


#### **Appendix 4:**

#### **Mary River Project 2011 Potential Quarry and Borrow Investigations**

## Mary River

### 2011 Potential Quarry and Borrow Investigations

			<i>E. Neufeld</i>	<i>R. Halim</i>	<i>J. Cleland</i>	
2011-11-21	B	Approved for Use	E. Neufeld	R. Halim	J. Cleland	
2011-10-04	A	Draft – For Internal Review	E. Neufeld	R. Halim	J. Cleland	
DATE	REV.	STATUS	PREPARED BY	CHECKED BY	APPROVED BY	APPROVED BY
						CLIENT

## Table of Contents

<b>1. Introduction .....</b>	<b>1</b>
<b>2. Background .....</b>	<b>1</b>
<b>3. Review of Previous Investigations and Laboratory Testing.....</b>	<b>2</b>
3.1 “Additional Quarry Sites” Report.....	2
3.2 “Summary of Aggregate Resource Evaluation Testing Program” Report.....	2
3.3 “Quarry Sites’ Screening” Report.....	3
3.4 “Interim ML/ARD Assessment of Railway Quarry Rock Samples” Report .....	3
3.5 Draft Environmental Impact Statement Report .....	4
<b>4. 2011 Investigation Program .....</b>	<b>4</b>
4.1 Overview .....	4
4.2 Rock Samples.....	5
4.3 Bedrock Depth and Type .....	6
4.4 RQD .....	6
4.5 Point Load Testing.....	7
<b>5. Laboratory Testing .....</b>	<b>7</b>
<b>6. Preliminary Recommendations.....</b>	<b>7</b>
<b>7. References.....</b>	<b>8</b>

## Tables

Table 1: Quarry Summary

Table 2: Borehole Statistics

## Figures

Figure 1: Quarry Sites

Figure 2: Quarry Sites

Figure 3: Bedrock Properties

## Appendices

### Appendix A: Sketches

Sketch 1: Quarry and Borrow Areas at Mary River Mine Site (Sourced from Figure 3-2.3 from DEIS)

Sketch 2: Quarry Areas at Steensby Inlet Port Site (Sourced from Figure 3-2.9 from DEIS)

### Appendix B: Borehole Logs

### Appendix C: Point Load Test Data

## 1. Introduction

Baffinland Iron Mines Corporation (Baffinland) is planning to develop the Mary River iron ore deposits on North Baffin Island in the Qikiqtani Region of Nunavut. This project will require a large volume of rockfill and borrow material for the development of the site infrastructure at the Mary River mine site and Steensby Inlet port site, as well as for the construction of an approximately 140 km railway between the mine site and the port site.

Site investigations have been conducted by Thurber Engineering Ltd (Thurber), under the direction of Hatch Ltd. (Hatch) at the quarries and borrow areas in the summer of 2011.

This report reviews previous studies and investigations for the potential borrow and quarry areas, summarizes the 2011 site investigations and presents recommendations for further development of these quarry and borrow areas.

## 2. Background

The following supporting documents have been identified that refer to borrow and quarry areas for the Mary River project:

- Canarail Consultants Inc. (Canarail), “Additional Quarry Sites”, TDM No. 159952-5000-121-032 [Ref 1].
- AMEC Americas Limited (AMEC), “Summary of Aggregate Resource Evaluation Testing Program, Baffinland Iron Mine Mary River Project” [Ref 2].
- Knight Piésold Consulting (KP), “Mary River Project Quarry Sites’ Screening” [Ref 3].
- AMEC, “Interim ML/ARD Assessment of Railway Quarry Rock Samples, Baffinland Mary River Project – Issued for DEIS” [Ref 4].

These documents were used in the preparation of the Draft Environmental Impact Statement (DEIS), issued in December 2010 [Ref. 5].

Very limited drilling investigations were undertaken at these sites, but the investigation programs from 2006 to 2008 included some drilling to collect samples for preliminary laboratory testing to assess the suitability of the material for granular fill and ballast uses. Preliminary testing was also completed to assess the material’s potential for Acid Rock Drainage (ARD). No specific testing was done to assess the material’s suitability for use as concrete aggregate, except for a limited number of physical tests on rock samples for use as potential ballast material. In particular, it does not appear that any testing for alkali-aggregate reactivity has been conducted to determine the suitability of the rocks from the quarry and borrow area at the Mary River site and potential quarries at Steensby Inlet, where concrete will be used during construction of the project. Identified quarry and borrow areas at Mary River (QMR2 and Borrow Area #3) and at Steensby Inlet (QS-1, QS-2, QS-3 and QS-3A) are shown in Appendix A on figures from the DEIS [Ref 5]. A

potential borrow area around Station 128 + 000 was identified in 2011 and may be the closest potential source for fine aggregates for Steensby Inlet. The location of this borrow area is not shown on these figures.

### **3. Review of Previous Investigations and Laboratory Testing**

#### **3.1 “Additional Quarry Sites” Report**

Canarail’s “Additional Quarry Sites” report [Ref 1] identifies 43 quarry sites to be added to the quarry sites previously identified by KP. The report also states that twelve of KP’s quarry sites were removed based on access difficulties, haul lengths, long upgrade haul distances, construction methodologies, quarry sizes and potential capacities. It was not expected that all of the 69 quarry sites would be developed, but it was felt that it would be better to identify too many rather than too few quarry sites. The report provided estimated volumes for all the sites and stated that Q133 + 500, also known as BAL-1, in the southern portion of the line will be used as ballast material. This report uses data from existing reports and field reconnaissance.

#### **3.2 “Summary of Aggregate Resource Evaluation Testing Program” Report**

AMEC’s “Summary of Aggregate Resource Evaluation Testing Program” report [Ref 2] summarizes laboratory testing done on potential sources of ballast, sub-ballast and rockfill material.

The test results on the ballast sources BAL-1 (Q133 + 500), BAL-2 (Q138 + 100), BAL-3 (Q139 + 600) and BAL-4 (Q139 + 600) are summarized as follows:

- All samples met the ballast specification requirements for degraded aggregate cement value (CV), bulk specific gravity, magnesium sulphate soundness and absorption.
- None of the samples met the shape factor requirements.
- All samples except one sample from BAL-1 (Q133 + 500) had losses that exceeded that maximum allowable limit in the Los Angeles abrasion test.
- All samples for BAL-2 (Q138 + 100), BAL-3 (Q139 + 600) and BAL-4 (Q139 + 600) met the requirements for the mill abrasion (MA) test, but only one sample from BAL-1 (Q133 + 500) met the test requirements.
- All the samples are below the maximum Petrographic Number (PN) requirement specified in Ontario provincial standards for high quality coarse aggregate used in highway road construction [Ref 6].

These results indicate that none of the BAL sources meet all the specification requirements for ballast stone. However, some of the poor test results may have been affected by the limitations of the method of sample preparation. All the samples show good resistance to freezing and thawing conditions.

Limited testing on sub-ballast and rockfill sources was undertaken at Q50 + 000, Q56 + 750, Q64 + 400, Q67 + 200, Q77 + 200 and Q82 + 700. The test results showed that the fines are non-plastic. Micro-deval abrasion testing on the coarse and fine aggregate fractions showed that the mass loss was below maximum losses specified in Ontario provincial standards [Ref 7]. These results indicate that the material could potentially be suitable for railway construction, although it will require additional testing.

The report recommended that additional testing be carried out on the ballast and sub-ballast sources to confirm these test results. In particular, the report recommends that testing be done at the quarry areas at Steensby Inlet, QS1, QS2, QS3 and QS3A, to assess their potential as concrete aggregate, ballast, sub-ballast, rockfill or other construction aggregate material. Also, it recommends that the fine aggregate source at Mary River be tested to assess its suitability for concrete fine aggregate uses.

The report did not identify or discuss any potential ballast materials at the northern end of the railway line. It is not known why the ballast materials were only planned to be sourced from the south end of the railway line. It was understood, however, that the southern end of the railway line passes through a number of rock cuts consisting of granitic rock, which might have been assumed to be hard and durable rock sources suitable for ballast material.

### 3.3 “Quarry Sites’ Screening” Report

KP’s “Mary River Project Quarry Sites’ Screening” report [Ref 3] shows preliminary bounds and volumes for road and railway construction for 68 of the 69 quarry sites recommended by Canarail’s “Additional Quarry Sites” report [Ref 1]. Outer boundaries are shown for each quarry for land tenure purposes and to provide flexibility to the contractor during quarry development. The report shows the areas of excavation that would be required for 1.5 times the volume excavation, assuming one 10 m lift is used, as well as proposed permit areas, areas of potential access to quarries and substitute quarry locations in environmentally sensitive areas.

The report did not discuss Q40 + 600 or QS3, and did not provide volumes for QS1, QS2 or QS3A, because these sources were not anticipated to be used at the time. The report also does not provide volumes of material for material which could potentially be obtained from construction of the tunnels or the ore loading dock.

### 3.4 “Interim ML/ARD Assessment of Railway Quarry Rock Samples” Report

AMEC’s “Interim ML/ARD Assessment of Railway Quarry Rock Samples” Report [Ref 4] assesses the metal leaching and acid rock drainage (ML/ARD) characteristics of rock samples collected by KP in 2008 along the rail alignment. Surface saw-cut sampling was used for ballast material suitability testing, chip sampling and core sampling for geochemical testing and surface grab sampling for durability testing. AMEC only used core samples for the ML/ARD testing, but the remainder of the samples are still stored at the SGS lab in Lakefield, Ontario.

The report provides a geological survey along the rail alignment, stating that the most northern approximately 29 km portion of the rail alignment crosses Paleozoic sedimentary rock and Precambrian rock. The southern 120 km portion of the rail alignment crosses only Precambrian

rock. The Paleozoic sedimentary rock includes sandstones, and the Precambrian rock includes gneisses.

Thirty-eight core samples from 26 boreholes were tested by SGS, taken every 3 to 7 km along the rail alignment. These included 7 sandstone, 6 granite and 25 granitoid gneiss samples.

The testing included acid-base accounting (ABA), Net Acid Generation (NAG), total metals by aqua regia digestion with ICP-MS finish, leachable metals, and mineralogy.

The report concluded that:

- The three types of materials tested are all suitable for quarry source material with a low potential for ML/ARD;
- Two gneiss samples had neutralization potential ratio (NPR) results that were unacceptably low, but the remainder of the samples did not show low NPR results. These samples were located at Station 64 + 400 and Station 95 + 200. However, samples between these stations had acceptable NPR results, indicating that a continuous area of low NPR likely does not exist between Station 64 + 400 and Station 95 + 200. This area should be investigated further;
- The sandstone and intrusive granite rocks are more homogeneous and may require less characterization prior to detailed development, but the less homogenous gneissic rocks should be characterized in more detail, especially around Cockburn Lake, to ensure that the rock does not have potential for ML/ARD;
- ML/ARD conditions may be caused by low sulphide content in the rock, and that sulphide contents should be identified in future sampling; and
- The regional geological information should be better characterized to identify potential ML/ARD areas.

### 3.5 **Draft Environmental Impact Statement Report**

Table A1 of Appendix 10D-6 in the DEIS provides volumes for most of the quarries. The DEIS adds Q40 + 600 and provides volumes for QS2 to the list given by KP in the “Quarry Sites’ Screening” report [Ref 3], but still does not include QS1, QS3A or QS3. The volumes are unchanged from the KP report. The DEIS also does not provide volumes of material for material which could potentially be obtained from construction of the tunnels or the ore loading dock.

Appendix 3D of the DEIS also contains drawings of possible access roads to the quarry sites prepared by Dillon Consulting.

## 4. **2011 Investigation Program**

### 4.1 **Overview**

The DEIS identifies 69 potential quarry locations, outlined in Table 1 below. It was not possible to drill at each of the 69 quarry locations due to time limitations. It should be noted that some of the potential quarry sites were located in areas where water sources for drilling were not

available nearby. Where water sources for drilling were not available nearby, helicopters were used to transport and supply drilling water, affecting the utilization of the helicopter time for the other drilling programs, and hence jeopardizing the completion of the remaining 2011 drilling investigations. In the interests of completing as many boreholes as possible, many of the potential quarry areas without nearby water sources were not drilled. Visual observation of these sites indicated that there are abundant sources of rock materials.

During the 2011 investigation program, boreholes were drilled at 35 quarry locations which had not previously been exploited or tested, at four additional locations along the proposed tunnels and at five locations at the ore loading dock at Steensby Inlet. Drilling was carried out under field supervision from Thurber. As drilling had never occurred at most of the sites, the purpose of the 2011 drilling program at the quarry sites was to confirm the availability of bedrock at the site, to confirm the rock depth and collect samples for physical and ARD tests. At Mary River and Steensby Inlet, samples were also collected for alkali-aggregate reactivity (AAR) tests to assess the material's potential for use as concrete aggregates.

Quarry QS3 is not discussed in the DEIS, but was mentioned in a 2008 KP investigation report [Ref 8]. This site is located on a bedrock knob adjacent to QS3A and it could be developed for sources of rock material at Steensby Inlet. It has been included in Table 1 for completeness.

Rock excavated during the construction of two tunnels in the southern portion of the rail alignment and at the ore loading dock at Steensby Inlet has also been identified as a potential source of rockfill materials. The four boreholes drilled along the tunnel locations and the five boreholes at the ore loading dock are included in Tables 1 and 2.

Table 1 also includes the approximate stations and coordinates of the potential quarry and borrow areas, the volumes of material anticipated to be sourced from the quarry and borrow areas, and whether a borehole was drilled at the quarry or borrow area in 2011. Table 2 summarizes the boreholes drilled in 2011, along with the quarry and borrow samples sent for lab testing. Borehole logs from the 2011 drilling program at the potential quarry and borrow sites are found in Appendix B [Ref 9].

Concrete will be required for construction of the Mary River and Steensby Inlet infrastructure. Material from potential quarries at the Mary River and Steensby Inlet sites will be tested to assess their suitability for concrete coarse aggregate in addition to their suitability as ballast and rockfill material. Additional samples were collected from Borrow Area #3 and from a potential borrow area in the vicinity of Station 128 + 000 for testing to meet suitability requirements for concrete fine aggregates.

## 4.2 Rock Samples

In general, the 16 quarry boreholes that were selected for sampling were chosen based on the distance from rail line and depth to bedrock. Boreholes closer to the railway and with bedrock at shallower depth were preferred over boreholes farther from the railway and with greater amounts of overburden.



The depth of samples for physical characteristic testing from the 16 boreholes was selected randomly. A section of core was selected that was representative of the average bedrock conditions.

The depth of samples for ARD testing was based on rock mineralogy. Sections of rock were selected that appeared to contain sulfide minerals such as pyrite. Where no evidence of sulfide minerals was found, the sample depth was selected randomly.

### 4.3 Bedrock Depth and Type

Table 2 and Figure 3 show the depth to bedrock in each borehole.

Bedrock was encountered in 42 of the 44 boreholes. At boreholes Q28 + 400 and Q31 + 500, bedrock was not encountered at end of drilling depths of 24.2 m and 13.2 m, respectively.

Sandstone bedrock was found at boreholes Q4 + 100, Q7 + 500 and Q18 + 100, and granitic gneiss bedrock was encountered at the remaining 39 boreholes. This confirms the geologic characterization of the railway line found in AMEC's "Interim ML/ARD Assessment of Railway Quarry Rock Samples" Report [Ref 4], as discussed in Section 3.4 of this report.

The depth to bedrock encountered in the boreholes varied along the rail alignment. Thirty-one boreholes had depths to bedrock of 5 m or less, six boreholes had depths to bedrock between 5 m and 10 m, and seven boreholes had depths to bedrock greater than 10 m. The maximum depth to bedrock recorded was 35 m. The boreholes with a depth to bedrock greater than 10 m are all found in the northernmost 40 km along the rail alignment. South of this point, the average depth is much lower at 2.5 m, indicating that the area has more prominent bedrock outcrops along the railway line.

### 4.4 RQD

The bedrock may be used for rail embankment material or ballast material if it has the required physical characteristics and does not show potential for ML/ARD. For ballast material, it is important that the rock be of good quality.

The Rock Quality Designation (RQD) gives an indication of rock soundness and quality. Rock with RQD values between 75% and 100% is classified as good to excellent, while rock with RQD values between 0% to 75% is classified as very poor, poor and fair. Table 2 summarizes the total thickness and percent in each borehole of very poor to fair rock. There is no discernible correlation between depth of rock and rock strength. Figure 3 summarizes the % of very poor to fair rock in each borehole.

Of the potential ballast sites which were previously investigated, only site BAL-1 (Q133 + 500) was not investigated in 2011. The borehole advanced in the vicinity of BAL-2 (Q138 + 100) consisted of 41% poor to fair rock, with RQD values ranging from 47% to 73%. The borehole advanced in the vicinity of BAL-3 (Q139 + 600) and BAL-4 (Q139 + 600), had no very poor to fair rock, confirming that it could be a very suitable source for ballast material.

#### 4.5 Point Load Testing

Point load tests were conducted on the rock, with results provided in Appendix C. In general, the most weak rock is found between Q7 + 500 and Q22 + 500, with a few weak areas between Q42 + 000 and Q53 + 700, and at Q88 + 800, QTR-12 (Station 101 + 100) and NTUN-DH03 (Station 102 + 930). Between these weaker areas, and from Station 114 + 600 to the end of the rail alignment, the rock is almost exclusively very strong to extremely strong (100 MPa to 330 MPa).

### 5. Laboratory Testing

Laboratory testing programs have been previously carried out by AMEC and SGS Group (SGS) using samples collected by KP. It was determined that testing of the 2011 investigation material should be carried out by AMEC and SGS to provide continuity with previous programs. The samples that were collected are summarized in Table 2. Sand samples were also collected from Borrow Area #3 and from a potential borrow area in the vicinity of Station 128 + 000 for testing to meet suitability requirements for concrete fine aggregates.

The tests to be conducted will be the same as those done in previous programs [Ref 2 and Ref 4], as the 2011 investigation program for potential rock quarry sites was intended to provide additional information on the previously identified potential quarries. Physical and geochemical testing results are not available at this time and will be reported separately by AMEC.

### 6. Preliminary Recommendations

The 2011 drilling program has increased the knowledge about the potential quarry and borrow areas by better characterizing the subsurface conditions at the potential sites and by taking samples for testing.

As the next step after the 2011 drilling program and prior to the final design, the following activities are recommended. It should be noted that these recommendations are only based on limited field test results from drilling; laboratory test results are not yet available and may change the recommendations indicated below:

- Additional investigations should be conducted to complete drilling at the potential quarries outlined in Table 1 which were not drilled in 2011. It is not necessary to investigate the quarries used exclusively for construction of the access road;
- Alternative quarry sites should be identified to replace sites where the 2011 drilling program showed deep bedrock or no bedrock. Prior to this, it should be determined whether the borehole results were anomalies, and the sites should be checked for the presence of bedrock outcrops. If so, additional drilling should be conducted at these sites;
- When physical and geochemical testing is completed and reported by AMEC, the potential quarries will need to be re-evaluated on the basis of the test results. The quarries should be revisited, alternative quarries may be added if necessary and volumes of the quarries should

be adjusted based on the information from the current investigation program. The volume of material that can be extracted from rock cut areas and tunnels will be provided by Canarail/EBA Engineering;

- Additional investigations should be conducted at a number of quarry sites to confirm the thickness of overburden, particularly for quarries which will potentially supply large volumes of materials;
- When the laboratory test results are completed and reported, potential ballast sources should be assessed at QMR2, around Station 22 + 500 and 50 + 000, and at QS1, QS2 and QS3A;
- Borrow areas containing large boulders which could be crushed into rock aggregates should be considered, particularly in the most northern 35 km portion of the railway line; and
- Additional rock samples from the 2011 investigation program may need to be tested for ARD and physical characteristics, such as samples from boreholes drilled at other potential rock cut areas which could be used as rockfill material.

## 7. References

1. Canarail Consultants Inc, "Additional Quarry Sites", TDM No. 159952-5000-121-032 Rev. C, December 5, 2008.
2. AMEC Americas Limited, "Summary of Aggregate Resource Evaluation Testing Program, Baffinland Iron Mine Mary River Project", File No. TC83911 Phase 2000, January 13, 2009.
3. Knight Piésold Consulting, "Mary River Project Quarry Sites' Screening", File No., NB102-181/25-A.01, Cont. No. NB10-00321, June 25, 2010.
4. AMEC Americas Limited, "Interim ML/ARD Assessment of Railway Quarry Rock Samples, Baffinland Mary River Project – Issued for DEIS", File No. TC 101507, December 14, 2010.
5. Knight Piésold Consulting, "Mary River Project, Draft Environmental Impact Statement", December 2010.
6. Ontario Provincial Standard Specification, "Material Specification for Aggregates – Hot Mix Asphalt", OPSS 1003, November 2006.
7. Ontario Provincial Standard Specification, "Material Specification for Aggregates – Base, Subbase, Select Subgrade, and Backfill Material", OPSS 1010, November 2003.
8. Knight Piésold Consulting, "Baffinland Iron Mines Corporation, Mary River Project, Rail Alignment, Steensby Port Site Infrastructure & Borrow Sources, 2008 Site Investigations Summary Report", Ref. No. NB102-181/24-2, December 31, 2010.
9. Thurber Engineering Limited, "2011 Geotechnical Investigation – Factual Report", under preparation.

E. Neufeld  
EN:ehs

## Tables

Table 1: Quarry Summary

Quarry Name	Approximate Station <sup>1</sup>	Northing Coordinates	Easting Coordinates	Volume with Contingency (m <sup>3</sup> ) <sup>2</sup>			Drilled in 2011
				Railway	Road	Total	
QMR2	Mary River	7,914,203	560,128	491,079	47,052	538,130	Yes
Q-0+500	- 0+500	7,911,899	563,668	826,508	196,174	1,022,683	Yes
Q4+100	4+100	7,909,418	566,698	570,968	199,833	770,801	Yes
Q7+500	7+500	7,907,667	569,432	619,585	174,374	793,959	Yes
Q10+250	10+250	7,905,378	572,883	1,007,536	206,620	1,214,157	Yes
Q14+500	14+200	7,904,382	575,868	1,232,091	240,446	1,472,537	Yes
Q18+100	18+100	7,902,853	578,804	1,463,455	263,943	1,727,398	Yes
Q22+500	22+500	7,901,663	583,415	1,484,696	230,582	1,715,278	Yes
Q25+500	25+500	7,900,221	586,954	755,019	124,801	879,820	Yes
Q28+400	28+400N	7,898,617	588,240	158,886	104,426	263,312	Yes
Q31+500	31+500N	7,897,863	590,944	53,356	128,885	182,242	Yes
Q35+000	35+000N	7,896,866	594,445	77,971		77,971	
Q35+500	35+500N	7,896,244	595,477	248,555	118,172	366,726	Yes
Q38+700	38+700N	7,893,140	596,368	342,908	116,780	459,687	Yes
Q40+600	40+600N	7,889,375	596,009	812,884	153,489	966,373	
Q42+000	42+000N	7,890,881	598,151	305,177	96,131	401,308	Yes
Q44+300	44+300N	7,888,054	598,208	302,919	62,185	365,104	Yes
Q44+000	44+000	7,885,927	596,138	178,149	38,931	217,080	Yes
Q45+000	45+000	7,884,724	596,201	54,862	29,020	83,882	Yes
Q45+800	45+800	7,884,147	596,990	34,368	42,430	76,798	
Q48+000	48+000	7,882,597	598,495	28,241	58,397	86,637	
Q50+000	50+000	7,881,100	597,357	134,915	70,757	205,672	Yes
Q53+700	53+700	7,877,567	597,616	339,267	78,350	417,616	Yes
Q56+750	56+750	7,875,280	598,852	426,916	87,668	514,583	Yes
Q60+000	60+000	7,871,954	599,087	327,131	102,084	429,214	
Q64+400	64+400	7,868,565	600,221	203,898	94,957	298,854	
Q67+200	67+200	7,865,619	600,161	156,728	79,560	236,288	
Q71+000	71+000	7,863,169	602,398	161,614	71,915	233,530	
Q74+200	74+200	7,860,226	603,469	109,863	63,161	173,024	
Q77+200	77+200	7,857,588	604,840	86,660	65,983	152,642	
Q79+600	79+600	7,855,411	605,366	145,051	77,616	222,666	
Q82+700	82+700	7,852,449	605,710	166,692	90,198	256,890	Yes
Q85+200	85+200	7,850,087	606,073	227,871	89,196	317,067	Yes
Q88+800	88+800	7,846,674	605,956	238,151	63,999	302,150	Yes
QTR21	90+400	7,845,379	605,707		51,239	51,239	
Q92+000	92+000	7,843,535	605,816	98,287		98,287	
QTR22	92+000	7,843,330	605,243		47,682	47,682	
QTR23	93+600	7,841,721	606,018		33,456	33,456	
Q95+400	95+150	7,840,905	607,500	16,898	56,143	73,041	
Q96+100	96+100	7,840,533	608,580	17,031	149,531	166,562	
Q96+700	96+700	7,839,908	608,976	6,493		6,493	
QTR10	97+300	7,839,328	611,431		203,081	203,081	
QTR11	98+700	7,838,013	611,995		134,433	134,433	
QTR6	98+900	7,836,409	611,377		101,012	101,012	
QTR13	100+700	7,833,967	609,448		165,509	165,509	Yes

Quarry Name	Approximate Station <sup>1</sup>	Northing Coordinates	Easting Coordinates	Volume with Contingency (m <sup>3</sup> ) <sup>2</sup>			Drilled in 2011
				Railway	Road	Total	
QTR12	101 + 100	7,836,190	610,857		162,040	162,040	Yes
NTUN-DH01	102 + 540	7,835,656	605,976				Yes
NTUN-DH03	102 + 930	7,835,382	605,698				Yes
NTUN-DH05	103 + 140	7,835,245	605,535				Yes
STUN-DH03	108 + 180	7,832,812	601,490				Yes
QTR7	108 + 300	7,832,685	608,302		132,606	132,606	
QTR17	105 + 700	7,832,984	603,944		948,392	948,392	
QTR14	104 + 300	7,831,608	607,681		171,297	171,297	
QTR15	105 + 200	7,830,326	606,224		227,039	227,039	
QTR16	106 + 200	7,830,731	603,228		905,620	905,620	
Q110 + 200	110 + 200	7,831,193	600,359	253,809		253,809	
QTR8	112 + 000	7,830,182	602,012		603,136	603,136	
Q114 + 600	114 + 600	7,827,828	597,850	382,501		382,501	Yes
QTR9	116 + 500	7,826,260	600,261		361,991	361,991	Yes
Q116 + 800	116 + 800	7,826,194	597,422	764,455		764,455	Yes
QTR18	120 + 600	7,822,808	599,870		536,571	536,571	
QTR4A	123 + 000	7,820,410	598,555	958,066	636,598	1,594,664	Yes
QTR19	126 + 900	7,816,806	597,863		451,609	451,609	
Q127 + 800	127 + 800	7,815,755	598,770	545,218		545,218	
Q128 + 000	128 + 000	7,813,922	598,828		222,278	222,278	
Q131 + 100	131 + 100	7,813,509	600,177	112,666	191,240	303,906	Yes
Q133 + 500	133 + 500	7,811,052	601,482				
QTR20	134 + 100	7,810,467	598,087		169,565	169,565	
Q138 + 100	138 + 100	7,807,612	598,865		104,996	104,996	Yes
Q139 + 600	139 + 600	7,806,105	598,727		119,999	119,999	Yes
QS3A	Steensby	7,800,000	595,698				Yes
QS3	Steensby	7,799,349	597,500				
QS2	Steensby	7,801,066	595,200		300,000	300,000	Yes
QS1	Steensby	7,803,054	593,500				Yes
SI-OLD-004	Steensby	7,798,314	592,879				Yes
SI-OLD-005	Steensby	7,798,331	592,860				Yes
SI-OLD-006	Steensby	7,798,409	592,876				Yes
SI-OLD-007	Steensby	7,798,424	592,840				Yes
SI-OLD-008	Steensby	7,798,489	592,891				Yes

**Notes:**

<sup>1</sup> Two sets of stationing are used along the rail alignment. Following the Ravn River realignment, which extends from approximately station 26 + 100 to station 46 + 582.93, the stationing resets to 43 + 830 to be consistent with the stationing used prior to the Ravn River realignment. To avoid confusion, stationing along the Ravn River realignment has an "N" suffix.

