1 INTRODUCTION

The following is a consolidated document of key engineering and environmental design considerations. For the purpose of the Application, consistent with the Nunavut Water Board (NWB or the Board) Guide "final drawings must be considered complete and include the statement "*issued for construction*" or *other similar statement*." Agnico Eagle has submitted drawings, signed, stamped, and dated by the appropriate engineer registered with NAPEG as "issued for permitting". See Main Application Document, Appendix J for list of drawings pertinent for Project Expansion infrastructure.

If the Project is approved, final design and construction drawings, accompanied with a detailed report, will be provided to the NWB in accordance with the schedule in the Type A Water Licence for infrastructure stipulated in the licence, including water works and waste disposal facilities. Agnico Eagle will submit to the Board for review and acceptance, in a timeframe set by the Board, final design and for-construction drawings, stamped and signed by a Professional Engineer, for all infrastructure and/or facilities designed to contain, withhold, divert, or retain water and/or waste.

Agnico Eagle is committed to compliance with appropriate territorial and federal legislative requirements for the provision of design details recognizing that facility infrastructure design information may be required by multiple regulators. Agnico Eagle will construct and operate the proposed Mine and associated infrastructure and facilities in accordance with all applicable legislation and industry standards. Agnico Eagle will submit a final design and construction drawings as per Type A Water Licence 2AM-WTP1826 Part D and a construction Summary Report to the Board, within ninety (90) days following the completion of any structure designed to contain, withhold, divert or retain Waters or Wastes.

2 ENVIRONMENTAL CONSIDERATIONS

2.1 Climate

2.1.1 Temperature

The Environment Canada climate normals provide a means to assess the long-term temperature trends at the Project. The annual daily average air temperature recorded at the Baker Lake A Station was -11.3 degrees Celsius (°C), and daily averages ranged from -31.3°C in January to +11.6°C in July. The "Unfrozen" period (daily average temperatures above 0°C), occurs from June through September. During this period, the daily average temperature is approximately 7°C. The "Frozen" period (months having daily average temperatures below 0°C), occurs from October through May. During the period considered (1981 to 2010), the average daily temperature is approximately -20.6°C.

2.1.2 Winds

The wind pattern is predominantly from the northwest quadrant. However, winds originating from the north-northwest and north are also common.

2.1.3 Precipitation

Precipitation data is recorded at the Environment Canada Baker Lake A station. The average annual total precipitation is 249 mm, with most of the rainfall occurring from May through October. The average annual snowfall is approximately 1265 mm, with most snowfall occurring from October through May. Some months experience mixed precipitation. Precipitation data are not recorded at the Meadowbank Mine meteorology station.

As deemed possible based on scheduling, infrastructure pads, diversion ditches, onland saddle dams and crossings of water courses will be constructed during the winter to reduce permafrost degradation and susceptibility. The Arctic design of the buildings will be reflected notably in heavy insulation and special foundation requirements, use of heated utilidors (insulated, enclosed utilities corridors) to connect the buildings and extreme space-heating requirements. To reduce heating needs, waste heat from the process plant at Meadowbank Mine will continue to be used. Space-heating requirements will be met through recovery of exhaust waste heat from the diesel engines driving the power generators. Auxiliary glycol/water-heating boilers will be provided for heating requirements in extreme conditions and emergencies. Another climate-driven feature will be heat-tracing of fuel and water lines, as deemed necessary.

3 PERMAFROST AND GEOTHERMAL CONSIDERATIONS

The Whale Tail Pit Project is in the zone of continuous permafrost. The land surface of the Whale Tail Pit Project is underlain by permafrost except under the lake where water is too deep to freeze to the bottom during winter. Taliks (areas of unfrozen ground) are expected beneath a water body where the water depth is greater than the ice thickness. Closed talik formations show a depression in the permafrost table below relatively shallower and smaller lakes. Open talik formations that penetrate through the permafrost and connect the lake waterbody with the sub-permafrost regime are to be expected for relatively deeper and larger lakes in the Project area.

