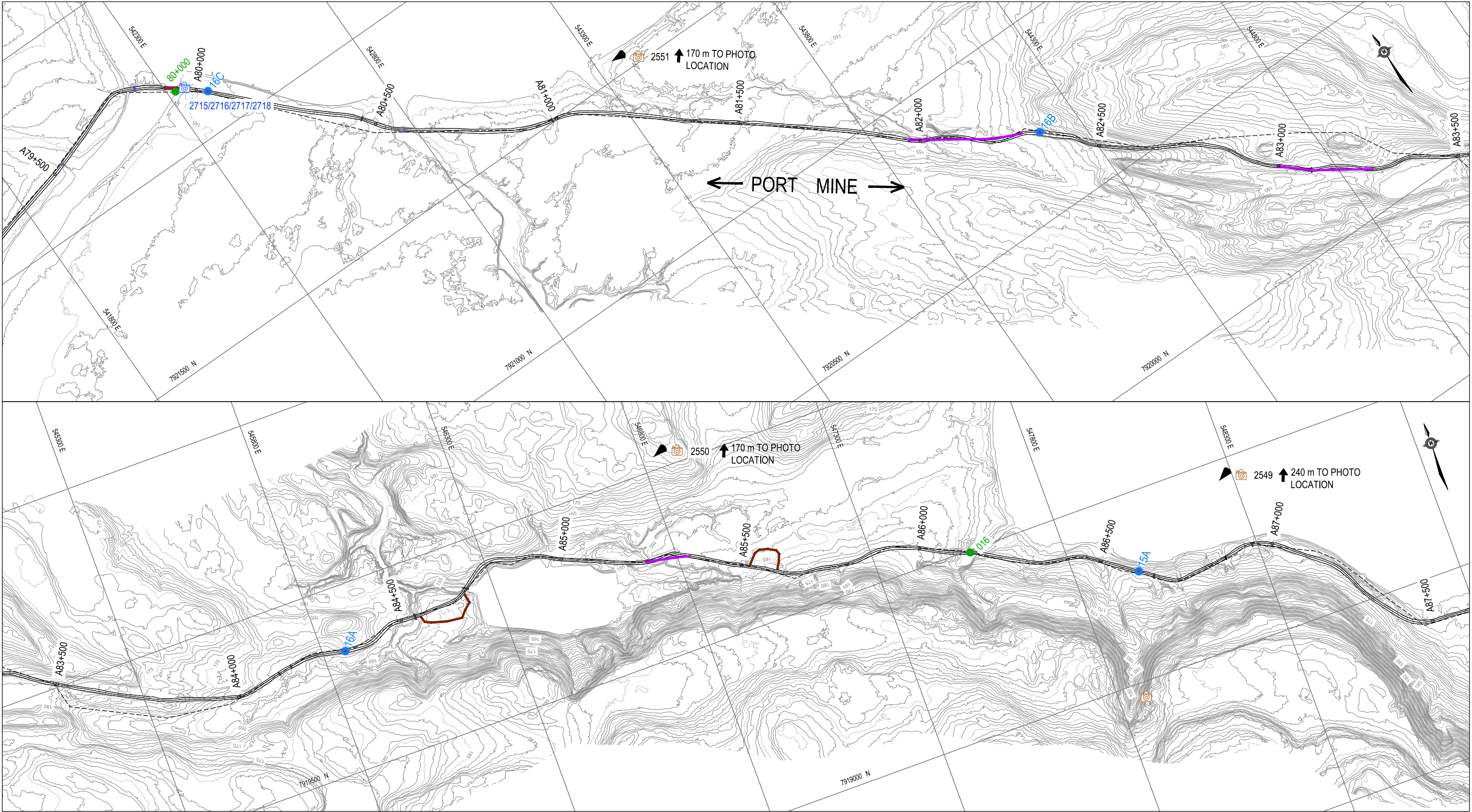


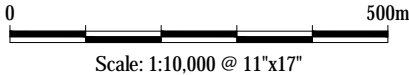
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- LEGEND:
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  - 100 m - DIRECTION AND DISTANCE TO AERIAL PHOTOGRAPH LOCATION WHEN TAKEN OFF THE FIGURE WINDOW
  - 2775 - 2019 GROUND PHOTOGRAPH LOCATION WITH REPORT PHOTO NUMBER(S)
  - 2009 WAYPOINT / BORROW LOCATION
  - NEW OR BORROW SOURCE NOT REPORTED ON, IN 2009
  - APPROXIMATE EXTENT OF BORROW AND LAYDOWN LOCATIONS
  - APPROXIMATE ROAD REALIGNMENT
  - HISTORIC ORIGINAL ALIGNMENTS NOT USED