<sup>2</sup> Volumes obtained from the DEIS [Ref 5].

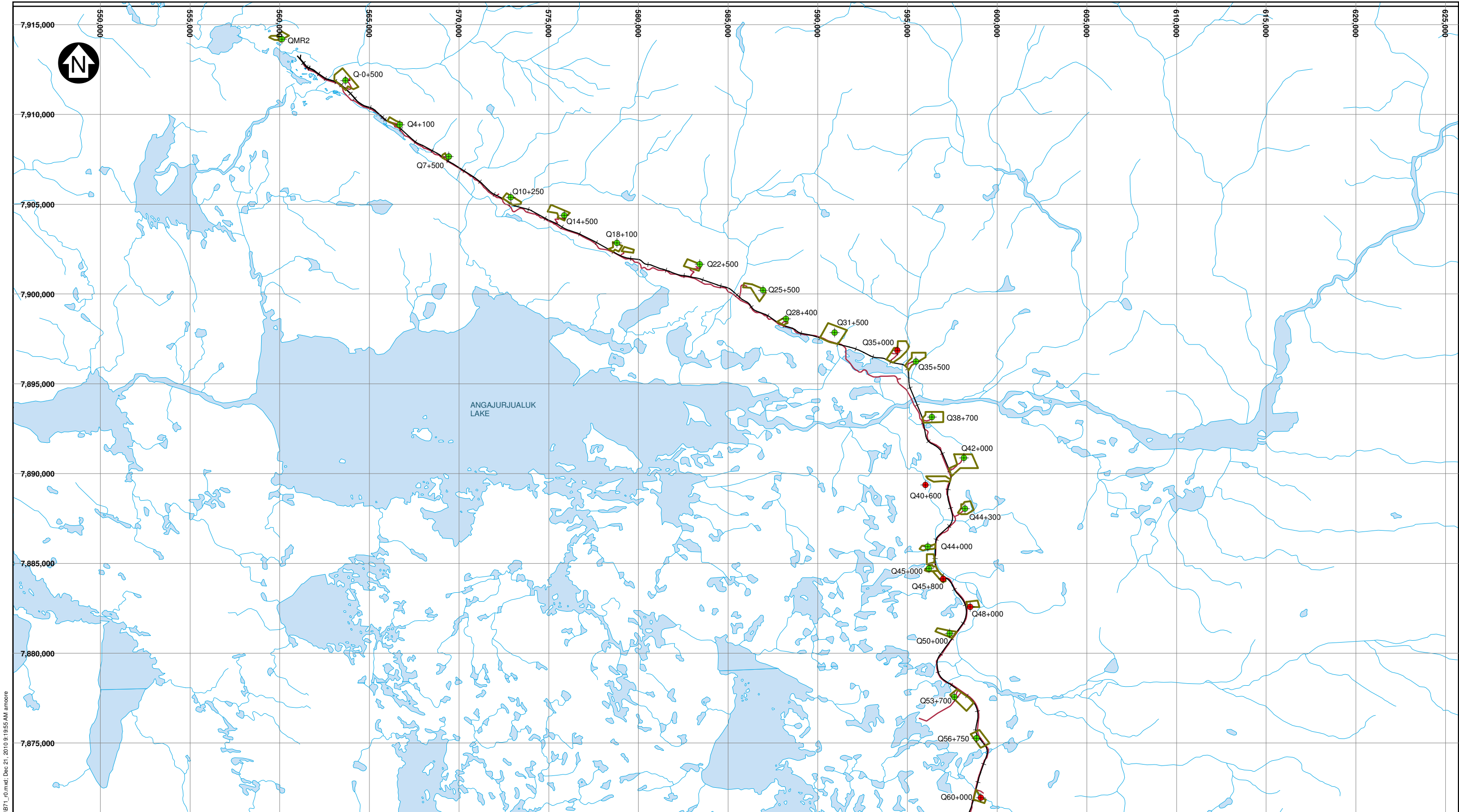
**Table 2: Borehole Statistics**

Borehole Name	Date Started	Date Completed	Borehole Depth (m)	Bedrock Depth (m)	Very Poor to Fair Rock Thickness (m)	% Very Poor to Fair Rock	Rock Type	Physical Testing	Depth of Physical Sample (m)	Depth of ARD Sample (m)
QMR2	7-Aug-11	7-Aug-11	26.0	0.90	1.1	4%	Granitic Gneiss	yes	2.0 to 7.0	11.25 to 11.35
Q0 + 500	30-Jul-11	31-Jul-11	30.0	16.15	13.85	100%	Granitic Gneiss			26.7 to 26.8
Q4 + 100	29-Jul-11	30-Jul-11	62.0	35.00	27	100%	Sandstone			53.43 to 53.5
Q7 + 500	28-Jul-11	29-Jul-11	50.0	21.80	19.2	68%	Sandstone			40.9 to 41
Q10 + 250	25-Jul-11	26-Jul-11	41.0	8.00	33	100%	Granitic Gneiss			22.9 to 23
Q14 + 500	24-Jul-11	24-Jul-11	47.2	11.20	33	92%	Granitic Gneiss			25.1 to 25.2
Q18 + 100	23-Jul-11	23-Jul-11	38.0	5.00	No data	No data	Sandstone			17.6 to 17.7
Q22 + 500	22-Jul-11	22-Jul-11	62.0	2.00	3	5%	Granitic Gneiss			20.4 to 20.5
Q25 + 500	22-Jul-11	22-Jul-11	32.3	5.34	2.9	11%	Granitic Gneiss	yes	8.34 to 13.3	6.7 to 6.8
Q28 + 400	18-Jul-11	18-Jul-11	24.2	> 24.2	-	-	No bedrock found			
Q31 + 500	19-Jul-11	19-Jul-11	13.2	> 13.2	-	-	No bedrock found			
Q35 + 500	17-Jul-11	17-Jul-11	26.2	2.45	11.75	49%	Granitic Gneiss			19.53 to 19.65
Q38 + 700	16-Jul-11	16-Jul-11	50.4	31.20	7.2	38%	Granitic Gneiss			38.15 to 38.24
Q42 + 000	16-Jul-11	16-Jul-11	23.4	5.67	8.73	49%	Granitic Gneiss	yes	12.75 to 18.0	21.57 to 21.67
Q44 + 300	15-Jul-11	15-Jul-11	14.4	1.90	-	0%	Granitic Gneiss			5.24 to 5.33
Q44 + 000	15-Jul-11	15-Jul-11	23.0	5.00	18	100%	Granitic Gneiss	yes	8.0 to 12.9	18.8 to 18.9
Q45 + 000	14-Jul-11	14-Jul-11	14.3	2.20	-	0%	Granitic Gneiss			7.45 to 7.56
Q50 + 000	14-Jul-11	14-Jul-11	16.9	0.44	-	0%	Granitic Gneiss	yes	7.79 to 13.1	14.8 to 14.9
Q53 + 700	13-Jul-11	13-Jul-11	29.2	7.70	-	0%	Granitic Gneiss	yes	14.2 to 19.4	23.2 to 23.3
Q56 + 750	11-Jul-11	11-Jul-11	11.2	4.41	-	0%	Granitic Gneiss	yes	4.7 to 9.7	10.8 to 10.9
Q82 + 700	22-Jul-11	22-Jul-11	16.0	1.65	9.35	65%	Granitic Gneiss	yes	4.55 to 9.5	4.2 to 4.3
Q85 + 200	20-Jul-11	21-Jul-11	25.9	2.58	8.52	37%	Granitic Gneiss			
Q88 + 800	17-Jul-11	18-Jul-11	15.0	0.10	4.4	30%	Granitic Gneiss			7.89 to 7.98

Borehole Name	Date Started	Date Completed	Borehole Depth (m)	Bedrock Depth (m)	Very Poor to Fair Rock Thickness (m)	% Very Poor to Fair Rock	Rock Type	Physical Testing	Depth of Physical Sample (m)	Depth of ARD Sample (m)
QTR13	14-Jul-11	14-Jul-11	15.1	5.00	3.1	31%	Granitic Gneiss	yes	5.2 to 10.2	11.3 to 11.45
QTR12	15-Jul-11	15-Jul-11	20.5	2.85	11.65	66%	Granitic Gneiss			8.8 to 8.92
NTUN-DH01	3-Aug-11	3-Aug-11	34.8	6.45	1.35	5%	Granitic Gneiss			13.0 to 13.4
NTUN-DH03	1-Aug-11	13-Aug-11	121.6	4.50	0.00	0%	Granitic Gneiss			109.2 to 109.6
NTUN-DH05	30-Jul-11	30-Jul-11	82.3	2.99	3.00	4%	Granitic Gneiss			
STUN-DH03	29-Jul-11	31-Jul-11	87.45	3.30	0.00	0%	Granitic Gneiss			63.0 to 63.4
Q114+600	25-Jul-11	25-Jul-11	33.6	2.30	-	0%	Granitic Gneiss	yes	9.4 to 14.52	16.95 to 17.05
QTR9	25-Jul-11	26-Jul-11	32.4	0.30	2.1	7%	Granitic Gneiss			29.75 to 29.86
Q116+800	26-Jul-11	26-Jul-11	32.4	1.93	-	0%	Granitic Gneiss	yes	12.75 to 17.39	9.1 to 9.2
QTR4A	4-Aug-11	4-Aug-11	32.45	0.51	14	44%	Granitic Gneiss	yes	19.9 to 24.9	15.5 to 15.6
Q131+100	5-Aug-11	5-Aug-11	20.48	8.35	3.15	26%	Granitic Gneiss	yes	11.48 to 16.1	17.6 to 17.7
Q138+100	16-Jul-11	17-Jul-11	30.93	0.00	12.7	41%	Granitic Gneiss	yes	17.29 to 22.18	6.1 to 6.2
Q139+600	19-Jul-11	19-Jul-11	13.9	0.00	-	0%	Granitic Gneiss			
QS1	6-Aug-11	6-Aug-11	17.52	0.00	6.02	34%	Granitic Gneiss	yes	9.6 to 14.52	15.6 to 15.75
QS2	24-Jul-11	25-Jul-11	30.0	0.00	3.2	11%	Granitic Gneiss			5.9 to 6.04
QS3A	23-Jul-11	23-Jul-11	15.1	0.00	11.8	78%	Granitic Gneiss	yes	4.97 to 10.0	15.6 to 15.75
SI-OLD-004	8-Aug-11	8-Aug-11	32.23	0.00	3	9%	Granitic Gneiss			
SI-OLD-005	9-Aug-11	10-Aug-11	31.96	0.00	5	16%	Granitic Gneiss			7.5 to 7.9
SI-OLD-006	10-Aug-11	11-Aug-11	38.38	0.00	6	16%	Granitic Gneiss			
SI-OLD-007	11-Aug-11	11-Aug-11	32.4	0.00	11.4	35%	Granitic Gneiss			8.1 to 8.5
SI-OLD-008	12-Aug-11	12-Aug-11	35.61	0.00	13.4	38%	Granitic Gneiss			



## Figures



SAVED: I:\1102\00181\25\AG\IS\Ac\Views\figures\B71\_10.mxd; Dec 21, 2010 9:19:55 AM ammore

**LEGEND:**

-  PROPOSED RAILWAY ALIGNMENT

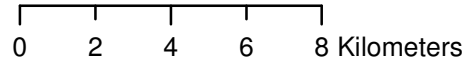
 PROPOSED ACCESS ROAD

 PROPOSED QUARRY LEASE BOUNDARY AREA

 WATER BODY
-  QUARRIES DRILLED IN 2011

 QUARRIES NOT DRILLED IN 2011

 RIVER/STREAM/DRAINAGE



**NOTES:**

1. BASE MAP 1:250,000 © HER MAJESTY THE QUEEN IN RIGHTS OF CANADA, DEPARTMENT OF NATURAL RESOURCES (2004).
2. COORDINATE GRID IS SHOWN IN UTM (NAD83) ZONE 17 AND IS IN METRES.
3. QUARRY LEASE BOUNDARY AREAS AND ACCESS ROAD PROVIDED BY KNIGHT PIESOLD.
4. RAILWAY ALIGNMENT PROVIDED BY CANARAIL CONSULTANTS INC. (AUGUST 12, 2010).
5. THE RAILWAY AND CONSTRUCTION ACCESS ROAD PRELIMINARY DESIGNS, INCLUDING THE LOCATION OF ALL WATERCOURSE CROSSINGS, ARE PRESENTED AS PLAN AND PROFILE DRAWINGS BY CANARAIL IN VOLUME 3 APPENDICES TO THE EIS.

BAFFINLAND IRON MINES CORPORATION

MARY RIVER PROJECT

**2011 GEOTECHNICAL INVESTIGATIONS  
QUARRY SITES**

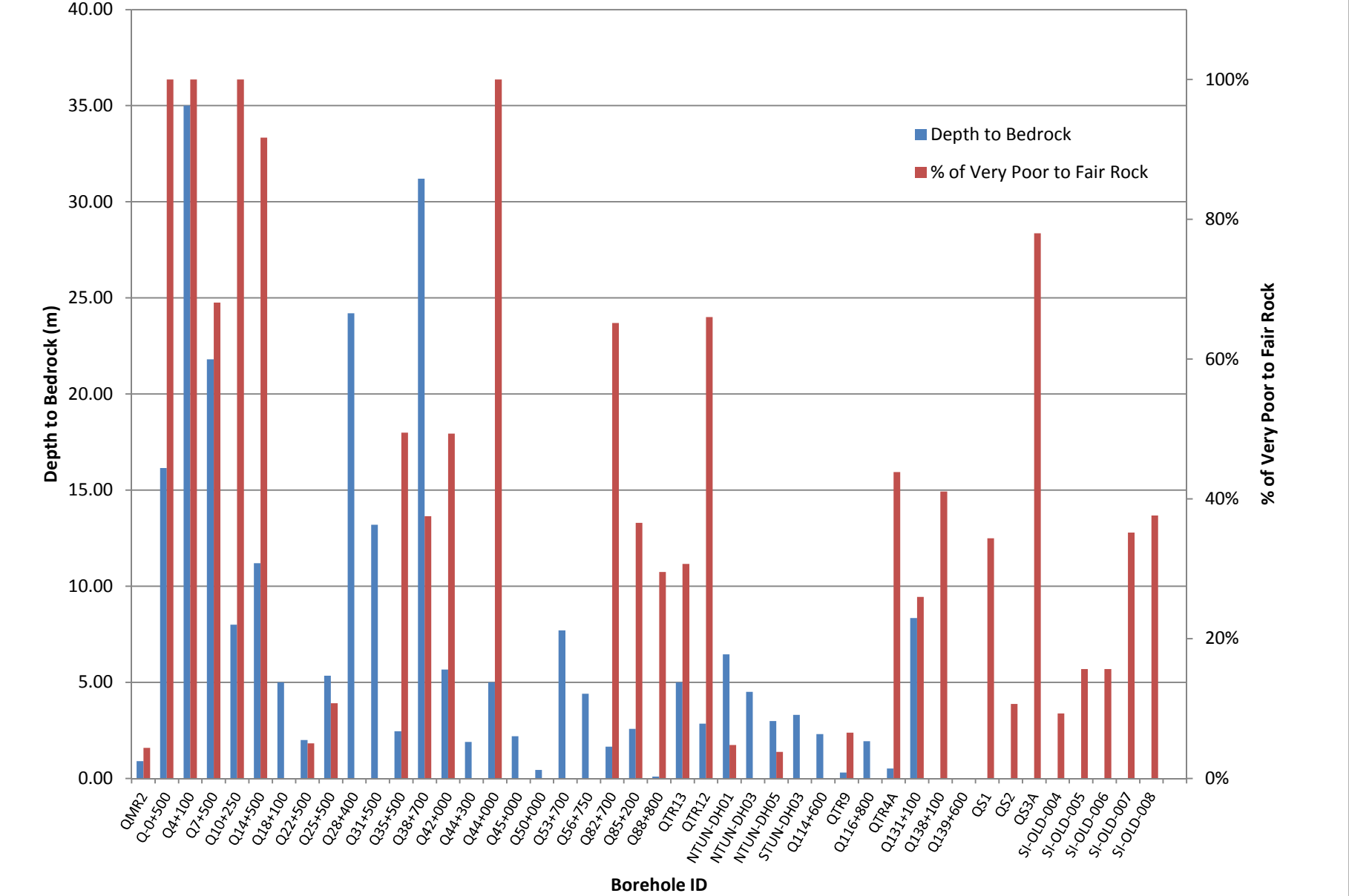
**Figure 1**

P/A NO.	REF NO.
	REV A

REV	DATE	DESCRIPTION	DESIGNED	DRAWN	CHK'D	APP'D

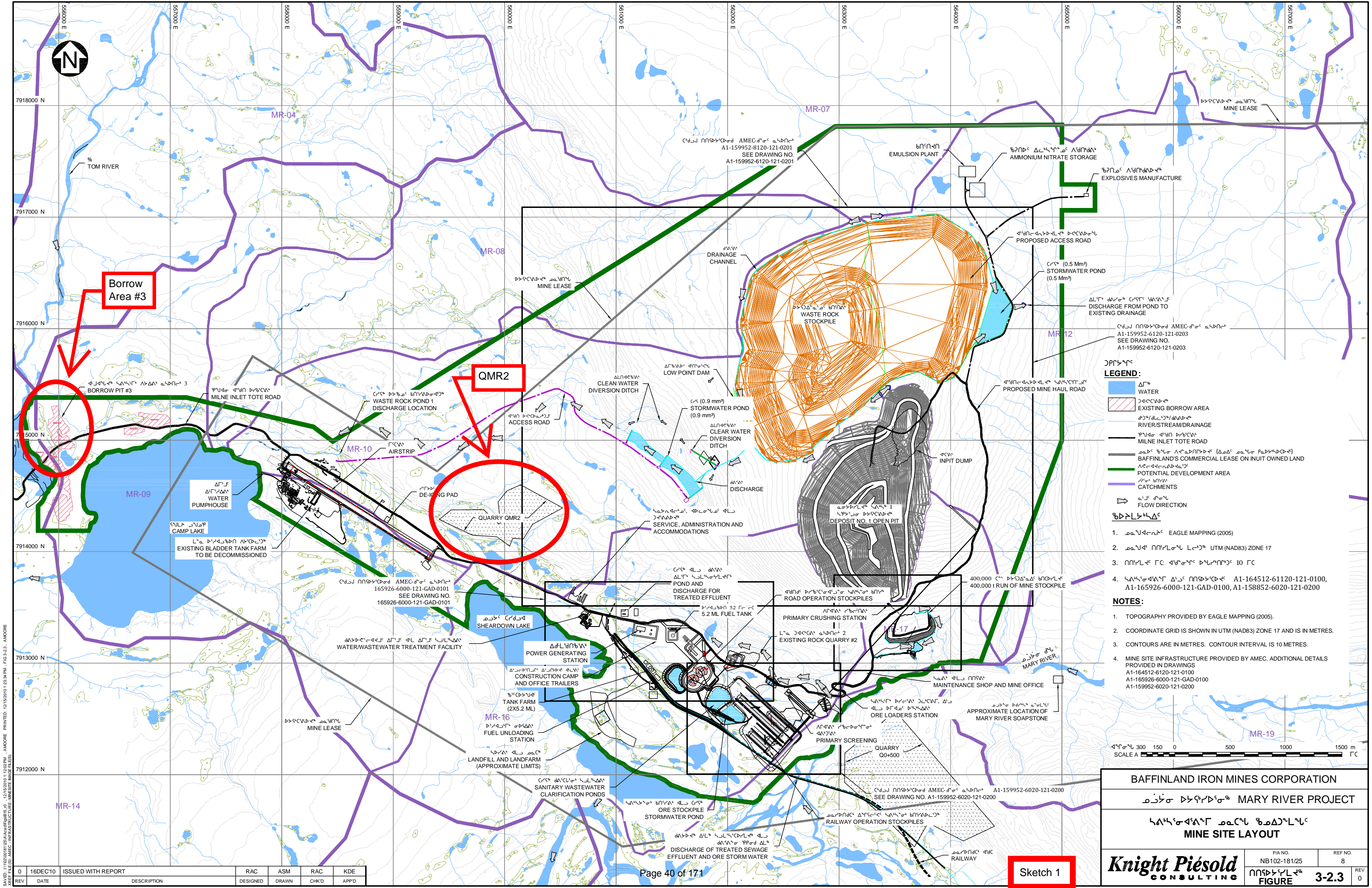


Figure 3: Bedrock Properties



## Appendix A: Sketches





**LEGEND:**

- Water
- Existing Borrow Area
- River/Stream/Drainage
- Milne Inlet Tote Road
- Baffinland's Commercial Lease on Inuit Owned Land
- Potential Development Area
- Catchments
- Flow Direction

**NOTES:**

- Topography provided by Eagle Mapping (2005).
- Coordinate grid is shown in UTM (NAD83) Zone 17 and is in metres.
- Contours are in metres. Contour interval is 10 metres.
- Mine site infrastructure provided by AMEC. Additional details provided in drawings:  
A1-164512-6120-121-0100  
A1-165926-6000-121-GAD-0100  
A1-159952-6020-121-0200

Baffinland Iron Mines Corporation

Mary River Project

Mine Site Layout

PIA NO.

REF NO.