Based on the thermal modelling and the available thermistor data, the permafrost characteristics in the Project area are summarized below (Golder 2019d):

- The depth of permafrost outside of the influence of lakes is estimated to be between 452 m and 522 m based on thermal gradients and ground temperatures at the lowest portions of the thermistor strings. The depth of permafrost increases with increasing distance from lakes with talik.
- Considering the 2D thermal modelling and 3D block model, the assessment indicated that:
  - Under the northern portion of the lake below Whale Tail Pit, there is likely a closed talik formation.
  - Open talik conditions are probable in the southern portion of the lake where the Whale Tail Lake becomes wider
  - Permafrost depth is between 480 m and 550 m for ground away from the Whale Tail Lake, and between 350 m and 450 m below surface in portions beneath the Whale Tail Lake where a closed talik is present.
  - The cryopeg thickness is likely between 20 m to 30 m.

Review of the 2D thermal analysis and 3D block model indicates that the predicted closed and open talik is consistent with the conceptual hydrogeological conditions adopted in the FEIS Addendum.The active layer thickness at the end of the summer season is expected to range from 1 to 3 m of the ground surface. Where the bedrock is less than 2 m below surface, the foundations of buildings and heavy equipment will rest directly on bedrock without the need for piles or extensive structural fill. For concrete foundations, experience at the Meadowbank Mine has shown that bedrock tends to shatter when excavated in Arctic climates. Therefore, allowances will be made for pouring lean concrete and using rock dowels to stabilize these foundations. Where the bedrock is deeper than 3 m, the foundations will be supported by pilings, as deemed necessary.

4 HYDROGEOLOGICAL CONSIDERATIONS

Since the completion of the FEIS hydrogeological assessment for the Expansion Project (Golder 2018), investigations have been carried out to collect additional site-specific data, as requested in the Project Certificate No. 008, Term and Condition No. 15 for the Approved Project. A summary of the results of these investigations is presented below (Golder 2019d).

A hydrogeological testing program was conducted between 7 and 9 of December 2018 (Golder 2019b) to collect data near the underground below IVR pit. The testing was conducted in deep bedrock in the sub-permafrost zone over a depth interval of about 375 to 626 metres below ground surface (mbgs). Each of the three tests conducted within this interval resulted in estimated hydraulic conductivities of less than 1 x 10-10 m/s (due to limitations of the testing equipment, hydraulic conductivities of less than 1 x 10-10 m/s could not be quantified). The calculated geometric average of the test data below 200 mbgs decreased from 1 x 10-9 m/s to 8 x 10-10 m/s.

On another hand, groundwater sampling and hydraulic head measurements of the Westbay multi-level system was undertaken in November 2018 (Golder 2019a). The 2018 sampling data supplements previous data collected from the Westbay multi-level system in 2016. Hydraulic heads measurements indicate a downward hydraulic gradient was present (magnitude of 0.008 m/m), which is consistent with the conceptual understanding of pre-development groundwater flow directions and predicted conditions post-closure following the formation of the Whale Tail Pit Lake (Golder 2018). Gradients measured pre-development are considered a reasonable interpretation of what long-term gradients could be post-closure following the formation of the pit lake.

Considering the approximate area of Whale Tail Pit (0.5 km^2), the updated geometric average of the deep sub-permafrost bedrock (8 x 10-10 m/s), and the measured downward gradient (0.008 m/m), the data would indicate long term groundwater flux from the pit lake would be approximately 0.3 m^3/day. This value is lower than discharge measurements predicted in the FEIS Addendum for the Environmental Assessment (EA) Scenario of 1.5 m^3/day (Golder 2018).

Except for the refinement of the hydraulic conductivity for the deep sub-permafrost bedrock, field data collected in 2018 is consistent with the conceptual hydrogeological model presented in the FEIS Addendum and no changes were made to the interpreted flow conditions.

Hydraulic head monitoring conducted in November 2018 confirmed the downward direction of the vertical hydraulic gradient predicted by the model below Whale Tail Lake. Review of the 2D thermal analysis and 3D block model indicates that predicted closed and open taliks are consistent with the conceptual hydrogeological conditions adopted in the FEIS Addendum (closed talik in the northern portion of Whale Tail lake and open talik in the southern portion). The 3D block model was compared to the permafrost in the numerical model, and minor adjustments were made in the simulated permafrost depth along the margins of Whale Tail Lake (slightly smaller in extent).