NOTES  
BASED ON DRAWING DATA PROVIDED BY  
KNIGHT PIESOLD CONSULTING  
BASE DATA: 1:10 000

ISSUED FOR USE



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Mary River Tote Road Quarry Reclamation  
Mary River, Baffin Island

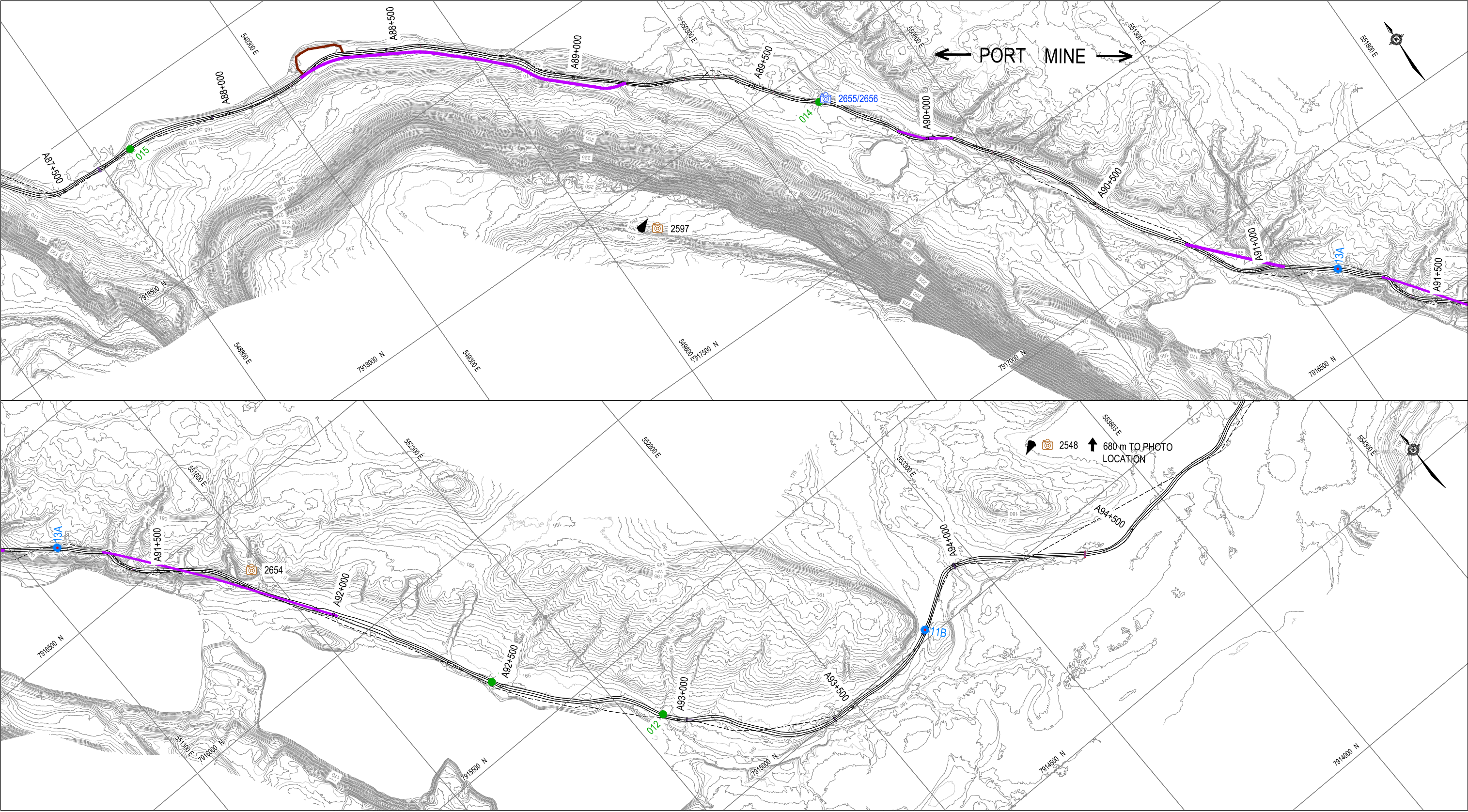
PLAN PHOTO LOCATIONS  
SHEET 11 OF 13

PROJECT NO. ENG.EARC03171-01	DWN BR/EP	CKD KJ	REV 0
OFFICE EDM	DATE December 2019		

Figure 11

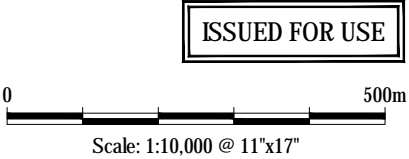


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- LEGEND:
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  - NEW OR BORROW SOURCE NOT REPORTED ON, IN 2009
  - APPROXIMATE EXTENT OF BORROW AND LAYDOWN LOCATIONS
  - APPROXIMATE ROAD REALIGNMENT
  - HISTORIC ORIGINAL ALIGNMENTS NOT USED

NOTES  
BASED ON DRAWING DATA PROVIDED BY  
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BASE DATA: 1:10 000



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Mary River Tote Road Quarry Reclamation  
Mary River, Baffin Island

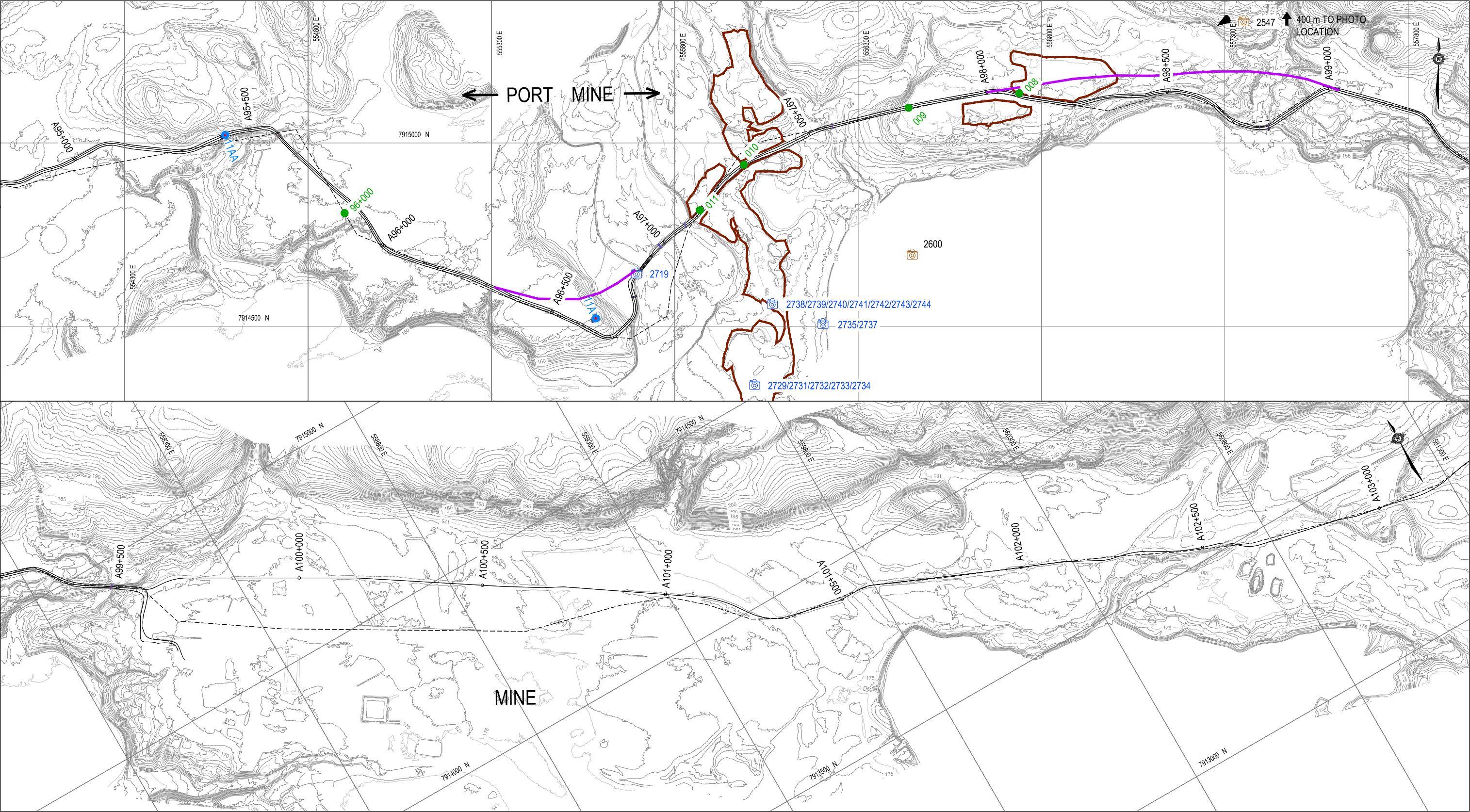
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SHEET 12 OF 13