NB102-181/25

8

Knight Piésold

CONSULTING

FIGURE

3-2.3

REV

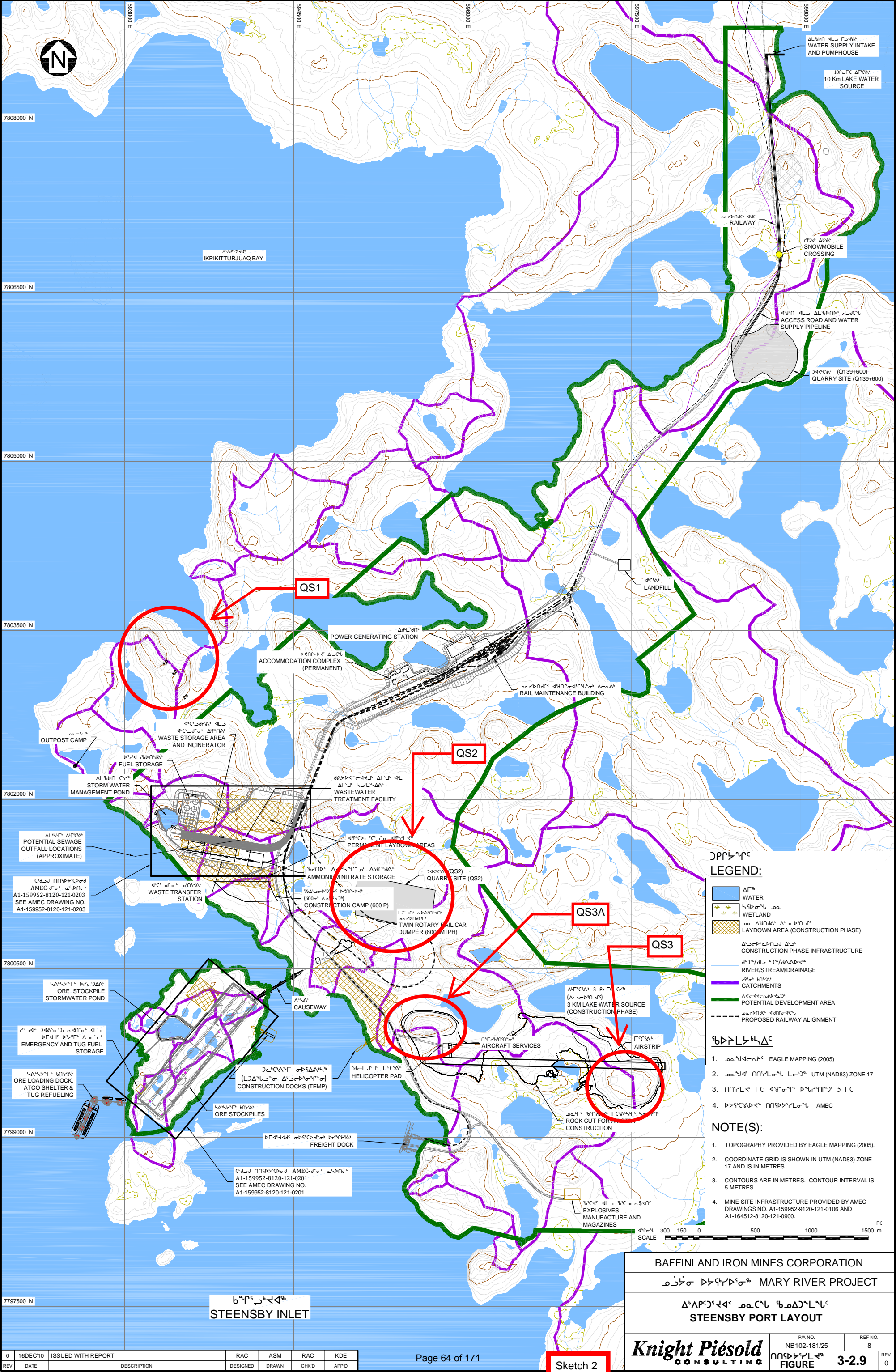
0

SAVED: 11/02/2010 12:34:34 PM - AMOORE PRINTED: 12/15/2010 11:22:02 PM - AMOORE FIG 3-2.3 - AMOORE  
FIG 3-2.3 - AMOORE - INFRASTRUCTURE LAYOUT - MARY RIVER PROJECT

REV	DATE	DESCRIPTION	DESIGNED	DRAWN	CHK'D	APP'D
0	16DEC'10	ISSUED WITH REPORT	RAC	ASM	RAC	KDE

Sketch 1





0	16DEC'10	ISSUED WITH REPORT	RAC	ASM	RAC	KDE
REV	DATE	DESCRIPTION	DESIGNED	DRAWN	CHK'D	APP'D

Baffinland Iron Mines Corporation

Mary River Project

Steensby Port Layout

**Knight Piésold**  
CONSULTING

P/A NO.  
NB102-181/25

REF NO.  
8

FIGURE

3-2.9

REV  
0

## Appendix B: Borehole Logs



# RECORD OF BOREHOLE QMR-2

PROJECT : Mary River Project  
 LOCATION : Mary River - Quarry  
 STARTED : August 7, 2011  
 COMPLETED : August 7, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 914 203 E 560 128

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	RECOVERY %						
		GROUND SURFACE		0.00									
1		COBBLES (< 100mm), granitic, fines washed out		0.90	1	RUN		TCR=70% SCR=70% RQD=70%				FI 1	0.90
2		GRANITIC GNEISS, slightly weathered to fresh, very strong, quartz-rich, grey										1	
3					2	RUN		TCR=100% SCR=100% RQD=95%				1	
4												1	
5												1	
6					3	RUN		TCR=100% SCR=100% RQD=100%				1	
7												1	
8												2	
9					4	RUN		TCR=100% SCR=100% RQD=90%				3	
10												1	
11												1	
12												1	
13					5	RUN		TCR=100% SCR=100% RQD=99%				1	
14		some quartz veins (100mm)										1	
15					6	RUN		TCR=100% SCR=100% RQD=94%				1	
16												2	
17		large plagioclase crystal at 17.0m to 17.69m										2	
18												2	
19					7	RUN		TCR=97% SCR=97% RQD=93%				2	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Boucher/Clarke  
 CHECKED : KS



# RECORD OF BOREHOLE QMR-2

PROJECT : Mary River Project  
 LOCATION : Mary River - Quarry  
 STARTED : August 7, 2011  
 COMPLETED : August 7, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 914 203 E 560 128

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040		
	NQ Diamond Drill											
21												
22					8	RUN		TCR=100% SCR=100% RQD=100%			1	
23											6	
24					9	RUN		TCR=100% SCR=100% RQD=94%				
25												
26		END OF BOREHOLE AT 26.00m.		26.00								
27												
28												
29												
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Boucher/Clarke  
 CHECKED : KS



# RECORD OF BOREHOLE Q-000+500

PROJECT : Mary River Project  
 LOCATION : -0+500 - Quarry  
 STARTED : July 30, 2011  
 COMPLETED : July 31, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 911 899 E 563 668

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	RECOVERY %				
		GROUND SURFACE		0.00							
		no recovery									
1					1	RUN	0				
2				2.15							
3		GRAVEL, granitic, fines washed out			2	RUN	13				
4											
5				5.15							
6		SAND and GRAVEL, trace to some silt, grey to brown			3	RUN	70				
7		layer of grey sand (120mm)									
8		ICE and SAND inclusions (Vs) at 6.50m to 7.00m									
9		some zones of frozen soil (Nbn) at 7.20m to 8.15m (Vr)			4	RUN	90				
10											
11		some cobbles									
12					5	RUN	57				
13											
14				14.15							
15		GRAVEL, sandy, trace to some silt and clay, some fractures with clay infilling, with completely weathered shale fragments, brown to red			6a	RUN	80				
16				16.15							
17		GRANITIC GNEISS, moderately weathered, some fibrous minerals, some clay gouges, red, grey			6b	RUN		TCR=92% SCR=75% RQD=13%			
18		clay at 17.12m (30mm)									
19		highly weathered, massive, highly broken, medium strong, reddish brown			7	RUN		TCR=53% SCR=15% RQD=7%			

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Clarke/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q-000+500

PROJECT : Mary River Project  
 LOCATION : -0+500 - Quarry  
 STARTED : July 30, 2011  
 COMPLETED : July 31, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 911 899 E 563 668

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMIST/ GROUND COND. <div>FROZEN<div></div> UNFROZEN<div></div> UNCERTAIN<div></div></div>	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT					
										wp	w	wl			ice
21	NQ Diamond Drill	residual to completely weathered, extremely to very weak, very dark reddish brown	<div></div>		8	RUN		TCR=50% SCR=10% RQD=0%							
22															
23		silty sand, trace clay (300mm)													
24		residual to highly weathered, very weak to weak													
25															
26															
27															
28		becoming slightly weathered, massive, medium strong, pinkish grey			10	RUN		TCR=67% SCR=29% RQD=22%							
29															
30															
30		END OF BOREHOLE AT 30.00m.		30.00	11	RUN		TCR=100% SCR=29%							
31															
32															
33															
34															
35															
36															
37															
38															
39															

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Clarke/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q004+100

PROJECT : Mary River Project  
 LOCATION : 4+100 - Quarry  
 STARTED : July 29, 2011  
 COMPLETED : July 30, 2011

DRILLER: BOART LONGYEAR  
 N 7 909 418 E 566 698

Project No. 19-1605-126

SHEET 1 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		<b>GRAVEL and COBBLES</b> (<160mm), granitic, angular to subrounded, fines washed out			1	RUN	25						
2		no recovery fines washed away											
3					2	RUN	0						
4													
5				5.11									
6		<b>SAND</b> , medium to coarse grained, some gravel (< 45mm), pink to grey											
7					3	RUN	32	Grain Size Analysis: Gr 16%/Sa 84%/ Si & Cl 0%					
8		gravelly											
9		ICE with soil inclusions at 9.00m to 9.23m			4	RUN	29						
10				9.61									
11		<b>GRAVEL</b> , with some zones of silty fine sand, some cobbles (180mm), some fines washed out			5	RUN							
12		boulders (270mm to 300mm) with some cobbles, medium to coarse sand			6	RUN	100						
13		cobbles (<170mm)			7	RUN	63						
14				14.00									
15		<b>SAND and GRAVEL</b> (<40mm), grey, some fines washed out			8	RUN	33						
16													
17				17.00									
18		<b>GRAVEL and COBBLES</b> , granitic, grey, fines washed out											
19					9	RUN	25						

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Clarke/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q004+100


PROJECT : Mary River Project  
 LOCATION : 4+100 - Quarry  
 STARTED : July 29, 2011  
 COMPLETED : July 30, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 909 418 E 566 698

SHEET 2 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.		
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040			10203040	
21	NQ Diamond Drill	no recovery		20.20										
22						10	RUN		0					
23														
24														
25														
26														
27														
28														
29														
30														
31		SILT, clayey, trace gravel, wet, brown		30.80	13	RUN		43						
32		SAND and GRAVEL, occasional boulders (< 300mm), with silty clay interbeds		32.00										
33														
34														
35		SANDSTONE, residual to completely weathered, extremely to very weak, brown		35.00										
36		completely to moderately weathered, massive, medium strong, light greyish brown			15	RUN			TCR=33% SCR=33% RQD=4%					
37														
38														
39		residual to highly weathered, medium strong			16	RUN			TCR=13% SCR=6% RQD=0%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Clarke/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q004+100




PROJECT : Mary River Project  
 LOCATION : 4+100 - Quarry  
 STARTED : July 29, 2011  
 COMPLETED : July 30, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 909 418 E 566 698

SHEET 3 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. <div><div>FROZEN</div><div>UNFROZEN</div><div>UNCERTAIN</div></div>	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT					
										wp	w	wl			ice
41	NQ Diamond Drill	highly broken						TCR=5% SCR=18% RQD=6%							
42															
43															
44															
45															
46															
47															
48															
49															
50															
51															
52															
53	moderately weathered							TCR=67% SCR=44% RQD=25%							
44															
45															
46															
47															
48															
49															
50															
51															
52															
53															
54															
55	slightly weathered, alternating brown and grey							TCR=81% SCR=58% RQD=21%							
48															
49															
50															
51															
52															
53															
54															
55															
56															
57															
58															
59															

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Clarke/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q004+100

PROJECT : Mary River Project  
 LOCATION : 4+100 - Quarry  
 STARTED : July 29, 2011  
 COMPLETED : July 30, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 909 418 E 566 698

SHEET 4 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040		
61					23	RUN		TCR=75% SCR=62% RQD=14%				
62		END OF BOREHOLE AT 62.00m.		62.00								
63												
64												
65												
66												
67												
68												
69												
70												
71												
72												
73												
74												
75												
76												
77												
78												
79												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Clarke/Ametrano  
 CHECKED : KS





# RECORD OF BOREHOLE Q007+500

PROJECT : Mary River Project  
 LOCATION : 7+500 - Quarry  
 STARTED : July 28, 2011  
 COMPLETED : July 29, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 907 667 E 569 432

Project No. 19-1605-126

SHEET 1 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE			SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT					ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. <div>FROZEN<div></div></div> <div>UNFROZEN<div></div></div> <div>UNCERTAIN<div></div></div>		
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %		WATER CONTENT, PERCENT								
										wp	ice	w	wl					
		GROUND SURFACE		0.00														
1	NQ Diamond Drill	COBBLES (<100mm), granitic, fines washed out, very poor recovery			1	RUN	8											
2																		
3		SAND, trace silt, trace gravel, trace cobbles, brown		3.00	2	RUN	33											
4		cobbles (~75mm)																
5																		
6																		
7																		
8		cobbles (<190mm)																
9																		
10																		
11																		
12		cobbles (<140mm)																
13																		
14																		
15																		
16						6	RUN		17									
17		BOULDER, sandstone, completely weathered, very weak, greyish brown		16.90														
18																		
19				7	RUN		TCR=100% SCR=43% RQD=28%											
	20.00																	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q007+500

PROJECT : Mary River Project  
 LOCATION : 7+500 - Quarry  
 STARTED : July 28, 2011  
 COMPLETED : July 29, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 907 667 E 569 432

Project No. 19-1605-126

SHEET 2 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
21	NQ Diamond Drill	SILT, clayey, trace sand, firm, grey, moist		21.80	8	RUN	100						
22		SANDSTONE, moderately to faintly weathered, medium strong to strong, light greyish brown											
23												3	
24												3	
25		faintly weathered, strong			9	RUN		TCR=100% SCR=95% RQD=57%				2	
26												2	
27		vertical joints from 27.7m to 27.9m										0	
28					10	RUN		TCR=100% SCR=93% RQD=81%				2	
29												1	
30		vertical joints from 29.7m to 30.0m, 31.4m to 31.6m										6	
31					11	RUN		TCR=100% SCR=65% RQD=57%				7	
32												3	
33		faintly weathered to fresh										5	
34					12	RUN		TCR=100% SCR=97% RQD=92%				1	
35		faintly weathered, closely to moderately spaced, medium strong										1	
36												2	
37					13	RUN		TCR=100% SCR=59% RQD=49%				2	
38												8	
39					14	RUN		TCR=87% SCR=72% RQD=34%				>25	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q007+500

PROJECT : Mary River Project  
 LOCATION : 7+500 - Quarry  
 STARTED : July 28, 2011  
 COMPLETED : July 29, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 907 667 E 569 432

Project No. 19-1605-126

SHEET 3 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT		
	NQ Diamond Drill											
41												
42												
43					15	RUN			TCR=100% SCR=46% RQD=30%			
44												
45					16	RUN			TCR=100% SCR=94% RQD=78%			
46												
47												
48					17	RUN			TCR=100% SCR=95% RQD=74%			
49												
50		END OF BOREHOLE AT 50.00m.		50.00								
51												
52												
53												
54												
55												
56												
57												
58												
59												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q010+250

PROJECT : Mary River Project  
 LOCATION : 10+250 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 905 378 E 572 883

Project No. 19-1605-126

SHEET 1 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMIST/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		SILT, sandy, trace gravel (inferred), occasional boulders (<400mm) and cobbles, coarse to medium grained, subrounded, grey to light brown and pink, fines washed out			1	RUN	49						
2													
3		cobbles (100mm to 200mm)			2	RUN	47						
4													
5		sandy silt, trace gravel from 5.0m to 5.5m											
6													
7					3	RUN	48						
8				8.00									
9		GRANITIC GNEISS, moderately weathered, medium to coarse grained, highly to moderately fractured, grey to pink			4	RUN		TCR=100% SCR=100% RQD=63%					
10													
11													
12					5	RUN		TCR=100% SCR=100% RQD=73%					
13													
14													
15													
16					6	RUN		TCR=100% SCR=100% RQD=47%					
17													
18													
19					7	RUN		TCR=100% SCR=100% RQD=37%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q010+250

PROJECT : Mary River Project  
 LOCATION : 10+250 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 905 378 E 572 883

Project No. 19-1605-126

SHEET 2 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND. FROZEN UNFROZEN UNCERTAIN		
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT						
										wp	w	wl			ice	
21	NQ Diamond Drill	moderately to slightly weathered, closely to moderately spaced, very to extremely strong			8	RUN			TCR=100% SCR=100% RQD=60%							
22																
23																
24																
25																
26																
27																
28																
29																
30						moderately weathered										
31																
32																
33																
34																
35																
36																
37																
38																
39																

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q010+250

PROJECT : Mary River Project  
 LOCATION : 10+250 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 905 378 E 572 883

Project No. 19-1605-126

SHEET 3 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10			20
41		END OF BOREHOLE AT 41.00m.		41.00								2	
42												>25	
43												2	
44													
45													
46													
47													
48													
49													
50													
51													
52													
53													
54													
55													
56													
57													
58													
59													

## GROUNDWATER ELEVATIONS

SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q014+500

PROJECT : Mary River Project  
 LOCATION : 14+500 - Quarry  
 STARTED : July 24, 2011  
 COMPLETED : July 24, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 904 382 E 575 868

Project No. 19-1605-126

SHEET 1 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				THERMIST/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %					
		GROUND SURFACE		0.00									
1		BOULDERS and COBBLES, grey to pink, fines washed out			1	RUN		47					
2													
3					2	RUN		100					
4													
5		BOULDERS, granitic, dark brown to red, highly weathered, highly fractured		5.20									
6													
7					3	RUN		100					
8													
9		clayey silt, some gravel, brown, moist from 9.50m to 9.70m			4	RUN		100					
10													
11		GRANITIC GNEISS, medium to coarse grained, highly weathered, highly fractured, brownish red to pink		11.20									
12					5	RUN			TCR=100% SCR=43% RQD=43%				
13		moderately weathered, moderately spaced											
14					6	RUN			TCR=100% SCR=100% RQD=37%				
15													
16													
17													
18					7	RUN			TCR=100% SCR=100% RQD=53%				
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q014+500

PROJECT : Mary River Project  
 LOCATION : 14+500 - Quarry  
 STARTED : July 24, 2011  
 COMPLETED : July 24, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 904 382 E 575 868

Project No. 19-1605-126

SHEET 2 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	RECOVERY %						
21		slightly weathered			8	RUN		TCR=100% SCR=100% RQD=70%					
22													
23													
24					9	RUN		TCR=100% SCR=100% RQD=47%					
25													
26													
27					10	RUN		TCR=100% SCR=100% RQD=77%					
28													
29		slightly to moderately weathered			11	RUN		TCR=100% SCR=100% RQD=45%					
30													
31													
32					12	RUN		TCR=100%					
33													
34													
35													
36					13	RUN		TCR=100%					
37													
38		strong to very strong, massive											
39					14	RUN		TCR=100% SCR=49% RQD=40%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS





# RECORD OF BOREHOLE Q014+500


PROJECT : Mary River Project  
 LOCATION : 14+500 - Quarry  
 STARTED : July 24, 2011  
 COMPLETED : July 24, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 904 382 E 575 868

Project No. 19-1605-126

SHEET 3 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040		
41	Diamond Drill	slightly weathered		47.20				TCR=100% SCR=77% RQD=62%			5	
42										2		
43					15	RUN				3		
44										1		
45										8		
46					2	5				1		
47					16	RUN		TCR=100% SCR=66% RQD=41%			3	
48		END OF BOREHOLE AT 47.20m.									2	
49												
50												
51												
52												
53												
54												
55												
56												
57												
58												
59												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q018+100

PROJECT : Mary River Project  
 LOCATION : 18+100 - Quarry  
 STARTED : July 23, 2011  
 COMPLETED : July 23, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 902 853 E 578 804

Project No. 19-1605-126

SHEET 1 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
		GROUND SURFACE		0.00										
1		GRAVEL, some cobbles, granitic, grey, fines washed out			1	RUN		50						
2		cobbles (<110mm)												
3					2	RUN		50						
4														
5		SANDSTONE, slightly to moderately weathered, weak to medium strong, bedded, brown		5.00										
6					3	RUN			TCR=100%					
7														
8														
9														
10		becoming reddish brown			4	RUN			TCR=100%					
11														
12														
13					5	RUN			TCR=100%					
14														
15														
16					6	RUN			TCR=100%					
17														
18														
19		uncemented sand interbeds from 19.2m to 19.3m and 19.8m to 19.9m			7	RUN			TCR=100%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ametrano  
 CHECKED : MB



# RECORD OF BOREHOLE Q018+100


PROJECT : Mary River Project  
 LOCATION : 18+100 - Quarry  
 STARTED : July 23, 2011  
 COMPLETED : July 23, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 902 853 E 578 804

Project No. 19-1605-126

SHEET 2 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN		
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040			10203040	
21	NQ Diamond Drill				8	RUN			TCR=100%					
22														
23														
24														
25														
26														
27														
28														
29														
30														
31														
32														
33														
34														
35														
36														
37														
38														
39														

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ametrano  
 CHECKED : MB



# RECORD OF BOREHOLE Q018+100

PROJECT : Mary River Project  
 LOCATION : 18+100 - Quarry  
 STARTED : July 23, 2011  
 COMPLETED : July 23, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 902 853 E 578 804

Project No. 19-1605-126

SHEET 3 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10			20
41		END OF BOREHOLE AT 41.00m.		41.00									
42													
43													
44													
45													
46													
47													
48													
49													
50													
51													
52													
53													
54													
55													
56													
57													
58													
59													

## GROUNDWATER ELEVATIONS

SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ametrano  
 CHECKED : MB



# RECORD OF BOREHOLE Q022+500

PROJECT : Mary River Project  
 LOCATION : 22+500 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 901 663 E 583 415

Project No. 19-1605-126

SHEET 1 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		<b>BOULDERS</b> , granitic, moderately weathered, moderately spaced, grey to pink			1	RUN	75						
2		<b>GRANITIC GNEISS</b> , fresh, moderately fractured, medium to coarse grained, grey to pink		2.00									
3					2	RUN		TCR=100% SCR=100% RQD=80%					
4													
5													
6					3	RUN		TCR=100% SCR=100% RQD=100%					
7													
8													
9					4	RUN		TCR=100% SCR=100% RQD=98%					
10													
11		moderately weathered											
12													
13					5	RUN		TCR=100% SCR=100% RQD=73%					
14		faintly weathered											
15													
16					6	RUN		TCR=100% SCR=100% RQD=98%					
17		fresh											
18													
19					7	RUN		TCR=100% SCR=100% RQD=97%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q022+500

PROJECT : Mary River Project  
 LOCATION : 22+500 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 901 663 E 583 415

Project No. 19-1605-126

SHEET 2 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
21					8	RUN		TCR=100% SCR=100% RQD=80%					
22													
23		slightly fractured											
24					9	RUN		TCR=100% SCR=100% RQD=95%					
25													
26													
27					10	RUN		TCR=100% SCR=100% RQD=92%					
28													
29													
30					11	RUN		TCR=100% SCR=100% RQD=89%					
31													
32													
33					12	RUN		TCR=100% SCR=100% RQD=92%					
34													
35													
36					13	RUN		TCR=100% SCR=100% RQD=90%					
37													
38													
39					14	RUN		TCR=100% SCR=100% RQD=93%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q022+500

PROJECT : Mary River Project  
 LOCATION : 22+500 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 901 663 E 583 415

Project No. 19-1605-126

SHEET 3 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
41													
42					15	RUN		TCR=100% SCR=100% RQD=92%					
43													
44													
45					16	RUN		TCR=100% SCR=100% RQD=90%					
46													
47													
48					17	RUN		TCR=100% SCR=100% RQD=92%					
49													
50													
51					18	RUN		TCR=100% SCR=100% RQD=95%					
52													
53													
54					19	RUN		TCR=100% SCR=100% RQD=97%					
55													
56													
57					20	RUN		TCR=100% SCR=100% RQD=100%					
58													
59													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q022+500

PROJECT : Mary River Project  
 LOCATION : 22+500 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 901 663 E 583 415

Project No. 19-1605-126

SHEET 4 OF 4

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10			20
61					21	RUN		TCR=100% SCR=100% RQD=97%					
62		END OF BOREHOLE AT 62.00m.		62.00									
63													
64													
65													
66													
67													
68													
69													
70													
71													
72													
73													
74													
75													
76													
77													
78													
79													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS





# RECORD OF BOREHOLE Q025+500



PROJECT : Mary River Project  
 LOCATION : 25+500 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 900 221 E 586 954

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTOR/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		BOULDERS, granitic, medium to coarse grained, trace cobbles, subrounded, dark grey to grey and pink, moderately fractured			1	RUN	76						
2													
3													
4					2	RUN		TCR=100% SCR=100% RQD=95%					
5													
6		GRANITIC GNEISS, slightly weathered, medium to coarse grained, slightly fractured, grey to pink		5.34									
7					3	RUN		TCR=100% SCR=100% RQD=95%					
8													
9		fresh											
10					4	RUN		TCR=100% SCR=100% RQD=93%					
11													
12													
13					5	RUN		TCR=100% SCR=100% RQD=97%					
14													
15													
16					6	RUN		TCR=100% SCR=100% RQD=100%					
17													
18		very thickly foliated, very to extremely strong											
19		horizontal break at 18.2m and 19.9m			7	RUN		TCR=100% SCR=85% RQD=79%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q025+500

PROJECT : Mary River Project  
 LOCATION : 25+500 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 900 221 E 586 954

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN <div></div> UNFROZEN <div></div> UNCERTAIN <div></div>			
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT							
										wp	w	wl			ice		
21	NQ Diamond Drill	dark greyish to black foliations from 23.34m to 23.54m and 23.86m to 24.13m	<div></div>		8	RUN		TCR=100% SCR=97% RQD=95%						1			
22														1			
23														0			
24														0			
25														1			
26														1			
27														0			
28														3			
29														0			
30														1			
31														1			
32	2	1	0	0	1	0	0	0	0	2	3	>25	>25	0	1	1	
33		END OF BOREHOLE AT 32.34m.															
34																	
35																	
36																	
37																	
38																	
39																	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Ametrano  
 CHECKED : KS



# RECORD OF BOREHOLE Q028+400

PROJECT : Mary River Project  
 LOCATION : 28+400 - Quarry  
 STARTED : July 18, 2011  
 COMPLETED : July 18, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 898 617 E 588 240

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.			
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT				FROZEN <div></div>	UNFROZEN <div></div>	UNCERTAIN <div></div>	
		GROUND SURFACE		0.00													
1	NQ Diamond Drill	GRAVEL, occasional cobbles and boulders, grey to pink, subangular to subrounded, fines washed out	<div></div>		1	RUN	15										
2				2.40													
3		no recovery															
4					2	RUN	0										
5				5.40													
6		GRAVEL, some cobbles, occasional boulders (<480mm), granitic, angular to subangular, grey to pink	<div></div>		3	RUN	25										
7				8.40													
8																	
9		SAND, some silt, some gravel, brown, frozen (Nbn)	<div></div>		4	RUN	57										
10				11.40													
11																	
12		SAND and GRAVEL (INFERRED), with cobbles and boulders (350mm), grey to pink, angular to subangular	<div></div>		5	RUN	41										
13																	
14		coarse sand, some gravel, some cobbles, trace silt, frozen (Nf)	<div></div>		6	RUN	73										
15																	
16																	
17																	
18		COBBLES and BOULDERS, granitic, trace to some coarse sand, grey to pink	<div></div>	18.00													
19			<div></div>		7	RUN	100										

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q028+400

PROJECT : Mary River Project  
 LOCATION : 28+400 - Quarry  
 STARTED : July 18, 2011  
 COMPLETED : July 18, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 898 617 E 588 240

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10 20 30 40		
21												
22					8	RUN	71					
23												
24					9	RUN	100					
24.20		END OF BOREHOLE AT 24.20m.										
25												
26												
27												
28												
29												
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q031+500



PROJECT : Mary River Project  
 LOCATION : 31+500 - Quarry  
 STARTED : July 19, 2011  
 COMPLETED : July 19, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 897 863 E 590 944

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND. <div>FROZEN<div></div></div> <div>UNFROZEN<div></div></div> <div>UNCERTAIN<div></div></div>		
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp</div><div></div><div>w</div><div></div><div>wl</div><div>10203040</div></div>						
		GROUND SURFACE		0.00												
1	NQ Diamond Drill	<b>BOULDERS</b> and <b>COBBLES</b> , some gravel, pink to dark grey, angular to sub-angular, fines washed out			1	RUN	54									
2																
3		no recovery														
4					2	RUN	0									
5																
6		<b>GRAVEL</b> and <b>COBBLES</b> , some boulders, trace sand, pink to light grey and dark grey, fines washed out			3	RUN	66									
7																
8																
9																
10					4	RUN	76									
11																
12					5	RUN	66									
13		END OF BOREHOLE AT 13.20m.		13.20										13.20		
14																
15																
16																
17																
18																
19																