Agnico Eagle's design basis taking into account hydrogeological considerations is detailed in the next section.

5 WATER MANAGEMENT AND GEOTECHNICAL CONSIDERATIONS

The design basis for geotechnical and water management infrastructure is as follows:

- Based on the guidelines provided in the Canadian Dam Association (CDA) Dam Safety Guidelines (CDA 2014), the return period of the design earthquake is 1:2,500 for high consequence of failure structures, and ranges between 1:100 and 1:1000 for signicant consequence of failure structures.
- Seepage control installations will be designed to take into account the risk of piping.
- The minimum dike setback from the pit rim shall be 100 m.
- Minimum Applicable Factors of Safety (MERN 2016) safety factors (SF) for the WRSF are as follows (Agnico Eagle 2018):
    - Local stability (for each bank). Static analysis – long term = 1.2
    - Global stability (deep rupture or rupture in foundation):
        - Static analysis – long term = 1.5
        - Pseudo-static analysis = 1.1 to 1.3
    - Based on the guidelines provided in the CDA Dam Safety Guidelines (CDA 2014), the most likely consequence of failure classification for the Expansion Project IVR dikes IVR D-1, D-2, D-3 Dikes would be “Significant”. This dike classification will be confirmed through the 60-Day Construction Notice and design report to be submitted to regulators, per Water License condition.

Table 1 through Table 9 provide a summary of water management considerations.

Table 1: Various Parameters for Surface Runoff Estimation for a Mean Precipitation Year and 1 in 2 Return Rainfall

Item	Value
Total annual precipitation for a mean precipitation year	249 mm
Total annual rainfall for a mean precipitation year	156 mm
Total annual water equivalent snowfall for a mean precipitation year	93 mm
Total estimated snow sublimation	72 mm
Monthly rainfall distribution	21% in June, 25% in July 27% in August, 27% in September
Estimated monthly lake surface evaporation	9 mm in June, 99 mm in July 100 mm in August, 40 mm in September
Estimated monthly natural land surface evapotranspiration	3 mm in June, 32 mm in July 32 mm in August, 13 mm in September
24-hour duration rainfall for 1 in 2 years of return period	27 mm
5-minute duration rainfall for 1 in 2 years of return period	2 mm
1-hour duration rainfall for 1 in 2 years return period	6 mm

Source: Agnico Eagle (2019)

mm = millimetres; % = percent.

Table 2: Average Annual Freshwater Requirements during Operations

Item	Volume (m^3)
Freshwater for road dust control	0*
Freshwater for truck shop	37,630
Freshwater for main camp	28,400 – 70,100
Freshwater for open pits	13,200 – 17,600

Souce: Golder 2019e
m^3 = cubic metres; * assumed

Updated predictions of groundwater inflow (quantity) are provided for the Base Case and EA Scenario. The Base Case Scenario represents the best estimate of groundwater inflow and groundwater TDS based on the measured data. The EA Scenario is designed to be a reasonable, yet more conservative, assessment of potential groundwater inflow quantity than values that might be adopted for mine operation planning (i.e., Base Case Scenario). Results from the more conservative EA Scenario are used in the Updated Site-Wide Water Balance and Water Quality model (Golder 2019d).

Table 3: Predicted Groundwater Discharge to North Basin of Whale Tail Lake during Dewatering – Base Case Scenario

Phase	Base Case Scenario
	Groundwater Discharge (m^3/day)
Lake Dewatering (Q1-Q3 2019)	1320

Table 4: Predicted Groundwater Inflow during Mining – Base Case Scenario – Whale Tail Pit and Underground

Phase	Time Period	Whale Tail Pit	Underground	Whale Tail Attenuation Pond		North Basin of Whale Tail Lake (within the diked area)
		Groundwater Inflow (m^3/day)	Groundwater Inflow (m^3/day)	Groundwater Inflow (m^3/day)	Surface Water Outflow (m^3/day)	Groundwater Discharge to Surface (m^3/day)
Mining	August-December 2019[1]	970	NA	350	180	650
	2020	1160	20	120	860	720
	2021	1310	30	90	1040	730
	2022	1340	110	90	1080	720
	2023	1340	180	90	1080	720
	2024	1340	170	90	1080	720
	2025	1340	130	90	1080	720

Notes:

IVR Pit is located in permafrost and was therefore not modelled. Interception of runoff / direct precipitation accounted for in Site Wide Water Balance.