PROJECT NO. ENG.EARC03171-01	DWN BR/EP	CKD KJ	REV 0
OFFICE EDM	DATE December 2019		

Figure 12



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**LEGEND:**

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- 100 m - DIRECTION AND DISTANCE TO AERIAL PHOTOGRAPH LOCATION WHEN TAKEN OFF THE FIGURE WINDOW
- 2775 - 2019 GROUND PHOTOGRAPH LOCATION WITH REPORT PHOTO NUMBER(S)
- 2009 WAYPOINT / BORROW LOCATION
- NEW OR BORROW SOURCE NOT REPORTED ON, IN 2009
- APPROXIMATE EXTENT OF BORROW AND LAYDOWN LOCATIONS
- APPROXIMATE ROAD REALIGNMENT
- HISTORIC ORIGINAL ALIGNMENTS NOT USED

**NOTES**

BASED ON DRAWING DATA PROVIDED BY KNIGHT PIESOLD CONSULTING

BASE DATA: 1:10 000

**ISSUED FOR USE**

0 500m

Scale: 1:10,000 @ 11"x17"

**CLIENT**

**Baffinland**  
Iron Mines Corporation

**TETRA TECH**

**Mary River Tote Road Quarry Reclamation  
Mary River, Baffin Island**

**PLAN PHOTO LOCATIONS  
SHEET 13 OF 13**

PROJECT NO. ENG.EARC03171-01	DWN BR/EP	CKD KJ	REV 0
OFFICE EDM	DATE December 2019		

**Figure 13**



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**Photo 1 (2014 Photo 1612):**

Priority C pit at KM 6.6 (Figure 1) lies on a gravel terrace with well-established ice wedge polygons. Removal of material from the active layer in the pit has initiated thaw and ponding but it has not changed since 2009 and remarkably is not currently impacting the embankment.



**Photo 2 (2014 Photo 1585):**

Pit at KM 20.7 (Figure 2), linear sinkholes from ice wedge melt out, see also Photo 3.





**Photo 3 (2014 Photo 1697):**

Pit at KM 20.7 (Figure 3) is threatening the stability of the road embankment as thermal erosion from water is occurring through the ice wedges exposed in the adjacent pit.



**Photo 4 (2014 Photo 1645):**

Pit at KM 71.6 (Figure 10), typical settlement from thaw of segregated ice common in finer-grained lacustrine soils.





**Photo 5 (DWH 2009 Photo 593):**  
Pit at KM 61.7 (Figure 8) in 2009, compare to Photo 5A.



**Photo 5A (2593):**  
Pit at KM 61.7 (Figure 8), in 2019, note extensive settlement in southern portion (right side) of the developed pit indicative of the presence of massive ice in portions of this deposit. Note road realignment to improve grade.





**Photo 6 (2014 Photo 1576):**

Pit at KM 32.4 (Figures 4 and 5), typical Priority C pit in thaw stable soils that only requires regrading.



**Photo 7 (2531):**

Pit at KM 7.2 with water on both sides of the new road alignment and extensive thaw of massive and wedge ice.





**Photo 8 (2578):**

Pit at KM 7.2 – Note extensive thaw ponds on the west (right) side of the road. The road has been significantly realigned since 2014 and thaw has progressed dramatically since then. Road is likely founded on permafrost that contains massive ice thereby raising concern that thaw due to the ponded water could lead to failure.



**Photo 9 (2536):**

Pit at KM 7.9 showing extensive settlement and ponding that is leading to embankment instability.