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q035+500

PROJECT : Mary River Project  
 LOCATION : 35+500 - Quarry  
 STARTED : July 17, 2011  
 COMPLETED : July 17, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 896 244 E 595 477

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN <div><div></div></div> UNFROZEN <div><div></div></div> UNCERTAIN <div><div></div></div>
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp</div><div></div><div>w</div><div></div><div>wl</div></div>				
		GROUND SURFACE		0.00										
		COBBLES, organics, subangular to subrounded		0.30										
1		BOULDERS, trace cobbles, slightly weathered, fines washed out			1	RUN		70						
2														
3		GRANITIC GNEISS, medium to coarse grained, slightly weathered, highly to moderately jointed, dark grey to pink		2.45										2.45
4					2	RUN			TCR=83% SCR=83% RQD=47%					
5														
6														
7					3	RUN			TCR=100% SCR=100% RQD=53%					
8														
9														
10					4	RUN			TCR=100% SCR=100% RQD=45%					
11														
12														
13					5	RUN			TCR=100% SCR=100% RQD=88%					
14														
15														
16					6	RUN			TCR=100% SCR=100% RQD=77%					
17														
18		becoming fresh, slightly jointed												
19					7	RUN			TCR=100% SCR=100% RQD=78%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q035+500

PROJECT : Mary River Project  
 LOCATION : 35+500 - Quarry  
 STARTED : July 17, 2011  
 COMPLETED : July 17, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 896 244 E 595 477

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040		
21		slightly weathered, closely jointed	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div>&lt;</div>									

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia/Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q038+700

PROJECT : Mary River Project  
 LOCATION : 38+700 - Quarry  
 STARTED : July 16, 2011  
 COMPLETED : July 16, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 893 140 E 596 368

Project No. 19-1605-126

SHEET 1 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.																						
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT				FROZEN <div></div>	UNFROZEN <div></div>	UNCERTAIN <div></div>																				
								wp ———— w ———— wl																												
		GROUND SURFACE		0.00																																
1	NQ Diamond Drill	GRAVEL, COBBLES and BOULDERS, granitic, angular to subangular, grey, fines washed out	<div></div>	2.40	1	RUN		28																												
2																																				
3		no recovery, all material washed out		5.40	2	RUN		0																												
4																																				
5		GRAVEL, trace sand, grey, very poor recovery, most material washed	<div></div>	11.40	3	RUN		4																												
6																																				
7																																				
8																																				
9		no recovery		17.40	4	RUN		8																												
10																																				
11																																				
12																																				
13					5	RUN		0																												
14																																				
15																																				
16																																				
17					6	RUN		0																												
18																																				
19																																				
		GRAVEL, some cobbles (<105mm), granitic, subangular, grey, fines washed out	<div></div>		7	RUN		14																												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS





# RECORD OF BOREHOLE Q038+700

PROJECT : Mary River Project  
 LOCATION : 38+700 - Quarry  
 STARTED : July 16, 2011  
 COMPLETED : July 16, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 893 140 E 596 368

Project No. 19-1605-126

SHEET 2 OF 3

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN <div><div></div></div> UNFROZEN <div><div></div></div> UNCERTAIN <div><div></div></div>
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT wp <div><div></div><div>10203040</div></div> w <div><div></div><div>10203040</div></div> wl				
	NQ Diamond Drill		<div><div></div></div>	20.40										
21		no recovery												
22		artesian condition encountered (2m above ground surface) at 22.0m			8	RUN		0						
23														
24		GRAVEL, some cobbles (<70mm), granitic, subrounded to subangular, fines washed out	<div><div></div></div>	23.40										
25					9	RUN		4						
26														
27														
28					10	RUN		10						
29														
30		SAND, some gravel, some silt, some cobbles, brown	<div><div></div></div>	29.40										
31					11	RUN		84						
32		GRANITIC GNEISS, slightly to faintly weathered, medium to coarse grained, closely jointed, pink to grey	<div><div></div></div>	31.20										
33														
34				12	RUN			TCR=100% SCR=100% RQD=64%						
35														
36														
37				13	RUN			TCR=100% SCR=93% RQD=57%						
38														
39	becoming fresh, moderately to widely jointed	<div><div></div></div>												
					14	RUN			TCR=100% SCR=100% RQD=78%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q038+700

PROJECT : Mary River Project  
LOCATION : 38+700 - Quarry  
STARTED : July 16, 2011  
COMPLETED : July 16, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR, LM-55  
N 7 893 140 E 596 368

SHEET 3 OF 3

DATUM: CGVD28

[illegible]

## GROUNDWATER ELEVATIONS

 SHALLOW/SINGLE INSTALLATION  
WATER LEVEL (date)

 DEEP/DUAL INSTALLATION  
WATER LEVEL (date)

LOGGED : Schneider  
CHECKED : KS



THURBER2S(5126) 5126.GPJ 11/9/11

# RECORD OF BOREHOLE Q042+000

PROJECT : Mary River Project  
 LOCATION : 42+000 - Quarry  
 STARTED : July 16, 2011  
 COMPLETED : July 16, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 890 881 E 598 151

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. <div><div>FROZEN</div><div>UNFROZEN</div><div>UNCERTAIN</div></div>
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT wp ———— w ———— wl 10    20    30    40				

		GROUND SURFACE		0.00													
1	NQ Diamond Drill	GRAVEL, COBBLES and BOULDERS, granitic, subrounded to subangular, grey to pink, fines washed out			1	RUN	11										
2																	
3																	
4																	
5																	
6		GRANITIC GNEISS, fresh, medium to coarse grained, moderately spaced joints, grey to pink		5.67											3	RUN	TCR=100% SCR=82% RQD=65%
7																	
8																	
9		moderately to widely spaced															
10							4								RUN	TCR=80% SCR=71% RQD=59%	
11																	
12		widely spaced															
13							5								RUN	TCR=100% SCR=100% RQD=74%	
14																	
15																	
16							6								RUN	TCR=100% SCR=100% RQD=82%	
17																	
18																	
19							7								RUN	TCR=100% SCR=100% RQD=87%	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q042+000

PROJECT : Mary River Project  
 LOCATION : 42+000 - Quarry  
 STARTED : July 16, 2011  
 COMPLETED : July 16, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 890 881 E 598 151

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. <div>FROZEN<div></div> UNFROZEN<div></div> UNCERTAIN<div></div></div>
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp</div><div></div><div>w</div><div></div><div>wl</div><div>10203040</div></div>		
21												
22					8	RUN		TCR=100% SCR=100% RQD=78%				
23				23.37								
24		END OF BOREHOLE AT 23.37m.										
25												
26												
27												
28												
29												
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q044+300


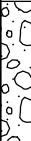

PROJECT : Mary River Project  
 LOCATION : 44+300 - Quarry  
 STARTED : July 15, 2011  
 COMPLETED : July 15, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 888 054 E 598 208


Project No. 19-1605-126


SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT 	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.									
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT													
		GROUND SURFACE		0.00																			
1	NQ Diamond Drill	GRAVEL and COBBLES, some sand, trace silt, brown		1.90	1	RUN	65																
2		GRANITIC GNEISS, slightly weathered, medium to coarse grained, closely jointed, grey to pink																					
3																							
4		2														RUN	TCR=100% SCR=100% RQD=80%						
5																							
6																							
7		3														RUN	TCR=100% SCR=99% RQD=81%						
8																							
9		becoming faintly weathered, moderately to widely jointed																					
10																					4	RUN	TCR=99% SCR=99% RQD=81%
11																							
12																							
13		5														RUN	TCR=100% SCR=98% RQD=84%						
14																							
15	END OF BOREHOLE AT 14.40m.		14.40																				
16																							
17																							
18																							
19																							

## GROUNDWATER ELEVATIONS

 SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

 DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q044+000



PROJECT : Mary River Project  
 LOCATION : 44+000 - Quarry  
 STARTED : July 15, 2011  
 COMPLETED : July 15, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 885 927 E 596 138

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		GRAVEL and COBBLES, granitic, subrounded to subangular, grey, fines washed out			1	RUN	9						
2													
3		GRANITIC GNEISS, slightly weathered, medium grained, closely spaced joints, grey to pink  becoming fresh, moderately spaced		5.00	2	RUN	26						
4													
5					3	RUN		TCR=93% SCR=50% RQD=50%					
6													
7													
8					4	RUN		TCR=97% SCR=97% RQD=55%					
9													
10					5	RUN		TCR=98% SCR=98% RQD=57%					
11													
12													
13													
14					6	RUN		TCR=100% SCR=25% RQD=7%					
15													
16													
17													
18					7	RUN		TCR=100% SCR=92% RQD=59%					
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q044+000

PROJECT : Mary River Project  
 LOCATION : 44+000 - Quarry  
 STARTED : July 15, 2011  
 COMPLETED : July 15, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 885 927 E 596 138

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTOR/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10			20
		moderately to widely spaced											
21					8	RUN		TCR=100% SCR=95% RQD=64%					
22													
23		END OF BOREHOLE AT 23.00m.		23.00									
24													
25													
26													
27													
28													
29													
30													
31													
32													
33													
34													
35													
36													
37													
38													
39													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q045+000

PROJECT : Mary River Project  
 LOCATION : 45+000 - Quarry  
 STARTED : July 14, 2011  
 COMPLETED : July 14, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 884 724 E 596 201

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
		GRAVEL, light grey to pink, sub-angular to sub-rounded		0.11									
1		COBBLES and BOULDERS, some gravel, granitic, angular, grey			1	RUN	79						
2				2.20									
3		GRANITIC GNEISS, faintly weathered, fine to medium grained, moderately to widely jointed, black to light grey			2	RUN		TCR=99% SCR=98% RQD=89%					
4													
5													
6													
7					3	RUN		TCR=100% SCR=100% RQD=85%					
8													
9													
10					4	RUN		TCR=100% SCR=100% RQD=89%					
11													
12													
13					5	RUN		TCR=100% SCR=97% RQD=88%					
14		END OF BOREHOLE AT 14.30m.		14.30									
15													
16													
17													
18													
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS





# RECORD OF BOREHOLE Q050+000

PROJECT : Mary River Project  
 LOCATION : 50+000 - Quarry  
 STARTED : July 14, 2011  
 COMPLETED : July 14, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 881 100 E 597 357

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
		GRAVEL and BOULDER		0.44									0.44
1		GRANITIC GNEISS, slightly weathered, medium to coarse grained, strong, black, grey, red			1	RUN	87						
2													
3					2	RUN		TCR=100% SCR=91% RQD=84%					
4													
5													
6					3	RUN		TCR=100% SCR=85% RQD=93%					
7													
8		becoming fresh, moderately jointed											
9					4	RUN		TCR=97% SCR=97% RQD=78%					
10													
11		closely jointed											
12					5	RUN		TCR=99% SCR=99% RQD=89%					
13													
14		faintly weathered, closely to moderately jointed											
15					6	RUN		TCR=100% SCR=99% RQD=84%					
16													
17		END OF BOREHOLE AT 16.90m.		16.90									
18													
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Van Luver/Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q053+700

PROJECT : Mary River Project  
 LOCATION : 53+700 - Quarry  
 STARTED : July 13, 2011  
 COMPLETED : July 13, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 877 567 E 597 616

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		<b>COBBLES</b> and <b>BOULDERS</b> , some gravel, granitic, angular to subangular, grey			1	RUN	43						
2		<b>SAND</b> and <b>GRAVEL</b> , some cobbles, some silt, wet, brown, some fines washed out		2.20									
3					2	RUN	57						
4													
5		<b>BOULDERS</b> and <b>COBBLES</b> , angular to subangular, some gravel, pink and dark grey, some fines washed out		4.70									
6					3	RUN	76						
7													
8		<b>GRANITIC GNEISS</b> , faintly weathered to fresh, medium to coarse grained, moderately jointed, grey to pink		7.70									
9					4	RUN		TCR=100% SCR=100% RQD=100%					
10													
11													
12					5	RUN		TCR=98% SCR=98% RQD=86%					
13													
14													
15					6	RUN		TCR=99% SCR=99% RQD=96%					
16													
17													
18													
19					7	RUN		TCR=100% SCR=100% RQD=98%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Van Luver/Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q053+700

PROJECT : Mary River Project  
 LOCATION : 53+700 - Quarry  
 STARTED : July 13, 2011  
 COMPLETED : July 13, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 877 567 E 597 616

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040		
21	NQ Diamond Drill				8	RUN		TCR=100% SCR=100% RQD=90%				
22												
23												
24												
25				9	RUN		TCR=98% SCR=98% RQD=88%					
26												
27												
28				10	RUN		TCR=100% SCR=100% RQD=99%					
29		END OF BOREHOLE AT 29.20m.		29.20								
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Van Luver/Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q056+750

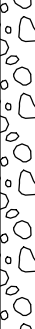


PROJECT : Mary River Project  
 LOCATION : 56+750 - Quarry  
 STARTED : July 11, 2011  
 COMPLETED : July 11, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 875 280 E 598 852

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.  FROZEN <div></div> UNFROZEN <div></div> UNCERTAIN <div></div>	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp</div><div></div><div>wl</div><div>10203040</div></div>					
		GROUND SURFACE		0.00											
1	NQ Diamond Drill	COBBLES and BOULDERS, some gravel, angular to subangular, pink to dark grey			1	RUN	78								
2															
3															
4															
5		GRANITIC GNEISS, faintly weathered, closely jointed, medium grained, pink to grey		4.41	2	RUN	100								
6															
7															
8		fresh, moderately jointed, medium to very coarse grained, pink to dark grey			3	RUN									TCR=100% SCR=100% RQD=76%
9															
10															
11		END OF BOREHOLE AT 11.20m.			4	RUN									TCR=100% SCR=100% RQD=87%
12															
13															
14															
15															
16															
17															
18															
19															

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Schneider  
 CHECKED : KS



# RECORD OF BOREHOLE Q082+700

PROJECT : Mary River Project  
 LOCATION : 82+700 - Quarry  
 STARTED : July 22, 2011  
 COMPLETED : July 22, 2011

DRILLER: WALKER DRILLING, D-50  
 N 7 852 449 E 605 710

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		GRAVEL, subrounded to subangular, pinkish grey			1	RUN	62						
2		GRANITIC GNEISS, slightly weathered, fine grained, highly fractured, grey to pink		1.65									
3					2	RUN		TCR=100% SCR=58% RQD=0%					
4					3	RUN		TCR=100% SCR=92% RQD=42%					
5		occasional quartz seams			4	RUN		TCR=100% SCR=100% RQD=67%					
6					5	RUN		TCR=97% SCR=97% RQD=82%					
7					6	RUN		TCR=100% SCR=100% RQD=52%					
8					7	RUN		TCR=100% SCR=100% RQD=52%					
9					8	RUN		TCR=97% SCR=29% RQD=29%					
10		highly weathered, highly fractured			9	RUN		TCR=99% SCR=72% RQD=52%					
11					10	RUN		TCR=100% SCR=100% RQD=100%					
12													
13													
14													
15													
16		END OF BOREHOLE AT 16.00m.		16.00									
17													
18													
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill  
 CHECKED : KS



# RECORD OF BOREHOLE Q085+200

PROJECT : Mary River Project  
LOCATION : 85+200 - Quarry  
STARTED : July 20, 2011  
COMPLETED : July 21, 2011

DRILLER: WALKER DRILLING, D-50  
N 7 850 087 E 606 073

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		GRAVEL and COBBLES, granitic, red/black/grey, fines washed out			1	RUN	45						
2					2	RUN	0						
				2.58	3a	RUN	33						
3		GRANITE, highly weathered, highly fractured, red/black			3b	RUN		TCR=100% SCR=0% RQD=0%					
4		vertical joint			4	RUN		TCR=95% SCR=22% RQD=12%					
5				5.40	5	RUN		TCR=46% SCR=0% RQD=0%					
6		GRANITIC GNEISS, slightly weathered, moderately fractured, grey, black, red			6	RUN		TCR=86% SCR=82% RQD=56%					
7					7	RUN		TCR=100% SCR=96% RQD=96%					
					8	RUN		TCR=100% SCR=100% RQD=100%					
					9	RUN		TCR=100% SCR=100% RQD=100%					
					10	RUN		TCR=100% SCR=98% RQD=98%					
8					11	RUN		TCR=100% SCR=45% RQD=36%					
9					12	RUN		TCR=99% SCR=99% RQD=92%					
10		moderately weathered, medium strong, grey/white and black, medium to coarse grained			13	RUN		TCR=100% SCR=87% RQD=75%					
11					14	RUN		TCR=100% SCR=78% RQD=78%					
12					15	RUN		TCR=100% SCR=77% RQD=77%					
13		slightly weathered			16	RUN		TCR=100% SCR=95% RQD=68%					
14					17	RUN		TCR=100% SCR=93% RQD=92%					
15		strong			18	RUN		TCR=100% SCR=97% RQD=93%					
16													
17													
18													
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
WATER LEVEL (date)

LOGGED : Letts  
CHECKED : KS



# RECORD OF BOREHOLE Q085+200

PROJECT : Mary River Project  
 LOCATION : 85+200 - Quarry  
 STARTED : July 20, 2011  
 COMPLETED : July 21, 2011

DRILLER: WALKER DRILLING, D-50  
 N 7 850 087 E 606 073

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN UNFROZEN UNCERTAIN
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10203040		
21					19	RUN		TCR=84% SCR=73% RQD=63%			1	
					20	RUN		TCR=100% SCR=100% RQD=100%			1	
22					21	RUN		TCR=99% SCR=96% RQD=80%			1	
23											1	
24					22	RUN		TCR=100% SCR=95% RQD=95%				
25											1	
					23	RUN		TCR=99% SCR=86% RQD=86%			2	
26		END OF BOREHOLE AT 25.89m.		25.89							2	
27											2	
28											2	
29											1	
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Letts  
 CHECKED : KS



# RECORD OF BOREHOLE Q088+800

PROJECT : Mary River Project  
 LOCATION : 88+800 - Quarry  
 STARTED : July 17, 2011  
 COMPLETED : July 18, 2011

DRILLER: WALKER DRILLING, D-50  
 N 7 846 674 E 605 956

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %					
		GROUND SURFACE		0.00									
		ORGANICS: (100mm)		0.10									
1		GRANITIC GNEISS, slightly weathered, medium grained, medium strong, black/pink/red			1	RUN			TCR=89% SCR=86% RQD=76%				
2					2	RUN			TCR=97% SCR=97% RQD=82%				
3					3	RUN							
4					4	RUN			TCR=73% SCR=69% RQD=60%				
5					5	RUN			TCR=88% SCR=67% RQD=26%				
6					6	RUN			TCR=100% SCR=100% RQD=92%				
7					7	RUN			TCR=100% SCR=83% RQD=48%				
8					8	RUN			TCR=100% SCR=100% RQD=100%				
9					9	RUN			TCR=96% SCR=92% RQD=81%				
10					10	RUN			TCR=58% SCR=33% RQD=0%				
11					11	RUN			TCR=100% SCR=88% RQD=70%				
12					12	RUN			TCR=100% SCR=100% RQD=68%				
13					13	RUN			TCR=94% SCR=94% RQD=91%				
14					14	RUN			TCR=100% SCR=100% RQD=78%				
15					15	RUN			TCR=79% SCR=79% RQD=74%				
16					16	RUN			TCR=100% SCR=98% RQD=96%				
17					17	RUN			TCR=85% SCR=80% RQD=80%				
18					18	RUN			TCR=100% SCR=100% RQD=100%				
19					19	RUN			TCR=100% SCR=100% RQD=100%				
		END OF BOREHOLE AT 15.00m.		15.00									

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Selman/Letts  
 CHECKED : KS





# RECORD OF BOREHOLE QTR-13

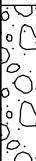




PROJECT : Mary River Project  
 LOCATION : 100+700 - Quarry  
 STARTED : July 14, 2011  
 COMPLETED : July 14, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 833 967 E 609 448

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS  DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div>50100150200250</div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.  FROZEN UNFROZEN UNCERTAIN		
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp10203040</div><div>ice</div><div>w</div><div>wl</div></div>						
		GROUND SURFACE		0.00												
1	NQ Diamond Drill	GRAVEL and COBBLES (< 100mm), red/dark grey, angular to sub-angular			1	RUN	64									
2																
3		COBBLES, BOULDERS (< 320mm) and GRAVEL, red/dark grey, angular to subangular		2.20												
4																
5		GRANITIC GNEISS, faintly weathered, medium to coarse grained, strong, red/dark grey		5.00												
6																
7		slightly to moderately weathered, medium strong			3	RUN									TCR=100% SCR=100% RQD=82%	
8																
9																
10					4	RUN									TCR=100% SCR=81% RQD=45%	
11																
12																
13						5	RUN									TCR=98% SCR=98% RQD=97%
14																
15			strong to very strong		15.10	6	RUN									TCR=100% SCR=100% RQD=94%
16		END OF BOREHOLE AT 15.10m.														
17																
18																
19																

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Young  
 CHECKED : KS



# RECORD OF BOREHOLE QTR-12








PROJECT : Mary River Project  
 LOCATION : 101+100 - Quarry  
 STARTED : July 15, 2011  
 COMPLETED : July 15, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 836 190 E 610 857

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		<b>COBBLES, GRAVEL and BOULDERS</b> ( $<450\text{mm}$ ), dark grey, pink/white, subangular to subrounded			1	RUN	95						
2													
3		<b>GRANITIC GNEISS</b> , faintly to slightly weathered, medium strong to strong, medium to coarse grained, dark grey, white, pink		2.85	2	RUN		TCR=100% SCR=97% RQD=43%				FI 10	
4													
5					3	RUN		TCR=100% SCR=100% RQD=92%				5	
6													
7					4	RUN		TCR=100% SCR=93% RQD=60%				10	
8													
9					5	RUN		TCR=100% SCR=74% RQD=74%				1	
10												1	
11		slightly to moderately weathered			6	RUN		TCR=103% SCR=84% RQD=81%				2	
12												2	
13					7	RUN		TCR=97% SCR=65% RQD=65%				1	
14												1	
15												0	
16												2	
17												1	
18												1	
19												0	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Young  
 CHECKED : KS



# RECORD OF BOREHOLE QTR-12

PROJECT : Mary River Project  
 LOCATION : 101+100 - Quarry  
 STARTED : July 15, 2011  
 COMPLETED : July 15, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 836 190 E 610 857

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10 20 30 40		
21		END OF BOREHOLE AT 20.50m.		20.50								
22												
23												
24												
25												
26												
27												
28												
29												
30												
31												
32												
33												
34												
35												
36												
37												
38												
39												

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Young  
 CHECKED : KS



# RECORD OF BOREHOLE NTUN-DH01



PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : August 3, 2011  
 COMPLETED : August 3, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 656 E 605 976

Project No. 19-1605-126


SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT				
		GROUND SURFACE		90.21										
		no recovery												
1	NQ Diamond Drill													
2														
3		SAND and GRAVEL, some cobbles, granitic, rounded to subangular, fines washed out		2.60	1	RUN	61							
4														
5		GRAVEL and COBBLES (<300mm), fines washed out		4.80	2A	RUN	100							
6														
7		END OF SAMPLING AT 6.45m, START CORING. FOR ROCK DETAILS PLEASE REFER TO NTUN-DH01(R).		6.45										
8														
9														
10														
11														
12														
13														
14														
15														
16														
17														
18														
19														

## GROUNDWATER ELEVATIONS

 SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

 DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : MB







# RECORD OF BOREHOLE NTUN-DH03

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : August 1, 2011  
 COMPLETED : August 2, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 382 E 605 698

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN <div><div></div></div> UNFROZEN <div><div></div></div> UNCERTAIN <div><div></div></div>
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp</div><div></div><div>w</div><div></div><div>wl</div><div>10203040</div></div>				
		GROUND SURFACE		146.68										
	NQ Diamond Drill	no sampling												
1		GRAVEL and COBBLES (<170mm), granitic, fine to medium grained, subangular to subrounded, pinkish red to greyish black	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div>&lt;</div>											

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : MB



# RECORD OF BOREHOLE NTUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : August 1, 2011  
 COMPLETED : August 2, 2011

INCLINATION: 44°      AZIMUTH: 63°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 382 E 605 698

Project No. 19-1605-126  
 SHEET 1 OF 6  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (m/min)	FLUSH % RETURN	COLOUR	FR-FRACTURE				F-FAULT				SM-SMOOTH				FO-FOLIATED				Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS								
									CL-CLEAVAGE		SH-SHEAR		VN-VEIN		J-JOINT		P-POLISHED		S-SLICKENSIDED		R-ROUGH		ST-STEPPED				PL-PLANAR		UE-UNEVEN		W-WAVY		C-CURVED	
									RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																					
									TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION																					
		GROUND SURFACE		146.68																														
				4.50	1B	45																												
5	NQ Diamond Drill	<b>GRANITIC GNEISS</b> , slightly weathered, strong, moderately to thinly spaced, pink with dark and light grey foliations			2																													
6																																		
7				3	19	75																												
8																																		
9				4	11	100																												
10																																		
11																																		
12				5	7	100																												
13																																		
14																																		
15		diagonal open joint at 14.42m			6	9	100																											
16																																		
17																																		
18																																		
19		biotite schist banding at 17.22m																																
20																																		
21																																		
22																																		
23		diagonal, irregular fresh joint at 18.10m horizontal, irregular slightly weathered joint at 18.26m sub-vertical open joint with rust at 18.57m			7	11	100																											
24		mechanical breaks																																
25																																		
26																																		
27																																		
28																																		
29																																		
30																																		
31																																		
32																																		
33																																		
34																																		
35																																		
36																																		
37																																		
38																																		
39																																		
40																																		
41																																		
42																																		
43																																		
44																																		
45																																		
46																																		
47																																		
48																																		

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB





RECORD OF BOREHOLE NTUN-DH03(R)			
PROJECT	: Mary River Project	INCLINATION: 44°	Project No. 19-1605-126
LOCATION	: Cockburn Lake - North Tunnel	AZIMUTH: 63°	
STARTED	: August 1, 2011	DRILLER: BOART LONGYEAR, LM-55	SHEET 2 OF 6
COMPLETED	: August 2, 2011	N 7 835 382 E 605 698	DATUM CGVD28

GROUNDWATER ELEVATIONS		 <b>THURBER</b>
<b>SHALLOW/SINGLE INSTALLATION</b> WATER LEVEL (date)	<b>DEEP/DUAL INSTALLATION</b> WATER LEVEL (date)	LOGGED : Dunstan/Hill  CHECKED : MB

