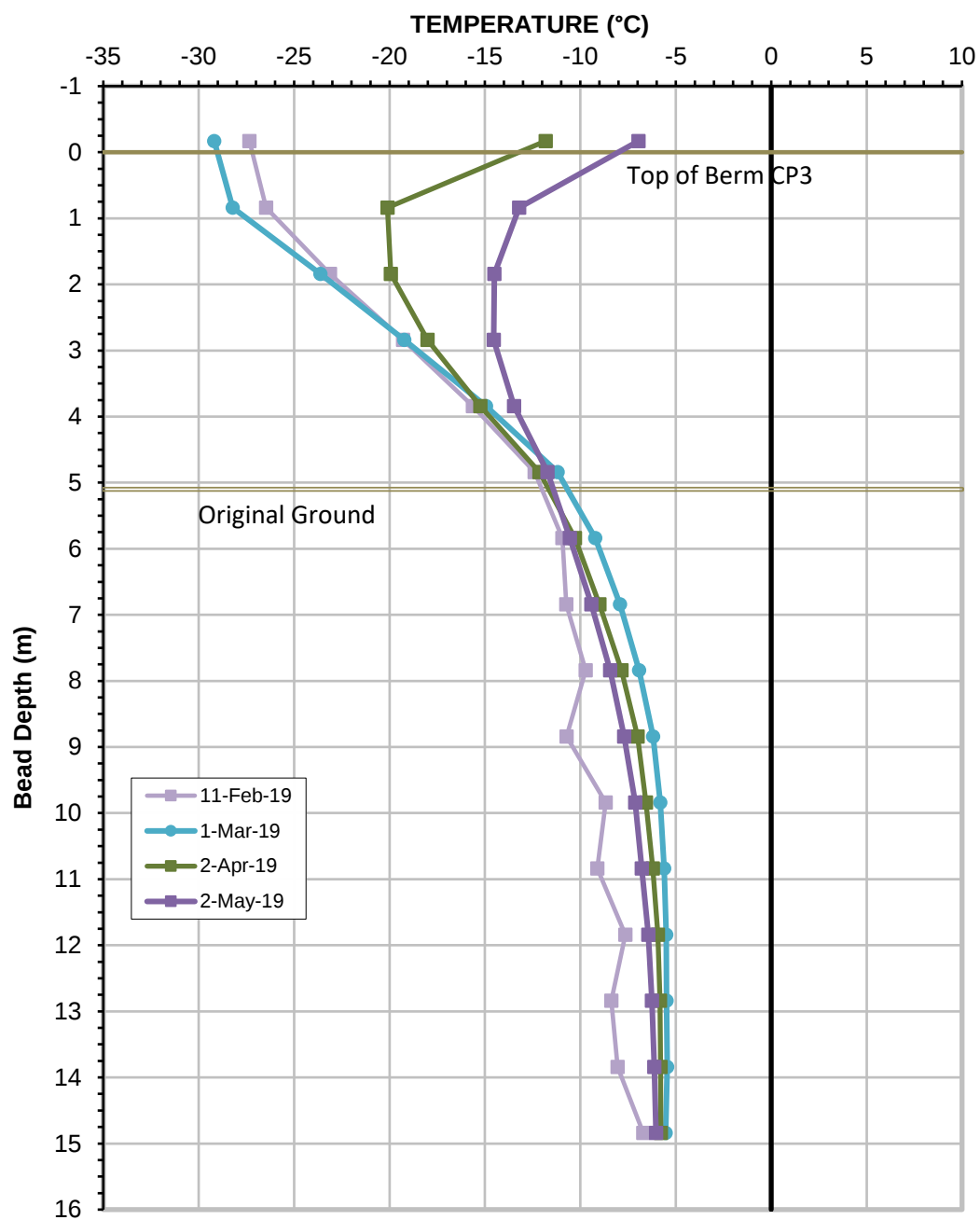
Serial No.: 2644
Date Installed: February 11, 2019

Ground Temperature Profile for Cable GTC-01
Berm CP3



Serial No.: 2645
Date Installed: February 11, 2019

Ground Temperature Profile for Cable GTC-02
Berm CP3



Serial No.: 2689
Date Installed: February 11, 2019

Ground Temperature Profile for Cable GTC-03
Berm CP3