**Photo 10 (2540):**

Pit at KM 19.8 (Figure 4), pit is deepening due to continued thaw of ice-rich ground. Side slopes are very steep and there is considerable cracking on the shoulder and side slopes indicating that thaw is progressing under the road embankment. See Photo 11 for an aerial view of this pit.



**Photo 11 (2571):**

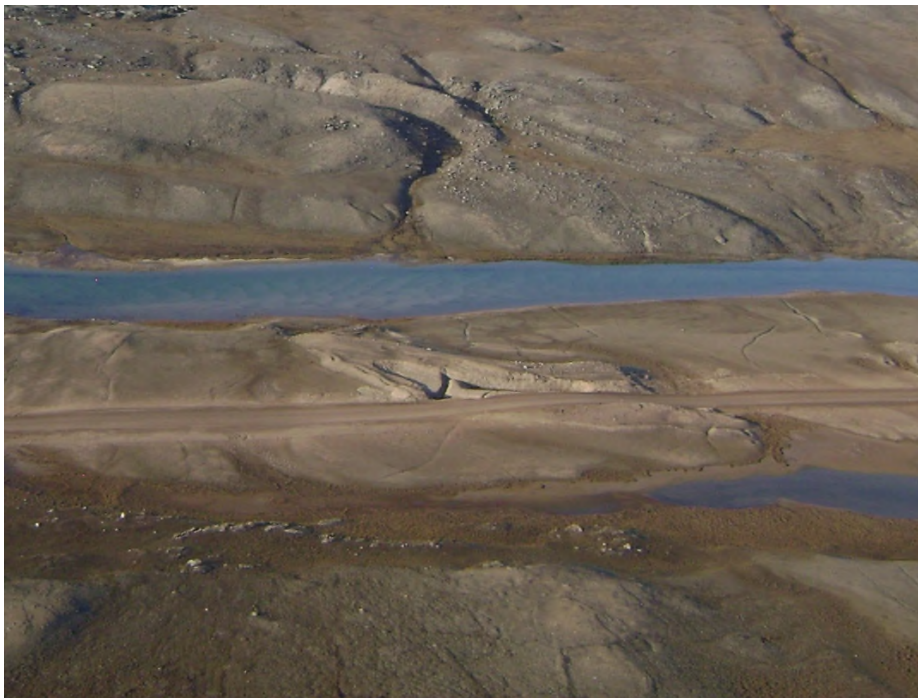
Pit at KM 19.8 (Figure 3), note ponded water resulting from thaw of ice-rich material and over-steepened side slopes where water is ponding.





**Photo 12 (2544):**

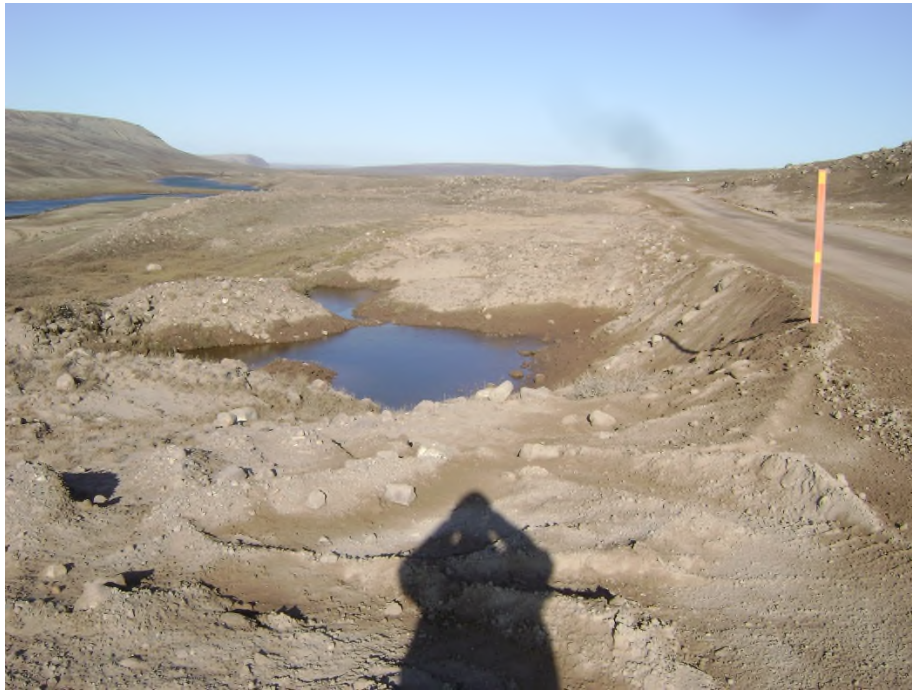
Pit at KM 20.7 (Figure 3), continued melt out of wedge ice since 2009, embankment side slope is at angle of repose and there is substantial cracking on the side slope. There is evidence that the wedge ice that extends under the road is thawing because there are dips on the road surface. See also Photo 3 for comparison to 2014 condition.