# RECORD OF BOREHOLE NTUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : August 1, 2011  
 COMPLETED : August 2, 2011

INCLINATION: 44°      AZIMUTH: 63°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 382 E 605 698

Project No. 19-1605-126  
 SHEET 3 OF 6  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FLUSH % RETURN	FR-FRACTURE		F-FAULT		SM-SMOOTH		FO-FOLIATED		Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS
									CL-CLEAVAGE		J-JOINT		R-ROUGH		UE-UNEVEN			
									SH-SHEAR		P-POLISHED		ST-STEPPED		W-WAVY			
									VN-VEIN		S-SLICKENSIDED		PL-PLANAR		C-CURVED			
RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA														
TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION														
80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80
50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150	50 100 150
		GROUND SURFACE																
45	NQ Diamond Drill				17	6	100											
46		pale white infill on joint surfaces																
47		slightly weathered, banded, fine grained, pinkish grey to greyish black																
48					18													
49																		
50																		
51					19	12	0											
52																		
53																		
54					20	11	0											
55																		
56																		
57					21	8	0											
58																		
59																		
60					22		0											
61																		
62																		
63					23		0											
64		soft white infill in joints up to 1mm thick																

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB




# RECORD OF BOREHOLE NTUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : August 1, 2011  
 COMPLETED : August 2, 2011

INCLINATION: 44°      AZIMUTH: 63°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 382 E 605 698

Project No. 19-1605-126  
 SHEET 4 OF 6  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH % RETURN	COLOUR % RETURN	FR-FRACTURE		F-FAULT		SM-SMOOTH		FO-FOLIATED		Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS																				
									CL-CLEAVAGE		J-JOINT		R-ROUGH		UE-UNEVEN																							
									SH-SHEAR		P-POLISHED		ST-STEPPED		W-WAVY																							
									VN-VEIN		S-SLICKENSIDED		PL-PLANAR		C-CURVED																							
RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																		
TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION																																		
65	NQ Diamond Drill	GROUND SURFACE																																				
66																				24	0																	
67																																						
68																																						
69																				25	0																	
70																																						
71																																						
72																				26	0																	
73																																						
74																																						
75																				27	0																	
76																																						
77																																						
78																				28	0																	
79																																						
80																																						
81																				29	0																	
82																																						
83																																						
84																				30	0																	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB





# RECORD OF BOREHOLE NTUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : August 1, 2011  
 COMPLETED : August 2, 2011

INCLINATION: 44°      AZIMUTH: 63°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 382 E 605 698

Project No. 19-1605-126  
 SHEET 6 OF 6  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FR-FRACTURE				F-FAULT				SM-SMOOTH				FO-FOLIATED				Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
								CL-CLEAVAGE		SH-SHEAR		VN-VEIN		J-JOINT		P-POLISHED		S-SLICKENSIDED		R-ROUGH		ST-STEPPED				PL-PLANAR		UE-UNEVEN		W-WAVY		C-CURVED																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
								RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
								TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																</

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB



# RECORD OF BOREHOLE NTUN-DH05


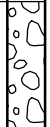
PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : July 30, 2011  
 COMPLETED : July 31, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 245 E 605 535


Project No. 19-1605-126


SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 	EXCESS ICE CONTENT, PERCENT				THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %					
		GROUND SURFACE		148.79									
		no sampling											
1	NQ Diamond Drill			1.30									
2		GRAVEL and COBBLES, granitic, boulders up to 710mm			1A	RUN		100					
3				2.99									
4		END OF SAMPLING AT 2.99m, START CORING. FOR ROCK DETAILS PLEASE REFER TO NTUN-DH05(R).											
5													
6													
7													
8													
9													
10													
11													
12													
13													
14													
15													
16													
17													
18													
19													

## GROUNDWATER ELEVATIONS

 SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

 DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : MB



# RECORD OF BOREHOLE NTUN-DH05(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : July 30, 2011  
 COMPLETED : July 31, 2011

INCLINATION: 70°      AZIMUTH: 45°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 245 E 605 535

Project No. 19-1605-126  
 SHEET 1 OF 4  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FLUSH	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN										F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED										SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR										FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED										Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																										
									RECOVERY					R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																												
									TOTAL CORE %		SOLID CORE %		TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																															
									80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20			80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20			80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB







# RECORD OF BOREHOLE NTUN-DH05(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - North Tunnel  
 STARTED : July 30, 2011  
 COMPLETED : July 31, 2011

INCLINATION: 70°      AZIMUTH: 45°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 835 245 E 605 535

Project No. 19-1605-126  
 SHEET 3 OF 4  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH % RETURN	COLOUR % RETURN	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN										F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED										SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR										FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED										Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																													
									RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																																		
									TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																																		
									80 60 40 20	80 60 40 20			80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20			80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB





# RECORD OF BOREHOLE STUN-DH03

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - South Tunnel  
 STARTED : July 29, 2011  
 COMPLETED : July 29, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 832 812 E 601 490

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS <small>DYNAMIC CONE PENETRATION RESISTANCE PLOT</small>	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT			
		GROUND SURFACE		111.22									
		no sampling											
1	NQ Diamond Drill												
2													
3													
4		END OF SAMPLING AT 3.30m, START CORING. FOR ROCK DETAILS PLEASE REFER TO STUN-DH03(R).		3.30									
5													
6													
7													
8													
9													
10													
11													
12													
13													
14													
15													
16													
17													
18													
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill  
 CHECKED : MB



# RECORD OF BOREHOLE STUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - South Tunnel  
 STARTED : July 29, 2011  
 COMPLETED : July 29, 2011

INCLINATION: 56°      AZIMUTH: 268°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 832 812 E 601 490

Project No. 19-1605-126  
 SHEET 1 OF 5  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH % RETURN	COLOUR	DISCONTINUITY DATA												Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
									RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
									TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
									80 60 40 20	80 60 40 20			15 10 5 20	DIP wrt Core Axis																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
		GROUND SURFACE		111.22																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						</

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : MB



DATUM CGVD28



THURBER

# RECORD OF BOREHOLE STUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - South Tunnel  
 STARTED : July 29, 2011  
 COMPLETED : July 29, 2011

INCLINATION: 56°      AZIMUTH: 268°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 832 812 E 601 490

Project No. 19-1605-126  
 SHEET 3 OF 5  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN	F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED	SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR	FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED	Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																														
														RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
														TOTAL CORE %	SOLID CORE %			DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								
														80 60 40 20	80 60 40 20			0 30 60 90																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
		GROUND SURFACE																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
-44	NQ Diamond Drill	sub-vertical fracture at 48.45m			14	3	25																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : MB



# RECORD OF BOREHOLE STUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - South Tunnel  
 STARTED : July 29, 2011  
 COMPLETED : July 29, 2011

INCLINATION: 56°      AZIMUTH: 268°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 832 812 E 601 490

Project No. 19-1605-126  
 SHEET 4 OF 5  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH % RETURN	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN	F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED	SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR	FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED	Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test						
														RECOVERY		R.Q.D. %	FRACT. INDEX PER 3 m	DISCONTINUITY DATA	
														TOTAL CORE %	SOLID CORE %			DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION
														80	80	80	80	80	80
60	60	60	60	60	60	60	60	60	60	60	60	60	60						
40	40	40	40	40	40	40	40	40	40	40	40	40	40						
20	20	20	20	20	20	20	20	20	20	20	20	20	20						
0	0	0	0	0	0	0	0	0	0	0	0	0	0						
		GROUND SURFACE																	
64	NQ Diamond Drill	horizontal, irregular, closed joint at 65.15m			21	9													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			
74	fresh, very wide spacing, medium grey with dark grey and pink sub-horizontal foliations			22	15	light grey 100													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			
74	diagonal, planar, weathered joints at 70.46 and 70.70m			23	13	light grey 100													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			
74	diagonal, irregular, closed joint at 79.65m			24	14	light grey 100													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			
74	becoming pinker			25	10	light grey 100													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			
74	diagonal, planar, weathered, open joints at 82.59 to 84.22m			26	12	light grey 100													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			
74				27	11	light grey 100													
65																			
66																			
67																			
68																			
69																			
70																			
71																			
72																			
73																			

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : MB



# RECORD OF BOREHOLE STUN-DH03(R)

PROJECT : Mary River Project  
 LOCATION : Cockburn Lake - South Tunnel  
 STARTED : July 29, 2011  
 COMPLETED : July 29, 2011

INCLINATION: 56°      AZIMUTH: 268°  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 832 812 E 601 490

Project No. 19-1605-126  
 SHEET 5 OF 5  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH	COLOUR % RETURN	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN	F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED	SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR	FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED	Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test
		GROUND SURFACE												
84	NQ Diamond Drill	sub-vertical, undulating to planar, weathered, open joint with mineral infill at 86.55m			28	10	light grey	100						
85														
86														
87														
88		END OF BOREHOLE AT 87.45m.		87.45										
89														
90														
91														
92														
93														
94														
95														
96														
97														
98														
99														
100														
101														
102														
103														

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : MB





# RECORD OF BOREHOLE Q114+600

PROJECT : Mary River Project  
 LOCATION : 114+600 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 25, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 827 828 E 597 850

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
		GROUND SURFACE		0.00										
		TOPSOIL		0.20										
1		BOULDERS and COBBLES, granitic, grey			1	RUN								
2				2.30										
3		GRANITIC GNEISS, slightly weathered, fine grained, slightly fractured, black, grey												
4					2	RUN			TCR=100% SCR=100% RQD=77%					
5														
6														
7					3	RUN			TCR=100% SCR=81% RQD=81%					
8														
9														
10					4	RUN			TCR=100% SCR=100% RQD=100%					
11														
12		layers of biotite interbeds												
13					5	RUN			TCR=100% SCR=100% RQD=100%					
14														
15		quartz crystal												
16					6	RUN			TCR=100% SCR=100% RQD=100%					
17		biotite schist at 16.90m												
18		biotite schist (100mm) at 18.2m												
19					7	RUN			TCR=100% SCR=100% RQD=92%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE Q114+600


PROJECT : Mary River Project  
 LOCATION : 114+600 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 25, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 827 828 E 597 850

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
21	NQ Diamond Drill	biotite interbeds			8	RUN			TCR=100% SCR=100% RQD=90%					
22														
23														
24														
25					9	RUN			TCR=100% SCR=100% RQD=77%					
26														
27														
28					10	RUN			TCR=100% SCR=100% RQD=100%					
29														
30														
31														
32					11	RUN			TCR=100% SCR=100% RQD=100%					
33														
34		END OF BOREHOLE AT 33.60m.		33.60										
35														
36														
37														
38														
39														

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Khabbaznia  
 CHECKED : KS



# RECORD OF BOREHOLE QTR-09

PROJECT : Mary River Project  
 LOCATION : 116+500 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 826 260 E 600 261

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. <div>FROZEN<div></div></div> <div>UNFROZEN<div></div></div> <div>UNCERTAIN<div></div></div>
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT <div><div>wp</div><div></div><div>w</div><div></div><div>wl</div><div>10203040</div></div>				
		GROUND SURFACE		0.00										
		COBBLES(150mm) and GRAVEL	<div><div></div><div></div></div>	0.30										0.30 <div></div>
1	NQ Diamond Drill	GRANITIC GNEISS, fresh, very strong, light grey and pink	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div></div>											

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Mediwake/Braverman  
 CHECKED : KS



# RECORD OF BOREHOLE QTR-09

PROJECT : Mary River Project  
 LOCATION : 116+500 - Quarry  
 STARTED : July 25, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 826 260 E 600 261

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
21	NQ Diamond Drill													
22					8	RUN			TCR=100% SCR=100% RQD=100%					
23														
24														
25					9	RUN			TCR=100% SCR=100% RQD=100%					
26														
27														
28					10	RUN			TCR=100% SCR=97% RQD=77%					
29														
30														
31					11	RUN			TCR=100% SCR=100% RQD=100%					
32		END OF BOREHOLE AT 32.40m.		32.40										
33														
34														
35														
36														
37														
38														
39														

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Mediwake/Braverman  
 CHECKED : KS



# RECORD OF BOREHOLE Q116+800

PROJECT : Mary River Project  
 LOCATION : 116+800 - Quarry  
 STARTED : July 26, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 826 194 E 597 422

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		COBBLES and GRAVEL, fines washed out			1	RUN	61						
2		GRANITIC GNEISS, fresh, moderately spaced, very strong, with diagonal foliations, horizontal to subhorizontal closed joints, grey, pink, white		1.93									
3													
4					2	RUN		TCR=100% SCR=92% RQD=92%					
5													
6													
7					3	RUN		TCR=100% SCR=94% RQD=94%					
8													
9													
10		open diagonal joint with black weathering at 9.20m			4	RUN		TCR=100% SCR=96% RQD=96%					
11													
12					5	RUN		TCR=100% SCR=97% RQD=94%					
13													
14					6	RUN		TCR=100% SCR=93% RQD=93%					
15		white quartzite banding at 14.8m											
16					7	RUN		TCR=100% SCR=93% RQD=93%					
17													
18		becoming widely spaced massive											
19					8	RUN		TCR=100% SCR=100% RQD=100%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : KS



# RECORD OF BOREHOLE Q116+800

PROJECT : Mary River Project  
 LOCATION : 116+800 - Quarry  
 STARTED : July 26, 2011  
 COMPLETED : July 26, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 826 194 E 597 422

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT <div><div></div><div>50100150200250</div></div>	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND. FROZEN <div><div></div></div> UNFROZEN <div><div></div></div> UNCERTAIN <div><div></div></div>			
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT							
										wp	w	wl					
21	NQ Diamond Drill	biotite schist interbeds from 22.32m to 22.49m  very widely spaced joints  closely spaced joints	<div></div>						TCR=100% SCR=96% RQD=96%						0		
22					9	RUN											0
23																	0
24																	1
25																	0
26																	3
27																	0
28																	3
29																	0
30																	1
31																	0
32																	0
33		END OF BOREHOLE AT 32.39m.		32.39										3			
34																	
35																	
36																	
37																	
38																	
39																	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : KS



# RECORD OF BOREHOLE QTR-04A

PROJECT : Mary River Project  
 LOCATION : 123+000 - Quarry  
 STARTED : August 4, 2011  
 COMPLETED : August 4, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 820 410 E 598 555

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTER/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	WATER CONTENT, PERCENT			
		GROUND SURFACE		0.00									
1	NQ Diamond Drill	GRAVEL, granitic, light grey to reddish brown, sub-angular to sub-rounded, fine to medium grained		0.51	1A	RUN							
2						1B	RUN		TCR=84% SCR=62%				
3													
4						2	RUN		TCR=100% SCR=98% RQD=69%				
5													
6													
7						3	RUN		TCR=100% SCR=100% RQD=71%				
8													
9				slightly weathered, moderately spaced									
10						4	RUN		TCR=100% SCR=98% RQD=82%				
11													
12													
13						5	RUN		TCR=100% SCR=94% RQD=71%				
14													
15				moderately to highly weathered, closely spaced joints, numerous open joints with silt/sand infill									
16						6	RUN		TCR=100% SCR=8% RQD=0%				
17													
18				heavily fractured from 17.45m to 21.05m									
19						7	RUN		TCR=100% SCR=100% RQD=90%				

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : KS



# RECORD OF BOREHOLE QTR-04A

PROJECT : Mary River Project  
 LOCATION : 123+000 - Quarry  
 STARTED : August 4, 2011  
 COMPLETED : August 4, 2011

DRILLER: BOART LONGYEAR, LM-55  
 N 7 820 410 E 598 555

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
21	NQ Diamond Drill												0	
22					8	RUN			TCR=100% SCR=90% RQD=81%				5	
23													3	
24													1	
25					9	RUN			TCR=100% SCR=89% RQD=78%				1	
26													0	
27													0	
28					10	RUN			TCR=100% SCR=90% RQD=85%				1	
29													0	
30		trace biotite schist banding sub-vertical joints with red silty infill from 30.0m to 30.3m											0	
31					11	RUN			TCR=100% SCR=95% RQD=84%				4	
32		END OF BOREHOLE AT 32.45m.		32.45									3	
33													0	
34													1	
35													0	
36													1	
37													0	
38													1	
39													0	

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : KS





# RECORD OF BOREHOLE Q131+100

PROJECT : Mary River Project  
 LOCATION : 131+100 - Quarry  
 STARTED : August 5, 2011  
 COMPLETED : August 5, 2011

DRILLER: BOART LONGYEAR  
 N 7 813 509 E 600 177

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1		SAND and GRAVEL (INFERRED), some cobbles (< 170mm), trace silt, occasional boulders (< 740mm), granitic, pinkish red to greyish black, moist			1	RUN	19						
2													
3													
4		SAND and GRAVEL, some silt, trace cobbles and boulders (< 210mm), granitic		5.48	2	RUN	67						
5													
6													
7		GRANITIC GNEISS, moderately weathered, fine grained, banded, strong		8.35	3	RUN	100						
8													
9													
10	NQ Diamond Drill	highly fractured zone at 11.1m (0.10m) with clayey infill			4	RUN		TCR=100% SCR=90% RQD=72%					
11													
12													
13		becoming moderately spaced, with white foliations			5	RUN		TCR=100% SCR=84% RQD=75%					
14													
15													
16					6	RUN		TCR=100% SCR=98% RQD=89%					
17													
18													
19					7	RUN		TCR=100% SCR=98% RQD=94%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : KS



# RECORD OF BOREHOLE Q131+100

PROJECT : Mary River Project  
 LOCATION : 131+100 - Quarry  
 STARTED : August 5, 2011  
 COMPLETED : August 5, 2011

Project No. 19-1605-126

DRILLER: BOART LONGYEAR  
 N 7 813 509 E 600 177

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT		ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.	
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m		RECOVERY %	10 20 30 40			10 20 30 40
		END OF BOREHOLE AT 20.48m.		20.48								0	
21													
22													
23													
24													
25													
26													
27													
28													
29													
30													
31													
32													
33													
34													
35													
36													
37													
38													
39													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Hill/Dunstan  
 CHECKED : KS



# RECORD OF BOREHOLE Q138+100


PROJECT : Mary River Project  
 LOCATION : 138+100 - Quarry  
 STARTED : July 16, 2011  
 COMPLETED : July 17, 2011

Project No. 19-1605-126

DRILLER: WALKER DRILLING  
 N 7 807 612 E 598 865

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1	NQ Diamond Drill	<b>GRANITIC GNEISS</b> , moderately weathered, medium to coarse grained, pink to grey			1	RUN		TCR=86% SCR=69% RQD=52%					
2		becoming moderately strong, slightly weathered			2	RUN		TCR=97% SCR=82% RQD=70%					
3													
4													
5					3	RUN		TCR=98% SCR=80% RQD=47%					
6		with black bands (mm scale), greenish clay infilled gouge at 6.24m											
7					4	RUN		TCR=99% SCR=99% RQD=80%					
8													
9					5	RUN		TCR=98% SCR=97% RQD=85%					
10					6	RUN		TCR=98% SCR=98% RQD=68%					
11		moderately strong, slightly weathered											
12					7	RUN		TCR=86% SCR=76% RQD=52%					
13													
14					8	RUN		TCR=97% SCR=87% RQD=81%					
15		medium strong to strong, moderately weathered											
16					9	RUN		TCR=100% SCR=96% RQD=96%					
17					10	RUN		TCR=92% SCR=81% RQD=73%					
18		strong, faintly weathered											
19					11	RUN		TCR=99% SCR=93% RQD=81%					
					12	RUN		TCR=97% SCR=90% RQD=79%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Singh  
 CHECKED : KS



# RECORD OF BOREHOLE Q138+100

PROJECT : Mary River Project  
 LOCATION : 138+100 - Quarry  
 STARTED : July 16, 2011  
 COMPLETED : July 17, 2011

Project No. 19-1605-126

DRILLER: WALKER DRILLING  
 N 7 807 612 E 598 865

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
21					13	RUN			TCR=98% SCR=92% RQD=88%					
22					14	RUN			TCR=99% SCR=95% RQD=90%					
23														
24		becoming strong, fresh			15	RUN			TCR=90% SCR=90% RQD=80%					
25														
26					16	RUN			TCR=97% SCR=93% RQD=70%					
27														
28					17	RUN			TCR=94% SCR=91% RQD=86%					
29														
30					18	RUN			TCR=100% SCR=93% RQD=91%					
31														
32					19	RUN			TCR=97% SCR=92% RQD=88%					
33														
34														
35														
36														
37														
38														
39														
		END OF BOREHOLE AT 30.95m.		30.93										

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Singh  
 CHECKED : KS



# RECORD OF BOREHOLE Q139+600

PROJECT : Mary River Project  
 LOCATION : 139+600 - Quarry  
 STARTED : July 19, 2011  
 COMPLETED : July 19, 2011

DRILLER: WALKER DRILLING  
 N 7 806 105 E 598 727

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %					
		GROUND SURFACE		0.00									
1	NQ Diamond Drill	<b>GRANITIC GNEISS</b> , slightly weathered, medium strong, medium to coarse grained, grey, pink, black			1	RUN			TCR=99% SCR=99% RQD=96%				
2					2	RUN			TCR=100% SCR=100% RQD=100%				
3					3	RUN			TCR=95% SCR=91% RQD=91%				
4					4	RUN			TCR=95% SCR=89% RQD=77%				
5					5	RUN			TCR=99% SCR=96% RQD=84%				
6					6	RUN			TCR=100% SCR=100% RQD=100%				
7					7	RUN			TCR=91% SCR=91% RQD=88%				
8					8	RUN			TCR=100% SCR=100% RQD=98%				
9					9	RUN			TCR=99% SCR=80% RQD=80%				
10		END OF BOREHOLE AT 13.89m.		13.89									
11													
12													
13													
14													
15													
16													
17													
18													
19													

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Santos/Singh  
 CHECKED : KS



# RECORD OF BOREHOLE QS-3A

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Quarry  
 STARTED : July 23, 2011  
 COMPLETED : July 23, 2011

DRILLER: WALKER DRILLING  
 N 7 800 000 E 595 698

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
		GROUND SURFACE		0.00										
1		GRANITIC GNEISS, highly weathered and fractured, coarse grained, weak, pinkish grey			1	RUN			TCR=37% SCR=25% RQD=25%					
2					2	RUN			TCR=98% SCR=56% RQD=40%					
3														
4		clay infill at 4.4m												
5														
6					4	RUN			TCR=100% SCR=74% RQD=41%					
7		moderately to highly weathered												
8					5	RUN			TCR=100% SCR=74% RQD=33%					
9					6	RUN			TCR=100% SCR=88% RQD=35%					
10		moderately weathered												
11		highly weathered			7	RUN			TCR=100% SCR=95% RQD=89%					
12		vertical joint, clay infill												
13					8	RUN			TCR=100% SCR=53% RQD=27%					
14														
15		slightly to moderately weathered			9	RUN			TCR=100% SCR=100% RQD=69%					
16														
17					10	RUN			TCR=100% SCR=94% RQD=92%					
18														
19														
		END OF BOREHOLE AT 15.13m.		15.13										

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Singh/Ramos  
 CHECKED : KS



# RECORD OF BOREHOLE QS-2

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Quarry  
 STARTED : July 24, 2011  
 COMPLETED : July 25, 2011

DRILLER: WALKER DRILLING  
 N 7 801 066 E 595 200

Project No. 19-1605-126

SHEET 1 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES			COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m RECOVERY %						
		GROUND SURFACE		0.00									
1	NQ Diamond Drill	GRANITIC GNEISS, faintly to slightly weathered, coarse grained, medium strong, black, grey, pink			1	RUN		TCR=100% SCR=100% RQD=95%					
2													
3					2	RUN		TCR=100% SCR=100% RQD=100%					
4													
5					3	RUN		TCR=98% SCR=98% RQD=98%					
6													
7													
8					4	RUN		TCR=100% SCR=97% RQD=97%					
9													
10					5	RUN		TCR=91% SCR=91% RQD=86%					
11													
12		slightly weathered, medium to coarse grained			6	RUN		TCR=100% SCR=100% RQD=92%					
13													
14		strong, fresh to faintly weathered			7	RUN		TCR=100% SCR=96% RQD=92%					
15													
16		massive			8	RUN		TCR=98% SCR=93% RQD=81%					
17													
18					9	RUN		TCR=99% SCR=95% RQD=91%					
19													
					10	RUN		TCR=100% SCR=92% RQD=88%					

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ramos/Singh  
 CHECKED : KS



# RECORD OF BOREHOLE QS-2

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Quarry  
 STARTED : July 24, 2011  
 COMPLETED : July 25, 2011

DRILLER: WALKER DRILLING  
 N 7 801 066 E 595 200

Project No. 19-1605-126

SHEET 2 OF 2

DATUM: CGVD28

DEPTH SCALE (metres)	BORING METHOD	SOIL PROFILE		SAMPLES				COMMENTS DYNAMIC CONE PENETRATION RESISTANCE PLOT 50 100 150 200 250	EXCESS ICE CONTENT, PERCENT				ADDITIONAL LAB. TESTING	THERMISTERS/ GROUND COND.
		DESCRIPTION	STRATA PLOT	ELEV. (m)	NUMBER	TYPE	BLOWS/0.3m	RECOVERY %						
21	NQ Diamond Drill	weak, highly weathered from 21.1m to 22.9m			11	RUN			TCR=100% SCR=53% RQD=47%					
22					12	RUN			TCR=100% SCR=0% RQD=0%					
23														
24					13	RUN			TCR=100% SCR=100% RQD=100%					
25														
26														
27					14	RUN			TCR=100% SCR=100% RQD=100%					
28														
29					15	RUN			TCR=100% SCR=91% RQD=79%					
30		END OF BOREHOLE AT 30.00m.		30.00										
31														
32														
33														
34														
35														
36														
37														
38														
39														

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Ramos/Singh  
 CHECKED : KS





## THURBER2S(5126) 5126.GPJ 11/9/11

Project No. 19-1605-126

SHEET 1 OF 1

DATUM: CGVD28

GROUNDWATER ELEVATIONS

CHECKED : KS



# RECORD OF BOREHOLE SI-OLD-004

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 8, 2011  
 COMPLETED : August 8, 2011

INCLINATION:      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 314 E 592 879