[1] Mining prior to Q4 2019 is within permafrost and groundwater inflow will be negligible.

NA = not applicable; m^3/day = cubic metres per day; % = percent.

Table 5: Predicted Groundwater Inflow during Reflooding - Base Case Scenario – Whale Tail Pit, Whale Tail Attenuation Pond, North Basin of Whale Tail Lake

Phase	Approximate Time Period	Water Level in Pit (masl)		Whale Tail Pit	Whale Tail Attenuation Pond		Dewatered North Basin of Whale Tail Lake (within the diked area)
		From	To	Net Groundwater Inflow/Outflow[1] (m^3/day)	Groundwater Inflow (m^3/day)	Surface Water Outflow (m^3/day)	Net Groundwater Discharge to Surface[1] (m^3/day)
Flooding	2026	-130	-76	NA	145	<5	340
	2027	-76	-39	NA	170	<5	340
	2028	-39	3	NA	180	<5	345
	2029	3	26	NA	185	<5	345
	2030	26	43	10	190	<5	345
	2031	43	61	60	180	10	345
	2032	61	73	90	170	30	345
	2033	73	87	120	160	45	340
	2034	87	101	130	155	50	340
	2035	101	111	700	125	505	330
	2036	111	124	1160	85	940	300
	2037	124	133	910	90	740	300
	2038	133	142	360	115	315	315
	2039	142	149	-30	70	135	370
	2040	149	153.5	-10	0	5	155

Phase	Approximate Time Period	Water Level in Pit (masl)		Whale Tail Pit	Whale Tail Attenuation Pond		Dewatered North Basin of Whale Tail Lake (within the diked area)
		From	To	Net Groundwater Inflow/Outflow[1] (m^3/day)	Groundwater Inflow (m^3/day)	Surface Water Outflow (m^3/day)	Net Groundwater Discharge to Surface[1] (m^3/day)
	2041	153.5	153.5	0	0	0	-10

Notes:

IVR Pit is located in permafrost and was therefore not modelled. Interception of runoff / direct precipitation accounted for in Site Wide Water Balance.

[1] Positive values indicate flow to the pit/pond and negative values indicate flow to bedrock.

NA = not applicable; m^3/day = cubic metres per day; % = percent.

Table 6: Predicted Groundwater Discharge to North Basin of Whale Tail Lake during Dewatering – EA Scenario

Phase	Base Case Scenario
	Groundwater Discharge (m^3/day)
Lake Dewatering (Q1-Q3 2019)	1330

Table 7: Predicted Groundwater Inflow during Mining - EA Scenario – Whale Tail Pit and Underground

Phase	Time Period	Whale Tail Pit			Underground		
		Groundwater Inflow (m^3/day)[3]	Portion of Inflow from Attenuation Pond (%)	Portion of Inflow from South Basin of Whale Tail Lake (%)	Net Groundwater Inflow (m^3/day)	Portion of Inflow from Attenuation Pond (%)	Portion of Inflow from South Basin of Whale Tail Lake (%)
Mining	August-December 2019[1]	970	1%	<1%	NA	NA	NA
	2020	1170	64%	<1%	60	<1%	<1%
	2021	1320	79%	3%	70	<1%	<1%
	2022	1360	81%	9%	250	<1%	<1%
	2023	1360	82%	12%	420	<1%	<1%
	2024	1350	82%	14%	410	<1%	<1%
	2025	1350	82%	15%	340	<1%	<1%

Notes:

IVR Pit is located in permafrost and was therefore not modelled. Interception of runoff / direct precipitation accounted for in Site Wide Water Balance.

[1] Mining prior to Q4 2019 is within permafrost and groundwater inflow will be negligible.

[2] TDS concentrations do not account for loading from lakes and Whale Tail Attenuation Pond. TDS from these sources to be accounted for in Site Wide Water Quality analysis.