**Photo 13 (2571):**

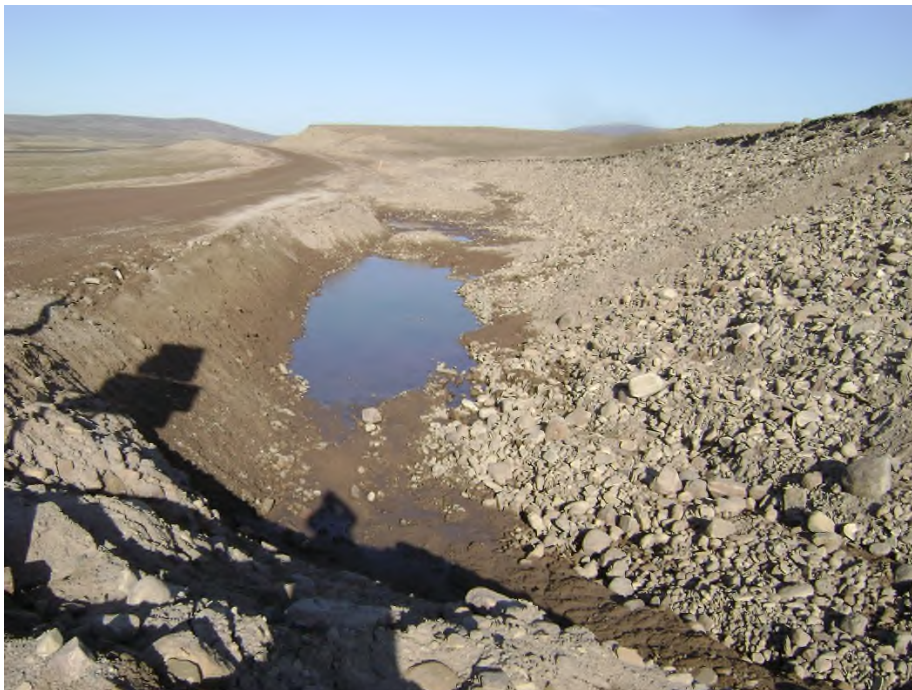
Pit at KM 20.7 (Figure 3), see Photo 13 for closeup of this pit.





**Photo 14 (2693):**

Pit at KM 21.9 (Figure 3), continued thaw of ice-rich material exacerbated by the ponding leading to steep side slope and instability of the embankment.



**Photo 15 (2693):**

Pit at KM 29.1 (Figure 4), continued thaw of ice-rich material exacerbated by the ponding leading to steep side slope and instability of the embankment. See also Photo 16. The central portion of this pit had been backfilled since 2014 dramatically improving embankment stability.





**Photo 16 (2695):**

Pit at KM 29.1 (Figure 4), continued thaw of ice-rich material exacerbated by the ponding leading to steep side slope and instability of the embankment. The central portion of this pit had been backfilled (bottom of the photo) since 2014 dramatically improving embankment stability.



**Photo 17 (2670):**

Pit at KM 56.8 (Figure 8), berms along this (right) side of the new road effectively push water away from the embankment providing thermal protection to the embankment foundation soils and enhancing stability.





**Photo 18 (2693):**

Pit at KM 56.8 (Figure 8), the left side of the new road has steeper side slopes particularly where it narrows at the culvert inlet (close to the KM marker). This increases the potential for instability. The water needs to be pushed back from the toe of the slope by constructing berms on this side of the road (see Photo 17).



**Photo 19 (2592):**

Pit at KM 56.8 (Figure 8), drainage at this location is clearly shown in this photo. The culvert under the old road (near the hairpin corner) is currently being relied upon to keep water levels low around the new road.