Project No. 19-1605-126  
 SHEET 1 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (n/min)	COLOUR % RETURN	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN	F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED	SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR	FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED	Unconfined Compressive Strength (Mpa)				FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																							
												FLUSH	RECOVERY		R.Q.D. %		FRACT. INDEX PER .3 m	DISCONTINUITY DATA																					
													TOTAL CORE %	SOLID CORE %				DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION																				
																				80 60 40 20	80 60 40 20	0 30 60 90																	
		GROUND SURFACE		4.54																																			
1	NQ Diamond Drill	<b>GRANITIC GNEISS</b> , faintly weathered, moderately wide horizontal foliation, strong, pink with dark and light grey foliations			1												J, open (25mm), irregular J, closed, planar, horizontal																						
2																																							
3					slightly weathered			2												J, closed, irregular, horizontal J, closed, irregular, horizontal, weathered J, closed, diagonal, black staining J, closed, black crystalline intrusion J, closed, black crystalline intrusion																			
4																																							
5																																							
6																																							
7								3												J, closed, irregular, diagonal, black staining																			
8								4												J, open, sub-vertical, weathered (5.48 to 5.69m) J, open, diagonal, weathered																			
9																																							
10					becoming coarse grained																																		
11																																							
12								5													J, closed, sub-horizontal J, 2mm aperture, stepped, vertical, silt infill (8.23 to 10.36m)																		
13																																							
14																																							
15								6													J, closed, horizontal, weathered J, sub-vertical, weathered (12.06 to 12.25m)																		
16																																							
17																																							
18								7													J, closed, horizontal, black staining J, closed, horizontal, black staining																		
19																																							

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB



# RECORD OF BOREHOLE SI-OLD-004


PROJECT : Mary River Project  
LOCATION : Steensby Inlet - Ore Loading Dock  
STARTED : August 8, 2011  
COMPLETED : August 8, 2011

INCLINATION:                      AZIMUTH:  
LLER: BOART LONGYEAR, LM-5  
N 7 798 314 E 592 879

Project No. 19-1605-126

SHEET 2 OF 2

DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FLUSH	RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DIP wrt Core Axis	DISCONTINUITY DATA		Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test
									TOTAL CORE %	SOLID CORE %				TYPE AND SURFACE DESCRIPTION			
		GROUND SURFACE															
21	NQ Diamond Drill	100mm white silt/sandy silt infill at 23.23m			8												
22																	
23																	
24																	
25																	
26																	
27																	
28																	
29																	
30																	
31																	
32		END OF BOREHOLE AT 32.23m.		32.23													
33																	
34																	
35																	
36																	
37																	
38																	
39																	

## GROUNDWATER ELEVATIONS

 SHALLOW/SINGLE INSTALLATION  
WATER LEVEL (date)

 DEEP/DUAL INSTALLATION  
WATER LEVEL (date)

LOGGED : Dunstan/Hill

CHECKED : MB



ROCKM(5126) 5126.GPJ 11/9/11

# RECORD OF BOREHOLE SI-OLD-005

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 9, 2011  
 COMPLETED : August 10, 2011

INCLINATION:      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 331 E 592 860

Project No. 19-1605-126  
 SHEET 1 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FLUSH	FR-FRACTURE CL-CLEAVAGE SH-SHEAR VN-VEIN												F-FAULT J-JOINT P-POLISHED S-SLICKENSIDED						SM-SMOOTH R-ROUGH ST-STEPPED PL-PLANAR				FO-FOLIATED UE-UNEVEN W-WAVY C-CURVED				Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
									RECOVERY				R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
									TOTAL CORE %	SOLID CORE %	R.Q.D. %	FRACT. INDEX PER .3 m			DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
																80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	45 30 15 0	45 30 15 0	0 30 60 90																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																														
		GROUND SURFACE		4.05																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																</

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan/Hill  
 CHECKED : MB



RECORD OF BOREHOLE SI-OLD-005																											
PROJECT : Mary River Project		INCLINATION:		AZIMUTH:		Project No. 19-1605-126																					
LOCATION : Steensby Inlet - Ore Loading Dock																											
STARTED : August 9, 2011		DRILLER: BOART LONGYEAR, LM-55				SHEET 2 OF 2																					
COMPLETED : August 10, 2011		N 7 798 331 E 592 860				DATUM CGVD28																					
DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (m/min)	COLOUR	FLUSH	% RETURN	FR-FRACTURE				F-FAULT				SM-SMOOTH				FO-FOLIATED				Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS
										CL-CLEAVAGE				J-JOINT				R-ROUGH				UE-UNEVEN					
										SH-SHEAR				P-POLISHED				ST-STEPPED				W-WAVY					
										RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA													
										TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION													
										80 80 80 80	40 40 40 40	80 80 80 80	0 0 0 0	15 15 15 15	0 0 0 0	0 0 0 0									50 100 150		
		GROUND SURFACE																									
21	NQ Diamond Drill				8																						
22																											
23																											
24																											
25																											
26																											
27																											
28																											
29																											
30																											
31																											
32				END OF BOREHOLE AT 31.96m.		31.96																					
33																											
34																											
35																											
36																											
37																											
38																											
39																											

**GROUNDWATER ELEVATIONS**

SHALLOW/SINGLE INSTALLATION  
WATER LEVEL (date)

DEEP/DUAL INSTALLATION  
WATER LEVEL (date)

LOGGED : Dunstan/Hill

CHECKED : MB

**THURBER**

ROCKM(5126) 5126.GPJ 11/9/11



# RECORD OF BOREHOLE SI-OLD-006

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 10, 2011  
 COMPLETED : August 11, 2011

INCLINATION:                      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 409 E 592 876

Project No. 19-1605-126  
 SHEET 1 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH	COLOUR % RETURN	DISCONTINUITY DATA												Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
									RECOVERY		R.Q.D. %	FRACT. INDEX PER 3 m	DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																											
									TOTAL CORE %	SOLID CORE %																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
									80 60 40 20	80 60 40 20					80 60 40 20	15 10 5 0	30 20 10 0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								
		GROUND SURFACE		9.09																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					

ROCKM(5126) 5126.GPJ 11/9/11

LOGGED : Hill  
 CHECKED : MB



# RECORD OF BOREHOLE SI-OLD-006

PROJECT : Mary River Project  
LOCATION : Steensby Inlet - Ore Loading Dock  
STARTED : August 10, 2011  
COMPLETED : August 11, 2011

INCLINATION:                      AZIMUTH:  
 LER: BOART LONGYEAR, LM-5  
 N 7 798 409 E 592 876

Project No. 19-1605-126


SHEET 2 OF 2

DATUM CGVD28

[illegible]

## GROUNDWATER ELEVATIONS

 SHALLOW/SINGLE INSTALLATION  
WATER LEVEL (date)

 DEEP/DUAL INSTALLATION  
WATER LEVEL (date)

LOGGED : Hill  
CHECKED : MB



# RECORD OF BOREHOLE SI-OLD-007

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 11, 2011  
 COMPLETED : August 11, 2011

INCLINATION:                      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 424 E 592 840

Project No. 19-1605-126  
 SHEET 1 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH % RETURN	COLOUR % RETURN	FR-FRACTURE		F-FAULT		SM-SMOOTH		FO-FOLIATED		Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test		
									CL-CLEAVAGE	SH-SHEAR	J-JOINT	P-POLISHED	R-ROUGH	UE-UNEVEN						
									VN-VEIN	S-SLICKENSIDED	ST-STEPPED	PL-PLANAR	W-WAVY	C-CURVED						
									RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA							
TOTAL CORE %		SOLID CORE %			DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION														
80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80	80 80 80 80		
		GROUND SURFACE		2.93																
1	NQ Diamond Drill	<b>GRANITIC GNEISS</b> , moderately weathered, closely spaced sub-horizontal foliation, strong, pinkish grey highly fractured at 0.45m to 0.64m			1															
2																				
3				highly fractured, some sand infill in open joints, sub-vertical joint running through 80% of run		2														
4																				
5																				
6																				
7						3														
8																				
9				dark grey biotite schist zones, slightly weathered																
10						4														
11				highly fractured biotite schist at 10.43m to 10.56m																
12				becoming slightly weathered, moderately spaced foliation																
13						5														
14																				
15																				
16						6														
17																				
18				becoming very coarse grained																
19				biotite seam with diagonal closed joint at 18.74 to 18.89m		7														

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : MB





# RECORD OF BOREHOLE SI-OLD-007

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 11, 2011  
 COMPLETED : August 11, 2011

INCLINATION:                      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 424 E 592 840

Project No. 19-1605-126  
 SHEET 2 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	FACILITY										Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test		
						PENETRATION RATE (mm/min)	FLUSH	COLOUR (mm)	% RETURN	FR-FRACTURE		F-FAULT		SM-SMOOTH				FO-FOLIATED	
										CL-CLEAVAGE	J-JOINT	R-ROUGH	UE-UNEVEN						
										SH-SHEAR	P-POLISHED	ST-STEPPED	W-WAVY						
VN-VEIN		S-SLICKENSIDED	PL-PLANAR	C-CURVED															
RECOVERY		R.Q.D. %	FRACT. INDEX PER .3 m	DISCONTINUITY DATA															
TOTAL CORE %	SOLID CORE %			TYPE AND SURFACE DESCRIPTION															
DIP wrt Core Axis																			
0 90 180 270																			
GROUND SURFACE																			
21	NQ Diamond Drill	white crystalline infill from 21.3m to 21.5m		8												J, closed, diagonal, black weathering			
22		heavily fractured vertical joint from 22.3m to 22.6m															J, closed, vertical, black weathering (20.40 to 20.48m)		
23		becoming faintly weathered															J, closed, horizontal, black weathering		
24					9												J, open, 3mm, white crystalline infill, weathered (21.34 to 21.55m)		
25																	J, heavily fractured, vertical, black weathering (22.33 to 22.63m)		
26																			
27					10												J, closed, planar, sub-vertical, red weathering (24.18 to 24.29m)		
28																	J, closed, irregular, horizontal, fresh		
29																	J, closed, irregular, horizontal, fresh		
30				biotite schist banding (<300mm)	11												J, closed, planar, sub-vertical, grey weathering (25.90 to 26.02m)		
31																	J, closed, planar, sub-vertical, black weathering (26.82 to 27.04m)		
32																	J, closed, irregular, horizontal, brown weathering		
33		END OF BOREHOLE AT 32.40m.		32.40												J, closed, irregular, horizontal, weathered			
34																J, closed, irregular, horizontal, weathered			
35																J, closed, sub-vertical, black weathering (31.30 to 31.62m)			
36																			
37																			
38																			
39																			

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : MB



# RECORD OF BOREHOLE SI-OLD-008

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 12, 2011  
 COMPLETED : August 12, 2011

INCLINATION:      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 489 E 592 891

Project No. 19-1605-126  
 SHEET 1 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	FLUSH	COLOUR % RETURN	FR-FRACTURE										F-FAULT				SM-SMOOTH				FO-FOLIATED				Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test																																																																																																																																																																																																																																																																																																																					
									CL-CLEAVAGE				SH-SHEAR				VN-VEIN		J-JOINT		P-POLISHED		S-SLICKENSIDED		R-ROUGH		ST-STEPPED		PL-PLANAR				UE-UNEVEN		W-WAVY		C-CURVED																																																																																																																																																																																																																																																																																																																
									RECOVERY		R.Q.D. %	FRACT. INDEX PER 3 m	DISCONTINUITY DATA																																																																																																																																																																																																																																																																																																																																								
									TOTAL CORE %	SOLID CORE %			DIP wrt Core Axis		TYPE AND SURFACE DESCRIPTION																																																																																																																																																																																																																																																																																																																																						
80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20

# RECORD OF BOREHOLE SI-OLD-008

PROJECT : Mary River Project  
 LOCATION : Steensby Inlet - Ore Loading Dock  
 STARTED : August 12, 2011  
 COMPLETED : August 12, 2011

INCLINATION:                      AZIMUTH:  
 DRILLER: BOART LONGYEAR, LM-55  
 N 7 798 489 E 592 891

Project No. 19-1605-126  
 SHEET 2 OF 2  
 DATUM CGVD28

DEPTH SCALE (metres)	BORING METHOD	DESCRIPTION	SYMBOLIC LOG	ELEV. (m)	RUN No.	PENETRATION RATE (mm/min)	COLOUR % RETURN	FLUSH	FR-FRACTURE	F-FAULT	SM-SMOOTH	FO-FOLIATED	Unconfined Compressive Strength (Mpa)	FIELD/LABORATORY TESTING RESULTS ● Point Load Test Diametral ▲ Point Load Test Axial ■ Laboratory UCS Test		
									CL-CLEAVAGE	J-JOINT	R-ROUGH	UE-UNEVEN				
									SH-SHEAR	P-POLISHED	ST-STEPPED	W-WAVY				
									VN-VEIN	S-SLICKENSIDED	PL-PLANAR	C-CURVED				
									DISCONTINUITY DATA							
RECOVERY		R.Q.D. %	FRACT. INDEX PER 3 m	DIP wrt Core Axis	TYPE AND SURFACE DESCRIPTION											
TOTAL CORE %	SOLID CORE %															
80 60 40 20	80 60 40 20	80 60 40 20	80 60 40 20	45 30 15 20	0 30 60 90			50	100	150						
21	NQ Diamond Drill	medium grained with dark grey diagonal foliations		9												
22																
23																
24				coarse grained, massive, occasional biotite schist banding	10											
25																
26																
27				very coarse grained	11											
28																
29																
30				medium grained, with fine seams of dark grey biotite schist bands	12											
31																
32																
33	dark grey biotite schist from 32.61m to 32.24m	13														
34																
35																
36		END OF BOREHOLE AT 35.61m.		35.61												
37																
38																
39																

## GROUNDWATER ELEVATIONS

▽ SHALLOW/SINGLE INSTALLATION  
 WATER LEVEL (date)

▼ DEEP/DUAL INSTALLATION  
 WATER LEVEL (date)

LOGGED : Dunstan  
 CHECKED : MB



## Appendix C: Point Load Test Data



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/28/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/30/2011  
**Core Size :** NQ **BH No :** Q7+500 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	10	26.0	D	0.5	47.5	138.3	5.2	Sandstone	Weak
2	13	35.1	D	0.5	47.5	110.5	5.2	Sandstone	Weak
3	13	37.5	D	3.5	47.5	114.8	36.4	Sandstone	Medium Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	7/25/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/30/2011
<b>BH No :</b>	Q10+250	<b>Tester :</b>	SH

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	4	8.0	D	21.5	47.5	88.8	223.5	Granitic Gneiss	Very Strong
2	4	8.1	A	24.5	47.5	48.8	206.9	Granitic Gneiss	Very Strong
3	4	9.7	D	0.5	47.5	85.0	5.2	Granitic Gneiss	Weak
4	5	12.5	D	-	47.5	102.8		Granitic Gneiss	
5	5	12.7	D	15.5	47.5	60.5	161.1	Granitic Gneiss	Very Strong
6	6	15.7	A	0.5	47.5	60.9	3.6	Granitic Gneiss	Very Weak
7	7	18.0	D	13.5	47.5	75.3	140.3	Granitic Gneiss	Very Strong
8	7	18.2	A	8.0	47.5	74.1	48.8	Granitic Gneiss	Medium Strong
9	8	20.4	D	11.5	47.5	63.7	119.5	Granitic Gneiss	Very Strong
10	8	22.3	A	16.0	47.5	68.5	103.8	Granitic Gneiss	Very Strong
11	9	23.5	D	6.5	47.5	78.1	67.6	Granitic Gneiss	Strong
12	9	23.5	A	12.0	47.5	54.0	93.6	Granitic Gneiss	Strong
13	10	26.5	A	16.5	47.5	73.9	101.0	Granitic Gneiss	Very Strong
14	10	26.6	D	16.5	47.5	51.2	171.5	Granitic Gneiss	Very Strong
15	10	27.0	A	11.5	47.5	53.1	90.9	Granitic Gneiss	Strong
16	10	28.1	D	18.0	47.5	85.3	187.1	Granitic Gneiss	Very Strong
17	11	30.5	A	-	47.5	62.0		Granitic Gneiss	
18	11	30.5	D	5.0	47.5	94.4	52.0	Granitic Gneiss	Strong
19	11	31.9	A	7.0	47.5	36.5	74.0	Granitic Gneiss	Strong
20	12	34.0	D	5.5	47.5	54.4	57.2	Granitic Gneiss	Strong
21	12	34.8	A	27.0	47.5	55.3	207.0	Granitic Gneiss	Very Strong
22	12	34.7	D	16.0	47.5	73.4	166.3	Granitic Gneiss	Very Strong
23	12	34.9	D	12.0	47.5	66.8	124.7	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/24/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/30/2011  
**Core Size :** NQ **BH No :** Q14+500 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	5	11.2	D	4.0	47.5	118.6	41.6	Granitic Gneiss	Medium Strong
2	6	14.8	D	23.0	47.5	88.7	239.1	Granitic Gneiss	Very Strong
3	7	17.8	D	1.0	47.5	113.7	10.4	Granitic Gneiss	Weak

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/23/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q18+100 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	3	7.8	D	0.5	47.5	129.8	5.2	Sandstone	Weak
2	5	11.4	D	0.5	47.5	116.1	5.2	Sandstone	Weak
3	6	14.1	D	0.5	47.5	133.0	5.2	Sandstone	Weak
4	6	14.5	D	0.5	47.5	126.8	5.2	Sandstone	Weak
5	6	15.0	D	0.5	47.5	171.0	5.2	Sandstone	Weak
6	6	16.0	D	3.5	47.5	141.6	36.4	Sandstone	Medium Strong
7	9	26.0	D	0.8	47.5	85.2	7.8	Sandstone	Weak
8	12	33.0	D	0.5	47.5	87.8	5.2	Sandstone	Weak
9	14	38.0	D	0.8	47.5	120.9	7.8	Sandstone	Weak

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.





## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/22/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q22+500 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	2.3	D	0.5	47.5	105.5	5.2	Granitic Gneiss	Weak
2	2	4.5	D	13.0	47.5	147.2	135.1	Granitic Gneiss	Very Strong
3	3	5.2	D	16.0	47.5	105.4	166.3	Granitic Gneiss	Very Strong
4	4	8.2	D	23.5	47.5	122.9	244.3	Granitic Gneiss	Very Strong
5	4	10.0	D	1.5	47.5	89.2	15.6	Granitic Gneiss	Weak
6	4	10.8	D	2.0	47.5	120.2	20.8	Granitic Gneiss	Weak
7	6	16.8	D	21.0	47.5	108.9	218.3	Granitic Gneiss	Very Strong
8	8	20.6	D	7.0	47.5	100.4	72.8	Granitic Gneiss	Strong
9	9	23.5	D	5.5	47.5	81.0	57.2	Granitic Gneiss	Strong
10	10	26.0	D	7.0	47.5	78.4	72.8	Granitic Gneiss	Strong
11	10	29.1	D	19.0	47.5	99.8	197.5	Granitic Gneiss	Very Strong
12	11	29.3	D	7.5	47.5	132.9	78.0	Granitic Gneiss	Strong
13	14	39.8	D	35.0	47.5	86.8	363.8	Granitic Gneiss	Extremely Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/22/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q25+500 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	3	5.4	D	30.0	47.5	74.2	311.8	Granitic Gneiss	Extremely Strong
2	5	11.6	D	26.0	47.5	88.3	270.3	Granitic Gneiss	Extremely Strong
3	6	14.3	D	17.5	47.5	86.8	181.9	Granitic Gneiss	Very Strong
4	11	32.1	D	18.0	47.5	108.6	187.1	Granitic Gneiss	Very Strong

- \* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$   
Long pieces of core can be tested diametrically to produce suitable lengths for axial testing
- \* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/17/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q35+500 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	2.6	D	14.5	47.5	190.0	150.7	Granitic Gneiss	Very Strong
2	3	7.5	D	13.5	47.5	101.2	140.3	Granitic Gneiss	Very Strong
3	5	11.5	D	13.0	47.5	131.7	135.1	Granitic Gneiss	Very Strong
4	5	14.4	D	18.5	47.5	125.9	192.3	Granitic Gneiss	Very Strong
5	7	18.2	D	16.0	47.5	190.0	166.3	Granitic Gneiss	Very Strong
6	8	21.5	D	25.0	47.5	76.0	259.9	Granitic Gneiss	Extremely Strong
7	9	25.5	D	14.0	47.5	155.0	145.5	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/16/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q38+700 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	11	32.3	D	25.0	47.5	126.5	259.9	Granitic Gneiss	Extremely Strong
2	12	33.0	D	22.5	47.5	135.3	233.9	Granitic Gneiss	Very Strong
3	14	38.5	D	16.5	47.5	161.0	171.5	Granitic Gneiss	Very Strong
4	14	41.5	D	21.0	47.5	77.2	218.3	Granitic Gneiss	Very Strong
5	16	47.1	D	14.5	47.5	99.7	150.7	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL \* ENVIRONMENTAL \* MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/16/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q42+000 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	3	7.8	D	20.5	47.7	113.9	211.7	Granitic Gneiss	Very Strong
2	3	8.2	D	11.5	47.5	89.8	119.5	Granitic Gneiss	Very Strong
3	6	14.5	D	15.8	47.5	164.0	163.7	Granitic Gneiss	Very Strong
4	7	19.5	D	0.5	47.5	107.0	5.2	Granitic Gneiss	Weak
5	8	20.5	D	22.5	47.5	97.5	233.9	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/15/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/27/2011  
**Core Size :** NQ **BH No :** Q44+000 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	7	17.4	D	5.5	47.5	106.6	57.2	Granitic Gneiss	Strong
2	7	18.6	D	30.0	47.5	84.9	311.8	Granitic Gneiss	Extremely Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/15/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/28/2011  
**Core Size :** NQ **BH No :** Q44+300 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	2.5	D	17.0	47.5	94.8	176.7	Granitic Gneiss	Very Strong
2	2	5.3	D	1.0	47.5	75.5	10.4	Granitic Gneiss	Weak
3	3	8.2	D	19.5	47.5	93.9	202.7	Granitic Gneiss	Very Strong
4	4	9.3	D	20.0	47.5	77.7	207.9	Granitic Gneiss	Very Strong
5	4	11.2	D	27.0	47.5	118.5	280.6	Granitic Gneiss	Extremely Strong
6	5	14.3	D	26.0	47.5	101.7	270.3	Granitic Gneiss	Extremely Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/14/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q45+000 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	2.5	D	21.5	47.5	105.5	223.5	Granitic Gneiss	Very Strong
2	2	4.9	D	2.0	47.5	147.2	20.8	Granitic Gneiss	Weak
3	3	5.1	D	29.0	47.5	105.4	301.4	Granitic Gneiss	Extremely Strong
4	4	9.5	D	0.5	47.5	122.9	5.2	Granitic Gneiss	Weak
5	5	11.8	D	13.0	47.5	89.2	135.1	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.