NA = not applicable; m^3/day = cubic metres per day; % = percent.

Table 8: Predicted Groundwater Inflow during Reflooding - EA Scenario – Whale Tail Pit, Whale Tail Attenuation Pond, North Basin of Whale Tail Lake

Phase	Approximate Time Period	Water Level in Pit (masl)		Whale Tail Pit			Whale Tail Attenuation Pond			North Basin of Whale Tail Lake (within the diked area)	
		From	To	Net Groundwater Inflow/Outflow[1] (m^3/day)	Portion of Inflow from Attenuation Pond (%)	Portion of Inflow from South Basin of Whale Tail Lake (%)	Groundwater Inflow (m^3/day)	Portion of Inflow from South Basin of Whale Tail Lake (%)	Pond Outflow (m^3/day)	Net Groundwater Inflow/Outflow[1] (m^3/day)	Portion of Inflow from South Basin of Whale Tail Lake (%)
Flooding	2026	-130	-76	NA	NA	NA	150	76%	<5	345	>99%
	2027	-76	-39	NA	NA	NA	170	84%	<5	345	>99%
	2028	-39	3	NA	NA	NA	180	89%	<5	345	>99%
	2029	3	26	NA	NA	NA	180	91%	<5	345	>99%
	2030	26	43	20	47%	41%	185	93%	<5	345	>99%
	2031	43	61	90	47%	41%	170	96%	25	345	>99%
	2032	61	73	130	44%	50%	160	97%	55	340	>99%
	2033	73	87	170	46%	53%	150	98%	80	340	>99%
	2034	87	101	170	50%	50%	145	98%	90	335	>99%
	2035	101	111	730	71%	29%	120	99%	530	330	>99%
	2036	111	124	1170	81%	19%	85	99%	950	300	>99%
	2037	124	133	910	82%	18%	90	99%	745	300	>99%
	2038	133	142	360	82%	18%	115	99%	315	315	>99%
	2039	142	149	-30	NA	NA	70	98%	140	370	>99%
	2040	149	153.5	-10	NA	NA	0	NA	10	155	>99%
	2041	153.5	153.5	0 to -5	NA	NA	0	NA	5	-10	NA

Notes:

IVR Pit is located in permafrost and was therefore not modelled. Interception of runoff / direct precipitation accounted for in Site Wide Water Balance.

[1] Positive values indicate flow to the pit/pond and negative values indicate flow to bedrock.

NA = not applicable; m^3/day = cubic metres per day; % = percent.

Table 9: Water Drainage Areas of Various Site Sectors

Facility	Ultimate Area (ha)
Main Camp Watershed	21.7
Whale Tail Pit Watershed	116.7
Whale Tail Pit Footprint	56.8
Whale Tail Attenuation Pond Watershed	102.7
Whale Tail WRSF Water Collection System Watershed	109.2
Whale Tail WRSF Footprint (in Whale Tail WRSF Water Collection System Watershed)	86.4
Whale Tail WRSF Footprint (in IVR Pit Watershed)	14.9
Whale Tail WRSF Footprint (in North Sector Watershed)	18.3
Whale Tail WRSF Footprint (Total)	119.6
IVR Pit Watershed	117.6
IVR Pit Footprint	35.6
IVR Attenuation Pond Watershed	131.6
IVR WRSF Water Collection System Watershed	45.9
IVR WRSF Footprint (in IVR WRSF Water Collection System Watershed)	32.6
IVR WRSF Footprint (in IVR Attenuation Pond Watershed)	45.7
IVR WRSF Footprint (Total)	78.3
North-East Sector watershed	68.4
South Whale Tail Lake Watershed	2231

Source: Golder (2019e)
ha = hectares.

5.1 Freeboard

The design criteria for minimum freeboard for the dikes are presented in Table 10: . The freeboard may change due to fluctuations in Whale Tail Lake and Ponds, or due to settlement in the dikes. Maintenance may be required to restore loss of freeboard due to settlement. The freeboard may also change during further advanced engineering phases.