**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/14/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q50+000 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	1.9	D	1.0	47.5	101.8	10.4	Granitic Gneiss	Weak
2	3	4.9	D	18.0	47.5	133.6	187.1	Granitic Gneiss	Very Strong
3	4	8.6	D	15.5	47.5	47.1	161.1	Granitic Gneiss	Very Strong
4	5	13.7	D	1.5	47.5	137.4	15.6	Granitic Gneiss	Weak
5	6	14.7	D	14.0	47.5	114.4	145.5	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/13/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/28/2011  
**Core Size :** NQ **BH No :** Q53+700 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	4	10.6	D	1.3	47.5	63.2	13.0	Granitic Gneiss	Weak

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/22/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/13/2011  
**Core Size :** NQ **BH No :** Q82+700 **Tester :** CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	6	9.8	D	23.5	47.5	107.8	244.3	Granitic Gneiss	Very Strong
2	6	9.8	A	30.0	47.5	39.4	299.1	Granitic Gneiss	Extremely Strong
3	7	11.1	D	26.0	47.5	61.3	270.3	Granitic Gneiss	Extremely Strong
4	7	11.1	A	22.8	47.5	33.7	256.2	Granitic Gneiss	Extremely Strong
5	7	12.1	D	12.0	47.5	85.2	124.7	Granitic Gneiss	Very Strong
6	7	12.1	D	15.8	47.5	85.2	163.7	Granitic Gneiss	Very Strong
7	7	12.1	A	22.5	47.5	39.3	224.8	Granitic Gneiss	Very Strong
8	9	14.8	D	24.0	47.5	85.7	249.5	Granitic Gneiss	Very Strong
9	9	14.8	A	28.0	47.5	41.8	266.3	Granitic Gneiss	Extremely Strong
10	10	15.9	D	27.0	47.5	102.5	280.6	Granitic Gneiss	Extremely Strong
11	10	15.9	A	22.5	47.5	46.1	198.4	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/18/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/27/2011  
**Core Size :** NQ **BH No :** Q88+800 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	1.4	D	11.5	47.5	182.0	119.5	Granitic Gneiss	Very Strong
2	3	3.4	D	18.0	47.5	152.2	187.1	Granitic Gneiss	Very Strong
3	6	5.0	D	20.8	47.5	102.1	215.7	Granitic Gneiss	Very Strong
4	13	9.6	D	17.5	47.5	122.5	181.9	Granitic Gneiss	Very Strong
5	15	12.5	D	2.0	47.5	151.6	20.8	Granitic Gneiss	Weak

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	7/25/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/12/2011
<b>BH No :</b>	Q114+600	<b>Tester :</b>	BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	2.5	D	21.0	47.5	89.3	218.3	Granitic Gneiss	Very Strong
2	2	2.5	A	31.0	47.5	47.8	266.0	Granitic Gneiss	Extremely Strong
3	2	3.8	D	13.3	47.5	101.6	137.7	Granitic Gneiss	Very Strong
4	2	3.8	A	26.5	47.5	50.7	217.1	Granitic Gneiss	Very Strong
5	2	5.0	D	18.5	47.5	100.0	192.3	Granitic Gneiss	Very Strong
6	2	5.0	A	26.5	47.5	51.3	215.3	Granitic Gneiss	Very Strong
7	3	5.8	D	19.8	47.5	78.8	205.3	Granitic Gneiss	Very Strong
8	3	5.8	A	24.5	47.5	44.8	220.9	Granitic Gneiss	Very Strong
9	3	7.1	D	11.8	47.5	86.5	122.1	Granitic Gneiss	Very Strong
10	3	7.2	A	31.0	47.5	45.4	276.6	Granitic Gneiss	Extremely Strong
11	3	8.0	D	20.5	47.5	88.6	213.1	Granitic Gneiss	Very Strong
12	3	8.0	A	24.0	47.5	39.0	241.1	Granitic Gneiss	Very Strong
13	4	9.3	D	21.3	47.5	88.6	220.9	Granitic Gneiss	Very Strong
14	6	14.5	D	18.0	47.5	92.4	187.1	Granitic Gneiss	Very Strong
15	6	15.6	A	27.0	47.5	52.2	216.3	Granitic Gneiss	Very Strong
16	6	16.6	D	16.7	47.5	93.9	173.6	Granitic Gneiss	Very Strong
17	6	16.6	A	23.0	47.5	44.3	209.3	Granitic Gneiss	Very Strong
18	7	17.7	D	19.5	47.5	74.6	202.7	Granitic Gneiss	Very Strong
19	7	17.8	A	28.0	47.5	42.6	262.5	Granitic Gneiss	Extremely Strong
20	7	18.5	D	15.3	47.5	76.3	158.5	Granitic Gneiss	Very Strong
21	7	18.6	A	12.5	47.5	47.5	107.7	Granitic Gneiss	Very Strong
22	7	19.6	A	34.0	47.5	58.8	248.3	Granitic Gneiss	Very Strong
23	7	19.7	D	19.8	47.5	84.5	205.3	Granitic Gneiss	Very Strong
24	8	20.7	A	19.3	47.5	79.7	111.1	Granitic Gneiss	Very Strong
25	8	20.7	D	24.3	47.5	42.0	252.1	Granitic Gneiss	Extremely Strong
26	8	21.6	A	24.5	47.5	49.0	206.3	Granitic Gneiss	Very Strong
27	8	21.8	D	19.0	47.5	86.5	197.5	Granitic Gneiss	Very Strong
28	8	22.2	A	15.8	47.5	48.5	133.6	Granitic Gneiss	Very Strong
29	8	22.5	D	23.0	47.5	87.5	239.1	Granitic Gneiss	Very Strong
30	9	23.9	D	19.8	47.5	91.9	205.3	Granitic Gneiss	Very Strong
31	9	23.9	A	35.0	47.5	47.9	299.9	Granitic Gneiss	Extremely Strong
32	9	24.6	D	19.5	47.5	90.3	202.7	Granitic Gneiss	Very Strong
33	9	24.7	A	29.0	47.5	50.5	238.4	Granitic Gneiss	Very Strong
34	9	25.7	D	12.0	47.5	113.6	46.3	Granitic Gneiss	Medium Strong
35	9	25.8	A	14.8	47.5	49.9	122.5	Granitic Gneiss	Very Strong

<b>36</b>	10	26.8	D	13.3	47.5	87.6	137.7	Granitic Gneiss	Very Strong
<b>37</b>	10	26.9	A	24.3	47.5	44.7	219.0	Granitic Gneiss	Very Strong
<b>38</b>	10	27.0	D	13.0	47.5	75.4	135.1	Granitic Gneiss	Very Strong
<b>39</b>	10	27.0	A	27.0	47.5	50.2	223.0	Granitic Gneiss	Very Strong
<b>40</b>	10	28.1	D	16.3	47.5	81.0	168.9	Granitic Gneiss	Very Strong
<b>41</b>	10	28.1	A	20.0	47.5	50.8	163.6	Granitic Gneiss	Very Strong
<b>42</b>	10	28.9	D	12.8	47.5	87.8	132.5	Granitic Gneiss	Very Strong
<b>43</b>	10	28.9	A	24.3	47.5	49.4	202.8	Granitic Gneiss	Very Strong
<b>44</b>	11	29.9	D	22.5	47.5	82.7	233.9	Granitic Gneiss	Very Strong
<b>45</b>	11	29.9	A	19.5	47.5	45.0	175.3	Granitic Gneiss	Very Strong
<b>46</b>	11	31.2	D	18.0	47.5	85.3	187.1	Granitic Gneiss	Very Strong
<b>47</b>	11	31.3	A	24.3	47.5	44.5	219.9	Granitic Gneiss	Very Strong
<b>48</b>	11	32.2	D	20.0	47.5	85.9	207.9	Granitic Gneiss	Very Strong
<b>49</b>	11	32.3	A	24.3	47.5	50.6	199.2	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	7/26/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/13/2011
<b>BH No :</b>	Q116+800	<b>Tester :</b>	BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	2.8	A	21.3	47.5	45.0	191.1	Granitic Gneiss	Very Strong
2	2	3.2	D	14.8	47.5	83.8	153.3	Granitic Gneiss	Very Strong
3	2	4.5	D	14.5	47.5	98.3	150.7	Granitic Gneiss	Very Strong
4	2	4.6	A	22.5	47.5	39.2	225.1	Granitic Gneiss	Very Strong
5	3	5.6	D	13.8	47.5	84.8	142.9	Granitic Gneiss	Very Strong
6	3	5.7	A	20.8	47.5	50.3	171.0	Granitic Gneiss	Very Strong
7	3	6.7	D	10.5	47.5	96.1	109.1	Granitic Gneiss	Very Strong
8	3	6.7	A	25.0	47.5	52.8	198.6	Granitic Gneiss	Very Strong
9	3	7.8	D	17.3	47.5	95.5	179.3	Granitic Gneiss	Very Strong
10	3	7.8	A	32.0	47.5	52.3	256.1	Granitic Gneiss	Extremely Strong
11	4	8.5	A	11.0	47.5	43.4	101.6	Granitic Gneiss	Very Strong
12	4	8.9	D	23.5	47.5	90.3	244.3	Granitic Gneiss	Very Strong
13	4	9.6	D	13.5	47.5	87.0	140.3	Granitic Gneiss	Very Strong
14	4	10.0	A	13.3	47.5	42.2	125.1	Granitic Gneiss	Very Strong
15	4	10.6	D	15.3	47.5	76.9	158.5	Granitic Gneiss	Very Strong
16	5	11.4	D	18.8	47.5	86.7	194.9	Granitic Gneiss	Very Strong
17	5	11.4	A	22.0	47.5	47.5	189.8	Granitic Gneiss	Very Strong
18	5	12.7	D	16.3	47.5	91.6	168.9	Granitic Gneiss	Very Strong
19	5	12.7	A	26.5	47.5	46.4	232.6	Granitic Gneiss	Very Strong
20	8	18.0	D	18.5	47.5	89.7	192.3	Granitic Gneiss	Very Strong
21	8	18.1	A	15.3	47.5	48.8	128.7	Granitic Gneiss	Very Strong
22	8	19.1	A	30.0	47.5	91.1	156.1	Granitic Gneiss	Very Strong
23	8	19.2	D	20.5	47.5	50.2	213.1	Granitic Gneiss	Very Strong
24	9	20.4	D	18.5	47.5	79.4	192.3	Granitic Gneiss	Very Strong
25	9	20.4	A	19.8	47.5	47.7	169.6	Granitic Gneiss	Very Strong
26	9	21.6	D	18.5	47.5	95.6	192.3	Granitic Gneiss	Very Strong
27	9	21.7	A	26.0	47.5	47.8	223.2	Granitic Gneiss	Very Strong
28	9	22.4	D	17.8	47.5	79.2	184.5	Granitic Gneiss	Very Strong
29	10	23.6	D	16.8	47.5	85.8	174.1	Granitic Gneiss	Very Strong
30	10	23.6	A	15.5	47.5	49.1	130.1	Granitic Gneiss	Very Strong
31	10	24.7	D	20.3	47.5	70.5	210.5	Granitic Gneiss	Very Strong
32	10	24.7	A	23.3	47.5	51.1	189.3	Granitic Gneiss	Very Strong
33	11	26.4	D	18.5	47.5	99.2	46.3	Granitic Gneiss	Medium Strong
34	11	26.4	A	21.8	47.5	49.8	180.8	Granitic Gneiss	Very Strong
35	11	27.3	D	16.8	47.5	94.0	174.1	Granitic Gneiss	Very Strong

<b>36</b>	11	27.4	A	18.3	47.5	50.6	149.8	Granitic Gneiss	Very Strong
<b>37</b>	11	28.6	D	16.8	47.5	92.2	174.1	Granitic Gneiss	Very Strong
<b>38</b>	11	28.8	A	21.8	47.5	48.2	185.4	Granitic Gneiss	Very Strong
<b>39</b>	12	29.8	D	21.0	47.5	111.2	218.3	Granitic Gneiss	Very Strong
<b>40</b>	12	29.8	A	19.8	47.5	49.1	165.9	Granitic Gneiss	Very Strong
<b>41</b>	12	30.9	D	18.5	47.5	95.6	192.3	Granitic Gneiss	Very Strong
<b>42</b>	12	30.9	A	23.3	47.5	47.6	200.1	Granitic Gneiss	Very Strong
<b>43</b>	12	32.1	D	19.5	47.5	96.0	202.7	Granitic Gneiss	Very Strong
<b>44</b>	12	32.1	A	22.8	47.5	46.3	199.9	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.





## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 8/5/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/12/2011  
**Core Size :** NQ **BH No :** Q131+100 **Tester :** CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	4	8.6	D	21.3	47.5	104.3	220.9	Granitic Gneiss	Very Strong
2	4	8.7	A	25.7	47.5	39.0	257.9	Granitic Gneiss	Extremely Strong
3	4	9.8	D	24.0	47.5	102.3	249.5	Granitic Gneiss	Very Strong
4	4	9.8	A	28.0	47.5	44.2	255.3	Granitic Gneiss	Extremely Strong
5	4	10.7	D	22.8	47.5	77.0	236.5	Granitic Gneiss	Very Strong
6	4	10.8	A	18.5	47.5	34.9	202.5	Granitic Gneiss	Very Strong
7	6	16.3	D	16.0	47.5	108.0	166.3	Granitic Gneiss	Very Strong
8	6	16.3	A	14.5	47.5	46.2	127.7	Granitic Gneiss	Very Strong
9	6	17.3	D	24.3	47.5	79.9	252.1	Granitic Gneiss	Extremely Strong
10	6	17.3	A	26.5	47.5	44.3	241.3	Granitic Gneiss	Very Strong
11	7	18.5	D	17.8	47.5	106.8	184.5	Granitic Gneiss	Very Strong
12	7	18.5	A	18.0	47.5	43.1	167.4	Granitic Gneiss	Very Strong
13	7	19.4	D	22.0	47.5	101.1	228.7	Granitic Gneiss	Very Strong
14	7	19.5	A	27.5	47.5	47.5	237.2	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	7/16/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/12/2011
<b>BH No :</b>	Q138+100	<b>Tester :</b>	CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	0.3	D	11.3	47.5	130.3	116.9	Granitic Gneiss	Very Strong
2	1	0.6	A	25.8	47.5	48.1	219.9	Granitic Gneiss	Very Strong
3	2	1.5	D	10.0	47.5	97.2	103.9	Granitic Gneiss	Very Strong
4	2	1.5	A	16.8	47.5	42.9	156.1	Granitic Gneiss	Very Strong
5	2	2.6	D	9.0	47.5	111.8	93.5	Granitic Gneiss	Strong
6	2	2.6	A	25.5	47.5	50.8	208.7	Granitic Gneiss	Very Strong
7	2	3.5	D	11.0	47.5	89.2	114.3	Granitic Gneiss	Very Strong
8	2	3.5	A	17.5	47.5	41.2	168.5	Granitic Gneiss	Very Strong
9	3	4.6	D	11.0	47.5	100.4	114.3	Granitic Gneiss	Very Strong
10	3	4.6	A	10.0	47.5	41.9	95.1	Granitic Gneiss	Strong
11	3	5.6	D	17.3	47.5	80.2	179.3	Granitic Gneiss	Very Strong
12	3	5.6	A	9.5	47.5	30.8	114.5	Granitic Gneiss	Very Strong
13	4	6.7	D	15.0	47.5	90.6	155.9	Granitic Gneiss	Very Strong
14	4	6.8	A	21.8	47.5	46.1	191.8	Granitic Gneiss	Very Strong
15	4	7.9	D	16.0	47.5	74.7	166.3	Granitic Gneiss	Very Strong
16	4	7.8	A	18.0	47.5	47.0	156.3	Granitic Gneiss	Very Strong
17	5	8.8	D	12.3	47.5	77.0	127.3	Granitic Gneiss	Very Strong
18	5	8.8	A	20.0	47.5	42.0	189.6	Granitic Gneiss	Very Strong
19	6	9.8	D	13.5	47.5	95.1	140.3	Granitic Gneiss	Very Strong
20	6	9.8	A	25.5	47.5	44.4	231.5	Granitic Gneiss	Very Strong
21	6	10.8	D	10.0	47.5	82.8	103.9	Granitic Gneiss	Very Strong
22	6	10.8	A	16.5	47.5	36.6	173.9	Granitic Gneiss	Very Strong
23	7	11.9	D	15.3	47.5	85.8	158.5	Granitic Gneiss	Very Strong
24	7	11.9	A	19.8	47.5	34.6	217.7	Granitic Gneiss	Very Strong
25	8	13.1	D	11.0	47.5	82.6	114.3	Granitic Gneiss	Very Strong
26	8	13.0	A	16.3	47.5	34.0	181.5	Granitic Gneiss	Very Strong
27	8	13.7	D	16.5	47.5	101.6	171.5	Granitic Gneiss	Very Strong
28	8	13.8	A	19.8	47.5	43.4	182.4	Granitic Gneiss	Very Strong
29	9	15.3	D	12.0	47.5	97.2	124.7	Granitic Gneiss	Very Strong
30	9	15.4	A	15.5	47.5	30.1	190.4	Granitic Gneiss	Very Strong
31	10	16.2	D	19.5	47.5	98.2	202.7	Granitic Gneiss	Very Strong
32	10	16.2	A	34.0	47.5	48.6	288.0	Granitic Gneiss	Extremely Strong
33	10	17.1	D	14.0	47.5	85.0	145.5	Granitic Gneiss	Very Strong
34	10	17.1	A	15.3	47.5	35.9	46.3	Granitic Gneiss	Medium Strong
35	14	22.4	D	10.0	47.5	94.8	103.9	Granitic Gneiss	Very Strong

<b>36</b>	14	22.5	A	15.0	47.5	51.4	121.7	Granitic Gneiss	Very Strong
<b>37</b>	15	23.6	D	11.0	47.5	90.4	114.3	Granitic Gneiss	Very Strong
<b>38</b>	15	23.6	A	22.5	47.5	42.8	210.2	Granitic Gneiss	Very Strong
<b>39</b>	15	24.7	D	9.5	47.5	105.0	98.7	Granitic Gneiss	Strong
<b>40</b>	15	24.8	A	14.5	47.5	42.9	135.3	Granitic Gneiss	Very Strong
<b>41</b>	16	25.9	D	14.0	47.5	84.6	145.5	Granitic Gneiss	Very Strong
<b>42</b>	16	25.9	A	22.5	47.5	38.9	226.5	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/19/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** Q139+600 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	4	4.6	D	1.5	47.5	105.5	15.6	Granitic Gneiss	Weak
2	6	9.0	D	7.0	47.5	147.2	72.8	Granitic Gneiss	Strong
3	7	10.7	D	9.5	47.5	105.4	98.7	Granitic Gneiss	Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 8/7/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/13/2011  
**Core Size :** NQ **BH No :** QMR2 **Tester :** CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	1.3	A	20.3	47.5	112.0	89.8	Granitic Gneiss	Strong
2	1	1.3	D	22.8	47.5	48.7	236.5	Granitic Gneiss	Very Strong
3	3	7.2	A	-	47.5	94.1		Granitic Gneiss	
4	3	7.2	D	10.3	47.5	44.7	106.5	Granitic Gneiss	Very Strong
5	4	8.8	A	15.5	47.5	104.3	72.6	Granitic Gneiss	Strong
6	4	8.8	D	17.0	47.5	39.0	176.7	Granitic Gneiss	Very Strong
7	4	9.4	A	16.3	47.5	102.3	77.3	Granitic Gneiss	Strong
8	4	9.3	D	22.5	47.5	44.2	233.9	Granitic Gneiss	Very Strong
9	4	10.4	A	23.3	47.5	77.0	137.9	Granitic Gneiss	Very Strong
10	4	10.4	D	11.3	47.5	34.9	116.9	Granitic Gneiss	Very Strong
11	5	11.8	A	12.3	47.5	108.0	55.9	Granitic Gneiss	Strong
12	5	11.6	D	19.0	47.5	108.0	197.5	Granitic Gneiss	Very Strong
13	5	12.8	A	19.0	47.5	46.2	167.3	Granitic Gneiss	Very Strong
14	5	12.8	D	12.5	47.5	79.9	129.9	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

Job No : 19-1605-126 Client : HATCH  
Date Drilled : 6/8/2011  
Project Name : Mary River Project - Geotechnical Investigation Date Tested : 8/12/2011  
Core Size : NQ BH No : QS1 Tester : BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	1.2	D	16.0	47.5	47.5	166.3	Granitic Gneiss	Very Strong
2	1	1.2	A	20.0	47.5	47.5	172.3	Granitic Gneiss	Very Strong
3	1	2.4	D	21.3	47.5	47.6	220.9	Granitic Gneiss	Very Strong
4	1	2.4	A	23.3	47.5	47.5	200.3	Granitic Gneiss	Very Strong
5	2	3.4	D	17.0	47.5	71.0	176.7	Granitic Gneiss	Very Strong
6	2	3.4	A	24.5	47.5	46.0	216.4	Granitic Gneiss	Very Strong
7	2	4.3	D	19.5	47.5	79.4	202.7	Granitic Gneiss	Very Strong
8	2	4.3	A	25.3	47.5	45.8	224.1	Granitic Gneiss	Very Strong
9	2	4.7	D	21.3	47.5	88.4	220.9	Granitic Gneiss	Very Strong
10	2	4.9	A	30.0	47.5	55.9	227.9	Granitic Gneiss	Very Strong
11	3	6.1	D	15.3	47.5	85.2	158.5	Granitic Gneiss	Very Strong
12	3	6.2	A	22.0	47.5	44.5	199.4	Granitic Gneiss	Very Strong
13	3	7.2	D	16.8	47.5	80.9	174.1	Granitic Gneiss	Very Strong
14	3	7.2	A	27.0	47.5	51.0	220.4	Granitic Gneiss	Very Strong
15	3	8.4	D	19.8	47.5	92.7	205.3	Granitic Gneiss	Very Strong
16	4	10.6	D	19.3	47.5	76.0	200.1	Granitic Gneiss	Very Strong
17	4	10.7	A	19.0	47.5	53.8	148.7	Granitic Gneiss	Very Strong
18	4	11.4	D	20.3	47.5	85.4	210.5	Granitic Gneiss	Very Strong
19	6	16.0	D	20.3	47.5	72.9	210.5	Granitic Gneiss	Very Strong
20	6	16.0	A	25.3	47.5	48.4	214.5	Granitic Gneiss	Very Strong
21	6	17.3	D	17.5	47.5	85.2	181.9	Granitic Gneiss	Very Strong
22	6	17.4	A	13.3	47.5	49.5	110.6	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/24/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/26/2011  
**Core Size :** NQ **BH No :** QS2 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	2.0	D	24.0	47.5	88.3	249.5	Granitic Gneiss	Very Strong
2	1	2.0	A	2.0	47.5	42.9	18.7	Granitic Gneiss	Weak
3	3	5.0	D	24.5	47.5	84.9	254.7	Granitic Gneiss	Extremely Strong
4	3	5.0	A	22.0	47.5	39.1	220.5	Granitic Gneiss	Very Strong
5	4	11.2	D	22.0	47.5	110.0	228.7	Granitic Gneiss	Very Strong
6	5	12.2	D	24.0	47.5	101.1	249.5	Granitic Gneiss	Very Strong
7	9	17.5	D	21.2	47.5	73.8	220.4	Granitic Gneiss	Very Strong
8	13	22.9	D	25.0	47.5	94.0	259.9	Granitic Gneiss	Extremely Strong
9	14	26.3	D	19.7	47.5	142.8	205.2	Granitic Gneiss	Very Strong
10	15	29.5	D	16.8	47.5	145.0	174.6	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/11/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/13/2011  
**Core Size :** NQ **BH No :** QS3A **Tester :** CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	1.2	D	20.0	47.5	112.0	207.9	Granitic Gneiss	Very Strong
2	2	1.2	A	21.3	47.5	48.7	179.7	Granitic Gneiss	Very Strong
3	3	1.9	A	7.5	47.5	94.1	38.1	Granitic Gneiss	Medium Strong
4	3	3.8	D	23.0	47.5	44.7	239.1	Granitic Gneiss	Very Strong
5	3	4.7	D	19.0	47.5	104.3	197.5	Granitic Gneiss	Very Strong
6	8	11.0	D	23.5	47.5	39.0	244.3	Granitic Gneiss	Very Strong
7	8	11.1	A	25.0	47.5	102.3	118.9	Granitic Gneiss	Very Strong
8	8	12.2	D	20.3	47.5	44.2	210.5	Granitic Gneiss	Very Strong
9	9	12.5	A	18.0	47.5	77.0	106.8	Granitic Gneiss	Very Strong
10	10	13.5	D	19.5	47.5	34.9	202.7	Granitic Gneiss	Very Strong
11	10	13.5	A	25.5	47.5	108.0	116.3	Granitic Gneiss	Very Strong
12	10	14.5	D	11.0	47.5	46.2	114.3	Granitic Gneiss	Very Strong
13	10	14.5	A	21.5	47.5	79.9	123.9	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.





## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	8/4/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/12/2011
<b>BH No :</b>	QTR4-1	<b>Tester :</b>	BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	2.4	D	10.0	47.5	106.1	103.9	Granitic Gneiss	Very Strong
2	1	2.4	A	15.8	47.5	43.9	144.2	Granitic Gneiss	Very Strong
3	1	3.1	D	18.8	47.5	84.3	194.9	Granitic Gneiss	Very Strong
4	1	3.1	A	18.8	47.5	42.6	175.8	Granitic Gneiss	Very Strong
5	1	3.5	D	17.8	47.5	99.7	184.5	Granitic Gneiss	Very Strong
6	1	3.5	A	17.3	47.5	44.5	156.4	Granitic Gneiss	Very Strong
7	2	3.9	A	15.0	47.5	36.2	159.6	Granitic Gneiss	Very Strong
8	2	3.9	D	12.5	47.5	107.6	129.9	Granitic Gneiss	Very Strong
9	2	4.9	A	24.5	47.5	47.7	210.6	Granitic Gneiss	Very Strong
10	2	4.9	D	18.3	47.5	93.3	189.7	Granitic Gneiss	Very Strong
11	3	6.3	D	16.0	47.5	126.4	166.3	Granitic Gneiss	Very Strong
12	3	6.4	A	29.0	47.5	51.8	233.7	Granitic Gneiss	Very Strong
13	3	7.2	A	30.0	47.5	45.2	268.7	Granitic Gneiss	Extremely Strong
14	3	7.3	D	13.3	47.5	121.1	137.7	Granitic Gneiss	Very Strong
15	4	8.6	A	19.5	47.5	52.9	154.7	Granitic Gneiss	Very Strong
16	4	8.7	D	24.8	47.5	102.0	257.3	Granitic Gneiss	Extremely Strong
17	4	9.3	D	30.0	47.5	110.5	311.8	Granitic Gneiss	Extremely Strong
18	4	9.4	A	29.8	47.5	47.0	258.5	Granitic Gneiss	Extremely Strong
19	4	10.3	D	12.0	47.5	92.4	124.7	Granitic Gneiss	Very Strong
20	4	10.7	A	23.0	47.5	53.9	179.7	Granitic Gneiss	Very Strong
21	4	11.3	D	17.3	47.5	99.7	179.3	Granitic Gneiss	Very Strong
22	4	11.4	A	21.0	47.5	52.9	166.5	Granitic Gneiss	Very Strong
23	5	12.2	D	12.8	47.5	101.7	132.5	Granitic Gneiss	Very Strong
24	5	12.2	A	17.0	47.5	55.1	130.6	Granitic Gneiss	Very Strong
25	5	13.4	A	27.5	47.5	50.2	227.0	Granitic Gneiss	Very Strong
26	5	13.4	D	20.0	47.5	89.9	207.9	Granitic Gneiss	Very Strong
27	5	14.1	A	14.0	47.5	38.1	143.2	Granitic Gneiss	Very Strong
28	5	14.1	D	9.8	47.5	87.3	101.3	Granitic Gneiss	Very Strong
29	6	14.5	D	11.0	47.5	106.3	114.3	Granitic Gneiss	Very Strong
30	7	17.8	D	16.3	47.5	89.6	168.9	Granitic Gneiss	Very Strong
31	7	20.4	D	14.0	47.5	87.0	145.5	Granitic Gneiss	Very Strong
32	7	20.4	A	24.8	47.5	44.6	224.1	Granitic Gneiss	Very Strong
33	9	25.8	A	16.8	47.5	41.7	159.6	Granitic Gneiss	Very Strong
34	9	25.8	D	18.8	47.5	102.5	46.3	Granitic Gneiss	Medium Strong
35	10	27.0	D	13.5	47.5	67.1	140.3	Granitic Gneiss	Very Strong