Table 10: Design Minimum Freeboard

Structure	Minimum Freeeboard		
	Normal Operation (m)	Design Flood Conditions (m)	Actual Minimum Feeboard (m)
Whale Tail Dike	2.0	1.8	2.0
Mammoth Dike	No water	1.3	1.5
Whale Tail WRSF Dike	No water	0.60	0.70
Northeast Dike	No water	0.7	0.80
IVR D-1, D-2, D-3 Dikes	TBD	TBD	TBD

Source: Agnico Eagle (2019).
m = metres.

5.2 Operations

The following outlines the key criteria and constraints that will need to be observed and followed to operate the dewatering dikes in accordance with the design objectives, concepts, and assumptions. Based on the guidelines provided in the Canadian Dam Association Dam Safety Guidelines (CDA, 2014), it is estimated that the IVR D-1, D-2, D-3 dikes could have a classification of "Significant" considering that a failure of these dikes could flood the Road No.1, used as a haul road, resulting in associated threat to the safety of mine personnel, equipment, and other workings within the area. As such, the design of the dike (e.i. freeboard, crest elevation, spillway, etc.). will be conducted under rigourous engineering standards and construction drawings and detailed deign report will be submitted to regulators via the 60-day Construction Notice, per Water License Part D . The operation and maintenance of the dikes will be conducted in accordance to the The Meadowbank Mine OMS manual (Whale Tail Addendum).

The Meadowbank Mine OMS manual (Whale Tail Addendum) will be updated by Agnico Eagle before the operations of the dikes, reviewed on an annual basis and revised as necessary to accommodate changes in the condition and operations of the facilities or in management structure as per Part B, condition 13 of the Water License 2AM-WTP1826 . The OMS Manual will be an extension of the existing Meadowbank OMS Manual (Agnico Eagle 2017).

6 RECOMMENDED OPEN PIT SLOPE GEOMETRY

A summary of the recommendations is provided below (Knight Piésold 2017, 2019):

- Design BFA: 65 to 75°
- Design Bench Width: 10 to 16 m
- Bench Height: 14 to 21 m
- IRA: 4 32° to 53°

7 WASTE ROCK STORAGE FACILITY CONSIDERATIONS

The design basis and criteria for the development of the waste rock storage facilities assumed, the maximum height of the Whale Tail Waste Rock Storage Facility is 95 m and a storage capacity of 65.3 Mt (Agnico Eagle 2018). Additional design criteria is presented in Table 11. The IVR Waste Rock Storage Facility will have a maximum height of approximately 60 m.

Table 11: Design Criteria for the Waste Rock Storage Facilities

Parameters	WT WRSF Value
Side Slope	37° (angle of repose)
Bench Height	20.0m
Bench Setback S	20.0m
Lift Placement	5.0m
Factor of Safety – Static Loading (frozen foundation and stripping material, with 4.7m deep (maximum) active zone at the toe of first bench	1.21
Factor of Safety – Static Loading Global Failure	1.52
Factor of Safety – Pseudo Static Analysis – Global Failure	1.48

Source: SNC (2018)

8 NPAG STOCKPILE FACILITY CONSIDERATIONS

The NPAG stockpile will have and overall height of 20 m. It will be composed of a 15 m bench constructed in three (3) layers of 5 m of thickness, and a 5 m bench at the top. The 5m bench at the top will start at a setback distance of 20 m from the crest of the 15 m bench. The bench height and setback were selected based on slope stability and constructability constraints. Similarly than with the WRSF geometry, the NPAG stockpile geometry design was chosen to ensure proper stability of the structure during its construction until it is eventually removed and utilized for closure. The geometry of the NPAG stockpiles is positioned strategically as to contain any contact water from this structure within the attenuation pond to eliminate any risk of propagating TSS in surrounding lakes. (Agnico Eagle, 2018).

9 OVERBURDEN STOCKPILE FACILITY CONSIDERATIONS

The stockpile will have a maximum height of 10 m. and a crest width of 10 m. The slope of the pile will follow the repose angle of the frozen stripping material (maximum 25°). The geometry of the overburden stockpile is positioned strategically as to contain any contact water from this structure within the attenuation pond to eliminate any risk of propagating TSS in surrounding lakes (Agnico Eagle 2018).

10 REFERENCES

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