<b>36</b>	10	27.1	A	6.3	47.5	48.1	53.4	Granitic Gneiss	Strong
<b>37</b>	10	27.9	D	17.5	47.5	75.7	181.9	Granitic Gneiss	Very Strong
<b>38</b>	10	27.9	A	19.3	47.5	43.5	177.6	Granitic Gneiss	Very Strong
<b>39</b>	10	28.9	D	31.5	47.5	48.2	327.4	Granitic Gneiss	Extremely Strong
<b>40</b>	10	29.1	A	18.8	47.5	91.9	96.9	Granitic Gneiss	Strong
<b>41</b>	11	29.6	A	27.0	47.5	49.3	52.6	Granitic Gneiss	Strong
<b>42</b>	11	29.9	D	21.3	47.5	87.3	100.1	Granitic Gneiss	Very Strong
<b>43</b>	11	31.2	D	17.8	47.5	74.2	86.8	Granitic Gneiss	Strong
<b>44</b>	11	31.2	A	22.5	47.5	49.6	48.7	Granitic Gneiss	Medium Strong
<b>45</b>	11	32.3	A	23.5	47.5	49.6	99.0	Granitic Gneiss	Strong
<b>46</b>	11	32.3	D	17.3	47.5	91.3	46.3	Granitic Gneiss	Medium Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/25/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** QTR-9 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	1	0.7	D	18.5	47.5	87.3	192.3	Granitic Gneiss	Very Strong
2	1	1.6	D	28.0	47.5	66.3	291.0	Granitic Gneiss	Extremely Strong
3	1	5.5	D	23.0	47.5	86.6	239.1	Granitic Gneiss	Very Strong
4	5	12.9	D	17.5	47.5	94.7	181.9	Granitic Gneiss	Very Strong
5	5	15.7	D	15.5	47.5	86.9	161.1	Granitic Gneiss	Very Strong
6	8	20.3	D	20.5	47.5	132.5	213.1	Granitic Gneiss	Very Strong
7	8	23.5	D	16.5	47.5	106.8	171.5	Granitic Gneiss	Very Strong
8	11	30.7	D	8.0	47.5	79.5	83.2	Granitic Gneiss	Strong
9	11	30.8	D	23.5	47.5	103.0	244.3	Granitic Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	7/15/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	7/25/2011
<b>BH No :</b>	QTR-12	<b>Tester :</b>	BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	2	3.0	D	5.0	47.5	76.8	52.0	Granitic Gneiss	Strong
2	2	3.2	A	15.8	47.5	53.2	124.3	Granitic Gneiss	Very Strong
3	2	4.3	A	26.0	47.5	53.8	203.5	Granitic Gneiss	Very Strong
4	3	5.4	A	16.0	47.5	47.4	138.1	Granitic Gneiss	Very Strong
5	3	5.4	D	8.0	47.5	88.6	83.2	Granitic Gneiss	Strong
6	3	6.3	A	10.5	47.5	44.9	94.5	Granitic Gneiss	Strong
7	3	6.3	D	8.0	47.5	87.3	83.2	Granitic Gneiss	Strong
8	3	7.4	D	2.5	47.5	80.9	26.0	Granitic Gneiss	Medium Strong
9	3	7.5	A	14.3	47.5	47.1	123.6	Granitic Gneiss	Very Strong
10	4	8.5	D	0.5	47.5	66.1	5.2	Granitic Gneiss	Weak
11	4	8.5	A	3.0	47.5	43.4	27.8	Granitic Gneiss	Medium Strong
12	4	9.0	A	18.0	47.5	42.9	167.9	Granitic Gneiss	Very Strong
13	4	9.3	D	0.5	47.5	80.9	5.2	Granitic Gneiss	Weak
14	4	9.4	D	17.5	47.5	80.0	181.9	Granitic Gneiss	Very Strong
15	4	10.3	D	9.5	47.5	73.4	98.7	Granitic Gneiss	Strong
16	4	10.3	A	0.5	47.5	40.7	4.9	Granitic Gneiss	Very Weak
17	5	11.4	A	4.5	47.5	45.3	40.2	Granitic Gneiss	Medium Strong
18	5	11.4	D	0.5	47.5	91.1	5.2	Granitic Gneiss	Weak
19	5	12.5	A	9.5	47.5	43.5	87.7	Granitic Gneiss	Strong
20	5	12.5	D	11.5	47.5	85.9	119.5	Granitic Gneiss	Very Strong
21	5	13.7	A	9.5	47.5	48.8	80.2	Granitic Gneiss	Strong
22	5	13.7	D	3.0	47.5	87.5	31.2	Granitic Gneiss	Medium Strong
23	6	14.8	A	15.5	47.5	40.5	151.3	Granitic Gneiss	Very Strong
24	6	15.1	D	16.0	47.5	76.5	166.3	Granitic Gneiss	Very Strong
25	6	16.6	D	0.5	47.5	72.9	5.2	Granitic Gneiss	Weak
26	6	16.7	A	12.5	47.5	53.4	98.4	Granitic Gneiss	Strong
27	7	18.2	A	16.3	47.5	44.4	52.6	Granitic Gneiss	Strong
28	7	18.2	D	7.5	47.5	73.8	100.1	Granitic Gneiss	Very Strong
29	7	19.1	D	1.0	47.5	46.1	86.8	Granitic Gneiss	Strong
30	7	19.2	A	12.8	47.5	104.3	48.7	Granitic Gneiss	Medium Strong
31	7	20.3	A	20.0	47.5	53.1	99.0	Granitic Gneiss	Strong
32	7	20.3	D	25.5	47.5	85.0	46.3	Granitic Gneiss	Medium Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



**THURBER ENGINEERING LTD.**  
GEOTECHNICAL • ENVIRONMENTAL • MATERIALS

## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/14/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/29/2011  
**Core Size :** NQ **BH No :** QTR-13 **Tester :** AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	4	11.4	D	24.0	47.5	123.4	249.5	Granitic Gneiss	Very Strong
2	5	13.2	D	24.5	47.5	102.2	254.7	Granitic Gneiss	Extremely Strong
3	6	14.8	D	32.0	47.5	72.9	332.6	Granitic Gneiss	Extremely Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	8/2/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/18/2011
<b>BH No :</b>	NTUN-DH03	<b>Tester :</b>	Sharif

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
19	17	43.7	A	20.0	47.7	52.6	158.7	Gneiss	Very Strong
20	17	43.7	D	16.0	47.6	53.9	165.6	Gneiss	Very Strong
21	17	43.9	A	16.5	47.5	56.6	124.2	Gneiss	Very Strong
22	17	43.9	D	14.0	47.6	54.7	145.1	Gneiss	Very Strong
23	17	44.8	A	12.5	47.7	53.8	97.6	Gneiss	Strong
24	17	44.8	D	16.0	47.6	63.4	166.0	Gneiss	Very Strong
25	17	45.1	A	13.0	47.5	58.3	95.6	Gneiss	Strong
26	17	45.5	D	17.5	47.6	69.6	181.6	Gneiss	Very Strong
27	17	45.9	A	15.5	47.6	40.8	150.2	Gneiss	Very Strong
28	17	45.9	D	14.5	47.6	69.0	150.1	Gneiss	Very Strong
29	17	46.5	A	17.5	47.5	46.3	153.9	Gneiss	Very Strong
30	17	46.5	D	18.5	47.8	67.8	190.3	Gneiss	Very Strong
3	18	46.9	A	22.0	47.9	46.7	190.9	Gneiss	Very Strong
4	18	47.0	D	16.0	47.5	60.0	166.4	Gneiss	Very Strong
5	18	47.5	A	11.0	47.5	44.8	99.2	Gneiss	Strong
6	18	47.5	D	16.5	48.6	49.2	165.4	Gneiss	Very Strong
7	18	47.9	A	26.0	48.0	54.4	200.2	Gneiss	Very Strong
8	18	47.9	D	23.5	47.8	72.4	241.7	Gneiss	Very Strong
9	18	48.5	A	15.5	48.5	46.6	133.4	Gneiss	Very Strong
10	18	48.9	D	13.5	47.7	63.4	139.5	Gneiss	Very Strong
11	18	49.4	A	16.5	48.3	48.0	139.3	Gneiss	Very Strong
12	18	49.4	D	20.5	47.5	64.8	213.3	Gneiss	Very Strong
13	19	49.6	A	12.5	48.1	47.4	107.0	Gneiss	Very Strong
14	19	49.7	D	21.5	47.4	61.8	223.9	Gneiss	Very Strong
15	19	50.5	A	14.0	48.2	40.7	134.6	Gneiss	Very Strong
16	19	50.5	D	18.0	47.4	45.4	187.6	Gneiss	Very Strong
17	19	50.8	A	20.0	47.6	40.4	195.4	Gneiss	Very Strong
18	19	50.8	D	18.0	48.1	52.1	183.3	Gneiss	Very Strong
1	20	54.2	A	21.5	47.8	47.2	185.4	Gneiss	Very Strong
2	20	54.2	D	21.0	48.0	48.8	214.6	Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	8/2/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/18/2011
<b>BH No :</b>	NTUN-DH03	<b>Tester :</b>	Sharif

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
15	19	50.5	A	18.0	47.9	40.0	176.0	Gneiss	Very Strong
16	19	50.5	D	20.0	47.8	48.0	205.8	Gneiss	Very Strong
17	19	51.1	A	20.5	47.7	52.8	162.3	Gneiss	Very Strong
18	19	51.1	D	20.5	47.5	54.8	212.8	Gneiss	Very Strong
19	19	51.6	A	21.0	47.4	43.8	193.2	Gneiss	Very Strong
20	19	51.6	D	18.5	47.6	57.3	191.7	Gneiss	Very Strong
21	19	52.4	A	14.0	48.1	37.8	142.5	Gneiss	Very Strong
22	19	52.4	D	11.5	47.4	51.7	119.8	Gneiss	Very Strong
24	19	52.5	A	18.0	47.7	67.4	117.9	Gneiss	Very Strong
23	19	52.6	D	16.5	48.4	70.9	166.6	Gneiss	Very Strong
25	20	52.9	A	23.5	47.8	47.3	202.2	Gneiss	Very Strong
26	20	53.0	D	17.0	48.1	56.3	173.5	Gneiss	Very Strong
27	20	53.4	A	17.0	47.7	50.4	139.5	Gneiss	Very Strong
28	20	53.4	D	21.0	47.8	67.5	216.0	Gneiss	Very Strong
29	20	53.8	A	17.0	48.2	52.7	133.8	Gneiss	Very Strong
30	20	53.8	D	22.5	47.9	73.2	231.2	Gneiss	Very Strong
2	21	54.5	D	21.5	47.5	64.3	223.4	Gneiss	Very Strong
1	21	55.0	A	17.0	47.5	61.7	119.6	Gneiss	Very Strong
3	21	55.5	A	21.5	47.5	44.3	195.8	Gneiss	Very Strong
4	21	55.6	D	21.0	47.5	54.3	218.1	Gneiss	Very Strong
5	22	55.8	A	26.0	47.5	49.0	219.0	Gneiss	Very Strong
6	22	55.9	D	21.5	48.3	56.6	217.8	Gneiss	Very Strong
7	22	56.3	D	20.5	47.5	73.3	212.8	Gneiss	Very Strong
8	22	56.3	A	22.5	47.5	41.6	215.0	Gneiss	Very Strong
9	22	57.0	A	13.5	47.5	40.0	132.8	Gneiss	Very Strong
10	22	57.1	D	17.0	48.0	62.5	173.8	Gneiss	Very Strong
11	22	58.0	A	20.5	47.6	65.2	137.9	Gneiss	Very Strong
12	22	58.0	D	17.5	47.4	64.6	182.3	Gneiss	Very Strong
13	22	58.5	A	25.0	47.5	51.3	202.9	Gneiss	Very Strong
14	22	58.6	D	23.0	47.5	53.7	239.1	Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

Job No : 19-1605-126 Client : HATCH  
Date Drilled : 8/2/2011  
Project Name : Mary River Project - Geotechnical Investigation Date Tested : 8/17/2011  
Core Size : NQ BH No : NTUN-DH03 Tester : BT/CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
27	22	59.5	D	16.8	47.4	108.5	174.5	Gneiss	Very Strong
28	22	59.5	A	15.0	47.4	45.0	135.2	Gneiss	Very Strong
25	22	60.5	D	21.3	47.4	99.2	221.7	Gneiss	Very Strong
26	22	60.5	A	25.3	47.4	45.6	225.0	Gneiss	Very Strong
23	23	61.6	D	19.3	47.5	104.6	200.4	Gneiss	Very Strong
24	23	61.6	A	12.3	47.4	49.7	102.0	Gneiss	Very Strong
22	23	62.6	D	1.5	47.4	105.8	15.6	Gneiss	Weak
20	23	63.6	D	1.5	47.5	95.2	15.6	Gneiss	Weak
21	23	63.7	D	19.0	47.4	105.6	198.0	Gneiss	Very Strong
18	23	64.5	D	25.8	47.4	111.8	268.3	Gneiss	Extremely Strong
19	24	64.7	A	15.8	47.4	34.5	174.3	Gneiss	Very Strong
16	24	65.6	D	16.0	47.5	110.7	166.5	Gneiss	Very Strong
17	24	65.6	A	18.8	47.4	57.0	140.4	Gneiss	Very Strong
14	24	66.6	D	21.0	47.4	100.2	218.9	Gneiss	Very Strong
15	24	66.7	A	25.8	47.5	50.4	212.2	Gneiss	Very Strong
12	25	67.6	D	17.3	47.5	77.9	179.5	Gneiss	Very Strong
13	25	67.6	A	20.0	47.5	38.4	203.5	Gneiss	Very Strong
10	25	68.7	D	17.0	47.4	91.2	177.3	Gneiss	Very Strong
11	25	68.7	A	14.0	47.4	37.0	146.8	Gneiss	Very Strong
9	25	69.7	A	8.0	47.5	40.9	77.5	Gneiss	Strong
8	25	69.7	D	10.0	47.4	122.4	104.2	Gneiss	Very Strong
7	26	70.6	D	17.3	47.4	100.8	179.9	Gneiss	Very Strong
6	26	72.0	A	22.8	47.4	50.8	186.4	Gneiss	Very Strong
5	26	72.1	D	18.3	47.4	92.7	190.1	Gneiss	Very Strong
4	26	73.4	A	13.8	47.4	42.5	129.3	Gneiss	Very Strong
3	26	73.6	D	17.8	47.4	92.5	185.0	Gneiss	Very Strong
2	27	74.8	A	24.8	47.4	47.2	214.7	Gneiss	Very Strong
1	27	74.8	D	21.5	47.4	98.3	223.9	Gneiss	Very Strong
29									
30									

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.





## POINT LOAD TEST SHEET

Job No : 19-1605-126 Client : HATCH  
Date Drilled : 8/2/2011  
Project Name : Mary River Project - Geotechnical Investigation Date Tested : 8/14/2011  
Core Size : NQ BH No : NTUN-DH03 Tester : CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
25	26	74.9	A	14.5	47.5	36.0	154.9	Gneiss	Very Strong
24	26	75.0	D	24.3	47.5	84.2	252.4	Gneiss	Extremely Strong
23	27	76.0	A	17.5	47.6	36.9	183.2	Gneiss	Very Strong
22	27	76.0	D	23.0	47.5	88.8	239.1	Gneiss	Very Strong
20	28	77.2	D	21.5	47.5	106.0	223.6	Gneiss	Very Strong
21	28	77.2	A	16.3	47.5	44.8	146.4	Gneiss	Very Strong
18	28	78.2	D	22.0	47.5	92.6	228.8	Gneiss	Very Strong
19	28	78.3	A	24.5	47.6	41.8	232.9	Gneiss	Very Strong
16	28	79.1	D	19.5	47.5	94.6	203.0	Gneiss	Very Strong
17	28	79.2	A	21.0	47.5	41.8	200.2	Gneiss	Very Strong
15	29	80.4	A	19.5	47.5	51.5	157.9	Gneiss	Very Strong
14	29	80.4	D	13.0	47.5	101.4	135.0	Gneiss	Very Strong
13	29	81.6	A	19.0	47.5	40.9	184.0	Gneiss	Very Strong
12	29	81.6	D	18.5	47.6	86.1	191.9	Gneiss	Very Strong
11	29	82.5	D	19.3	47.5	97.0	200.2	Gneiss	Very Strong
9	30	83.4	D	17.0	47.4	89.1	177.1	Gneiss	Very Strong
10	30	83.4	A	14.5	47.5	38.4	147.5	Gneiss	Very Strong
7	30	84.5	D	19.5	47.6	91.0	202.2	Gneiss	Very Strong
8	30	84.6	A	16.5	47.5	35.5	178.0	Gneiss	Very Strong
5	30	85.4	D	13.0	47.6	104.4	134.8	Gneiss	Very Strong
6	30	85.5	A	12.0	47.6	36.7	126.2	Gneiss	Very Strong
3	31	86.6	D	18.0	47.2	117.9	188.9	Gneiss	Very Strong
4	31	86.7	A	18.5	47.1	53.3	146.7	Gneiss	Very Strong
1	31	87.7	D	27.0	47.6	104.3	279.7	Gneiss	Extremely Strong
2	31	87.7	A	23.0	47.6	50.3	189.2	Gneiss	Very Strong
26									
27									
28									
29									
30									

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

Job No : 19-1605-126 Client : HATCH  
Date Drilled : 8/2/2011  
Project Name : Mary River Project - Geotechnical Investigation Date Tested : 8/14/2011  
Core Size : NQ BH No : NTUN-DH03 Tester : CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
29	32	88.6	D	21.5	47.6	83.2	222.8	Gneiss	Very Strong
30	32	88.6	A	16.0	47.6	36.0	170.6	Gneiss	Very Strong
28	32	89.7	A	30.5	47.6	46.3	267.7	Gneiss	Extremely Strong
27	32	89.8	D	29.0	47.7	114.1	300.0	Gneiss	Extremely Strong
25	32	90.6	D	25.3	47.5	94.3	262.3	Gneiss	Extremely Strong
26	32	90.6	A	24.0	47.6	42.4	225.6	Gneiss	Very Strong
23	33	91.6	D	23.0	47.6	90.4	238.1	Gneiss	Very Strong
24	33	91.6	A	21.3	47.6	41.9	201.4	Gneiss	Very Strong
22	33	92.6	D	21.5	47.5	105.7	223.3	Gneiss	Very Strong
20	33	93.6	D	17.5	47.6	99.5	181.2	Gneiss	Very Strong
21	33	93.6	A	16.3	47.6	47.5	139.8	Gneiss	Very Strong
18	34	94.6	D	19.5	47.5	104.3	202.4	Gneiss	Very Strong
19	34	94.6	A	15.3	47.5	48.3	129.7	Gneiss	Very Strong
17	34	96.0	A	21.8	47.6	44.3	197.5	Gneiss	Very Strong
16	34	96.1	D	19.8	47.6	108.1	204.5	Gneiss	Very Strong
14	34	97.2	D	24.0	47.6	101.2	248.8	Gneiss	Very Strong
15	34	97.2	A	26.5	47.6	41.5	253.2	Gneiss	Extremely Strong
12	35	98.0	D	22.0	47.7	110.1	227.5	Gneiss	Very Strong
13	35	98.0	A	25.5	47.6	55.1	195.4	Gneiss	Very Strong
11	35	99.1	D	26.5	47.6	107.2	274.9	Gneiss	Extremely Strong
10	35	100.2	A	14.8	47.6	36.1	157.0	Gneiss	Very Strong
9	35	100.2	D	17.0	47.5	96.0	176.6	Gneiss	Very Strong
7	36	101.3	D	19.5	47.6	97.9	202.0	Gneiss	Very Strong
8	36	101.3	A	14.5	47.7	43.1	134.5	Gneiss	Very Strong
5	36	102.7	D	19.0	47.5	116.0	197.7	Gneiss	Very Strong
6	36	102.8	A	22.5	47.5	52.3	180.0	Gneiss	Very Strong
3	37	103.8	D	16.5	47.5	96.6	171.5	Gneiss	Very Strong
4	37	103.8	A	17.0	47.6	45.5	151.2	Gneiss	Very Strong
1	37	104.9	D	21.0	47.6	85.9	217.5	Gneiss	Very Strong
2	37	104.9	A	15.8	47.6	37.5	162.6	Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

<b>Job No :</b>	19-1605-126	<b>Client :</b>	HATCH
<b>Project Name :</b>	Mary River Project - Geotechnical Investigation	<b>Date Drilled :</b>	8/2/2011
<b>Core Size :</b>	NQ	<b>Date Tested :</b>	8/14/2011
<b>BH No :</b>	NTUN-DH03	<b>Tester :</b>	CC

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
29	37	105.8	D	23.3	47.6	97.7	241.0	Gneiss	Very Strong
30	37	105.8	A	21.3	47.6	46.0	187.7	Gneiss	Very Strong
27	38	107.0	D	19.8	47.5	108.8	205.3	Gneiss	Very Strong
28	38	107.0	A	21.0	47.6	52.8	166.7	Gneiss	Very Strong
25	38	108.2	D	16.0	47.5	102.9	166.4	Gneiss	Very Strong
26	38	108.2	A	22.5	47.6	47.5	193.8	Gneiss	Very Strong
23	38	109.1	D	16.8	47.6	94.3	173.3	Gneiss	Very Strong
24	38	109.1	A	15.0	47.6	47.2	129.8	Gneiss	Very Strong
21	39	110.4	D	21.8	47.7	108.9	224.6	Gneiss	Very Strong
22	39	110.4	A	16.0	47.6	50.6	131.0	Gneiss	Very Strong
20	39	111.2	A	35.0	47.6	52.6	278.3	Gneiss	Extremely Strong
19	39	111.3	D	23.0	47.6	110.3	238.3	Gneiss	Very Strong
17	40	112.6	D	24.0	47.5	76.4	249.1	Gneiss	Very Strong
18	40	112.6	A	19.3	47.6	35.5	207.5	Gneiss	Very Strong
15	40	113.7	D	20.5	47.7	95.6	212.0	Gneiss	Very Strong
16	40	113.7	A	22.0	47.7	43.0	204.5	Gneiss	Very Strong
13	40	114.6	D	21.0	47.6	100.5	217.6	Gneiss	Very Strong
14	40	114.7	A	13.8	47.7	45.5	122.2	Gneiss	Very Strong
11	41	115.6	D	22.0	47.6	98.2	227.6	Gneiss	Very Strong
12	41	115.6	A	27.5	47.6	48.0	234.7	Gneiss	Very Strong
8	41	117.1	D	20.0	47.7	83.7	206.7	Gneiss	Very Strong
10	41	117.1	A	17.0	47.7	46.2	149.4	Gneiss	Very Strong
9	41	117.1	A	13.5	47.7	37.7	138.9	Gneiss	Very Strong
7	41	118.5	D	23.5	47.6	87.5	243.3	Gneiss	Very Strong
6	42	118.6	A	24.0	47.6	44.3	217.8	Gneiss	Very Strong
5	42	118.6	D	16.3	47.6	80.2	168.1	Gneiss	Very Strong
3	42	119.5	D	21.0	47.7	95.0	217.1	Gneiss	Very Strong
4	42	119.6	A	6.0	47.7	33.2	68.0	Gneiss	Strong
1	42	120.8	D	19.0	47.6	108.3	197.2	Gneiss	Very Strong
2	42	120.9	A	16.5	47.6	36.0	176.0	Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

Job No : 19-1605-126 Client : HATCH  
Date Drilled :  
Project Name : Mary River Project - Geotechnical Investigation Date Tested : 8/27/2011  
Core Size : NQ3 BH No : NTUN-DH05 Tester : AS

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
1	22	65.4	D	7.0	47.2	98.6	73.5	gneiss	Strong
2	22	65.8	D	17.3	47.6	74.3	178.8	gneiss	Very Strong
3	22	66.2	D	17.5	47.2	181.0	183.8	gneiss	Very Strong
4									
5									
6									
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									
26									
27									
28									
29									
30									

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

**Job No :** 19-1605-126 **Client :** HATCH  
**Date Drilled :** 7/28/2011  
**Project Name :** Mary River Project - Geotechnical Investigation **Date Tested :** 8/2/2011  
**Core Size :** NQ **BH No :** STUN-DH03 **Tester :** BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
2	22	70.1	D	12.0	47.7	89.4	124.1	Gneiss	Very Strong
3	22	70.4	A	18.5	47.7	43.6	170.1	Gneiss	Very Strong
1	22	71.3	A	19.8	47.7	46.2	173.5	Gneiss	Very Strong
4									
5									
6									
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									
26									
27									
28									
29									
30									

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.



## POINT LOAD TEST SHEET

Job No : 19-1605-126 Client : HATCH  
Date Drilled : 7/29/2011  
Project Name : Mary River Project - Geotechnical Investigation Date Tested : 8/2/2011  
Core Size : NQ BH No : STUN-DH03 Tester : BT

Test No.	Run No.	Depth (m)	Axial or Diametral	Force (kN)	Diameter (mm)	Length (mm)	UCS (MPa)	Rock Type	Notes
30	23	71.3	D	18.8	47.6	106.7	194.0	Gneiss	Very Strong
28	23	72.5	D	12.0	47.6	95.5	124.2	Gneiss	Very Strong
29	23	72.5	A	20.3	47.6	42.0	191.6	Gneiss	Very Strong
26	24	73.6	D	15.5	47.7	88.5	160.2	Gneiss	Very Strong
27	24	73.6	A	17.0	47.7	47.5	146.2	Gneiss	Very Strong
24	24	74.5	D	18.0	47.6	88.6	186.4	Gneiss	Very Strong
25	24	74.5	A	25.0	47.6	54.8	192.5	Gneiss	Very Strong
22	25	75.5	D	15.0	47.6	109.1	155.6	Gneiss	Very Strong
23	25	75.5	A	13.0	47.6	46.0	114.7	Gneiss	Very Strong
21	25	76.4	D	9.5	47.6	74.5	98.3	Gneiss	Strong
19	25	77.7	D	16.5	47.6	78.0	170.8	Gneiss	Very Strong
20	25	77.7	A	24.0	47.6	43.2	222.3	Gneiss	Very Strong
17	26	79.1	D	17.0	47.6	102.6	176.0	Gneiss	Very Strong
18	26	79.2	A	24.0	47.7	46.8	208.9	Gneiss	Very Strong
15	26	80.0	D	16.5	47.6	98.9	170.7	Gneiss	Very Strong
16	26	80.1	A	21.5	47.7	47.4	185.2	Gneiss	Very Strong
13	26	80.8	D	16.5	47.7	93.1	170.4	Gneiss	Very Strong
14	26	80.8	A	23.0	47.7	47.3	198.2	Gneiss	Very Strong
11	27	82.0	D	18.5	47.7	79.4	190.9	Gneiss	Very Strong
12	27	82.1	A	19.5	47.7	42.0	184.2	Gneiss	Very Strong
9	27	82.7	D	17.0	47.7	78.7	175.6	Gneiss	Very Strong
10	27	82.7	A	20.0	47.7	47.7	171.4	Gneiss	Very Strong
7	27	83.9	D	18.0	47.7	88.5	185.8	Gneiss	Very Strong
8	27	84.0	A	21.3	47.7	43.5	195.6	Gneiss	Very Strong
5	28	85.1	D	21.0	47.7	88.3	216.7	Gneiss	Very Strong
6	28	85.1	A	16.0	47.7	42.4	150.2	Gneiss	Very Strong
3	28	86.5	D	18.0	47.7	80.6	185.7	Gneiss	Very Strong
4	28	86.5	A	26.5	47.7	47.2	228.7	Gneiss	Very Strong
1	28	87.0	D	16.5	47.7	78.1	170.3	Gneiss	Very Strong
2	28	87.1	A	23.0	47.7	40.1	225.4	Gneiss	Very Strong

\* It is ideal to perform axial test on core specimens with D/L ratio of  $1.1 \pm 0.1$

Long pieces of core can be tested diametrically to produce suitable lengths for axial testing

\* Diametral Test should have  $0.7 \times D$  on either side of test point.